ROOF LEVEL 8' - 1 1/8"





TBAE FIRM REGISTRATION NO.: 1452 TBPLS FIRM REGISTRATION NO.: 10065600 A202

TBPE FIRM REGISTRATION NO.: F-1416

305 East Huntland Drive Suite 200 Austin, Texas 78752 512.453.0767 512.453.1734

SHEET TITLE BUILDING ELEVATIONS -BUNKHOUSE SHEET NUMBER

TEXAS

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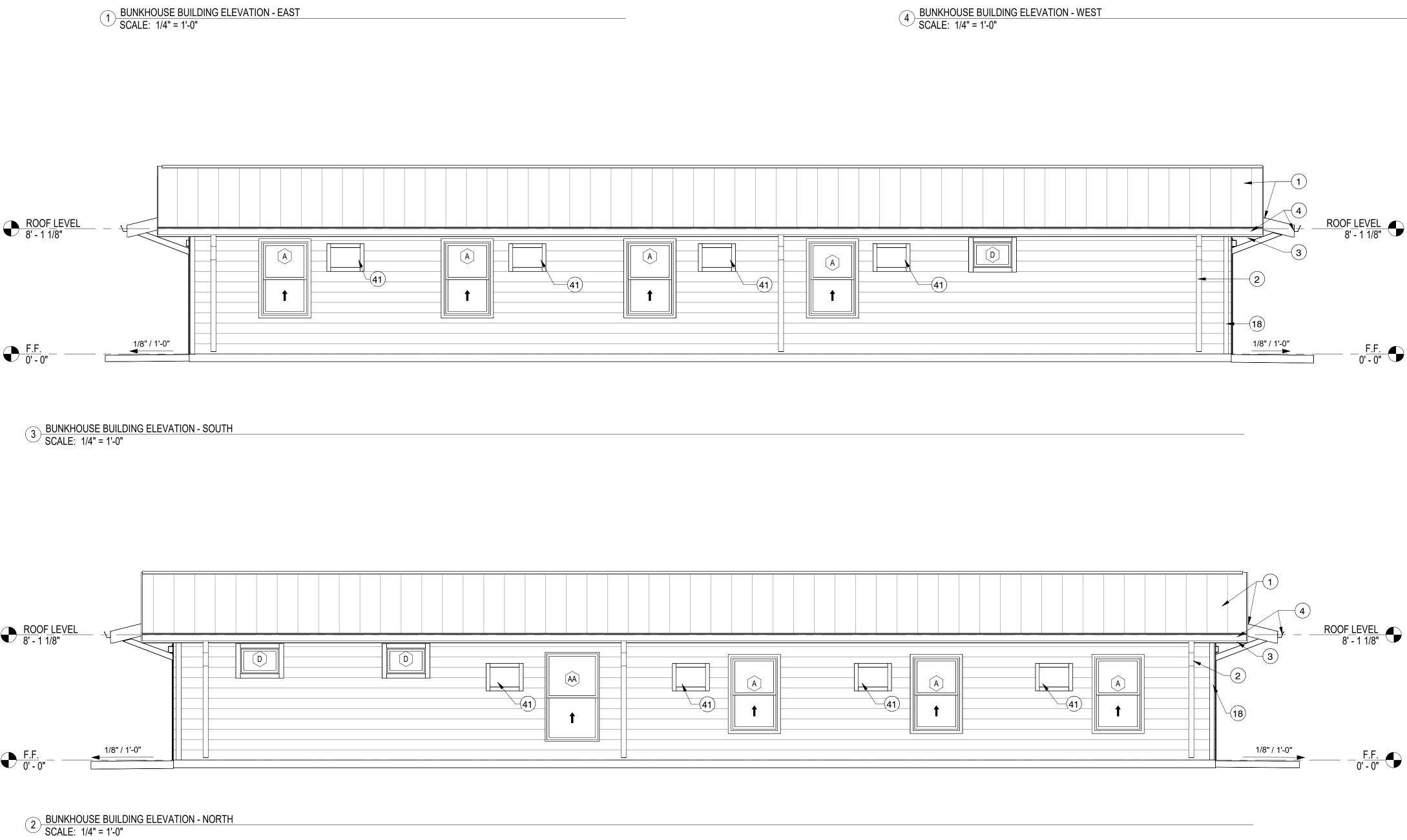
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TX FIRM NO. 1452

GUS ENGELING WMA
THERING LODGE AND BUNKHOUS

DATE: April 3, 2024 DESIGNED BY: MW DRAWN BY: EB REVIEWED BY: OH



ROOF LEVEL 8' - 1 1/8"

ROOF LEVEL 8' - 1 1/8"

F.F. 0' - 0"

7' - 0"

W113)

34 RE: ELECTRICAL 35 4" VTR 36 LINE OF WALL BELOW 37 STAINED 4" TIMBER SHELF 38 KITCHEN ISLAND RE:6-7-8/A4-2 39 1X6 STAINED T&G CEDAR SLATS WITH 1" SEPERATION, ST-3 40 CONTROL JOINT, TYP. PER 07/S3.1 41 THROUGHWALL AC UNIT RE:MECHANICAL 42 NON RATED CEILING HATCH 43 ROOF FRAME. REFER TO STRUCTURAL DRAWINGS AND SPECIFICATIONS 44 MECHANICAL DUCTWORK, PAINTED RE: MECH

KEYNOTES

3 FIBER CEMENT FACIA W/SEMI-TRANSPARENT STAIN TO MATCH TRIM

11 IRONSTONE HEARTH COURSING TO MATCH EXISTING CONFERENCE

6 STANDING SEAM METAL ROOF CANOPY OVER EXTERIOR DOOR,

9 SINK W/ COMPACT GARBAGE DISPOSAL, REF. PLUMBING

10 RUMFORD R4848 BACK TO BACK FIREPLACE

13 (3) 36" WIRE SHELVING UNITS, 60"H, 15"D.

18 HARDIE TRIM TYP AT EA ELEVATION CORNER

25 STEEL COLUMN WITH CEDAR CLADDING, STAINED ST-1

19 TRIM, PAINTED TO MATCH HARDIE SIDING

21 1X6 STAINED T&G CEDAR SLATS, ST-2

14 36" WIRE SHELVING UNITS, 60"H, 14"D.

12 4X8 MOUNTED PLYWOOD BOARD

15 WATER HEATR. REF. PLUMBING

20 THROUGH WALL FLASHING

22 WOOD COLUMN STAIN (ST-1)

24 MOUNTED AT 12' AFF, TYP

26 BUILT-IN TWIN XL BED RE:1/A402

27 BUILT-IN CLOSET RE:10/A402 28 BUILT-IN DESK RE: 4/A402

1 STANDING SEAM METAL ROOF, TYP.

2 PREFINISHED MTL DOWNSPOUT

4 PREFINISHED MTL GUTTER 5 WOOD BEAM STAIN, ST-1

7 CASED OPENING

16 MOP SINK

23 1X4 TRIM

29 46" CEILING FAN 30 CEILING FAN

31 GYP CEILING PAINTED 32 LINE OF COUNTER BELOW 33 MECH UNIT RE: MECHANICAL

17 STAIN CEDAR POST

ROOF LEVEL 8' - 1 1/8"

(W114)

8 ADA COMPLIANT SINK

2 GATHERING LODGE CROSS SECTION SCALE: 1/4" = 1'-0"

KEYNOTES

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41 THROUGHWALL AC UNIT RE:MECHANICAL 42 NON RATED CEILING HATCH

43 ROOF FRAME. REFER TO STRUCTURAL DRAWINGS AND SPECIFICATIONS

44 MECHANICAL DUCTWORK, PAINTED RE: MECH

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TX FIRM NO. 1452

S G WMA BUNKHOU GUS ENGELING THERING LODGE AND B

DATE: April 3, 2024 DESIGNED BY: MW DRAWN BY: EB REVIEWED BY: OH

SHEET TITLE
BUILDING SECTIONS GATHERING LODGE

SHEET NUMBER

305 East Huntland Drive

512.453.0767 512.453.1734

Suite 200 Austin, Texas 78752

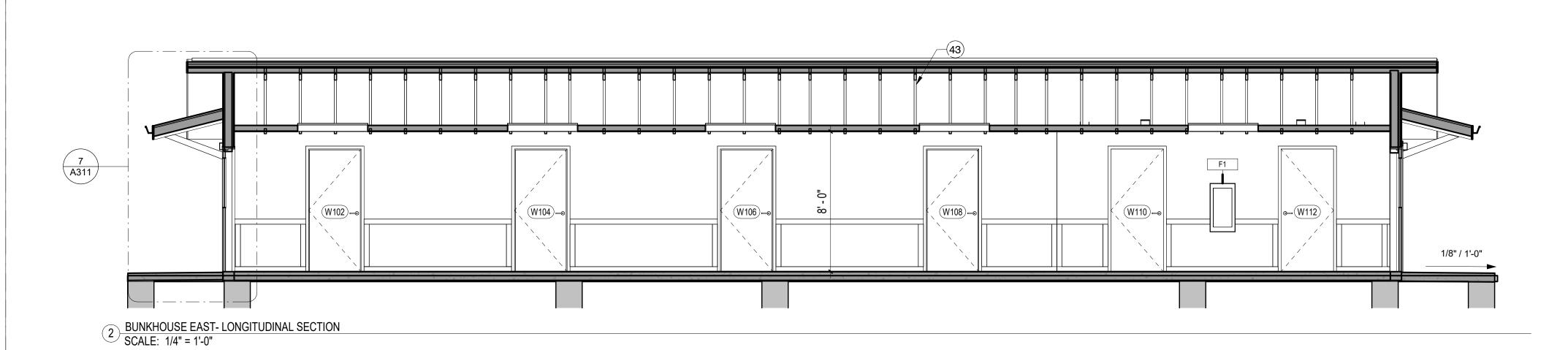
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TBPE FIRM REGISTRATION NO.: F-1416
TBPLS FIRM REGISTRATION NO.: 10065600

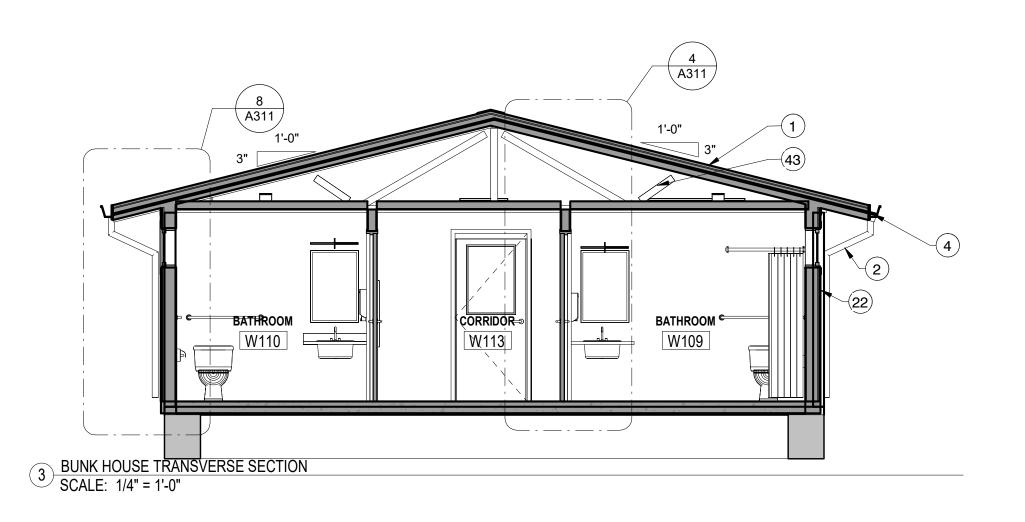
A301

Design Group

mwm

1 GATHERING LODGE LONGITUDINAL SECTION SCALE: 1/4" = 1'-0"







1 STANDING SEAM METAL ROOF, TYP.

2 PREFINISHED MTL DOWNSPOUT 3 FIBER CEMENT FACIA W/SEMI-TRANSPARENT STAIN TO MATCH TRIM

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GUS ENGELING WMA
THERING LODGE AND BUNKHOUS

ECT NUMBER:

SHEET TITLE **BUILDING SECTIONS -GATHERING LODGE**

A302

TBAE FIRM REGISTRATION NO.: 1452 TBPE FIRM REGISTRATION NO.: F-1416

TBPLS FIRM REGISTRATION NO.: 10065600

Design Group

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WALL SECTIONS

SHEET NUMBER

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SHEET TITLE WALL SECTIONS

SHEET NUMBER

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TBAE FIRM REGISTRATION NO.: 1452
TBPE FIRM REGISTRATION NO.: F-1416
TBPLS FIRM REGISTRATION NO.: 10065600

1 GATHERING LODGE PANTRY SECTION SCALE: 1/2" = 1'-0"

STANDING SEAM MTL ROOF WITH SHELF — ADHERING SHEET UNDER LAYMENT, TYP

OPEN CELL SPRAY OVER PRE-ENGINEERED —

WOOD TRUSS, TYP

CEILING SOFFIT

STAINED 1X6 T&G CEDAR WALL SIDING ----

ALIGN WITH CEILING SOFFIT

CEILING AS SCHED.

SPRAY FOAM INSULATION —

PAINTED GYP BD OVER 3/4" — PLYWOOD ON 2X WOOD FRAMING

STAINED 1X6 WOOD TRIM, TYP —

STAINED 1X6 WOOD BASE, TYP -

FOUNDATION. REF STRUCTURE -

STAINED 1X6 T&G CEDAR -

2 GATHERING LODGE FIRE PLACE SECTION SCALE: 1/2" = 1'-0"

CONTINUOUS CAP FLASHING -FLUID APPLIED MEMBRANE AIR — BARRIER OVER BRICK TYP MASONRY TIE AT 16" OC. EW. TYP. — CONTINUOUS THROUGH WALL -FLASHING WITH WEEPS AT 24" OC STANDING SEAM METAL ROOF — 8" CMU REF 🗀 STRUCTURE SPACE AROUND FLUES TYP

DAMPER

SMOOTH -

FACE STONE CEMENT FILL STEEL ANGLE RE: STRUCTURE

STRUCTURE

STEEL ANGLE RE: —

STAINED 4" TIMBER SHELF -

TR42 RUMFORD THROAT

8" CMU RE: STRUCTURE -

FIRE BRICK W/ HEATSTOP II -REFRACTORY MORTAR

> IRONSTONE -VENEER

RE: STRUCTURE ←

CONCRETE FOUNDATION ——

INTERIOR

ANCHOR TO MASONRY

SUPERIOR CLAY CORP 20"X20" -

MORTAR BED WITH WEEP HOLE -

CLAY POT

CASTSTONE TOP -

IRONSTONE VENEER, STONE TYPE & COURSING TO MATCH - ADJACENT CONFERENCE CENTER. STONE PROVIDED BY CONTRACTOR

<u>| |</u>-8' - 8"

FLUES LINERS SMOKE CHAMBER LG FLAT CASTIRON —

FLUID APPLIED MEMBRANE AIR —____ BARRIER OVER BRICK TYP BRICK GROUTED SOLID -SUPERIOR CLAY 1318 — MASONRY TIE AT 16" OC EW. TYP ---SUPERIOR CLAY 1318 —

HEATSTOP II

EXTERIOR

RUMFORD R4242 BACL TO BACK FIRE PLACE

2' - 2"

REFRACTORY MORTAR

- CONCRETE PAVEMENT RE: CIVIL

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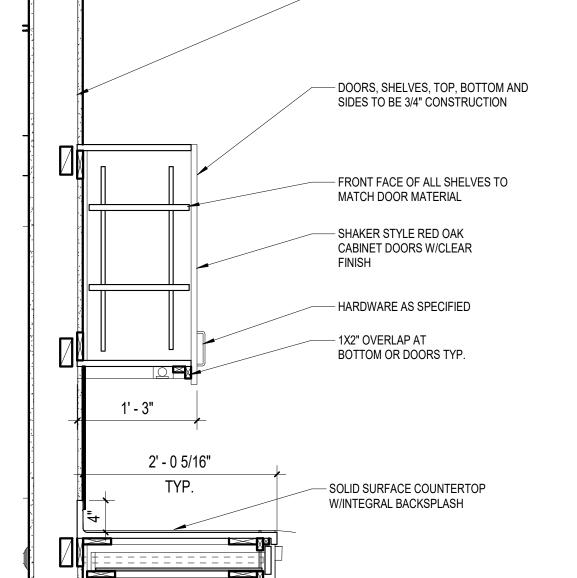
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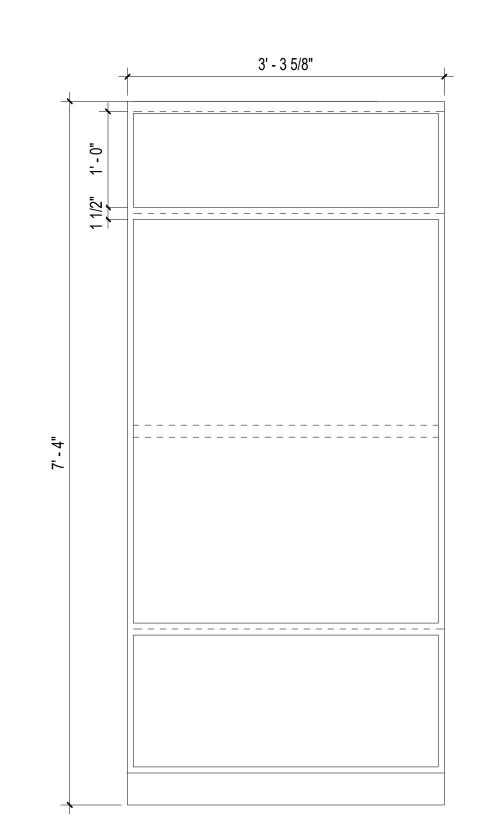
SHEET TITLE SECTION DETAILS

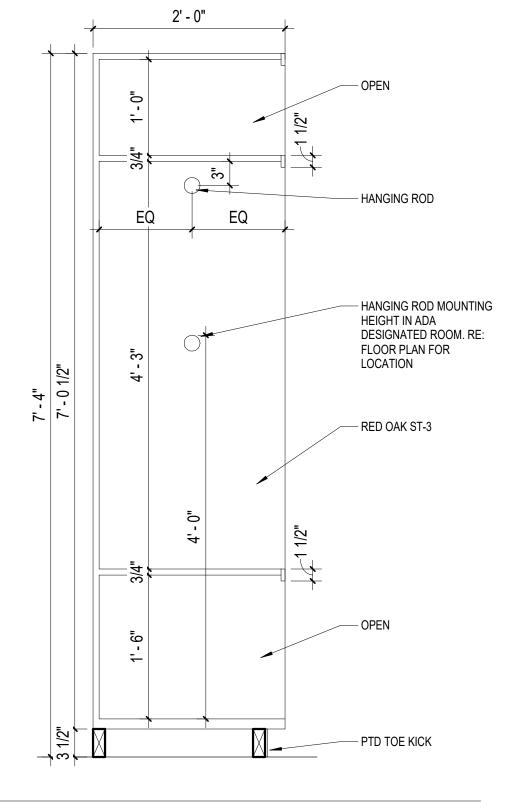
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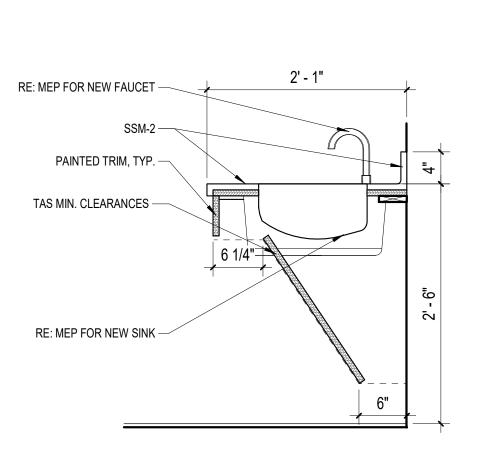
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TBAE FIRM REGISTRATION NO.: 1452 TBPE FIRM REGISTRATION NO.: F-1416 TBPLS FIRM REGISTRATION NO.: 10065600









SECTION @ BATHROOM SINK
SCALE: 1" = 1'-0"

1. PROVIDE BLOCKING, NAILERS, GROUNDS, FURRING AND OTHER SIMILAR ITEMS REQUIRED TO RECEIVE AND SUPPORT MILLWORK AND OTHER WORK

MILLWORK NOTES

MILLWORK TO BE FABRICATED AND INSTALLED IN ACCORDANCE WITH THE STANDARD OF THE AMERICAN WOODWORKERS INSTITUTE (AWI).

- 2. SUBMIT SHOP DRAWINGS FOR ALL MILLWORK ITEMS PRIOR TO FABRICATION.
- PROVIDE FINISHED END PANELS AND/OR END RETURNS AT OPEN ENDED OR EXPOSED CASEWORK
- PLYWOOD TO BE HIGH DENSITY.
- CONTRACTOR TO PROVIDE MILLWORK GROMMETS PER END USER'S DIRECTION AFTER COMPLETION OF WORK. GROMMET TO BE MOCKER, MM4 (CAP) AND MM4A-26D (LINER) IN BLACK
- COORDINATE LOCATIONS AND PROVIDE BACKING IN PARTITIONS FOR ALL CABINETS, COUNTERTOP AND SHELVING, BACKING TO BE 6" x 18 GA. MET. PLATE OR 3-5/8" x 18 GA. CONTINUOUS RUNNER WI FLANGES CUT AT STUDS.
- 7. SCRIBE COUNTERTOPS AND SPLASHES TO ADJACENT SURFACES.
- PROVIDE 1/2" X 1/2" SCRIBE WHERE CABINETS ABUT A PARTITION OR GYP. WALL BOARD SOFFIT. REVEAL COLOR TO MATCH CABINET, U.N.O.
- STANDARD MILLWORK HARDWARE TO BE MOCKER, DP78 COLOR: MATTE CHROME, U.N.O. SIZE MA
- 10. PROVIDE DOOR AND DRAWER PULLS AS DELINEATED.
- PROVIDE COUNTERTOPS OVER ALL BASE CABINETS AND KNEE SPACE APRONS.
- 12. PROVIDE ENDSPLASH WHEN COUNTERTOP IS ADJACENT TO WALLS AT SIDES, U.N.O.
- 13. PROVIDE FIRE TREATED WOOD BLOCKING WITHIN WALL FOR ALL WALL MOUNTED CABINETRY.
- 14. PROVIDE HANGING FILE HARDWARE WHEN MILLWORK IS NOTED WITH "F" IN STYLE DESIGNATION.
- 15. PROVIDE BRACING AT KNEE SPACE @ 3'-0" TYPICAL.
- 16. ALL CABINET SHALL BE CLEAR STAINED RED OAK VENEER WITH PARTICLE BOARD CONSTRUCTION. CABINET DOORS SHALL BE SHAKER STYLE FACE, FLAT CENTER PANEL AND FLAT FRAME WITH EUROPEAN (INVISIBLE) SELF CLOSING, 170 DEGREE OPENING HEAVY-DUTY HINGES AND 100 POUND RATED, FULL-EXTENSION, SIDE-MOUNTED DRAWER SLIDES, U.N.O. JOINTS 1/16" MAXIMUM WIDTH CONSISTENT THROUGHOUT. REF SPECIFICATION FOR ADDITIONAL INFORMATION.

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SHEET NUMBER

TBPLS FIRM REGISTRATION NO.: 10065600

512.453.1734

9 SECTION @ TYP. BASE & UPPER CABINETS SCALE: 1" = 1'-0"

— PTD TOE KICK

- HARDWARE AS SPECIFIED

DOORS W/CLEAR FINISH

SHAKER STYLE RED OAK CABINET

- DOORS, SHELVES, TOP, BOTTOM AND

SIDES TO BE 3/4" CONSTRUCTION

SECTION @ FULL HEIGHT CABINET SCALE: 1" = 1'-0"

TBAE FIRM REGISTRATION NO.: 1452 TBPE FIRM REGISTRATION NO.: F-1416

SHEET TITLE

MILLWORK DETAILS

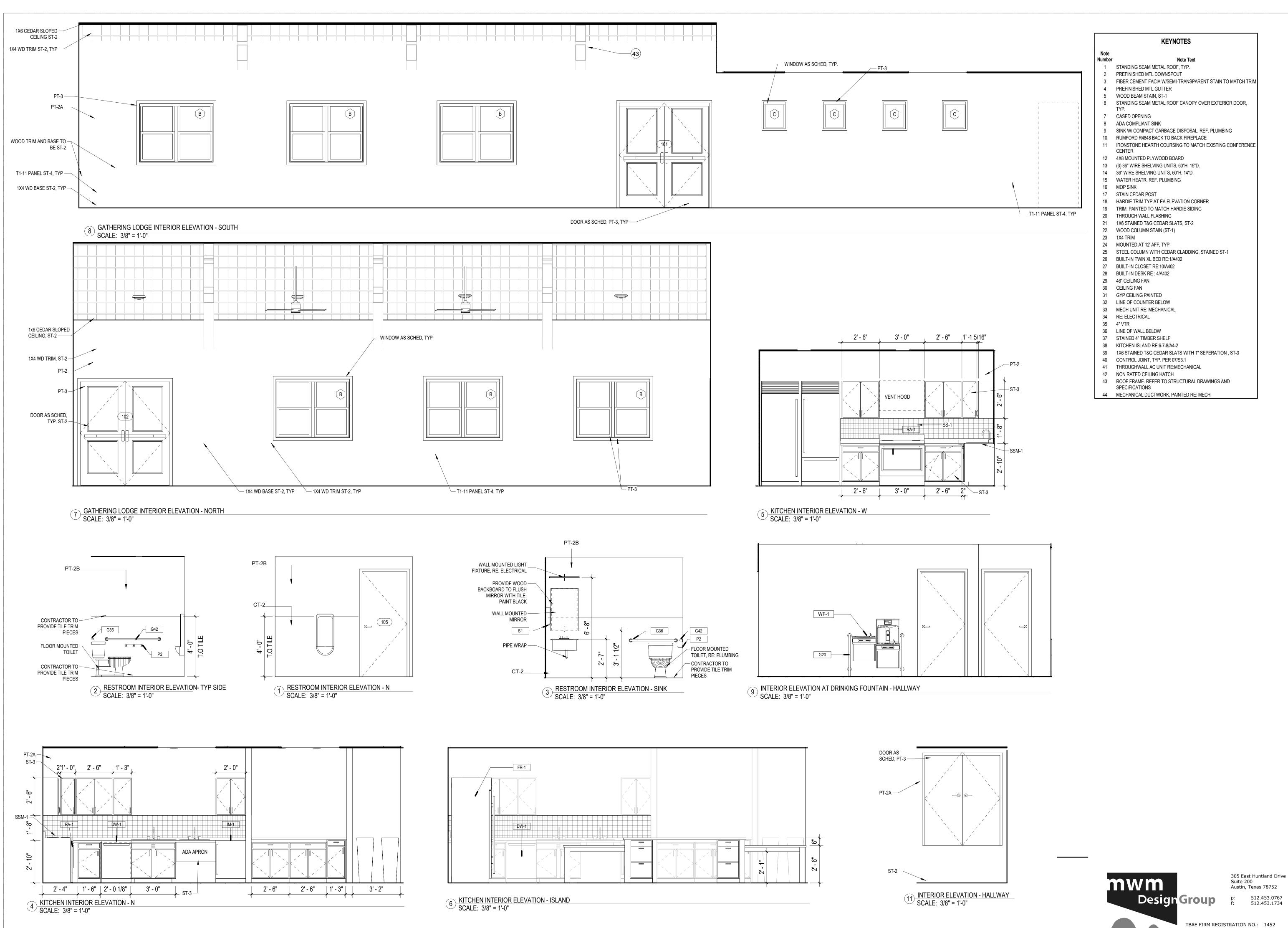
DATE: April 3, 2024

DESIGNED BY: MW

REVIEWED BY: OH

DRAWN BY: EB

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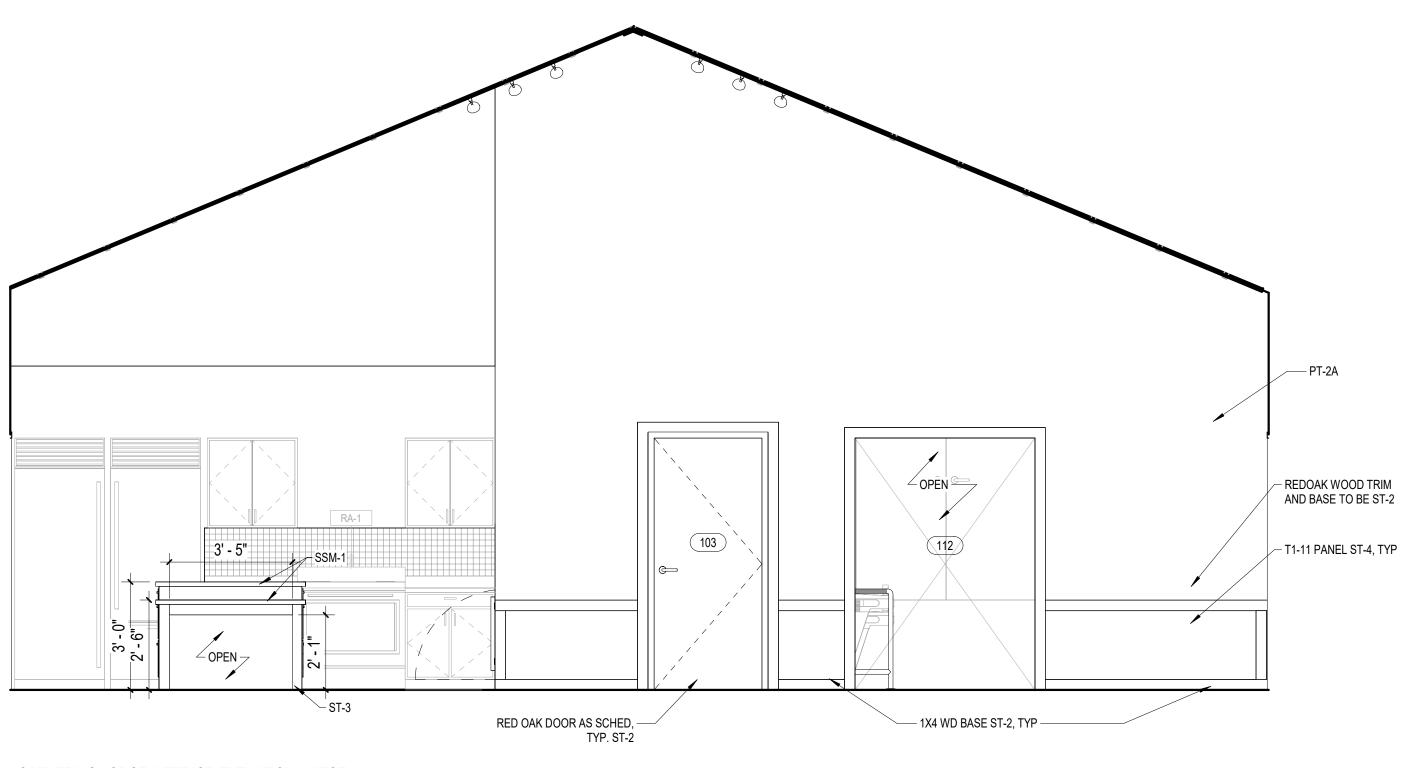
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> SHEET TITLE INTERIOR ELEVATIONS -GATHERING LODGE

SHEET NUMBER

A501

TBPE FIRM REGISTRATION NO.: F-1416 TBPLS FIRM REGISTRATION NO.: 10065600 GATHERING LODGE INTERIOR ELEVATION - EAST SCALE: 3/8" = 1'-0"



2 GATHERING LODGE INTERIOR ELEVATION - WEST SCALE: 3/8" = 1'-0"

KEYNOTES

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TX FIRM NO. 1452

S BUNKHO GUS ENGELING
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DATE: April 3, 2024 DESIGNED BY: MW DRAWN BY: EB REVIEWED BY: OH

> SHEET TITLE INTERIOR ELEVATIONS -GATHERING LODGE

SHEET NUMBER

305 East Huntland Drive

512.453.0767 512.453.1734

Suite 200 Austin, Texas 78752

A502

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KEYNOTES

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42 NON RATED CEILING HATCH 43 ROOF FRAME. REFER TO STRUCTURAL DRAWINGS AND

SPECIFICATIONS 44 MECHANICAL DUCTWORK, PAINTED RE: MECH

S BUNKHOU WMA GUS ENGELING THERING LODGE AND B

TEXAS

PARKS 8

WILDLIFE

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AGREEMENT FOR THIS

WORK.

TX FIRM NO. 1452

DATE: April 3, 2024 DESIGNED BY: MW DRAWN BY: EB REVIEWED BY: OH

> SHEET TITLE INTERIOR ELEVATIONS -BUNKHOUSE

SHEET NUMBER

305 East Huntland Drive

512.453.0767 512.453.1734

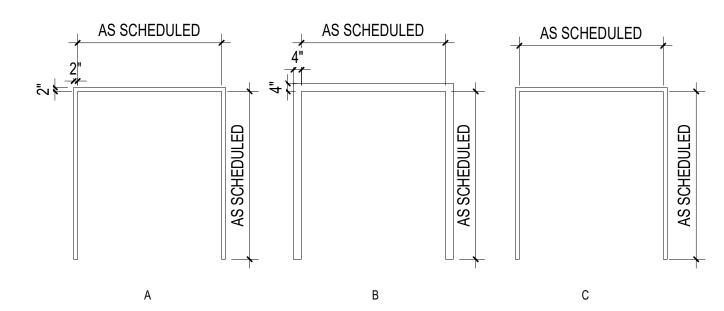
Suite 200 Austin, Texas 78752

TBAE FIRM REGISTRATION NO.: 1452 TBPE FIRM REGISTRATION NO.: F-1416 TBPLS FIRM REGISTRATION NO.: 10065600

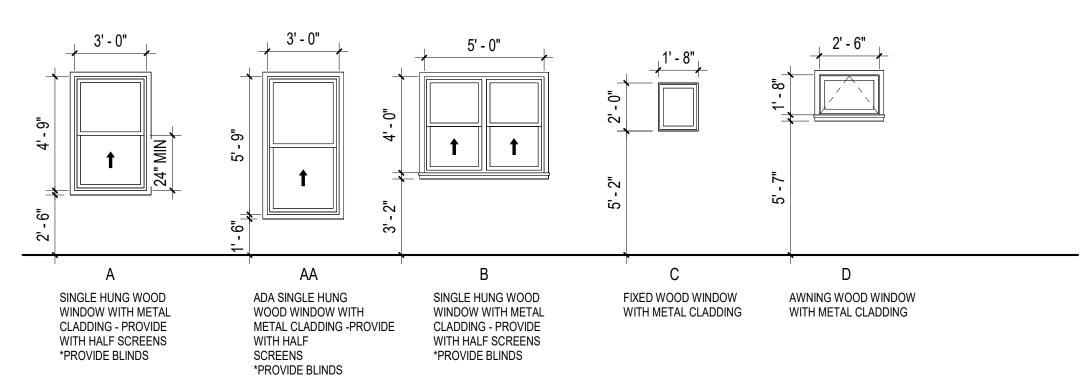
A503

DOOR TYPES

SCALE: 1/4" = 1'-0"



DOOR FRAME TYPES
SCALE: 1/4" = 1'-0"



WINDOW TYPES SCALE: 1/4" = 1'-0"

ROOM FINISH LEGEND

FLOOR FINISH

SC-1: CONCRETE SEALER | PROSOCO | CONSOLIDECK LS CT-2 : CERAMIC TILE | DALTILE | SYNCHRONIC | GREY SY33 | 12X24 TEXTURED

GT-2 : SANDED GROUT | POLYBLEND PLUS #647 BROWN VELVET RA-1: 2.5" WHEELED TRAFFIC ACCESSORY | JOHNSONITE | CTA-283-P | TOAST 283

PT-2A: SHERWIN WILLIAMS SW7036 | ACCESSIBLE BEIGE | EG-SHEL PT-1B: SHERWIN WILLIAMS SW7009 | PEARLY WHITE | EG-SHEL | PRE CATALYZED WATER BASED EPOXY

PT-3: SHERWIN WILLIAMS SW9174 | MOTH WING | SEMI GLOSS

PT-4: SHERWIN WILLIAMS SW6108 | LATTE | MATCH EXISTING PT-5 : SHERWIN WILLIAMS SW7041 | VAN DYKE BROWN | LOW LUSTRE

PT-6: SHERWIN WILLIAMS SW2816 | ROCKWOOD GREEN CT-1: CERAMIC TILE | DALTILE | STAGECRAFT | MATTE ARCTIC WHITE | 0790 | 3X12

GT-1: UNSANDED GROUT | POLYBLEND PLUS #640 ARCTIC WHITE TR-1 : TILE TRIM PIECE | DALTILE | STAGECRAFT | MATTE ARCTIC WHITE | JOLLY | 1/2 X 12

RB-1: 4" RUBBER BASE WITH TOE | JOHNSONITE | CTA-283-P | TOAST 283 ST-2 : CABOT | SEMI-SOLID | MATCH EXISTING CONFERENCE CENTER

CT-2-CB: CERAMIC TILE | DALTILE | COVE BASE | SYNCHRONIC | GREY SY33 | 6X12 TEXTURED

ST-1: | CABOT | SEMI-SOLID | MATCH EXISTING CONFERENCE CENTER ST-2: CABOT | SEMI-SOLID | MATCH EXISTING CONFERENCE CENTER PT-1A: SHERWIN WILLIAMS SW7009 | PEARLY WHITE | FLAT

PT-1B: SHERWIN WILLIAMS SW7009 | PEARLY WHITE | EG-SHEL | PRE CATALYZED WATER BASED EPOXY

WB-1: WINDOW BLINDS | HUNTER DOUGLAS | 2" FAUX WOOD BLINDS | MOCHA 936

CASEWORK FINISHES COUNTERTOPS

SSM-1: SOLID SURFACE | FORMICA | RIVER ROCK MOSAIC 656

MILLWORK FINISH ST-3: STAIN | CLEAR

CABINETRY HARDWARE

			DOOR				FRAME				
DOOR NUMBER	TYPE	WIDTH	HEIGHT	MATERIAL	FINISH	TYPE	MATERIAL	FINISH	FIRE RATING (MIN)	HARDWARE SET	COMMENTS
101	BB	6' - 0"	7' - 0"	WD/ALUM	PAINT	С	WD/ALUM	PAINT	- ,	714	
102	BB	6' - 0"	7' - 0"	WD/ALUM	PAINT	С	WD/ALUM	PAINT	-	714	
103	Α	3' - 0"	7' - 0"	WD	STAIN	С	WD	STAIN	-	207	
104	Α	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	-	341	
105	Α	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	-	341	
106		5' - 0"	6' - 0"								
107	Α	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	-	207	
108	Α	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	-	201	
109	BB	6' - 0"	7' - 0"	WD/ALUM	PAINT	С	WD/ALUM	PAINT	-	714	
110	BB	6' - 0"	7' - 0"	WD/ALUM	PAINT	С	WD/ALUM	PAINT	-	714	
111	AA	6' - 0"	7' - 0"	H.MTL	PAINT	Α	H.MTL	PAINT	-	214	
112	-	5' - 0"	7' - 0"	-	-	В	WD	STAIN		002	CASED OPENING
E101	Α	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	20 MIN	U907	
E102	A	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	20 MIN	U907	
E103	Α	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	20 MIN	U907	
E104	A	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	20 MIN	U907	
E105	Α	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	20 MIN	U907	
E106	A	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	20 MIN	U907	
E107	A	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	20 MIN	U907	
E108	Α	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	20 MIN	U907	
E109	A	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	-	343S	
E110	A	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	-	343S	
E111	A	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	_	207	
E112	A	3' - 0"	7' - 0"	WD	STAIN	В	WD	STAIN	_	343S	
E113	В	3' - 0"	7' - 0"	H.MTL	PT	Α	H.MTL	PT	-	715	
E114	В	3' - 0"	7' - 0"	H.MTL	PT	Α	H.MTL	PT	-	715	
W101	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	U907	
W102	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	U907	
W103	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	U907	
W104	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	U907	
W105	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	U907	
W106	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	U907	
W107	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	U907	
W108	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	U907	
W109	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	343S	
W110	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	343S	
W111	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	207	
W112	A	3' - 0"	7' - 0"		STAIN	В			20 MIN	343S	
W113	В	3' - 0"	7' - 0"		PT	A			20 MIN	715	
W114	В	3' - 0"	7' - 0"		PT	A			20 MIN	715	

					ROC	OM FINISH SC	HEDULE		
ROOM NO.	NAME	FLOOR	WALL BASE	WALL	CEILING	MILLWORK	COUNTERTOP	WINDOWS	COMMENTS
101	GATHERING	SC-1	ST-2	PT-2A, PT-3, ST-2, ST-4	ST-1,ST-2	-	-	-	
102	KITCHEN	SC-1	ST-2	PT-2A, PT-3, ST-2, ST-4	PT-1A	ST-3	SSM-1		
103	PANTRY	SC-1	RB-1	PT-2A	PT-1A	-	-	-	
104	RESTROOM	CT-2, GT-2 RA-1	CT-2-CB	CT-1, GT-1, TR-1, PT-1B,	PT-1B	-	-	-	CONTRACTOR TO UTILIZE ALL TILE ACCESSORIES PROVIDED BY MANUFACTURER
105	RESTROOM	CT-2, GT-2, RA-1	CT-2-CB	CT-1, GT-1, TR-1, PT-1B	PT-1B	-	-	-	CONTRACTOR TO UTILIZE ALL TILE ACCESSORIES PROVIDED BY MANUFACTURER
106	MECH	SC-1	RB-1	PT-2A	PT-1A	-	-	-	
107	STORAGE	SC-1	RB-1	PT-2A	PT-1A	-	-	-	
108	JANITOR	SC-1	RB-1	PT-2A	PT-1A	-	-	-	
109	HALLWAY	SC-1	ST-2	PT-2A, PT-3	PT-1A	-	-	-	
E101	BEDRROM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
E102	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3		WB-1	
E103	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
E104	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
E105	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
E106	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
E107	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
E108	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
E109	BATHROOM	CT-2, RA-1	CT-2-CB	CT-1, PT-2B	PT-1B	ST-3	SSM-1		CONTRACTOR TO UTILIZE ALL TILE ACCESSORY PIECES PROVIDED BY MANUFACTURE
E110	BATHROOM	CT-2, RA-1	CT-2-CB	CT-1, PT-2B	PT-1B	ST-3	SSM-1		CONTRACTOR TO UTILIZE ALL TILE ACCESSORY PIECES PROVIDED BY MANUFACTURE
E111	JAN/MECH	SC-1	RB-1	PT-2A, FRP-1	PT-1A	-	-		PTOVIDE 3'(W) X 6'(H) FRP AT BOTH SIDES OF MOP SINK WALLS
E112	BATHROOM	CT-2, RA-1	CT-2-CB	CT-1, PT-2B	PT-1B	ST-3	SSM-1		CONTRACTOR TO UTILIZE ALL TILE ACCESSORY PIECES PROVIDED BY MANUFACTURE
E113	CORRIDOR	SC-1	ST-2	PT-2A, PT-3, ST-2, ST-4	PT-1A	-	-		
W101	BEDRROM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
W102	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
W103	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
W104	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
W105	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
W106	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
W107	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
W108	BEDROOM	SC-1	ST-2	PT-2A	PT-1A	ST-3	-	WB-1	
W109	BATHROOM	CT-2, RA-1	CT-2-CB	CT-1, PT-2B	PT-1B	ST-3	SSM-1	-	CONTRACTOR TO UTILIZE ALL TILE ACCESSORY PIECES PROVIDED BY MANUFACTURI
W110	BATHROOM	CT-2, RA-1	CT-2-CB	CT-1, PT-2B	PT-1B	ST-3	SSM-1	-	CONTRACTOR TO UTILIZE ALL TILE ACCESSORY PIECES PROVIDED BY MANUFACTURI
W111	JAN/MECH	SC-1	RB-1	PT-2A,FRP-1	PT-1A	-	-	-	PROVIDE 3'(W) X 6'(H) FRP AT BOTH SIDES OF MOP SINK WALLS
W112	BATHROOM	CT-2, RA-1	CT-2-CB	CT-1, PT-2B	PT-1B	ST-3	SSM-1	-	CONTRACTOR TO UTILIZE ALL TILE ACCESSORY PIECES PROVIDED BY MANUFACTUR
W113	CORRIDOR	SC-1	ST-2	PT-2A, PT-3, ST-2, ST-4	PT-1A	_	-	_	

Type Mark	Description	Manufacturer	Model	Count	Comments
DR-1	DRYER	GENERAL ELECTRIC	GFD43ESSMWW	1	
DW-1	DISHWASHER	BOSCH	SGE53X55UC	1	
FR-1	FREEZER	WHIRLPOOL SIDEKICKS	WSZ57L18DM	1	
G20	Snap Flange (1-1/2" O.D) Peened - Wall to Floor, 18" x 33" (3875-P) (3875-P)	American Specialties Inc.	3875-P	2	DRINKING FOUNTAIN CANE DETECTION
G36	36 inch ADA compliant grab bar	KOHLER Co.	K-10544-S	1	
G42	42 inch ADA compliant grab bar	KOHLER Co.	K-10545-S	1	
IM-1	ICE MACHINE - UNDER COUNTER	SPT	IM-600US	1	
IM-2	ICE MACHINE - FREE STAINDING	MAINTOWOC ICE	ID-0302A	1	
M1	MIRROR			1	
2	SURFACE MOUNTE TOILET PAPER			1	
RA-1	OVEN RANGE	VERONA	VDFSEE365SS	1	
RF-1	REFRIGERATOR	WHIRLPOOL SIDEKICKS	WSR57R18DM	1	
S1	SURFACE MOUNTED SOAP DISPENSE	Bobrick Washroom Equipment, Inc.	B-2111	1	
WA-1	WASHER	GENERAL ELECTRIC	GFW430SSMWW	1	

Design Group

Suite 200 Austin, Texas 78752 512.453.1734

305 East Huntland Drive 512.453.0767

TBAE FIRM REGISTRATION NO.: 1452 TBPE FIRM REGISTRATION NO.: F-1416 TBPLS FIRM REGISTRATION NO.: 10065600 PARKS 8 WILDLIFE

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TEXAS

TX FIRM NO. 1452

AND BUNKHOUSE GUS ENGELING WMA HERING LODGE AND BUNKHC PROJECT NUMBER:

DATE: April 3, 2024 DESIGNED BY: MW DRAWN BY: EB REVIEWED BY: OH

SHEET TITLE

SHEET NUMBER

SCHEDULES

A601

WITH

WITHOUT

GENERAL STRUCTURAL NOTES

01-GENERAL

. THIS PROJECT SHALL MEET ALL REQUIREMENTS OF THE 2021 INTERNATIONAL

ROOF= 20 PSF ROOF SNOW LOAD FOR GROUND SNOW LOAD PF = 5 PSF FLOOR LOAD

COMMON LIGHT STORAGE 125 PSF 3.DESIGN WIND LOADS

RISK CATEGORY: BASIC WIND SPEED - 110 MPH

BUILDING CODE.

2. DESIGN LIVE LOADS

WIND EXPOSURE C APPLICABLE INTERNAL COEFFICIENT (GCpi): INCLUDED MWFRS (MAIN WIND FORCE RESISTING SYSTEM): PER ASCE 7-10 COMPONENTS AND CLADDING: (SUBJECT TO ASCE 7-10 ASD 0.6 FACTOR) DIMENSION A = 5'-0"

EXTERIOR WALLS (INWARD OR OUTWARD) 55 PSF (AREA = 10 SF) ZONE 5 65 PSF (AREA = 10 SF) ROOF (INWARD OR OUTWARD ZONE 1 50 PSF (AREA = 10 SF ZONE 2 85 PSF (AREA = 10 SF) ZONE 3 125 PSF (AREA = 10 SF) ZONE 2 (OH) =125 PSF (AREA = 10 SF) 201 PSF (AREA = 10 SF) ZONE 3 (OH) =

4.FOUNDATIONS ARE DESIGNED TO MEET THE RECOMMENDATIONS CONTAINED IN A REPORT PREPARED FOR THIS PROJECT BY TERRACON GEOREPORT DATED **NOVEMBER 16, 2018**

5.FOUNDATION DESIGN IS BASED ON AN ALLOWABLE BEARING VALUE OF 2000 PSF. AT A MINIMUM DEPTH OF 2 FEET BELOW FINAL GRADES 6.ALL FILL MATERIAL SHALL HAVE A MINIMUM PLASTICITY INDEX OF 5 AND A MAXIMUM PLASTICITY INDEX OF 15 WITH A LIQUID LIMIT OF 30 OR LESS, AND SHALL BE COMPACTED TO A MINIMUM DENSITY OF 95% OF ASTM D698 (STANDARD PROCTOR) AT ABOVE OPTIMUM MOISTURE CONTENT UNLESS OTHERWISE NOTED. BRACE ALL GRADE BEAMS AS REQUIRED DURING COMPACTION OPERATION.

.GRADE BEAM DEPTHS SHALL BE INCREASED, IF NECESSARY BY FIELD CHANGE, TO EXTEND A MINIMUM OF 6" BELOW FINISH GRADE/FROST LINE. 8.EXPOSED FACES OF GRADE BEAMS SHALL BE RUBBED WITHIN 24 HOURS AFTER POURING.

9.STRUCTURAL DRAWINGS MAY NOT BE USED AS SHOP DRAWINGS 10. ALL SHELF L'S AND BOLTS TO BE GALVANIZED. ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE ELEMENTS, OR SOIL SHALL BE GALVANIZED (G60) OR TREATED WITH HIGH PERFORMANCE PAINT PER THE SPECIFICATIONS. VERIFY THE SIZE AND LOCATION OF ALL MECHANICAL AND ELECTRICAL OPENINGS, AND VERIFY NO CONFLICT WITH STRUCTURAL ELEMENTS. CONSULT STRUCTURAL ENGINEER IF LOCATIONS OR WEIGHTS OF ROOF TOP UNITS OR OTHER MECHANICAL EQUIPMENT DIFFER FROM THOSE SHOWN ON PLAN 2.PROVIDE ALL CONCRETE PADS, TRAPS, BASINS, ETC., SHOWN ON MECHANICAL

DRAWINGS WHERE INDICATED TO BE SUPPLIED BY GENERAL CONTRACTOR. 13. VERIFY ALL DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL AND STRUCTURAL DRAWINGS AND EXISTING CONDITIONS. CONTRACTOR IS TO NOTIFY ARCHITECT AND OBTAIN CLARIFICATION IN WRITING PRIOR TO PROCEEDING.

02-CONCRETE

ALL CONCRETE AND METAL REINFORCEMENT SHALL BE FABRICATED AND PLACED IN CONFORMITY WITH THE "ACI STANDARD BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318-11)

2. POURED IN PLACE CONCRETE SHALL STRICTLY ADHERE TO THE PROPORTIONS, ESTABLISHED IN DESIGN MIXES, CONSISTING OF THE ACTUAL MATERIALS TO BE USED DURING CONSTRUCTION, FOR THE SEVERAL STRENGTHS AND USES INTENDED. THESE DESIGN MIXES ARE TO BE PREPARED BY A PRE-QUALIFIED LABORATORY, AND THE MATERIALS AND TEST RESULTS ARE TO BE REVIEWED BY THE ENGINEER AND OWNER'S LAB REPRESENTATIVE PRIOR TO USE.

3. POURED IN PLACE CONCRETE IS TO BE NORMAL WEIGHT AND IS TO DEVELOP COMPRESSIVE STRENGTH F'C AT 28 DAYS PER CLASS OF CONCRETE SCHEDULE

4. UNLESS OTHERWISE NOTED, METAL REINFORCEMENT FOR POURED IN PLACE CONCRETE IS TO BE ASTM A-615, GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. (GRADE 15 FOR COLUMN BARS #1 AND LARGER)

5. VERIFY THE PRESENCE, LOCATION, SIZES AND CORRECTNESS OF ALL OPENINGS, SLAB DEPRESSIONS AND EMBEDMENTS REQUIRED PRIOR TO CONCRETING. NO OPENINGS SHALL BE PERMITTED THROUGH BEAMS OR COLUMNS UNLESS SHOWN ON THE STRUCTURAL DRAWINGS OR REVIEWED BY THE STRUCTURAL ENGINEER.

6.PROPER ACCESSORIES/SUPPORTS ARE TO BE USED AS NOTED AND REVIEWED ON THE SHOP DRAWINGS. ALL REINFORCING TO BE SECURELY AND ACCURATELY HELD IN LOCATIONS SHOWN ON THE PLANS.

7. CONSTRUCTION JOINTS OF ALL TYPES MAY BE USED ONLY WHERE SHOWN ON THE FABRICATOR'S REVIEWED PLACING DRAWINGS.

8.PROVIDE CORNER BARS IN ALL GRADE BEAMS AND WALLS OF SAME SIZE AND SPACING AS ADJACENT BARS, UNLESS OTHERWISE NOTED. 9.PROVIDE STANDARD 90 DEGREE HOOKS ON ALL BARS AT BEAM ENDS. IF BEAM

DEPTH IS PROHIBITIVE, USE STANDARD 180 DEGREE HOOK.

10. BARS SCHEDULED AS CONTINUOUS SHALL BE SPLICED WITH A 'CLASS B LAP' AND SPLICED AS FOLLOWS: TOP BARS AT CENTERLINE OF ANY SPAN. BOTTOM BARS OVER ANY SUPPORT.

11. UNLESS OTHERWISE SHOWN, ALL SLABS AND STEPS ON FILL SHALL BE REINFORCED WITH #4 BARS AT 18" CENTERS EACH WAY, SUPPORTED TWO INCHES FROM THE TOP OF SLAB, LAP 22" AT SPLICES.

12.POUR SLABS ON GRADE IN SECTIONS NOT TO EXCEED 225 SF SEPARATED BY KEYED OR DOWELED CONSTRUCTION CONTROL JOINTS OR SAWED JOINTS. NO DIMENSION IN EITHER DIRECTION IS TO EXCEED 15'-0". CONTRACTOR SHALL PROVIDE PROPOSED LAYOUT OF JOINTS FOR REVIEW BY THE ARCHITECT AND RECEIVE WRITTEN APPROVAL PRIOR TO PLACING ANY SLAB.

13. VERIFY DEPTHS OF PIERS BEFORE PIER STEEL IS CUT. PIER STEEL SHALL BE DELIVERED TO THE JOB SITE IN STANDARD 60'-0" LENGTHS AND CUT AS REQUIRED. CLASS 'B' LAPS WILL BE ALLOWED IN THE PIER STEEL. NO MORE THAN 50% OF BARS ARE TO BE LAPPED IN ANY 5'-0" LENGTH OF THE PIER.

03-EARTH WORK

ALL EARTH WORK AND SITE PREPARATION SHALL BE PERFORMED IN STRICT ACCORDANCE WITH THE SPECIFICATIONS AND THE GEOTECHNICAL REPORT. ALL FOUNDATION EXCAVATIONS SHALL BE OBSERVED AND APPROVED BY THE

2.ANY EXISTING FILLS OR UNSUITABLE SOILS AS DETERMINED BY THE GEOTECH SHALL BE EXCAVATED AND REPLACED WITH PROPERLY COMPACTED SOIL.

3.PROPOSED BUILDING SHOULD BE UNDERCUT TO A MINIMUM DEPTH OF 18" BELOW EXISTING GRADES OR 30" BELOW BOTTOM OF BUILDING SLAB, WHICHEVER EXTENDS TO A LOWER LEVEL. A MINIMUM OF 30" OF "NON-EXPANSIVE" SELECT FILL SHOULD BE PLACED WITHIN THE BUILDING FOOTPRINT

4.UNDERCUTTING AND PLACEMENT OF "NON-EXPANSIVE" STRUCTURAL FILL SHOULD EXTEND A MINIMUM OF 5 FEET BEYOND BUILDING FOOTPRINT.

5. REFER TO GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION.

04-STRUCTURAL STEEL

?.THE MAIN FRAME IS TO BE A SIMPLE SYSTEM TYPE, WITH CONNECTIONS AS SHOWN

. ALL STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN CONFORMITY WITH THE REQUIREMENTS OF THE LATEST EDITION, AISC "MANUAL OF STEEL

ON AND NOTED ON THE DRAWINGS. UNLESS OTHERWISE SHOWN OR NOTED CONNECTIONS AT NONCONTINUOUS JOINTS ARE TO BE DETAILED IN CONFORMANCE WITH THE 14TH EDITION, AISC "MANUAL OF STEEL CONSTRUCTION". CONNECTIONS SHALL BE DESIGNED FOR THE END REACTION OF BEAMS AS TABULATED IN PART 3 (ASD) OR AS SHOWN ON DETAILS OR NOTED ON PLANS. FIELD CONNECTIONS AT NONCONTINUOUS JOINTS ARE TO BE BOLTED OR WELDED, SHOP CONNECTIONS TO BE WELDED OR BOLTED.

3.EXCEPT AS SHOWN OR NOTED, ALL STRUCTURAL SHAPES ARE TO BE ASTM A-992, GR. 50 MATERIAL. TUBE STEEL TO BE FY = 46 KSI, ASTM A500, GRADE B. STRUCTURAL PLATES AND MISCELLANEOUS STEEL SHALL BE ASTM A36 MATERIAL ALL WELDS TO BE A MINIMUM OF 3/16" FILLET CONTINUOUS WELDS UNLESS NOTED OTHERWISE

HWELDS SHALL BE MADE ONLY BY PREQUALIFIED WELDERS PER AWS DIJ CERTIFIED WITHIN THE LAST 12 MONTHS. ALL WELDS SHALL BE MADE USING ETO

5.ERECTION TOLERANCES SHALL CONFORM TO THE AISC CODE OF STANDARD PRACTICE EXCEPT THAT THE MAXIMUM TOTAL DISPLACEMENT AT THE CENTERLINES OF ANY COLUMN FROM THE ESTABLISHED COLUMN CENTERLINES SHALL NOT EXCEED 1/2 INCH AT ANY LEVEL.

6.A RECOGNIZED TESTING LABORATORY, REVIEWED BY THE STRUCTURAL ENGINEER SHALL BE ENGAGED FOR THE PURPOSE OF SHOP AND FIELD INSPECTION. THE LABORATORY SHALL ASSURE THAT APPROVED WELDING MATERIALS AND SEQUENCES ARE USED, AND SHALL CERTIFY IN WRITING THAT THE QUALITY AND STRENGTH REQUIREMENTS OF ALL CONNECTIONS HAVE BEEN ATTAINED AND THAT ALL TOLERANCES ARE WITHIN SPECIFIED LIMITS. .PROVIDE BOLTS AND PUNCH HOLES IN STRUCTURAL AND MISCELLANEOUS METAL

FOR ATTACHMENTS OF WOOD NAILERS AS REQUIRED ON THE ARCHITECTURAL, MECHANICAL OR STRUCTURAL DRAWINGS. 8.STRUCTURAL AND MISCELLANEOUS METAL IS TO BE CLEANED PRIOR TO SHOP

PAINTING AND SHIPMENT IN ACCORD WITH THE STRUCTURAL STEEL PAINTING COUNCIL REQUIREMENTS FOR THE FOLLOWING GRADE: POWER TOOL 9. PROVIDE MINIMUM 1/4 INCH CAP PLATE AT ENDS OF ALL TUBE STEEL MEMBERS, UNLESS OTHERWISE NOTED

10. ALL EXTERIOR EXPOSED STEEL SHALL BE GALVANIZED UNLESS NOTED AND/OR SPECIFIED OTHERWISE.

ALL MATERIAL TO BE #2 KD SOUTHERN PINE, OR BETTER, WITH A MINIMUM F6 = 1400 PSI AND E = 1,600,000 PSI, UNLESS NOTED OTHERWISE.

2. ALL TRUSSES TO BEAR DIRECTLY OVER A STUD. ADD ADDITIONAL STUDS IF

3.PROVIDE JOIST HANGERS AT FLUSH CONNECTIONS AND WHERE RAFTERS OR JOISTS DO NOT BEAR ON PLATES.

4.PROVIDE ONE JAMB STUD PLUS STANDARD STUD FOR HEADER SPANS 4'-6' OR LESS AND TWO JAMB STUDS PLUS STANDARD STUD FOR SPANS OVER 4'-6', UNLESS SHOWN OTHERWISE.

5.PROVIDE DOUBLE JOISTS UNDER ALL PARTITIONS PARALLEL TO FLOOR JOIST

6.ALL HEADERS TO BE: SEE SCHEDULE, UNLESS SHOWN OTHERWISE ON PLAN.

T.EXECUTION OF WOOD FRAMING SHALL CONFORM TO THE MANUAL FOR HOUSE FRAMING, NATIONAL LUMBER ASSOCIATION.

8.ALL SIMPSON (OR APPROVED EQUAL) WOOD FRAMING CONNECTORS SHALL BE INSTALLED WITH NAILS PER CATALOG. USE MINIMUM NUMBER PER CONNECTION UNLESS NOTED OTHERWISE.

06-PRE-FABRICATED/ENGINEERED **WOOD TRUSSES**

TRUSS JOISTS AT FLOORS AND ROOF TRUSSES TO BE DESIGNED FOR LIVE LOADS SHOWN BELOW PLUS ALL DEAD LOADS. FABRICATED TRUSSES ARE CONSIDERED A SYSTEM AND ARE TO BE DESIGNED BY AND ARE THE RESPONSIBILITY OF THE TRUSS FABRICATOR AND THEIR REGISTERED ENGINEER. THE TRUSS MANUFACTURER SHALL PROVIDE A SET OF THE CONSTRUCTION DOCUMENTS TO THE TRUSS DESIGN ENGINEER WHO SEALS THE CALCULATIONS FOR REVIEW AND REFERENCE IN THEIR DESIGN. TRUSS CALCULATIONS, TRUSS LAYOUT/PLAN, AND SHOP DRAWINGS ARE TO BE SEALED BY A REGISTERED PROFESSIONAL ENGINEER.

2. LOADS: a.ROOF LIVE = 20 PSF b.ROOF LIVE MEP = 50 PSF

3.DEFLECTIONS OF TRUSS FRAMING SHALL BE LIMITED TO THE FOLLOWING: a.FLOOR LIVE LOAD = L/480 b.FLOOR TOTAL LOAD = L/260

c.ROOF LIVE LOAD = L/360d.ROOF TOTAL LOAD = L/240

e.EFFECT OF E-VARIABLE SHALL BE CONSIDERED IN DEFLECTION CALC BY REDUCING PUBLISHED (E) BY 25%. f. A CREEP FACTOR SHALL BE CONSIDERED IN THE TOTAL DEFLECTION CALCULATION OF Ker = 1.5, MULTIPLIED BY THE INITIAL DEAD LOAD DEFLECTION.

4. TRUSS MANUFACTURER TO FIELD VERIFY ALL DIMENSIONS WITH ARCHITECTURAL PLANS AND CONFIRM DURING INSTALLATION. TRUSS PROFILE DIMENSIONS AND CONFIGURATION OF TOP AND BOTTOM CHORDS SHALL BE TAKEN FROM ARCHITECTURAL DOCUMENTS AND SECTIONS.

5.TRUSS MANUFACTURER/DESIGNER SHALL DESIGN/PROVIDE FOR AND SHOW ON SHOP DRAWINGS ALL ITEMS LISTED IN IBC SECTION 2303.4 WITH THE FOLLOWING CLARIFICATION: ALL PERMANENT BRIDGING/BRACING REQUIRED FOR LATERAL BRACING (BOTTOM CHORD OR OTHERWISE) OF WOOD TRUSSES PER IBC 2303.4.1, INCLUDING BRIDGING CONNECTION DETAILS TO TRUSSES AND MAIN SUPPORT STRUCTURE SHALL BE SHOWN AND SPECIFIED BY TRUSS MFR. BRACING FOR PIGGYBACK TRUSSES IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND THEIR ENGINEER TO DESIGN, SEAL AND SPECIFY

07-WOOD SHEATHING

ROOF SHEATHING SHALL BE 23/32" MINIMUM THICKNESS TONGUE AND GROOVE APA RATED STRUCTURAL I OR EXPOSURE I WITH A MINIMUM SPAN RATING OF 32/16 LONG DIMENSION SHALL BE PERPENDICULAR TO SUPPORTS AND BE CONTINUOUS OVER TWO OR MORE SPANS WITH A STAGGERED JOINT LAYOUT. ROOF SHEATHING SHALL BE NAILED WITH 10d COMMON NAILS AT 6" ON CENTER AT PANEL EDGES AND AT 12" ON CENTER AT ALL INTERMEDIATE SUPPORTS. ALL FASTENERS SHALL BE DRIVEN SO THAT THEIR HEAD OR CROWN IS FLUSH WITH THE SURFACE OF THE

2.5HEARWALL SHEATHING SHALL BE 15/32" MINIMUM THICKNESS APA RATED STRUCTURAL I OR EXPOSURE I WITH A MINIMUM SPAN RATING OF 32/16. LONG DIMENSION SHALL BE PERPENDICULAR TO SUPPORTS AND BE CONTINUOUS OVER TWO OR MORE SPANS WITH A STAGGERED JOINT LAYOUT. SHEARWALL SHEATHING SHALL BE NAILED WITH 100 COMMON NAILS ACCORDING TO THE SHEARWALL SCHEDULE. ALL FASTENERS SHALL BE DRIVEN SO THAT THEIR HEAD OR CROWN IS FLUSH WITH THE SURFACE OF THE SHEATHING.

3.PROVIDE 4'-Ø" WIDTH OF PLYWOOD SHEATHING MINIMUM AT ALL BUILDING CORNERS. NAIL TO SUPPORTS WITH 100 NAILS AT 6" ON CENTER AT PANEL EDGES, AND AT 6" ON CENTER AT ALL INTERMEDIATE SUPPORTS.

08-HEAVY TIMBER/ENGINEERED **LUMBER CONSTRUCTION**

. ALL LAMINATED BEAMS TO HAVE A MINIMUM Fb = 2000 PSI FV = 90 Fc+ = 245 PSI AND E = 1,600,000 PSI.

2. ALL GLULAM OR LAMINATED BEAMS TO HAVE MINIMUM MATERIAL PROPERTIES EQUAL TO OR BETTER THAN THE 24F-V3 LAYUP COMBINATION OR Fb = 2400 PSI. FV = 270 PSI, Fc += 740 PSI AND E = 1,800,000.

3. CONNECTIONS BETWEEN MEMBER OF HEAVY TIMBER TRUSSES, GLULAM TRUSSES OR LARGE GLULAM FRAMES/BEAMS SHALL BE DESIGN VERIFIED BY MANUFACTURER'S PROFESSIONAL ENGINEER FOR LOADS SPECIFIED

4.ALL COMPOSITE PSL BEAMS SHALL HAVE MINIMUM MATERIAL PROPERTIES EQUAL TO OR BETTER THAN ILEVEL 2.0E PARALLAM PSL BEAMS OR Fb = 2900 PSI, Fv = 290 PSI, Fc+ = 750 PSI AND E = 2,000,000 PSI.

5.ALL COMPOSITE LVL BEAMS SHALL HAVE MINIMUM MATERIAL PROPERTIES EQUAL TO OR BETTER THAN ILEVEL 1.9E MICROLLAM LVL OR Fb = 2600 P91, Fv = 285 PSI, Fc+ = 750 AND E = 1,900,000 PSI.

6.ALL COMPOSITE LSL BEAMS SHALL HAVE A MINIMUM MATERIAL PROPERTIES EQUAL TO OR BETTER THAN ILEVEL 1.55E TIMBERSTRAND LSL OR Fb = 2325 PSI, Fv = 310 PSI, Fc+ = 800 PSI, AND E = 1,550,000 PSI.

I.APPEARANCE OR FINISH OF ALL HEAVY TIMBER, GLULAM, OR COMPOSITE FRAMING SHALL BE VERIFIED AND COORDINATED WITH ARCHITECTURAL REQUIREMENTS.

09-POST INSTALLED ANCHORS

POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING REBAR. ALL POST INSTALLED ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURERS CURRENT PUBLISHED INSTALLATION INSTRUCTIONS (MPII). INSTALLATION OF ADHESIVE ANCHORS EITHER HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED ONLY BY ACI-CRSI CERTIFIED ADHESIVE ANCHOR INSTALLERS.

SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THEN THOSE LISTED BELOW, SHALL BE SUBMITTED TO THE ENGINEER WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER SHOWING THAT THE SUBSTITUTED PRODUCTS WILL ACHIEVE AN EQUIVALENT CAPACITY. UNLESS OTHERWISE NOTED ON THE PLANS, ANCHORS SHALL BE:

1. CONCRETE EXPANSION ANCHORS

a.ALL CONCRETE EXPANSION ANCHORS SHALL MEET THE REQUIREMENTS OF ACI 318, APPENDIX D AND SHALL BE ACCEPTABLE FOR BOTH CRACKED AND UNCRACKED CONCRETE. b. "STRONG-BOLT" BY SIMPSON STRONG-TIE CO. OF PLEASANTON, CA.

c.HILTI KWIK BOLT III EXP. ANCHORS BY HILTI CORP.

d.HILTI UNDERCUT ANCHOR BY HILTI CORP e.POWER-STUD + SD | BY POWERS FASTENERS

f. POWER-STUD + SD 2 BY POWERS FASTENERS

3. CONCRETE AND MASONRY SCREW ANCHORS

2. GROUTED MASONRY EXPANSION ANCHORS a."WEDGE-ALL" BY SIMPSON STRONG-TIE CO. OF PLEASANTON, CA. **b.KWIK BOLT III EXP. ANCHOR BY HILTI CORP** c.POWER-STUF + SD | BY POWERS FASTENERS

a.ALL SCREW ANCHORS SHALL BE INSTALLED IN DRY INTERIOR NON-CORROSIVE ENVIRONMENTS OR FOR TEMPORARY OUTDOOR

6. "TITEN HO" BY SIMPSON STRONG-TIE CO. OF PLEASANTON, CA. c.HUS-H SCREW ANCHOR BY HILTI CORP.

d.WEDGE-BOLT BY POWERS FASTENERS

4. ADHESIVE ANCHORS

a.ADHESIVE ANCHORS SHALL CONSIST OF AN INSERT AND AN ADHESIVE FORMULA SPECIFIED BY THE MANUFACTURER. INSERTS SHALL MEET THE REQUIREMENTS OF ASTM A301, A36, A193-B1 OR F1554 FOR THREADED RODS OR ASTM A615 OR A706 FOR REBAR UNLESS NOTED OTHERWISE.

LOADING. ONLY NON EPOXY BASED ADHESIVES SHALL BE USED WHEN BASE MATERIAL TEMPERATURES ARE BELOW 40 DEG. F. c.PROVIDE SCREEN TUBES OR APPROVED MANUFACTURER APPARATUS FOR

b.ALL ADHESIVE ANCHORS SHALL BE ACCEPTABLE FOR LONG TERM

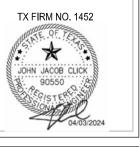
INSTALLATION IN UNGROUTED MASONRY OR BRICK VOID SPACE OR CELLS. d."SET EPOXY" BY SIMPSON STRONG-TIE CO. OF PLEASANTON, CA.

e."ACRYLIC-TIE" BY SIMPSON STRONG-TIE CO. OF PLEASANTON, CA. f. HILTI HIT-HY200 SAFE SET (ESR-3187)BY HILTI CORPORATION Q.HILTI HIT-HYTØ (ESR-2682) BY HILTI CORPORATION

h. HILTI RE 500v3 SAFE SET (ESR-3814) BY HILTI CORPORATION AC 100+ GOLD BY POWERS FASTENERS (FOR CONCRETE AND MASONRY) j. DEWALT/POWERS AC 100+ BY DEWALT (FOR CONCRETE AND MASONRY) k DEWALT/POWERS PURE 110+ BY DEWALT (FOR CONCRETE) 1. DEWALT AC200+ BY DEWALT (FOR CONCRETE)

TEXAS PARKS *N*ILDLIFI

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DATE: April 3, 2024 DESIGNED BY: DRAWN BY: REVIEWED BY:

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ONSTRUCTION

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SHEET TITLE

GENERAL

mwm **Design Group**

CLICK ENGINEERING

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TEXAS REG. NO. F-10142

CLICK DALLAS, TEXAS / P: 214.871.2302

SHEET NUMBER

S1.

OBSERVE PREPARATION OF GROUT SPECIMENS.

MORTAR SPECIMENTS. AND/OR PRISMS

REQUIRED SPECIAL INSPECTIONS - STRUCTURAL STEEL REQUIRED VERIFICATION AND INSPECTION OF STRUCTURAL STEEL (REF TABLE 1705.2.2 OF IBC) REFERENCED STANDARD CONTINUOUS PERIODIC **IBC REFERENCE VERIFICATION AND INSPECTION** MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS IDENTIFICATION MARKINGS TO CONFORM TO ASTM APPLICABLE ASTM MATERIAL STANDARDS SPECIFIED IN THE APPROVED STANDARDS: AISC 360. SECTION A3.3 CONSTRUCTION DOCUMENTS. MANUFACTURER'S CERTIFICATE OF COMPLIANCE APPLICABLE ASTM MATERIAL STANDARDS REQUIRED. 2. INSPECTION OF HIGH-STRENGTH BOLTING SNUG-TIGHT JOINTS. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCH MARKING, TWIST-OFF 1704.3.3 AISC 360. SECTION M2.5 BOLT OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITHOUT MATCH MARKING OR CALIBRATED WRENCH METHODS OF INSTALLATION 3. MATERIAL VERIFICATION OF STRUCTURAL STEEL AND COLD-FORMED STEEL DECK FOR STRUCTURAL STEEL, IDENTIFICATION A. MARKINGS TO CONFORM TO AISC 360. FOR OTHER STEEL, IDENTIFICATION MARKINGS TO ASTM A 6 OR ASTM A 568 9 B. CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS. C. MANUFACTURER'S CERTIFIED TEST REPORTS. 4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS IDENTIFICATION MARKING TO CONFORM TO AWS AISC 360. SECTION A3.5 AND A. SPECIFICATION IN THE APPROVED CONSTRUCTION APPLICABLE AWS A5 DOCUMENTS DOCUMENTS. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED. 5. INSPECTION OF WELDING A. STRUCTURAL STEEL AND/OR COLD-FORMED STEEL DECK: 1) COMPLETE AND PARTIAL PENETRATION GROOVE WELDS 2) MULTIPASS FILLET WELDS **AWS D1.1** 1704.3.1 3) SINGLE-PASS FILLET WELDS > 5/16" 4) PLUG AND SLOT WELDS 5) SINGLE-PASS FILLET WELDS < = 5/16" **AWS D1.3** 6) FLOOR/ROOF DECK WELDS/ PAFs B. REINFORCING STEEL: 1) VERIFICATION OF WELD ABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706 2) REINFORCING STEEL-RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL AWS D1.4 ACI 318: 3.5.2 MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAR WALLS AND SHEAR REINFORCEMENT. 3) SHEAR REINFORCEMENT 4) OTHER REINFORCING STEEL 6. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE A. DETAILS SUCH AS BRACING AND STIFFENING Χ B. | MEMBER LOCATIONS 1704.3.2 Χ APPLICATION OF JOINT DETAILS AT EACH C. LOCATION REQUIRED VERIFICATION AND INSPECTION OF MASONRY CONSTRUCTION (REF TABLE 1.18.3 ACI 530) VERIFICATION AND INSPECTION CONTINUOUS PERIODIC REFERENCED STANDARD IBC REFERENCE PROPORTIONS OF SITE MIXED MORTAR, GROUT, AND ACI 530: 1.18 1705.4 ANY PRESTRESSING GROUT FOR BONDED TENDONS COMPLIANCE OF SIZE AND LOCATION OF STRUCTURAL ACI 530: 1.18 1705.4 ELEMENTS. COMPLIANCE OF TYPE, SIZE, AND LOCATION OF ACI 530: 1.18 1705.4 ANCHORS INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION. COMPLIANCE OF PREPARATION, CONSTRUCTION, AND ACI 530: 1.18 1705.4 PROTECTION OF MASONRY DURING COD WEATHER (TEMPERATURE BELOW 40 DEGREES F) OR HOT WEATHER (TEMPERATURE ABOVE 90 DEGREES F). GRADE AND SIZE OF REINFORCEMENT AND ANCHOR ACI 530: 1.18 1705.4 BOLTS. PRESTRESSING TENDONS AND ANCHORAGES PLACEMENT OF MASONRY UNITS AND CONSTRUCTION ACI 530: 1.18 1705.4 OF MORTAR JOINTS PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ACI 530: 1.18 1705.4 PRESTRESSING TENDONS AND ANCHORS

ACI 530: 1.18

ACI 530: 1.18

ACI 530: 1.18

1705.4

1705.4

1705.4

GENERAL STRUCTURAL NOTES (CONT'D.)

10-CONCRETE MASONRY UNITS

ALL MASONRY TO HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH F'M OF 1900 PSI AT THE AGE OF 28 DAYS.

2. ALL CMU (CONCRETE MASONRY UNITS) TO HAVE A MINIMUM AVERAGE NET-AREA COMPRESSIVE STRENGTH OF 1900 PSI. ASTM C90, LIGHTWEIGHT AGGREGATE.

3. MORTAR SHALL BE TYPE S.

2000 PSI AT 28 DAYS.

4.CONSTRUCTION TO COMPLY WITH ACI 530,1 TMS 602 ASCE 6-13.

5.CONTRACTOR TO BE RESPONSIBLE FOR BRACING ALL MASONRY WALLS DURING CONSTRUCTION AND UNTIL ENTIRE STRUCTURE IS COMPLETE.

6.MASONRY DESIGN IS BASED ON THE CRITERIA THAT INSPECTION IS REQUIRED. INSPECTION SHALL COMPLY WITH SECTION 1704.5.2 AND TABLE 1704.5.1 OF IBC, AND SECTION 1.15.3 AND TABLE 1.15.2 OF ACI/ASCE 530.02

7. GROUT FOR BOND BEAMS AND GROUT FILLED CELLS SHALL MEET PROPORTION REQUIREMENTS OF ASTM C416 AND SHALL HAVE A COMPRESSIVE STRENGTH OF

8.GROUT POURS SHALL NOT EXCEED 4 FEET IN HEIGHT EXCEPT WHERE CLEAN OUTS ARE PROVIDED IN THE BOTTOM COURSE OF THE CELL TO BE FILLED.

#5 AT 8" O/C (FULLY GROUTED CELLS)

9. VERTICAL REINFORCING BARS SHALL BE ASTM A-615, GRADE 60 AND TO BE HELD IN PLACE UNTIL GROUT IS SET. PLACE IN CENTER OF WALL UNLESS NOTED OTHERWISE ON DRAWINGS. REINFORCE CMU WALLS IN GROUTED CELLS AS FOLLOWS, UNLESS NOTED OTHERWISE ON DRAWINGS:

10. STEEL LINTELS FOR OPENINGS GREATER THAN 6'-0" SHALL BE SHORED AT MID SPAN UNTIL THE SUPPORTED MASONRY/BRICK WALL HAS CURED.

PROVIDE A VERTICAL BAR, THAT MATCHES WALL VERTICAL REINFORCING SIZE, ADJACENT TO ALL OPENINGS (DOORS, ETC), AT ENDS OF WALLS, AND ADJACENT TO ALL VERTICAL MASONRY CONTROL JOINTS.

12.CONTROL JOINTS SHALL BE 2'-8" OR HALF THE DOOR WIDTH +/- WHICHEVER IS GREATER FROM DOOR. VERTICAL CONTROL JOINTS SHALL BE LOCATED AT 25'-0" MAX. ON CENTER.

13. FILL ALL CELLS BELOW GRADE WITH GROUT

AND EXPANSION JOINTS.

14. REINFORCE CONCRETE MASONRY UNIT JOINTS WITH LADDER TYPE HOT DIP GALVANIZED COLD-DRAWN STEEL CONFORMING TO ANSI/ASTM A82, REFER SPECS FOR SIZE (W2.8 SIDE RODS WITH W2.8 CROSS RODS AT TORNADO

A. SPACE JOINT REINFORCING AT 16" C/C TYPICAL, SPACE AT 8" C/C AT TORNADO SHELTER AND PARAPETS, UNLESS NOTED OTHERWISE.

B. LAP JOINT REINFORCING 14" AT SPLICES. C. JOINT REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS

D. PROVIDE PREFABRICATED JOINT REINFORCING CORNER PIECES AT ALL WALL CORNERS AND INTERSECTIONS.

11-SUBMITTALS

SUBMITTAL LIST AND SCHEDULE

E. DESIGN CALCULATIONS

THE GENERAL CONTRACTOR SHALL PREPARE A DETAILED LIST AND SCHEDULE OF ALL SUBMITTAL ITEMS TO BE SENT TO THE STRUCTURAL ENGINEER PRIOR TO THE START OF CONSTRUCTION. THIS LIST SHALL BE UPDATED AND REVISED AND KEPT CURRENT AS THE JOB PROGRESSES. THE SUBMITTAL LIST SHALL BE ORGANIZED AS SHOWN BELOW.

A. SHOP DRAWINGS

B. MANUFACTURERS LITERATURE FOR PRODUCTS, ASSEMBLIES, AND HARDWARE C. PRODUCTS, ASSEMBLIES AND HARDWARE

D. PRODUCT CERTIFICATIONS, MILL CERTIFICATES, AND AFFIDAVITS

SHOP DRAWINGS

A. THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS:

CONCRETE MIX DESIGN (Φ) 2) CONSTRUCTION JOINT LOCATIONS IN SLABS ON GRADE

3) MISCELLANEOUS STEEL (TRUSS PLATES) 4) REINFORCING STEEL

5) PRE-ENGINEERED WOOD TRUSSES (*)

(*) ITEMS MARKED THUS SHALL HAVE SHOP DRAWINGS SEALED BY A REGISTERED ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED PER THE PROJECT SPECIFICATIONS

(#) ITEMS MARKED THUS SHALL BE SUBMITTED TO ENGINEER FOR OWNERS RECORD ONLY AND WILL NOT HAVE THE ENGINEERS SHOP DRAWINGS

(Φ) ITEMS MARKED THUS SHALL BE SUBMITTED TO THE OWNERS TESTING AGENCY FOR THEIR REVIEW

B. ALL SHOP DRAWINGS MUST BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL

C. THE OMISSION FROM THE SHOP DRAWINGS OF ANY MATERIAL REQUIRED BY THE CONTRACT DOCUMENTS TO BE FURNISHED SHALL NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY OF FURNISHING AND INSTALLING SUCH MATERIALS, REGARDLESS OF WHETHER THE SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED

. MANUFACTURERS LITERATURE

SUBMIT MANUFACTURERS LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT

DESIGN CALCULATIONS

THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED FOR THE FOLLOWING ITEMS: A. PRE-ENGINEERED WOOD TRUSSES

THE USE OF ELECTRONIC FILES OR REPRODUCTION OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT, AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT MAY OCCUR HEREON.

12-DELEGATED DESIGN

. IN ACCORDANCE WITH THE SPECIFICATIONS, THE ITEMS LISTED BELOW ARE NOT INCLUDED IN THE CONTRACT DOCUMENTS. DESIGN OF THESE ELEMENTS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR, AND SHALL BE DESIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE HAVING JURISDICTION AT THE PROJECT SITE.

A. STEEL CONNECTIONS

B. METAL LADDERS

C. GUARDRAIL AND HANDRAIL SYSTEMS

D. COLD FORMED METAL FRAMING

E. EMBEDDED ASSEMBLIES AND INSERTS, CLAMPS, HANGERS, UNISTRUT, TRAPEZES, ETC. FOR THE SUPPORT OF MEP SYSTEMS.

F. EMBEDDED ASSEMBLIES, INSERTS, AND/OR HANGERS FOR FIRE SUPPRESSION

G. EXCAVATION SUPPORT AND PROTECTION

H. SPECIALTY RETENTION SYSTEMS

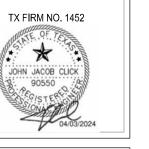
DESIGN OF THE ITEMS LISTED ABOVE SHALL BE IN ACCORDANCE WITH THE GENERAL BUILDING CODE, AND SHALL INCLUDE ALL ATTACHMENTS TO THE STRUCTURE.

REQUIRED VERIFICATION AND INSPECTION OF MASONRY CONSTRUCTION (REF TABLE 1 18 3 ACL 530) - CONTINUED

	(ILLI TABLE 1.10.5 ACI 550) - CONTINOLD						
	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE		
12.	SUBMITTALS FOR MATERIALS USED IN MASONRY CONSTRUCTION INDICATING COMPLIANCE WITH THE CONTRACT DOCUMENTS	-	Х	ACI 530: 1.18	1705.4		
13.	VERIFICATION OF I'm AND I'aac IN ACCORDANCE WITH ARTICLE 1.4B PRIOR TO CONSTRUCTION FOR EVERY 5000 SF OF CONSTRUCTION	X	-	ACI 530: 1.18	1705.4		
14.	VERIFICATIONS OF PROPORTIONS OF MATERIALS IN PREMIXED OR PREBLENDED MORTAR, GROUT, AND PRESTRESSING GROUT AS DELIVERED TO THE SITE	-	Х	ACI 530: 1.18	1705.4		



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DATE: April 3, 2024 DESIGNED BY: DRAWN BY: REVIEWED BY:

SHEET TITLE

GENERAL NOTES

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SHEET NUMBER

S1.2

	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1.	INSPECTION OF REINFORCING STEEL.	-	X	ACI 318: 3.5, 7.1-7.7	1913.4
2.	INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1704.3, ITEM 5b	-	Х	AWS D1.4 ACI318: 3.5.2	
3.	INSPECT BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED	-	-	ACI 318: 8.1.3, 21.2.8	1911.5, 1912.1
4.	INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE	-	X	ACI 318: 3.8.6, 8.1.3, 21.2.8	1912.1
5.	VERIFYING USE OF REQUIRED DESIGN MIX	-	X	ACI 318: CH. 4, 5.2-5.4	1904.2.2, 1913.2, 1913.3
6.	AT TIME OF FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X	-	ASTM C 172 ASTM C 31 ACI 31 8: 5.6, 5.8	1913.10
7.	INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	Х	-	ACI 318: 5.9, 5.10	1913.6, 1913.7, 1913.8
8.	INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	-	Х	ACI 318: 5.11-5.13	1913.9
9.	INSPECTION OF PRESTRESSED/POST-TENSIONED CONCRETE. INSPECT PLACEMENT/QUALITY, LOCATION OF PT TENDONS	-	Х		
	A. APPLICATION OF POST-TENSIONED/PRESTRESSING FORCES	Х	-	ACI 318: 18.20	
	B. GROUTING OF BONDED POST-TENSIONED/PRESTRESSING TENDONS IN SEISMIC-FORCE-RESISTING SYSTEM	X	-	ACI 318: 18.18.4	
10.	ERECTION OF PRECAST CONCRETE MEMBERS INCLUDING INSPECTION OF WELDS TO EMBEDS AND SUPPORTS	-	Х	ACI 318: CH. 16	
11.	VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS	-	Х	ACI 318: 6.2	
12.	INSPECT FORMWORK FOR SHAPE, LOCATIONS, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	Х	ACI 318: 6.1.1	

NOTES

- THE OWNER SHALL EMPLOY QUALIFIED SPECIAL INSPECTORS TO PERFORM INSPECTIONS IN ACCORDANCE WITH THE BUILDING CODE. INSPECTORS SHALL PERFORM ALL DUTIES AND RESPONSIBILITIES AS REQUIRED BY THE BUILDING CODE. JOB SITE VISITS BY THE STRUCTURAL ENGINEER DO NOT CONSTITUTE AND ARE NOT A SUBSTITUTE FOR SPECIAL INSPECTIONS.
- THE SCHEDULE CONTAINS A LIST OF THE SPECIAL INSPECTION ACTIVITIES RELATED TO THE QUALITY ASSURANCE PLAN REQUIRED BY THE BUILDING CODE (IBC CHAPTER 17) FOR THE FABRICATION, ERECTION, AND CONSTRUCTION OF THE STRUCTURAL SYSTEMS AS DESCRIBED IN THE SPECIFICATION AND DRAWINGS FOR THE PROJECT. ALL INSPECTORS SHALL BE QUALIFIED BY TRAINING AND EXPERIENCE FOR THE REQUIRED INSPECTIONS AND TEST PROCEDURES. REFER TO IBC CHAPTER 17 "STRUCTURAL TESTS AND SPECIAL INSPECTIONS," AND SPECIFICATION SECTION 01 45 23 "TESTS AND INSPECTIONS" FOR SPECIFIC TEST PROCEDURES. THE SCHEDULE IS INTENDED TO BE A "STATEMENT OF SPECIAL INSPECTIONS" ACCORDING TO IBC SECTION 1704.
- TESTING AND INSPECTION REPORTS SHALL BE PREPARED FOR EACH INSPECTION ITEM ON A PERIODIC OR DAILY BASIS WHENEVER INSPECTIONS ARE MADE ON THAT ITEM. REPORTS SHALL BE DISTRIBUTED TO THE OWNER, CONTRACTOR, ARCHITECT, STRUCTURAL ENGINEER, AND BUILDING OFFICIAL (IF REQUESTED), FOR THEIR REVIEW, COMMENTS, AND ACTION, AS NEEDED
- ARCHITECTURAL, MECHANICAL, AND ELECTRICAL COMPONENTS REQUIRING SPECIAL INSPECTIONS PER SECTION 1705 OF THE IBC HAVE NOT BEEN LISTED HERE. REFER TO ARCH/MEP FOR SPECIAL INSPECTION REQUIREMENTS FOR THESE COMPONENTS.
- THE OWNER WILL EMPLOY AND PAY FOR THE SPECIAL INSPECTIONS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE AS NEEDED TO ENSURE THESE TESTS AND INSPECTIONS ARE PERFORMED AT THE APPROPRIATE TIMES. THE OWNER'S INSPECTORS WILL PROVIDE INSPECTIONS FOR STEEL AND WOOD FRAMING ITEMS AND A TESTING LABORATORY WILL PROVIDE MATERIALS TESTING, SITE TESTING FOR ALL OTHER ITEMS.

FOR REQUIRED VERIFICATION AND INSPECTION OF STRUCTURAL STEEL, SEE DWG. S1.2

	REQUIRED VERIFICATION	N AND INSF	PECTION	N OF WOOD FRAMING	
	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1.	FABRICATOR CERTIFICATION/ QUALITY CONTROL PROCEDURES FOR WOOD TRUSS MANUFACTURER/ DESIGNER	-	Х	TPI-1, NDS	2303.4.1.4
2.	MATERIAL GRADING	-	X	NDS	2303
3.	CONNECTIONS, LIGHT GAGE METAL CLIPS/ HANGERS/ HOLD-DOWNS OR OTHERWISE. INSPECT INSTALLATION, QUANTITY, CONFIGURATION, NAILS, AND TYPE OF CONNECTOR	-	Х	NDS	2304
4.	DIAPHRAGM AND SHEARWALLS. INSPECT SIZE, CONFIGURATION, BLOCKING, FASTENING OF SHEARWALLS AND DIAPHRAGMS. VERIFY PANEL GRADE AND THICKNESS	-	Х	NDS	2304.6 2304.7
5.	PREFABRICATED WOOD TRUSSES. INSPECT FABRICATION OF WOOD TRUSSES AND INSTALLATION OF PERMANENT TRUSS BRACING.	-	Х	TPI-1, NDS	2303.4.1.4
6.	GENERAL FASTENING	-	X	NDS	2304.9

		BEAM, S (GRADE 6	LAB OR V 0 UNCOATE			
DAD	f'c=30	NORMAL V	WEIGHT CO		f'c=50	00 PSI
BAR SIZE	ld TOP	Id BOTT	ld TOP	Id BOTT	ld TOP	Id BOT
#3	1'-9"	1'-4"	1'-6"	1'-2"	1'-5"	1'-1"
#4	2'-4"	1'-10"	2'-1"	1'-7"	1'-10"	1'-5"
#5	3'-0"	2'-3"	2'-7"	2'-0"	2'-4"	1'-9"
#6	3'-7"	2'-9"	3'-1"	2'-4"	2'-9"	2'-1"
#7	5'-2"	4'-0"	4'-6"	3'-6"	4'-0"	3'-1"
#8	5'-11"	4'-7"	5'-2"	3'-11"	4'-7"	3'-6"
#9	6'-8"	5'-2"	5'-9"	4'-5"	5'-2"	4'-0"
#10	7'-6"	5'-10"	6'-6"	5'-0"	5'-10"	4'-6"
#11	8'-4"	6'-6"	7'-3"	5'-7"	6'-6"	5'-0"

I. TABULATED VALUES ARE APPLICABLE ONLY IF CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED IS NOT LESS THEN 'db', CLEAR COVER IS NOT LESS THAN 'db', AND STIRRUPS OR TIES THROUGHOUT 'Id' IS NOT LESS THAN CODE MINIMUM, OR CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED IS NOT LESS THAN 2*'db' AND CLEAR COVER IS NOT LESS THAN 'db'. FOR OTHER CASES, MULTIPLY TABULATED VALUES BY 1.5.

'TOP' BARS ARE HORIZONTAL REBARS WITH MORE THAN 12" OF FRESH CONCRETE CAST BELOW THE BARS AT THE DEVELOPMENT LENGTH. 3. FOR LIGHT WEIGHT CONCRETE MULTIPLY THE TABULATED VALUES BY 1.3.

BY 60 KSI.

4. FOR EPOXY COATED BARS, MULTIPLY THE TABULATED VALUES BY 1.5 FOR BOTTOM BARS, OR BY 1.3 FOR TOP BARS. 5. FOR REINFORCEMENT OTHER THAN GRADE 60, MODIFY THE TABULATED

VALUES BY THE RATIO OF THE REINFORCEMENT YIELD STRENGTH DIVIDED

	TENSION LAP SPLICES-CLASS "B" TOP BARS AND BOTTOM BARS (GRADE 60 UNCOATED BARS) NORMAL WEIGHT CONCRETE								
BAR	f'c=30	00 PSI	f'c=40	00 PSI	f'c=50	00 PSI			
SIZE	TOP	BOTT	TOP	BOTT	TOP	вотт			
#3	2'-4"	1'-9"	2'-0"	1'-6"	1'-10"	1'-5"			
#4	3'-1"	2'-4"	2'-8"	2'-1"	2'-5"	1'-10"			
#5	3'-10"	3'-0"	3'-4"	2'-7"	3'-0"	2'-4"			
#6	4'-8"	3'-7"	4'-0"	3'-1"	3'-7"	2'-9"			
#7	6'-9"	5'-2"	5'-10"	4'-6"	5'-3"	4'-0"			
#8	7'-9"	5'-11"	6'-8"	5'-2"	6'-0"	4'-7"			
#9	8'-8"	6'-8"	7'-6"	5'-9"	6'-9"	5'-2"			
#10	9'-10"	7'-6"	8'-6"	6'-6"	7'-7"	5'-10"			
#11	10'-11"	8'-4"	9'-5"	7'-3"	8'-5"	6'-6"			

1. TABULATED VALUES ARE APPLICABLE ONLY IF CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED IS NOT LESS THEN 'db', CLEAR COVER IS NOT LESS THAN 'db', AND STIRRUPS OR TIES THROUGHOUT 'Id' IS NOT LESS THAN CODE MINIMUM, OR CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED IS NOT LESS THAN 2*'db' AND CLEAR COVER IS NOT LESS THAN 'db'. FOR OTHER CASES, MULTIPLY TABULATED VALUES BY 1.5. 'TOP' BARS ARE HORIZONTAL REBARS WITH MORE THAN 12" OF FRESH

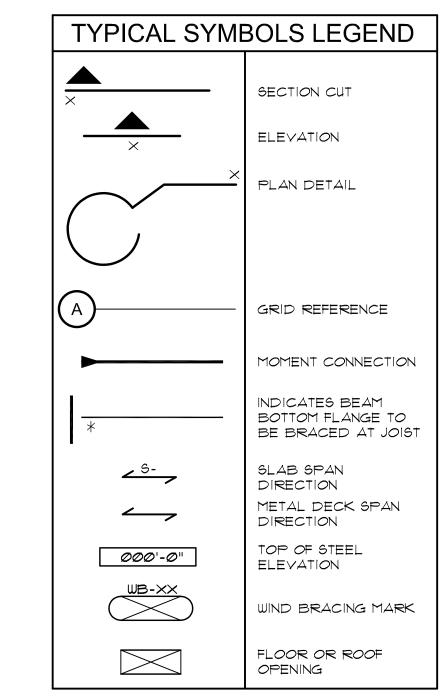
CONCRETE CAST BELOW THE BARS AT THE DEVELOPMENT LENGTH. FOR LIGHT WEIGHT CONCRETE MULTIPLY THE TABULATED VALUES BY 1.3. 4. FOR EPOXY COATED BARS, MULTIPLY THE TABULATED VALUES BY 1.5 FOR

BOTTOM BARS, OR BY 1.3 FOR TOP BARS. 5. FOR REINFORCEMENT OTHER THAN GRADE 60, MODIFY THE TABULATED VALUES BY THE RATIO OF THE REINFORCEMENT YIELD STRENGTH DIVIDED

6. FOR CLASS 'A' SPLICE (PERMITTED ONLY WHEN NOT MORE THAN HALF THE BAR SPLICES ARE STAGGERED BY THE DISTANCE OF THE SPLICE LENGTH) USE SAME AS TENSION DEVELOPMENT LENGTH.

CLASSES	OF CONCRETE S	CHEDU	JLE	
CONCRETE USAGE	MINIMUM COMPRESSIVE STRENGTH (f 'c)	SLUMP (IN)	CONCRETE TYPE	MAXIMUM AGGREGATE SIZE
SHALLOW FOUNDATIONS				
FOOTINGS	4000 PSI AT 28 DAYS	3-5	NWC	1 1/2"
GRADE BEAMS	4000 PSI AT 28 DAYS	3-5	NWC	1"
SLABS				
SLAB ON GRADE	4000 PSI AT 28 DAYS	3-5	NWC	1 1/2"

1. NWC REFERS TO NORMAL WEIGHT CONCRETE HAVING AIR DRY UNIT WEIGHT OF APPROXIMATELY 145 PCF (ASTM 33 AGGREGATE)



ITEMS SHOWN ON THIS SYMBOLS LEGEND LIST MAY OR MAY NOT APPLY TO THIS PROJECT. ITEMS ARE FOR REFERENCE ONLY



mwm Design Group

TEXAS PARKS (WILDLIFE

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BUNKHOU WM 27 GELIN GN 里

DATE: April 3, 2024 DESIGNED BY: DRAWN BY: REVIEWED BY:

ONSTRUCTION

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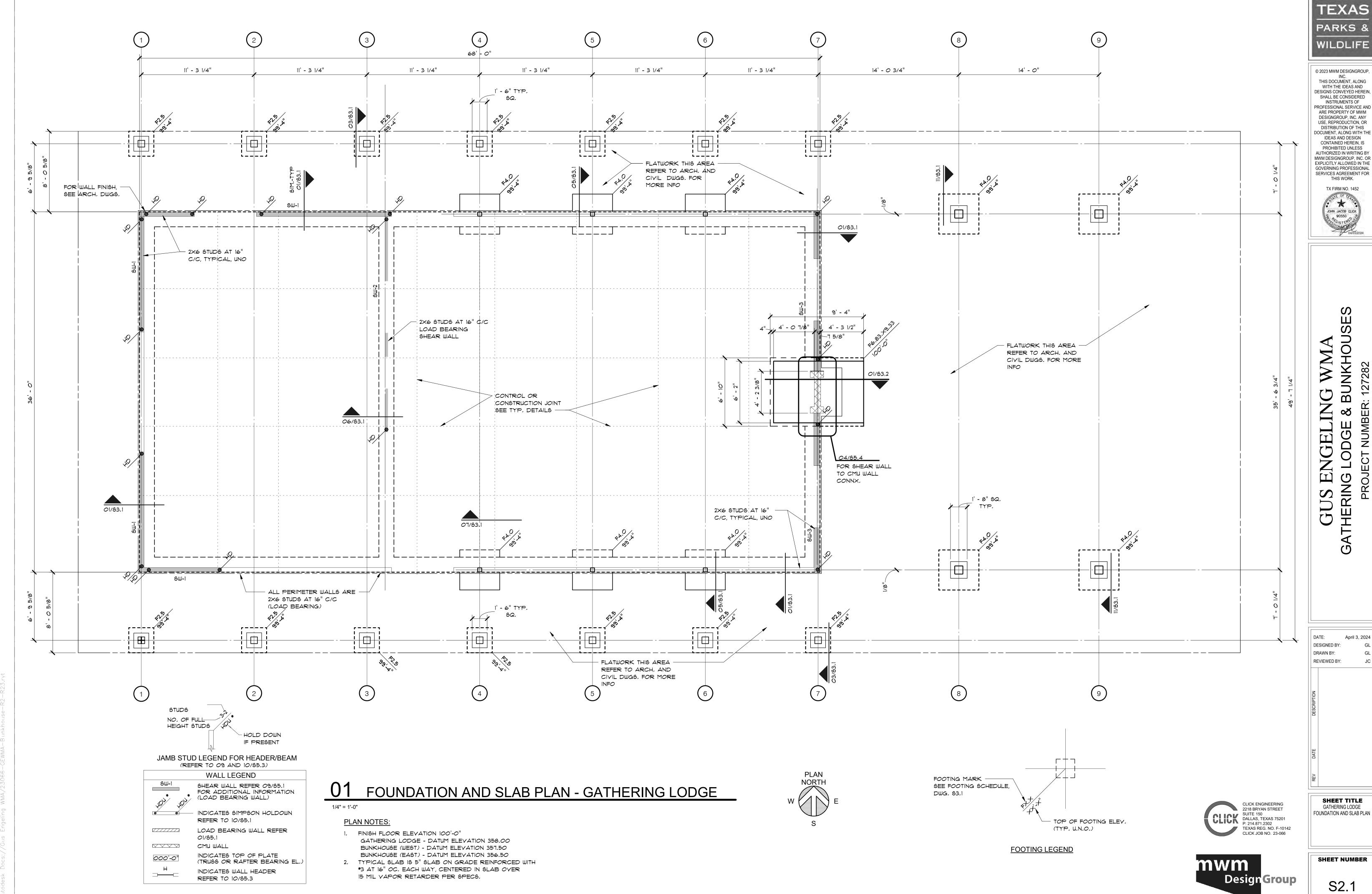
CD

SHEET TITLE

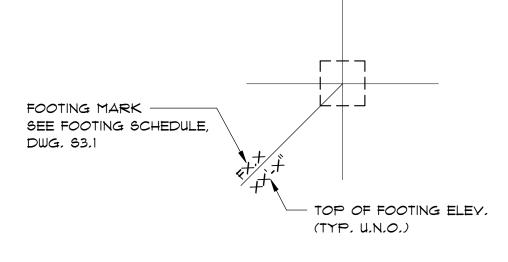
INSPECTION & SCHEDULES

SHEET NUMBER

S1.3



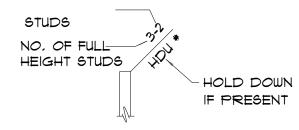
SHEET NUMBER



FOOTING LEGEND

PLAN NOTES:

- 1. FINISH FLOOR ELEVATION 100'-0" GATHERING LODGE - DATUM ELEVATION 358.00 BUNKHOUSE (WEST) - DATUM ELEVATION 357.50 BUNKHOUSE (EAST) - DATUM ELEVATION 356,50
- 2. TYPICAL SLAB IS 5" SLAB ON GRADE REINFORCED WITH #3 AT 16" OC. EACH WAY, CENTERED IN SLAB OVER 15 MIL VAPOR RETARDER PER SPECS.



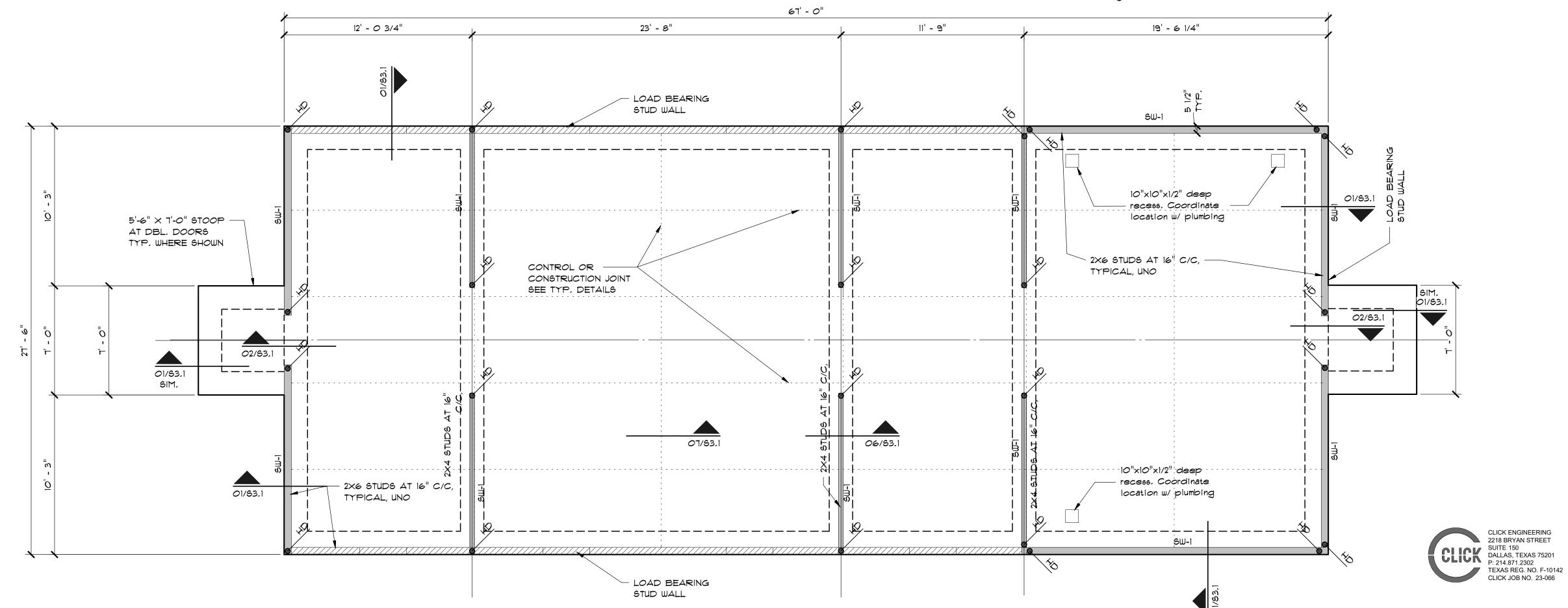
JAMB STUD LEGEND FOR HEADER/BEAM (REFER TO 09 AND 10/65.3)

	WALL LEGEND
SW-1	SHEAR WALL REFER 09/65.1 FOR ADDITIONAL INFORMATION (LOAD BEARING WALL)
	INDICATES SIMPSON HOLDOWN REFER TO 10/65,1
V/////	LOAD BEARING WALL REFER 01/65,1
	CMU WALL
000'-0"	INDICATES TOP OF PLATE (TRUSS OR RAFTER BEARING EL.
H □ □	INDICATES WALL HEADER REFER TO 10/65.3

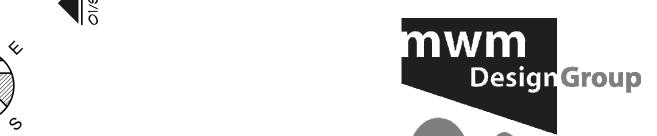
FOUNDATION AND SLAB PLAN - BUNKHOUSE EAST

1/4" = 1'-0"

LOAD BEARING STUD WALL



02 FOUNDATION AND SLAB PLAN - BUNKHOUSE WEST



TEXAS PARKS 8 WILDLIFE

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GUS ENGELING WMA

HERING LODGE & BUNKHOUSE

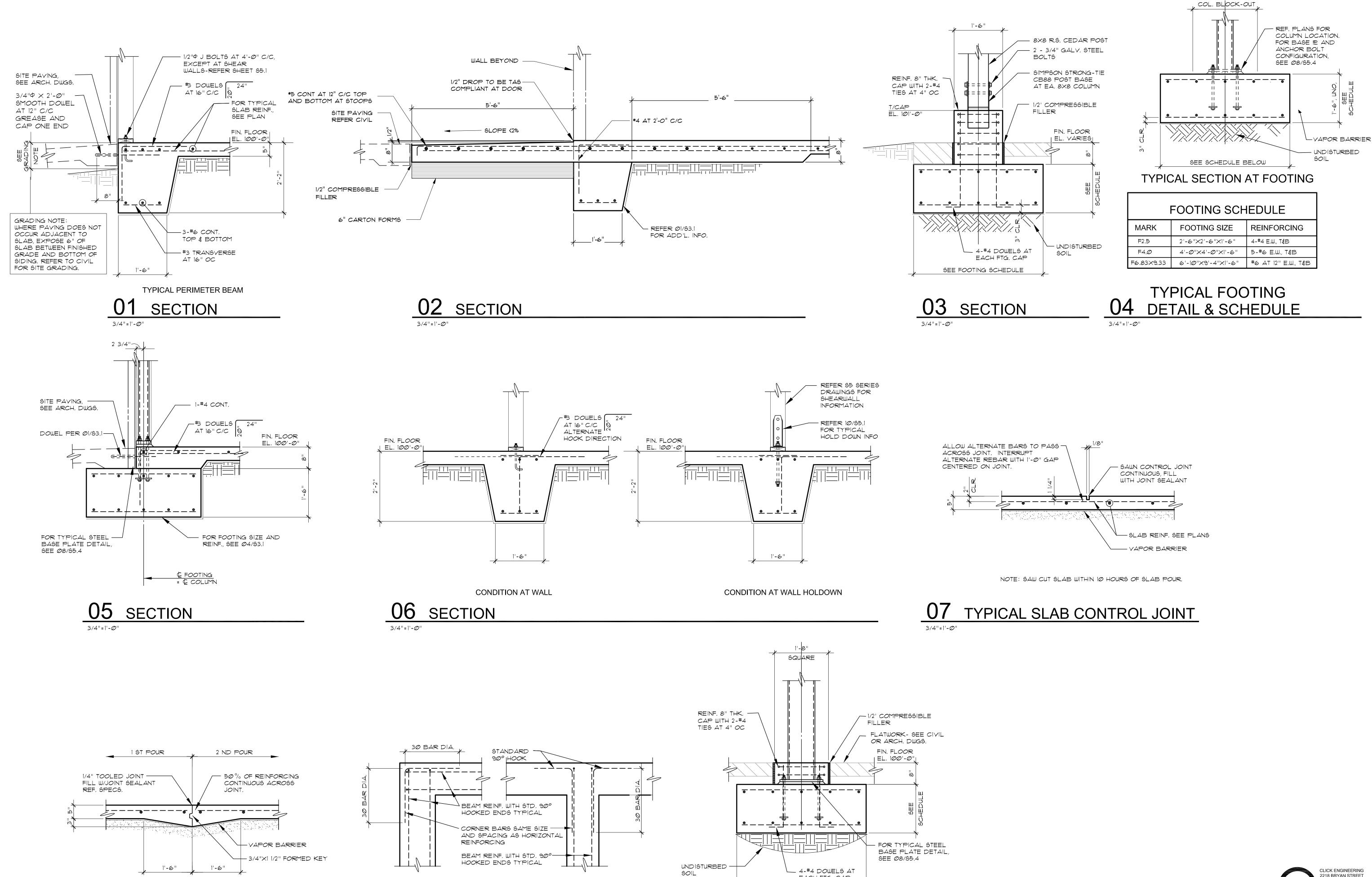
DATE: April 3, 2024 DESIGNED BY: DRAWN BY: REVIEWED BY:

SHEET TITLE

BUNKHOUSE FOUNDATION AND SLAB PLAN

SHEET NUMBER

S2.2



10 TYPICAL BEAM CORNER BARS

NOT TO SCALE

_ 4-#4 DOWELS AT EACH FTG. CAP

SEE FOOTING SCHEDULE

SECTION

3/4"=1'-Ø"

1'-6"

TYPICAL SLAB CONTROL JOINT

SHEET TITLE CLICK ENGINEERING 2218 BRYAN STREET FOUNDATION DETAILS SUITE 150
DALLAS, TEXAS 75201
P: 214.871.2302 TEXAS REG. NO. F-10142 CLICK JOB NO. 23-066

mwm Design Group S3.1

SHEET NUMBER

TEXAS

WILDLIFE

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THIS WORK

TX FIRM NO. 1452

S

G WMA BUNKHOUS

NGELING

GUS ENSATHERING

G

DATE: April 3, 2024

DOCUMENTS

CONSTRUCTION

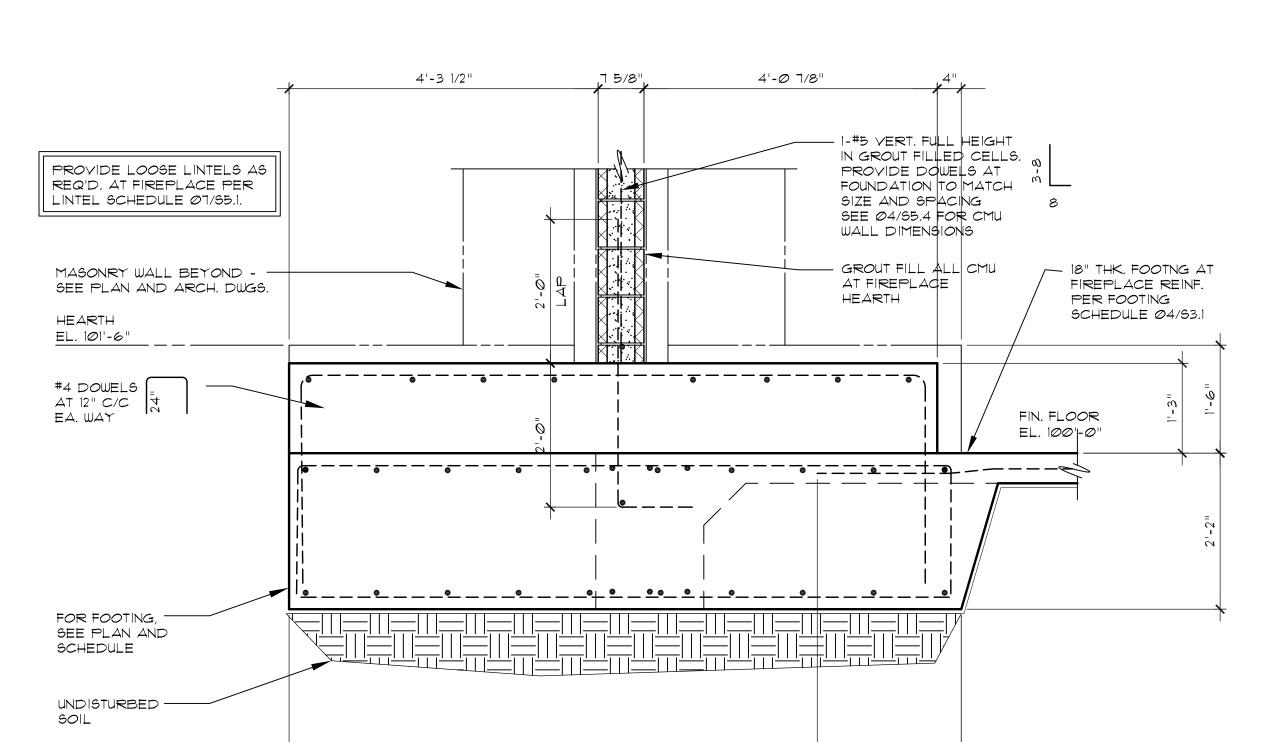
%56

DESIGNED BY:

REVIEWED BY:

PARKS

CD DOCUMENT



9'-4"

2'-Ø" LAP

SECTION AT FIREPLACE NOT TO SCALE



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GUS ENGELING WMA GATHERING LODGE & BUNKHOUSE ECT NUMBER:

REVIEWED BY:

CONSTRUCTION DOCUMENTS

CD DOCUMENT 95%

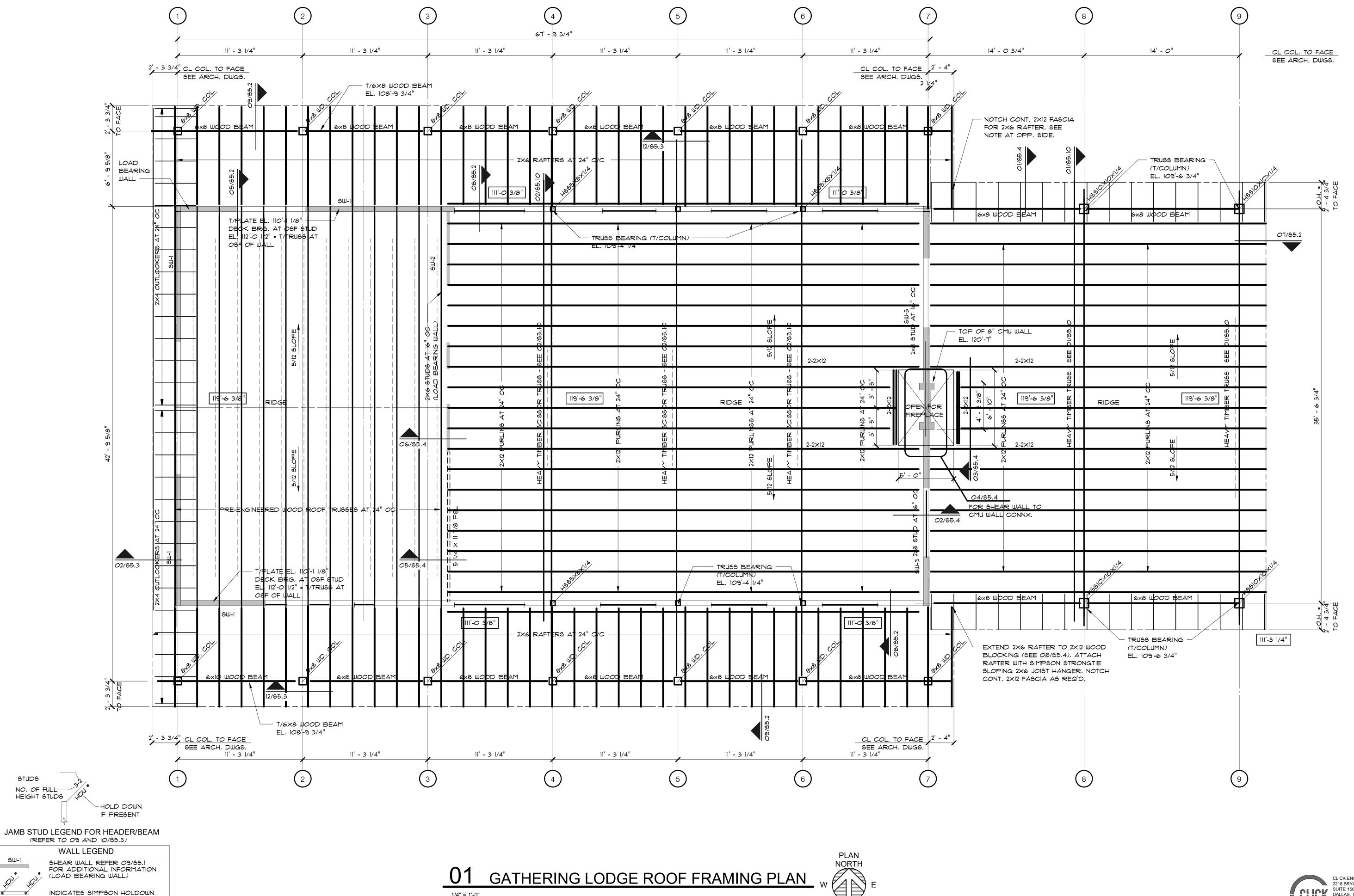
SHEET TITLE

FOUNDATION DETAILS

CLICK ENGINEERING
2218 BRYAN STREET
SUITE 150
DALLAS, TEXAS 75201
P: 214.871.2302
TEXAS REG. NO. F-10142
CLICK JOB NO. 23-066

mwm
Design Group

SHEET NUMBER



April 3, 2024 DESIGNED BY: DRAWN BY: REVIEWED BY:

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JOHN JACOB CLICK 90550 G/STE

S

GUS ENGELING WMA HERING LODGE & BUNKHOU

里

JECT NUMBER: 127282

1/4" = 1'-0"

PLAN NOTES: ELEVATIONS NOTED ON PLAN THUS XXX'-X" ARE TO DECK BEARING

ELEVATION AT ROOF. WOOD ROOF DECK TO BE 3/4" PLYWOOD, U.N.O.

FOR SHEAR WALLS, REFER TO SHEAR WALL SCHEDULE, DWG. 65.1. FOR HEADERS SHOWN ON PLAN, REFER TO HEADER SCHEDULE, 10/65.3.

D.S. TRUSS = DRAG STRUT TRUSS, CENTER OVER WALL BELOW, DESIGN TRUSS FOR HORIZ, LOAD OF 300 PLF.



SHEET TITLE GATHERING LODGE ROOF FRAMING PLAN

DATE:

SHEET NUMBER

S4.1

STUDS

 $\bigvee\bigvee\bigvee$

000'-0"

NO, OF FULL

REFER TO 10/65.1

01/65.1

CMU WALL

LOAD BEARING WALL REFER

INDICATES TOP OF PLATE

INDICATES WALL HEADER

REFER TO 10/65,3

(TRUSS OR RAFTER BEARING EL.)

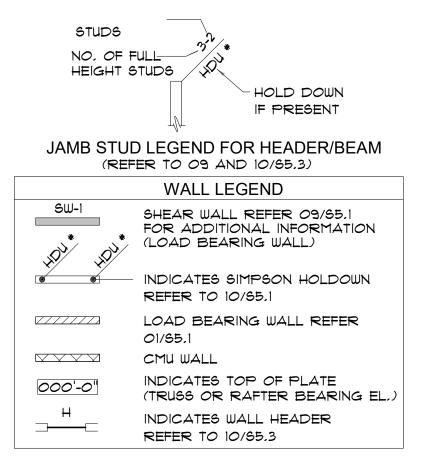
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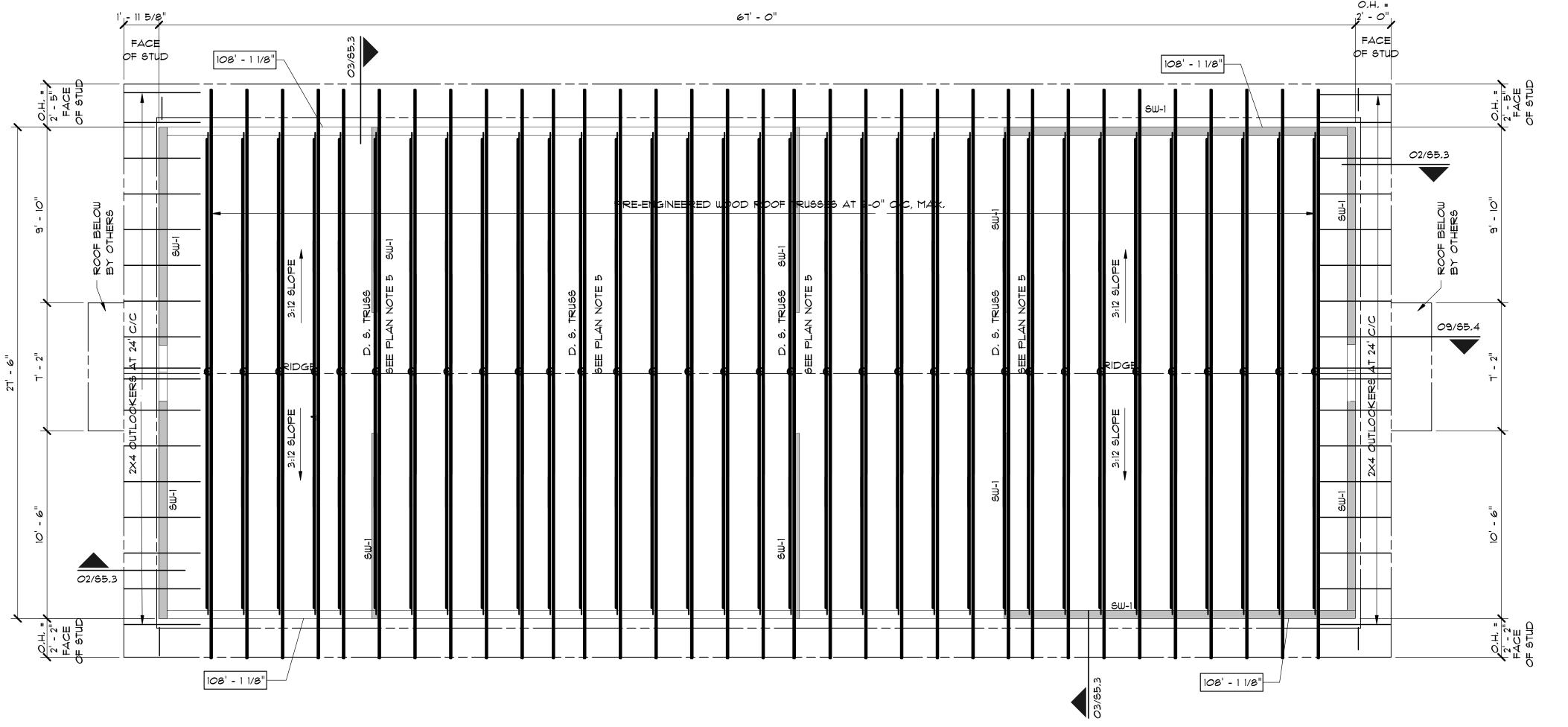
PLAN NOTES:

ELEVATIONS NOTED ON PLAN THUS XXX'-X" ARE TO DECK BEARING ELEVATION AT ROOF.

WOOD ROOF DECK TO BE 3/4" PLYWOOD, U.N.O. FOR SHEAR WALLS, REFER TO SHEAR WALL SCHEDULE, DWG. 65.1.

FOR HEADERS SHOWN ON PLAN, REFER TO HEADER SCHEDULE, 10/65.3. D.S. TRUSS = DRAG STRUT TRUSS, CENTER OVER WALL BELOW, DESIGN TRUSS FOR HORIZ, LOAD OF 300 PLF.







SHEET TITLE BUNKHOUSE ROOF FRAMING PLANS

DATE:

DESIGNED BY:

REVIEWED BY:

DRAWN BY:

April 3, 2024

SHEET NUMBER

TEXAS

PARKS 8

WILDLIFE

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GUS ENGELING WMA
THERING LODGE & BUNKHOUSE

JECT NUMBER: 127282

02 ROOF FRAMING PLAN - BUNKHOUSE WEST

mwm Design **Group**

1/4" = 1'-0"

-BOUNDARY NAILING

PLATE SPLICE

REFER DETAIL

- 2-16d FACE NAIL FIRST

NON-LOAD BEARING HEADER

SCHEDULES, AND JAMB STUD

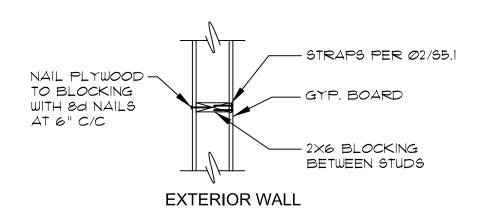
SCHEDULE, SEE PLAN

PLATE TO STUD

-STRAPS PER 02/95.1 -NAIL SHEATHING TO HEADER PER PANEL EDGE REQUIREMENTS IN SHEAR WALL SCHEDULE

SECTION

NOT TO SCALE



SECTION

CONDITION AT WINDOW

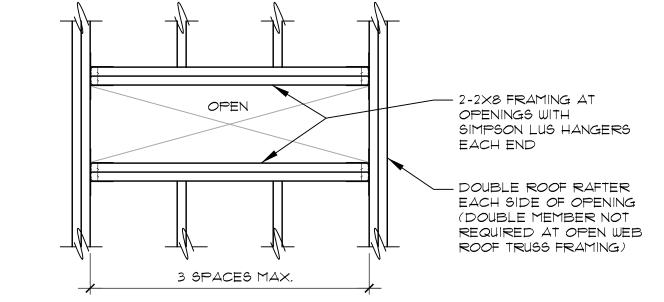
02B

NOT TO SCALE

— 2×6 BLOCKING —

SHEAR WALL

AT STRAP TYPICAL



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THIS WORK. TX FIRM NO. 1452

BUNKHOUSE

127282

WMA

NGELING

GUS E

THERING

DATE: February 29, 2024

DESIGNED BY:

REVIEWED BY:

01 TYPICAL EXTERIOR AND INTERIOR BEARING WALL FRAMING

02 TYPICAL SHEAR WALL OPENINGS NOT TO SCALE

NOT TO SCALE

PROVIDE HOLD DOWN AT EACH

END OF ALL SHEAR WALLS,

CONDITION AT DOOR

SEE 06/S3.1 \$ 06/S5.1

V₂×6 TYPICAL -SIMPSON BA HANGERS TYPICAL

TYPICAL FRAMING AT ROOF OPENING

LUMBER PROPERTIES SCHEDULE

04 TYPICAL ROOF OPENING

NOT TO SCALE

NAILING SCHEDULE					
CONNECTION TYPE	NAILING TYPE	NAILS			
FLOOR JOIST TO SILL	TOENAIL	3-8D			
FLOOR JOIST TO GIRDER	TOENAIL	3-8D			
BRIDGING TO JOIST	TOENAIL EA END	2-8D			
SOLE PLATE TO JOIST	FACE NAIL	16D AT 16" C/C			
SOLE PLATE TO BLOCKING	FACE NAIL	16D AT 16" C/C			
TOP PLATE TO STUD	END NAIL	2-16D			
STUD TO SOLE PLATE	TOE NAIL	3-8D			
STUD TO SOLE PLATE	END NAIL	2-16D			
DOUBLE STUDS	FACE NAIL EA. FACE	2 ROWS 16D AT 16" C/C			
TRIPLE STUDS	FACE NAIL EA. FACE	2 ROWS 30D AT 16" C/C			
DOUBLE TOP PLATES	FACE NAIL	16D AT 16" C/C			
TOP PLATES AT INTERSECTIONS	FACE NAIL	16D AT 16" C/C			
CONTINUOUS HEADER (TWO PIECES)	FACE NAIL EA EDGE	16D AT 16" C/C			
CONTINUOUS HEADER TO STUD	END NAIL	6-16D			
CEILING JOIST TO PLATE	TOE NAIL	3-8D			
CEILING JOIST AT LAPS OVER PARTITIONS	FACE NAIL	3-16D			
CEILING JOIST TO PARELLEL RAFTERS	FACE NAIL	3-16D			
RAFTER TO PLATE	TOE NAIL	3-8D			
I" BRACE TO PLATE	FACE NAIL	2-8D			
IX8 SHEATHING TO EACH BEARING	FACE NAIL	2-8D			
WIDER THAN IX8 SHEATHING TO EACH BEARING	FACE NAIL	2-8D			
BUILT UP CORNER STUDS	FACE NAIL	16D AT 24" C/C			
PLYWOOD ROOF & WALL SHEATHING AT EDGES	FACE NAIL	10D GALV. AT 6"			
PLYWOOD ROOF & WALL SHEATHING AT INT. SUPPORTS	FACE NAIL	100 GALV. AT 12" C/C			

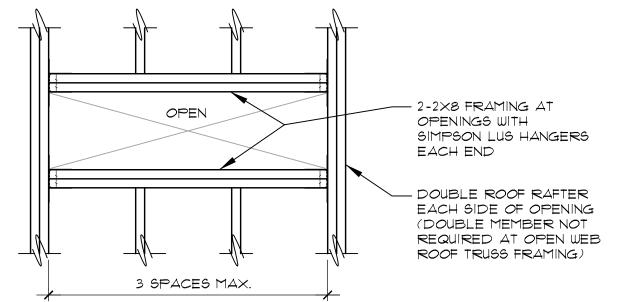
SOUTHER	Y PINE	3	•	•	
2×4	NO. 2	1300	180	1,200,000	1650
2×6	NO. 2	1300	180	1,200,000	1600
2×8	NO. 2	1300	180	1,200,000	1550
2×10	NO. 2	1300	180	1,200,000	1500
2×12	NO. 2	1300	180	1,200,000	1450
		-			
ENGINEER	RED WOOD -	LVL OR EQUA	7Г		
1.75×9.5		2600	285	1,900,000	2500
1.75×14		2600	285	1,900,000	2500
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SHOWN OR NOTED ON DRAWINGS. SPAN LIMITS ANGLE SIZE \perp 4 × 4 × 1/4" 4'-1" TO 5'-6" \perp 4 × 4 × 5/16 5'-7" TO 7'-6" $L6 \times 4 \times 5/16$ " (LLV) 7'-7" TO 10'-0" $L 6 \times 4 \times 3/8$ " (LLV) ANGLES EXPOSED TO WEATHER SHALL BE GALVANIZED. MINIMUM BEARING SHALL BE 8" EACH END.

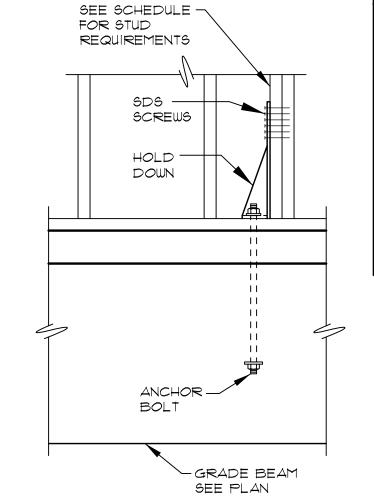
LOOSE LINTEL SCHEDULE - ANGLES

THIS SCHEDULE IS FOR MATERIAL NOT OTHERWISE

LOOSE LINTEL SCHEDULE



TYPICAL FRAMING AT ROOF OPENING NOT TO SCALE



	HOLD DOWN SCHEDULE						
	SDS SCREWS	HOLDOWN	ANCHOR BOLT AT CONCRETE	ANCHOR BOLT AT WOOD FLOOR	N <i>O. O</i> F SDS 1/4"x2 1/2" SCREWS		
		HDU-5	AB1Ø	5/8"\$ THREADED ROD	14		
	/ HOLD	-	-	-	-		
4	DOWN	-	-	-	-		
	11						
	* ANCHOR						
-	BOLT						

1. CONTRACTOR TO PROVIDE NUTS AND WASHERS FOR ANCHOR BOLTS (HARDENED WASHERS)

2. CONTRACTOR OPTION TO REQUEST POST INSTALLED

NOT TO SCALE

1-SIMPSON CS22 STRAP ONE SIDE OF WALL WITH

TWO ROWS OF 8d NAILS

(NO SPLICE ALLOWED IN STRAPS TYPICAL)

AT 4 1/8" C/C (STAGGERED

SHEAR WALL BLOCKING NOTE: 2-2X BLOCKING AT PLYWOOD

PANEL EDGES REQUIRED AT

8d NAILS SPACED 2 1/2" C/C

2. 10d NAILS SPACED 3" OR

FOLLOWING CONDITIONS

OR LESS

ANCHOR	A	ANCHOR BOLT SCHEDULE for HOLD DOWNS				
BOLT	<u>Ψ</u> Ω	MARK	DIA.	P = PROJECTION	L	
		AB10	5/8"	5"	SEE NOTE 1	
GRADE BEAM		-	-	-	-	
SEE PLAN		-	-	-	-	
	#	PLATE 1/	/2×4×4 TA	CK WELD TO NUT		
HOLDOWN AT CONCRETE	1. ANCHOR BOLTS TO BE EMBED OR LENGTH REG					

CLICK ENGINEERING
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P: 214.871.2302

TEXAS REG. NO. F-10142

DETAILS AND SCHEDULES SHEET NUMBER

S5.1

mwm Design Group

<u> </u>	NAILING SCHEDULE	

LUMBER PROPERTIES SCHEDULE NOT TO SCALE

SHEAR/BRACED WALL SCHEDULE						
MARK	WALL CONSTR.	TYPE OF MAT'L.	NAILING	HOLD DOWNS	SOLE PLATE	INTERMEDIATE ANCHOR BOLTS
SWI	BLOCKED 2×6 AT 16" C/C	15/32" WALL SHEATHING PER GENERAL NOTES AT OUTSIDE FACE	10d GALY. NAILS SPACED AT 6" C/C AT PANEL EDGE AND 12" C/C AT INTERMEDIATE SUPPORT	1 - HDU5 AT EACH END	2×6	1/2" DIA. $ imes$ 4 $1/2$ " THRU BOLTS OR ANCHOR BOLTS AT 48" C/C
SW2	BLOCKED 2×6 AT 16" C/C	15/32" WALL SHEATHING PER GENERAL NOTES AT BOTH FACES	10d GALY. NAILS SPACED AT 4" C/C AT PANEL EDGE AND 16" C/C AT INTERMEDIATE SUPPORT	2 - HDU5 AT EACH END	2×6	1/2" DIA. $ imes$ 4 $1/2$ " THRU BOLTS OR ANCHOR BOLTS AT 48" C/C
SW2	BLOCKED 2×8 AT 16" C/C	15/32" WALL SHEATHING PER GENERAL NOTES AT BOTH FACES	10d GALV. NAILS SPACED AT 4" C/C AT PANEL EDGE AND 16" C/C AT INTERMEDIATE SUPPORT	2 - HDU5 AT EACH END	2×8	1/2" DIA. X 4 1/2" THRU BOLTS OR ANCHOR BOLTS AT 32" C/C

NOTE: PROVIDE BLOCKING AT ALL PLYWOOD EDGES.

SHEAR WALL / BRACED WALL SCHEDULE

NOT TO SCALE

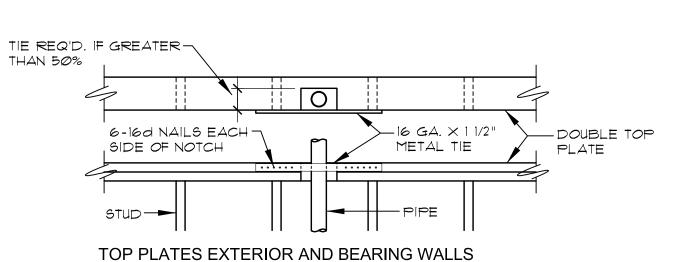
TYPICAL SHEAR WALL HOLD-DOWN DETAIL

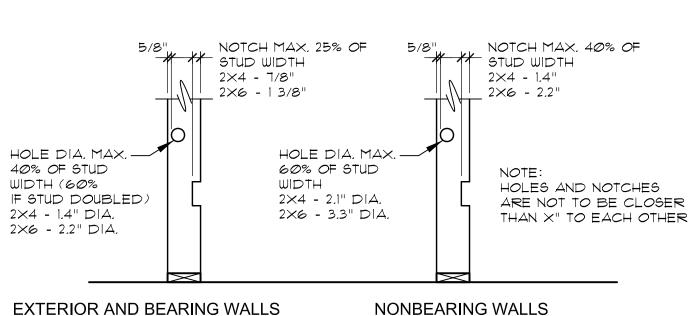
NOT TO SCALE

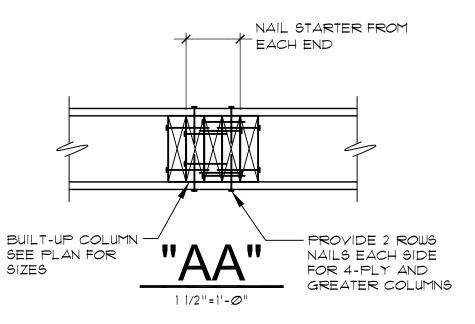
SHEET TITLE TYPICAL FRAMING

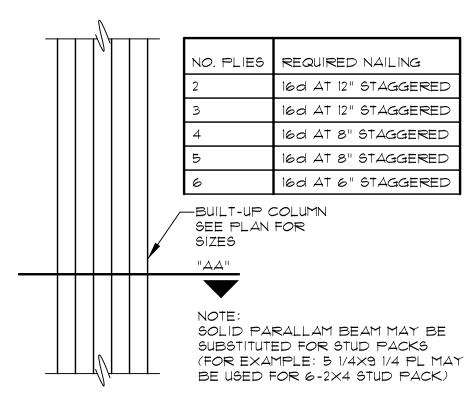
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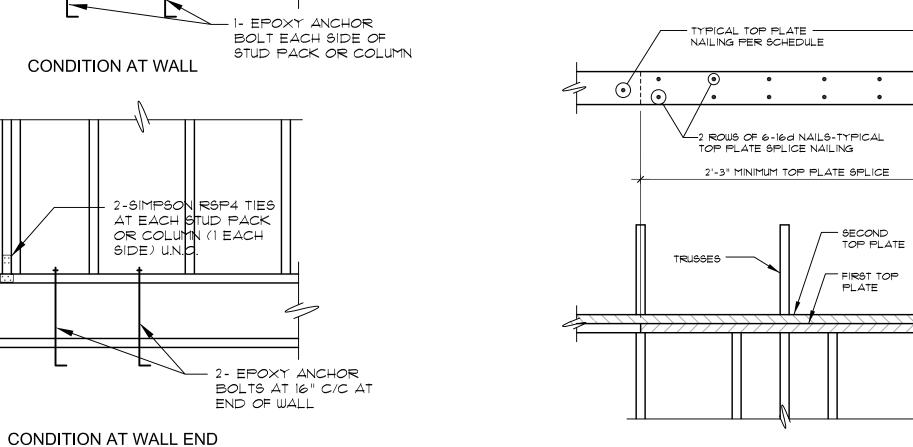
CD











-STUD PACK OR

GENERAL NOTES

2-SIMPSON RSP4 TIES AT EACH STUD PACK

OR COLUMN (1 EACH

COLUMN SEE

SIDE) U.N.O.

REFER NAILING -

SCHEDULE SHT.

S5.1 FOR STUD TO PLATE NAILS

BOTTOM-

PLATE

NAIL EACH STUD

SCHEDULE

IN STUD PACK TO

PLATE PER NAILING

REFER TO ELEVATION

ABOVE FOR ADD'L.

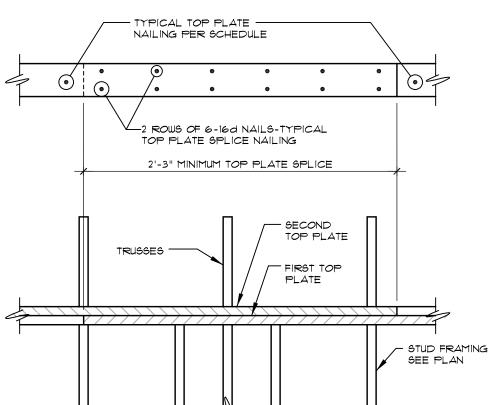
1/2"\$\times4 1/4" SIMPSON SET EPOXY ANCHOR

WITH NUT AND WASHER

TYPICAL PLATE

TO CONCRETE

AT 4'-0" C/C



TYP. HOLES AND NOTCHES DETAIL NOT TO SCALE

NOTE: STRAPS USED FOR THESE CONDITIONS ARE FOR PROTECTION

THE STUD IF IT IS OVER CUT.

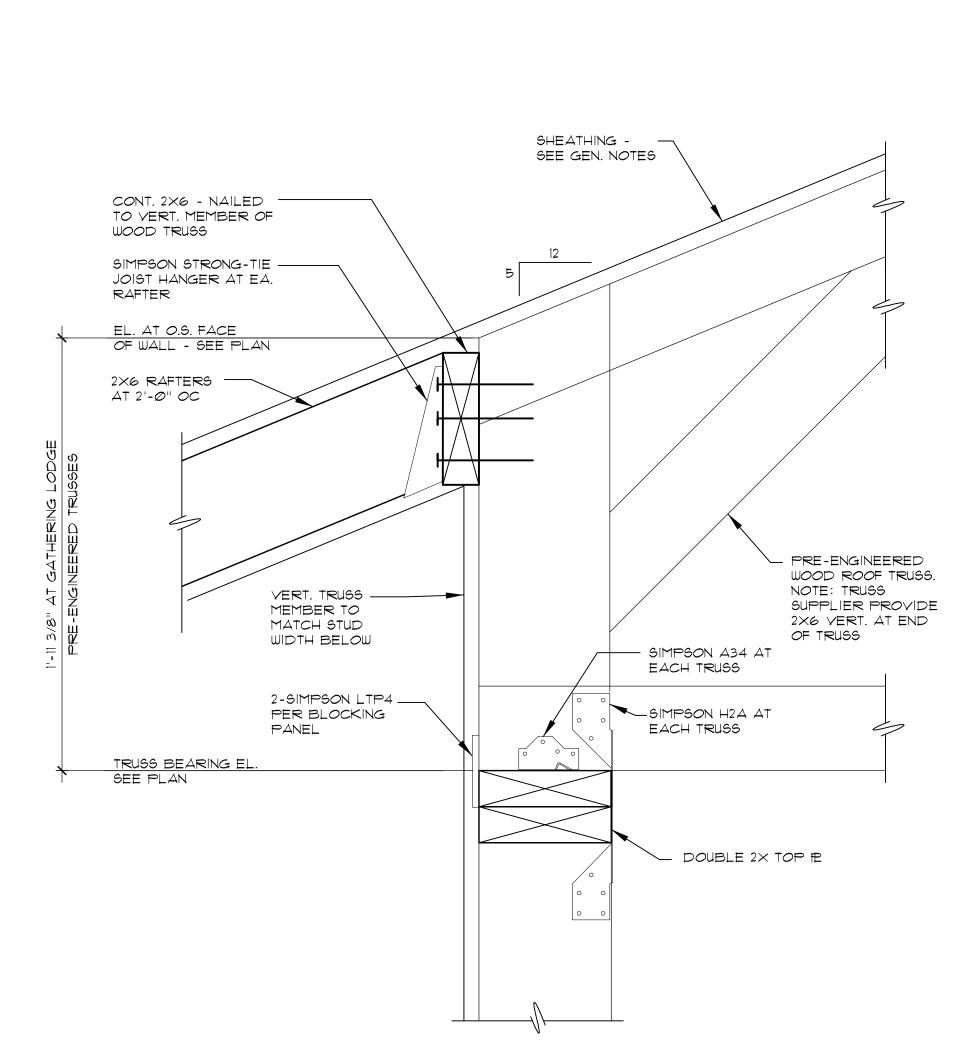
OF THE PIPES FROM NAIL PUNCTURE. HOLES AND NOTCHES EXCEEDING

THESE CONDITIONS REQUIRE STRAPS THAT ARE INTENDED TO REINFORCE

TYPICAL STUD PACK DETAIL NOT TO SCALE

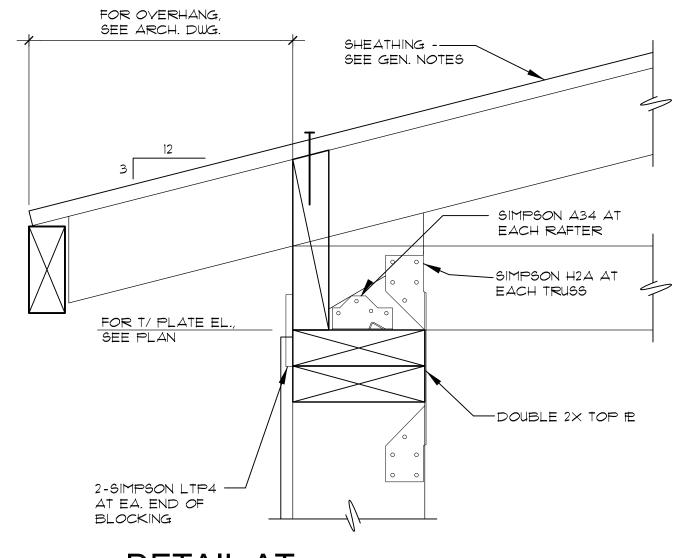
TYP. STUD PACK/COL. TO PLATE

TYPICAL TOP PLATE SPLICE NOT TO SCALE

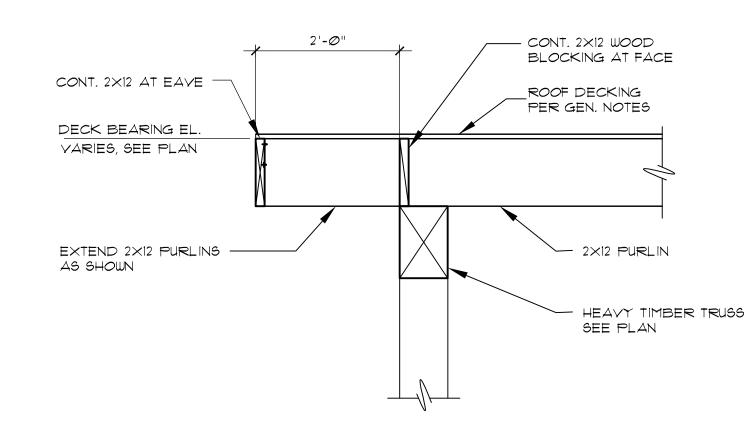


DETAIL AT DEEP SEAT TRUSS BEARING

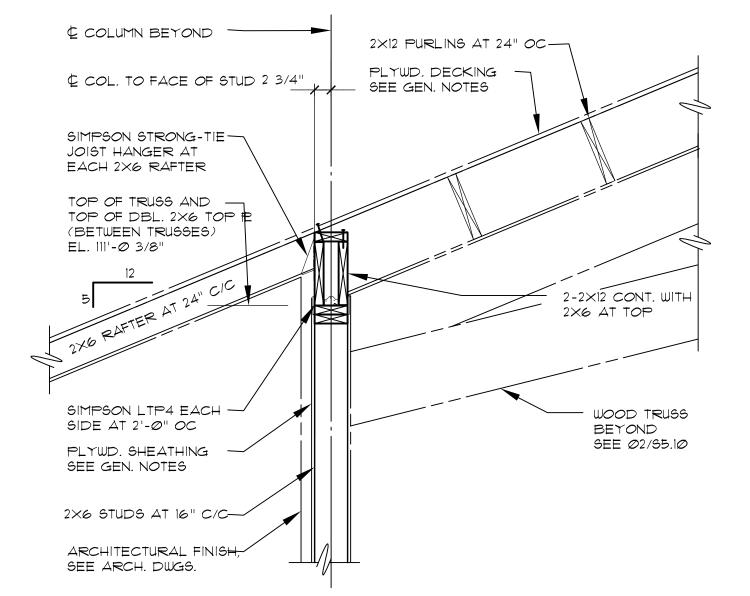
3"=1'-Ø"

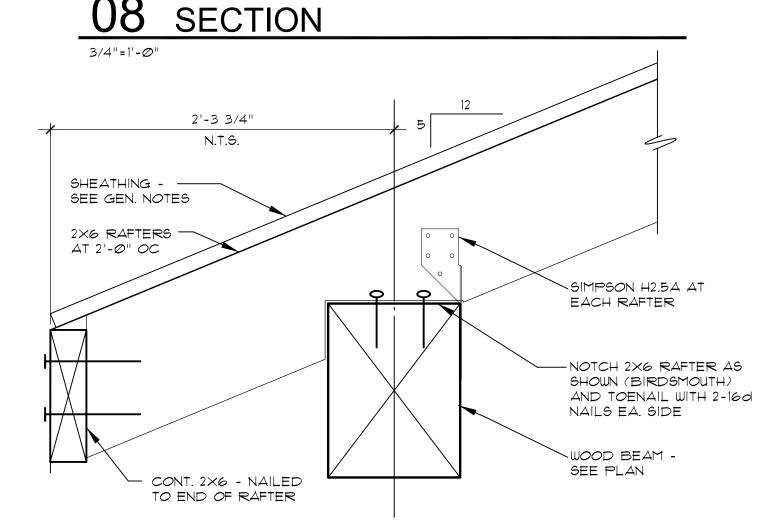


DETAIL AT STANDARD TRUSS BEARING 3"=1'-Ø"



SECTION 3/4"=1'-Ø"





SECTION

3"=1'-Ø"



mwm **Design** Group 127282

TEXAS

PARKS 8

WILDLIFE

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BUNKHOUSE

WMA

NGELING

GUS E

DATE: February 29, 2024 DESIGNED BY: DRAWN BY: REVIEWED BY:

SHEET TITLE

TYPICAL FRAMING SECTIONS AND DETAILS

SHEET NUMBER

S5.2

CD

GREATER THAN 10'-0" SEE PLAN FOR HEADER SIZE

NON-LOAD BEARING WALL HEADER SCHEDULE

2×4 STUDS

2-2×6

2-2×6

 $2 - 2 \times 12$

HEADER

3-2×6

3-2×6

 $3-2\times12$

PSL

3 1/2× 11 1/4 5 1/4× 11

GREATER THAN 10'-0" SEE PLAN FOR HEADER SIZE

2×6 STUD:

OPENING WIDTH HEADER

UP TO 4'-0"

4'-1" TO 6'-0"

6'-1" TO 8'-0"

3'-1" TO 10'-0"

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SHEET TITLE

SHEET NUMBER

TYPICAL FRAMING

SECTIONS AND

DETAILS

TYPICAL HEADER DETAIL

2'-Ø"

PLATE LINE

SIMPSON MTS18 -EACH SIDE OF

BRACING TRUSS

REFER Ø1/65.3

TYPICAL BEAM BEARING ON WALL NOT TO SCALE

BEAM SIZE FOR DETAIL AT COLUMN BASE, -8×8 R.S CEDAR SEE Ø3/S3.1

NOT TO SCALE

TYPICAL PORCH BEAM ON R.S. WOOD COLUMN

mwm **Design** Group

TYPICAL HEADER SUPPORT

FULL HEIGHT

NOT TO SCALE

STUDS SEE

PLAN

B.O. HEADER

REF. ARCH'L

-REFER PLAN FOR

JAMB STUDS SEE GENERAL

NOTES AND JAMB STUD

SCHEDULE, 10/S5.03

MEMBER SIZE

STAGGERED EACH

2X MEMBER

TYPICAL WOOD BEAM TO STUD WALL

Ø6/S5.2

SIMPSON A34 AT

SIMPSON H2A AT

TRUSS GIRDERS

UNDER TRUSSES

SEE ARCH DWGS

REFER Ø4/65.1

SIMPSON MTS30 AT

FOR SPLICE DETAIL

-2×6 STUDS AT 16" C/C

ARCHITECTURAL FINISH

PROVIDE STUD DIRECTLY

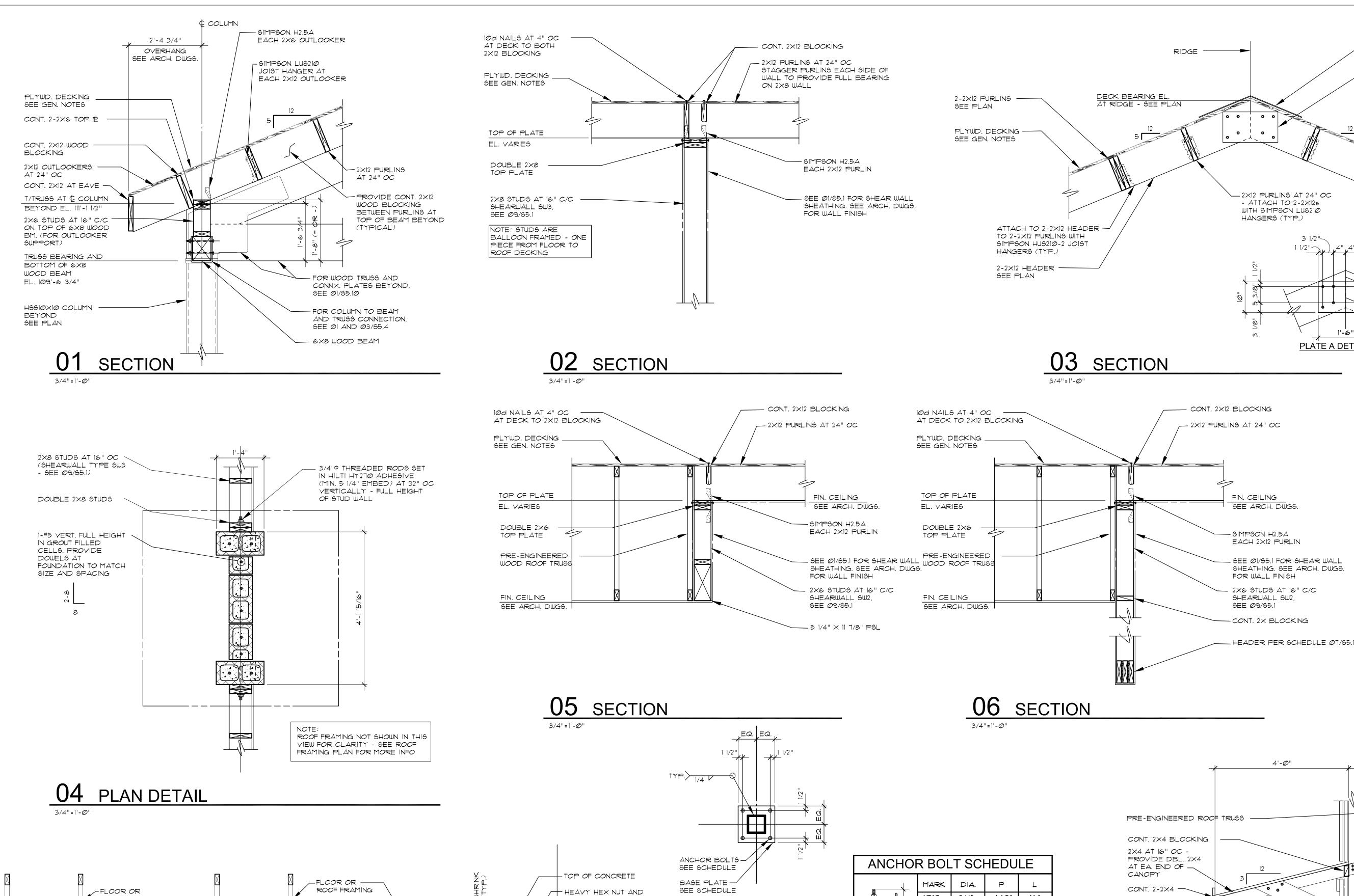
EACH TRUSS

EACH TRUSS

SEE PLAN FOR _SIMPSON CC68 COLUMN CAP CONNECTOR WITH 4- 5/8" THRU BOLTS IN BEAM AND 5/8" THRU BOLTS IN COLUMN

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S5.3



4'-Ø" - CONT. 2×6 ATTACHED TO TRUSSES SIMPSON A34 AT EA. RAFTER - TOP OF DBL. 2×6 TOP PLATE CONT. 2-2×4 EL. 108'-1 1/8" EL. 108'-0" 2X6 STUDS AT 16" C/C ADD STUDS UNDER EA. 4X6 WOOD BRACE -RAFTER FOR SUPPORT BEYOND AT EA. END OF CANOPY. ATTACH WITH 3-1/2" P GALY. THRU BOLTS AT EA. SUPPORT - SEE ARCH. DBL. 2×6 TOP PE DWGS. FOR MORE INFO HEADER SCHEDULE, ATTACH 4X6 WOOD BRACE-SEE PLAN TO WALL WITH SIMPSON A33 ANGLES EACH SIDE OF EA. BRACE CLICK ENGINEERING 2218 BRYAN STREET CLICK SUITE 150
DALLAS, TEXAS 75201
P: 214.871.2302 FOR FINISH, TEXAS REG. NO. F-10142 SEE ARCH. DWGS. CLICK JOB NO. 23-066

PLATE A DETAIL

SECTION AT BUNKHOUSE CANOPY mwm **Design** Group

TEXAS PARKS 8 WILDLIFE

SIMPSON LSTA36 STRAP -

EACH SIDE OF RIDGE

-<u>Plate "a"</u> See detail bel*o*w

ATTACH TO 2-2X12 HEADER

— ₱1/4×1Ø× 1'-6"

WITH 9/16"4 HOLES

AS SHOWN FOR 1/2"

A3Ø1 THRU BOLTS

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S BUNKHOU 27 NGELIN GUS E THERING

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SHEET TITLE

FRAMING

SECTIONS AND DETAILS

CD

SHEET NUMBER S5.4

N.T.S.

SIMPSON STCT

AT EACH TRUSS

-WALL STUDS -

PARTITION PERPENDICULAR

TO CEILING RAFTERS

TYPICAL NON-LOAD BEARING WALL ATTACHMENT TO FRAMING

TRUSS (OR AT

AT PARTITIONS)

NOTCH TOP PLATE AT

CONTRACTORS OPTION,

TOP PLATE MAY BE IX

ROOF FRAMING

-SIMPSON STCT

AT 4'-0" C/C

2×4 AT 4'-Ø" C/C

- PROVIDE IX AT TOP

PLATE OR NOTCH-SEE

WITH 2-16d NAILS

EACH END

NOTCH NOTE

WALL STUDS

PARTITION PARALLEL

WITH CEILING RAFTERS

TYPICAL BASE PLATE DETAILS

LEVELING LUG

PLATE 3/8×3×3

WITH NUT AT ABIO

PLATE 1/2×4×4 = WITH NUT AT ABII

OR SHIMS AS

REQ'D.

PLATE WASHER WITH

TYPE A

ANCHOR BOLTS -SEE SCHEDULE

BASE PLATE -

SEE SCHEDULE

STD. HOLE

ANCHOR-

BOLT SEE

SCHEDULE

TACK WELD>

HSS 5×5 3/4×11×11 4-AB10 TYPE, HSS 5×5 /4× 5 1/2 ×11 TYPE E 4-AB1Ø AT PERIMETER 455 10×10 ×17×1′-5″ 4-AB11

TYPICAL BASE PLATE SCHEDULE

AB10

ABII

ANCHOR BOLTS TO BE F1554 GRADE 36

PER AISC.

COL. SIZE

1. USE OVERSIZED HOLES FOR ANCHOR BOLTS

. PLATE WASHERS WITH STANDARD HOLES SHALL BE INSTALLED OVER OVERSIZED

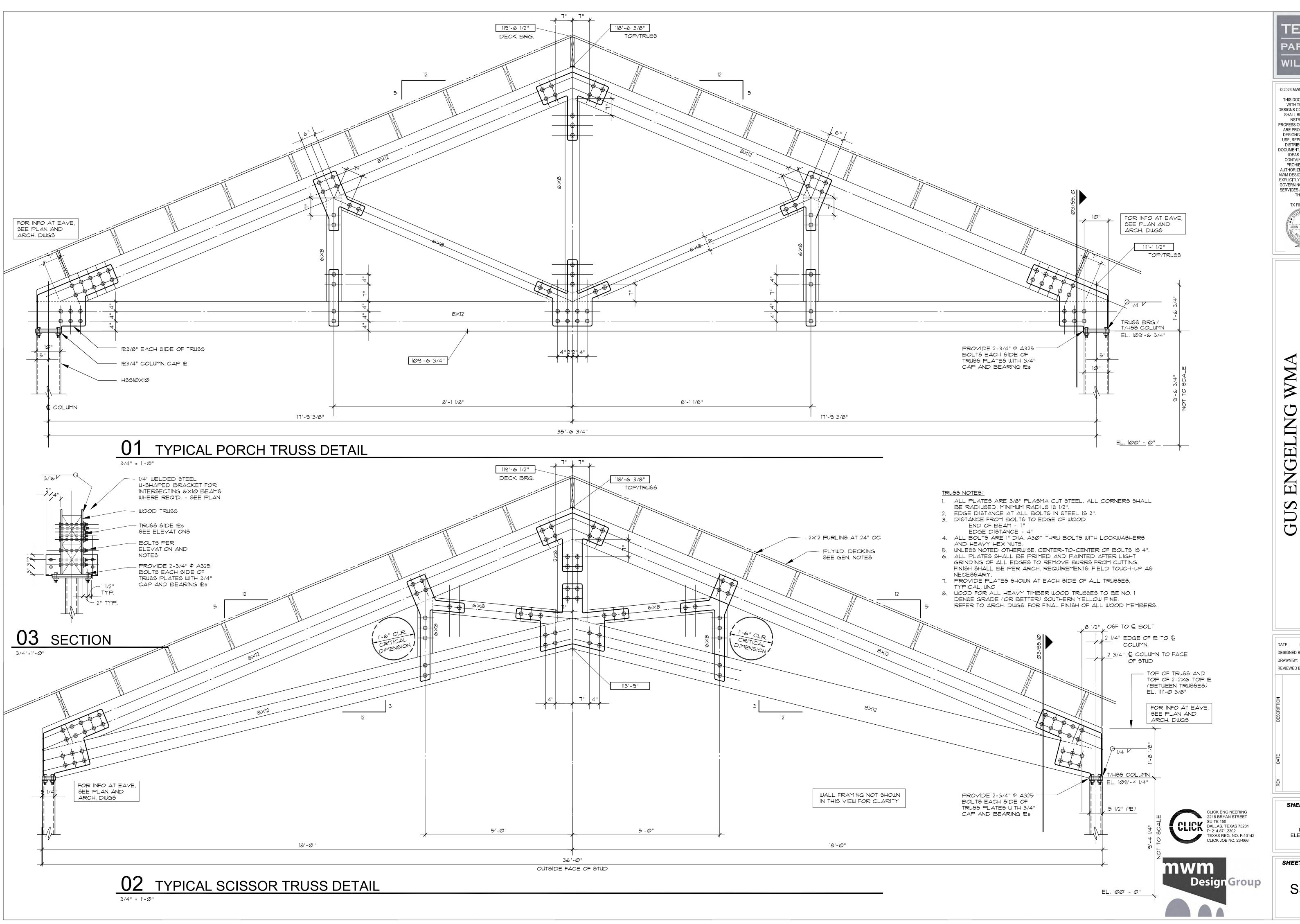
PLATE SIZE

3/4"

4 1/2"

4 1/2"

ANCHOR BOLTS



TEXAS
PARKS &
WILDLIFE

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GUS ENGELING WMA
THERING LODGE & BUNKHOUSES

DATE: February 29, 2024
DESIGNED BY: GL
DRAWN BY: GL
REVIEWED BY: JC

DOCUMENT

DOC

CD

SHEET TITLE

TRUSS ELEVATIONS HEET NUMBER

SHEET NUMBER





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04/03/24

SNGELING
NG LODGE & BUNK

GUS EJ GATHERIN

KEY NOTES NOTE: REFERENCE NUMBER INSIDE HEXAGON

GENERAL NOTES

UTILITY & CIVIL ENGINEER.

A. COORDINATE ELECTRIC UTILITY REQUIREMENTS WITH UTILITY COMPANY.

B. COORDINATE EXACT TRANSFORMER LOCATION AND CLEARANCES. POUR PAD PER UTILITY REQUIREMENTS.

C. COORDINATE PRIMARY, INCLUDING CONDUITS SIZE & ROUTE PATH WITH

D. COORDINATE TELEPHONE SERVICE REQUIREMENTS WITH COMMUNICATION

E. COORDINATE CABLE TV SERVICE REQUIREMENTS WITH VENDOR.

F. REFER TO CIVIL DRAWINGS FOR ADDITIONAL REQUIREMENTS AND

G. REFER TO SHEET E7.01 & P7.02 FOR SYMBOL LEGEND.

- 1. PRIMARY UNDERGROUND SERVICE LATERAL BY TRINITY VALLEY ELECTRIC
- 2. ELECTRICAL UNDERGROUND SECONDARY SERVICE LATERAL FROM TVEC PAD MOUNTED TRANSFORMER TO MAIN BUILDING WIREWAY. PROVIDE CONDUIT AND CABLE

- 6. MTS- STATE "BASE BID INCLUDES ELECTRICAL AND FIRE ALARM SERVICES. TO THIS
- LOCATED WITHIN UTILITY YARD FENCE. SEE ELECTRICAL ONE-LINE DIAGRAM. 9. ELECTRICAL UNDERGROUND SERVICE LATERAL FROM WIREWAY MOUNTED AT MAIN
- ONE-LINE DIAGRAM.
- DATA SERVICE CABLE.
- 13. UNDERGROUND SERVICE LATERAL FROM UNI-STRUT MOUNTED SERVICE DISCONNECT

COORDINATION.

- COOPERATIVE (TVEC). E.C. TO PROVIDE TRENCHING, 4' SCHEDULE 90 PVC AND BACKFILLING PER TVEC REQUIREMENTS. COORDINATE EXACT ROUTING WITH TPWD
- AS PER ELECTRICAL ONE-LINE DIAGRAM.
- 3. 2" CONDUIT, WITH PULL CORD, FOR FIRE ALARM WIRING TO MAIN FACP.
- 4. EXISTING POWER COMPANY POWER POLES AND EXISTING SERVICE.
- 5. ROUTE 1" CONDUIT FROM BUNKHOUSE FIRE ALARM PANELS TO MAIN BUILDING FIRE
- ALARM PANEL.
- POINT AND CAPPING FOR FUTURE USE ON EAST BUNKHOUSE". 7. WIREWAY AS INDICATED ON ELECTRICAL ONE-LINE DIAGRAM. MOUNT TO SIDE OF
- 8. BUILDING SERVICE DISCONNECT SWITCH MOUNTED ON UNI-STRUT RACK AND
- BUILDING UNI-STRUT RACK. PROVIDE CONDUIT AND CABLE AS PER ELECTRICAL
- 10. PROVIDE UNDERGROUND 2-INCH SCHEDULE 80 PVC WITH PULL CORD FOR FUTURE
- PROVIDE 24 INCH BY 36 INCH BY 48 INCH DEEP PULL BOX FOR FUTURE DATA SERVICE. PROVIDE LID WITH "DATA" INSCRIBED.
- 12. STUB UP INTO BUILDING AT WALL MOUNTED TELEPHONE TERMINAL BOARD.

TO PANELBOARD INDICATED. REVER TO ONE-LINE DIAGRAM FOR ADDITIONAL

DRAWN BY: REVIEWED BY:

DESIGNED BY:

04/03/2024

SHEET TITLE

SITE PLAN - MEP

SHEET NUMBER

mwm





____2-WAY CO.

₹ 3" DCW SERVICE. REFER

TO CIVIL

CIVIL

4" SS REFER TO

ISOLATION VALVE LOCATED IN LODGE — ROOM 107.

4" SS REFER TO CIVIL

PAD MOUNTED TVEC
TRANSFORMER. SEE ELECTRICAL ——

ISOLATION VALVE LOCATED IN LODGE -

ROOM 107

ONE-LINE DIAGRAM FOR

ADDITIONAL INFORMATION.