

FOUNDATION PART PLAN

1.) FOOTING DESIGN BASED ON AN SOIL BRG. CAPACITY OF 2000 PSF.

(GEOTECHNICAL REPORT BY TERRACON (PROJECT NO. 72245090, DATED 20 DEC 2024)

IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY THE ENGINEER OF RECORD

IF UNSTABLE, ORGANIC, WEAK OR OTHERWISE UNACCEPTABLE SOIL CONDITIONS ARE

ENCOUNTERED DURING EXCAVATIONS OR SUBSEQUENT GEOTECHNICAL INVESTIGATIONS.

2.) ELEV. NOTED (-) ARE BELOW REFERENCE FINISHED FLOOR TO TOP/FOOTING. FINISHED FLOOR ELEVATION = (+36.00'), TYP., U.O.N. SEE CIVIL DRAWINGS.

- 3.) SLAB ON GRADE IS NORMAL WEIGHT CONCRETE WITH REINFORCED WITH 6x6 W1.4 x W1.4 WWM ON A 4" NO. 57/67 WASHED STONE AND 15 MIL POLY VAPOR BARRIER, TYP. U.O.N.
- 4.) ALL CONCRETE SHALL BE A MINIMUM STRENGTH OF 3000 PSI MEETING ACI 301
 AND ACI 318. ALL CONCRETE SHALL BE MIXED, HANDLED, SAMPLED, TESTED,
 AND PLACED IN ACCORDANCE WITH ACI STANDARDS. ALL SAMPLES SUBJECT TO
 PUMPING SHALL BE TAKEN AT THE EXIT END OF THE PUMP AT THE ELEVATION
 OF PLACEMENT. (REFERENCE ACI MANUAL OF CONCRETE PRACTICE).

5.) ALL REINFORCING BARS SHALL BE GRADE 60 CONFORMING TO ASTM 615. LAP

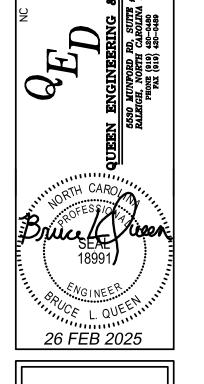
- BARS WHERE REQUIRED USING CLASS B TENSION LAP SPLICES, OR 40 BAR DIAMETERS.
 DEVELOPMENT LENGTHS SHALL BE CRSI MINIMUM UON.
 6.) SEE S1101 FOR COLUMN FOOTING SCHEDULE AND ADDITIONAL
- NOTES THAT APPLY.
- 7.) REFERENCE ARCHITECTURAL AND PLUMBING DRAWINGS FOR COORDINATION OF SLOPED FLOORS AT FLOOR DRAINS, AND DEPRESSED FLOOR SLAB LOCATIONS.
- 8.) LOCATE ALL WALLS AND MASONRY OPENINGS PER ARCHITECTURAL DRAWINGS.
- 9.) ARCHITECTURAL BACKGROUND IS SHOWN FOR REFERENCE ONLY. REFER TO ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS OF WALLS.
- 10.) THE CONTRACTOR SHALL COORDINATE AND VERIFY ALL DIMENSIONS & ELEVATIONS PRIOR TO STARTING CONSTRUCTION AND ANY DISCREPANCY SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD.
- 11.) SEE ARCHITECTURAL DRAWINGS FOR ALL POURED RAMP LOCATIONS AND DIMENSIONS, TYPICAL.
 SEE ARCHITECTURAL DRAWINGS FOR ALL POURED STAIR LOCATIONS AND DIMENSIONS, TYPICAL.
 CONSTRUCT ALL POURED RAMPS AND STAIRS IN ACCORDANCE WITH STRUCTURAL SECTIONS
 7 & 16/S1101 AND ARCHITECTURAL DRAWINGS. SEE 10/S1101 FOR BOLLARD DETAIL. LOCATE
 ALL BOLLARDS PER ARCHITECTURAL DRAWINGS.

12.) ALL COLUMNS = HSS $8 \times 8 \times \frac{1}{4}$, TYP. U.O.N.

No, Date: Revision

b'' = 1'-0"

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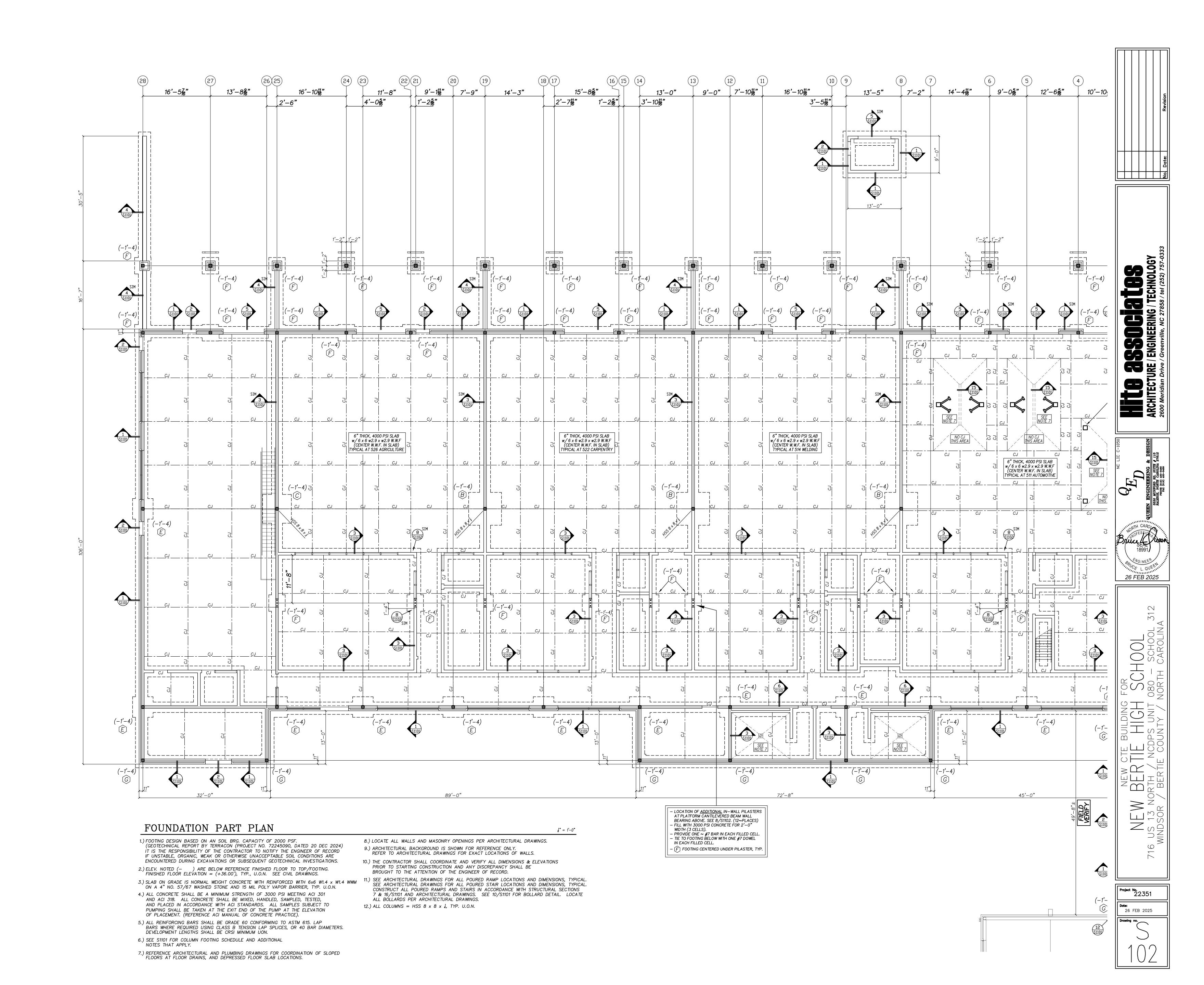


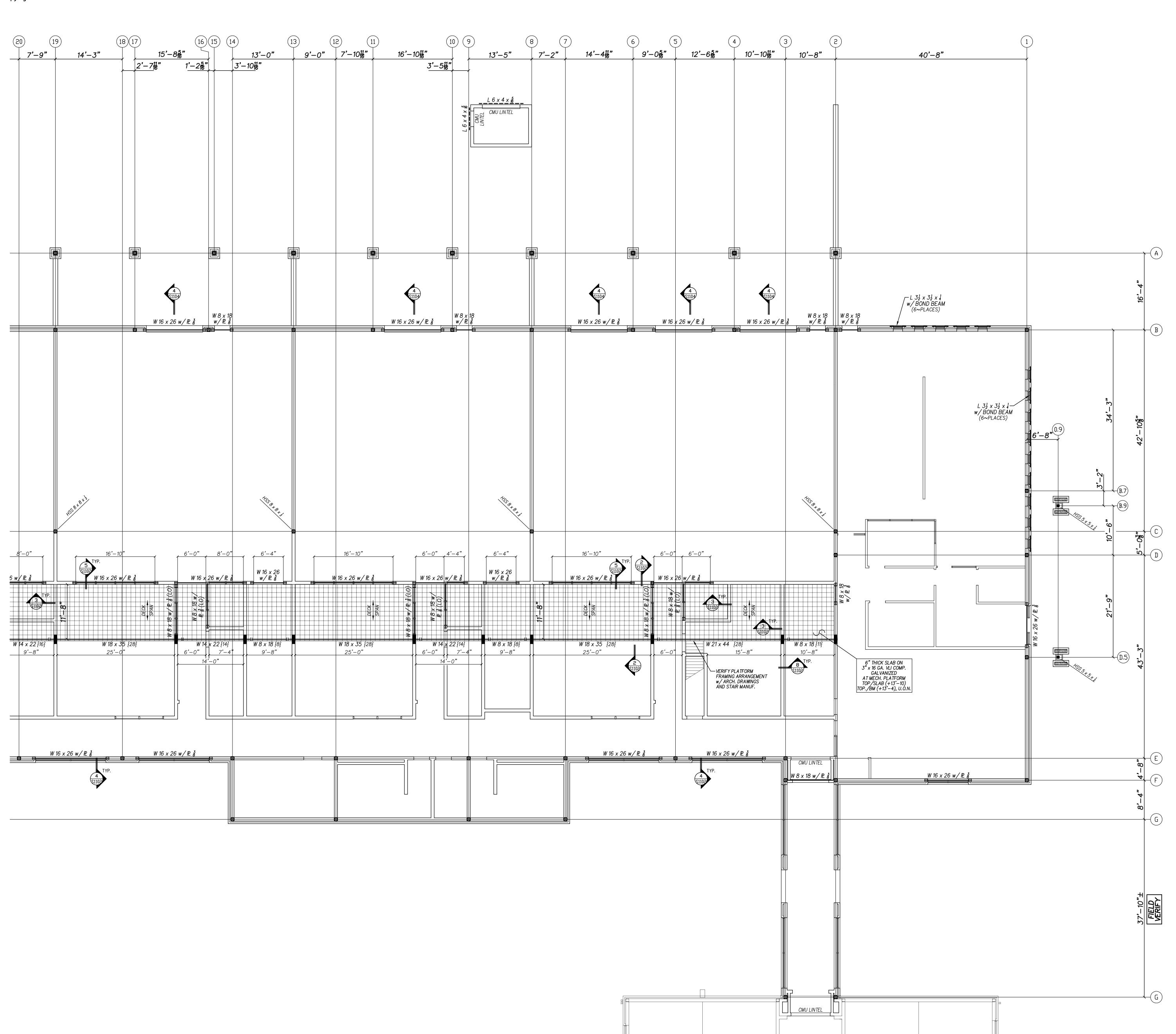
New CIE BUILDING FOR NEW CIE BUILDING FOR JULIA SCHOOL 3 UNDSOR / BERTIE COUNTY / NORTH CAROLINA

Project No. 22351

Date: 26 FEB 2025

Drawing no. 1





INTERMEDIATE PART PLAN

1.) SEE PLAN FOR TOP/UTILITY PLATFORM SLAB ELEVATION

2.) SEE S1102 FOR METAL FLOOR DECK FASTENING PATTERN, WHERE REQUIRED.

3) SEE PLAN FOR SLAB THICKNESS AND FLOOR DECK SIZE. REINFORCE ALL NEW PLATFORM SLABS WITH 6x6 w2.9 x w2.9 wwm.

4.) ARCHITECTURAL BACKGROUND IS SHOWN FOR REFERENCE ONLY. REFER TO ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS OF WALLS.

5.) REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATION OF ALL LINTELS.

6.) CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AS REQUIRED FOR NEW COMPOSITE DECK SPANS GREATER THAN 10'-6"

7.) LOCATE ALL WALLS AND MASONRY OPENINGS PER ARCHITECTURAL DRAWINGS. WHERE REQUIRED, SEE S1102 FOR ALL NEW CMU LINTELS.

8.) NO FABRICATION OR ERECTION SHALL COMMENCE PRIOR TO THE APPROVAL OF ALL STRUCTURAL STEEL SHOP DRAWINGS BY THE ENGINEER OF RECORD.

9.) COORDINATE AND VERIFY ALL DECK EDGE LOCATIONS AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

BEAM SIZE — INDICATES NO. OF \$\frac{3}{1}\tilde{\phi} \times 4\frac{3}{2}\tilde{\phi}\$ STUDS EQUALLY SPACED BETWEEN BEAMS (ONE NUMBER INDICATES STUDS EQUALLY SPACED ALONG CENTERLINE OF ENTIRE BEAM)

C=\frac{3}{2}

INDUCED CAMBER AT MIDSPAN (NO CAMBER IF BLANK)

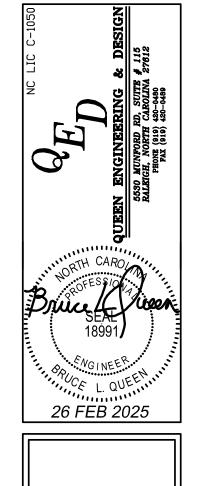
BEAM LEGEND

10.) REFERENCE ARCHITECTURAL AND PLUMBING DRAWINGS FOR COORDINATION OF SLOPED FLOORS AT FLOOR DRAINS, AND DEPRESSED FLOOR SLAB LOCATIONS.

No, Date: Revision

b'' = 1'-0''

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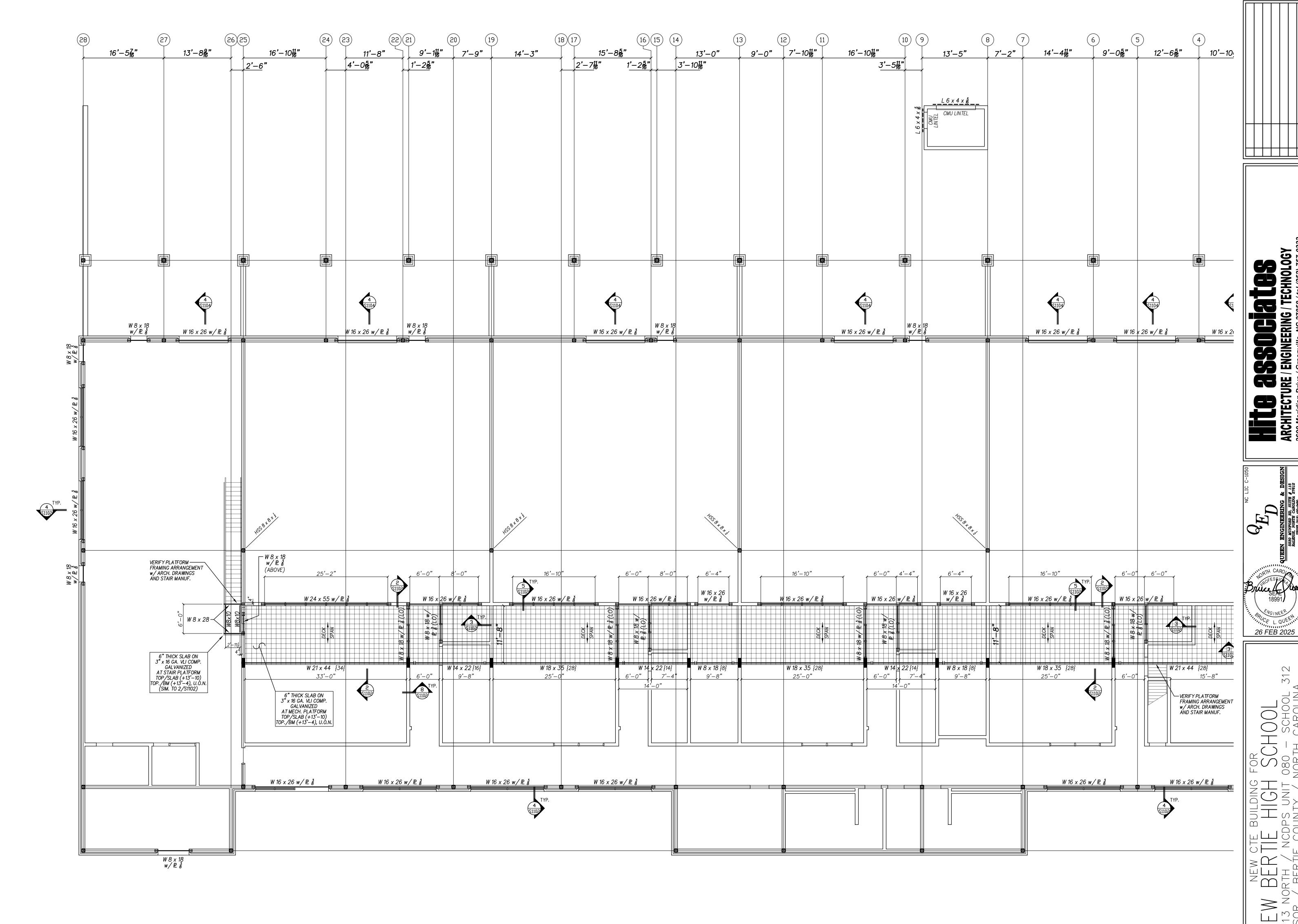
NEM BERTIE HIGH SCHOOL
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NDSOR / BERTIE COUNTY / NORTH CAROLINA

Project No. 22351

Date: 26 FEB 2025

Drawing no. 7



INTERMEDIATE PART PLAN

1.) SEE PLAN FOR TOP/UTILITY PLATFORM SLAB ELEVATION
2.) SEE S1102 FOR METAL FLOOR DECK FASTENING PATTERN, WHERE REQUIRED.
3) SEE PLAN FOR SLAB THICKNESS AND FLOOR DECK SIZE.

REINFORCE ALL NEW PLATFORM SLABS WITH 6x6 w2.9 x w2.9 WWM.

4.) ARCHITECTURAL BACKGROUND IS SHOWN FOR REFERENCE ONLY.
REFER TO ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS OF WALLS.

5.) REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATION OF ALL LINTELS.

6.) CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AS REQUIRED FOR

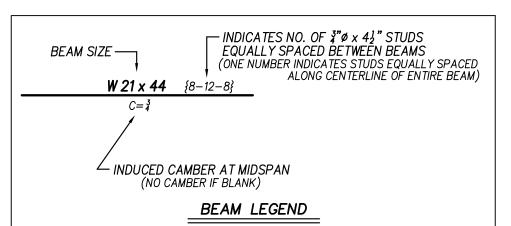
7.) LOCATE ALL WALLS AND MASONRY OPENINGS PER ARCHITECTURAL DRAWINGS.
WHERE REQUIRED, SEE S1102 FOR ALL NEW CMU LINTELS.

8.) NO FABRICATION OR ERECTION SHALL COMMENCE PRIOR TO THE APPROVAL OF ALL STRUCTURAL STEEL SHOP DRAWINGS BY THE ENGINEER OF RECORD.

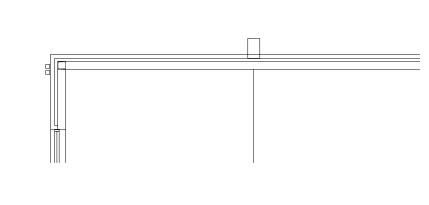
9.) COORDINATE AND VERIFY ALL DECK EDGE LOCATIONS AND DIMENSIONS

WITH ARCHITECTURAL DRAWINGS.

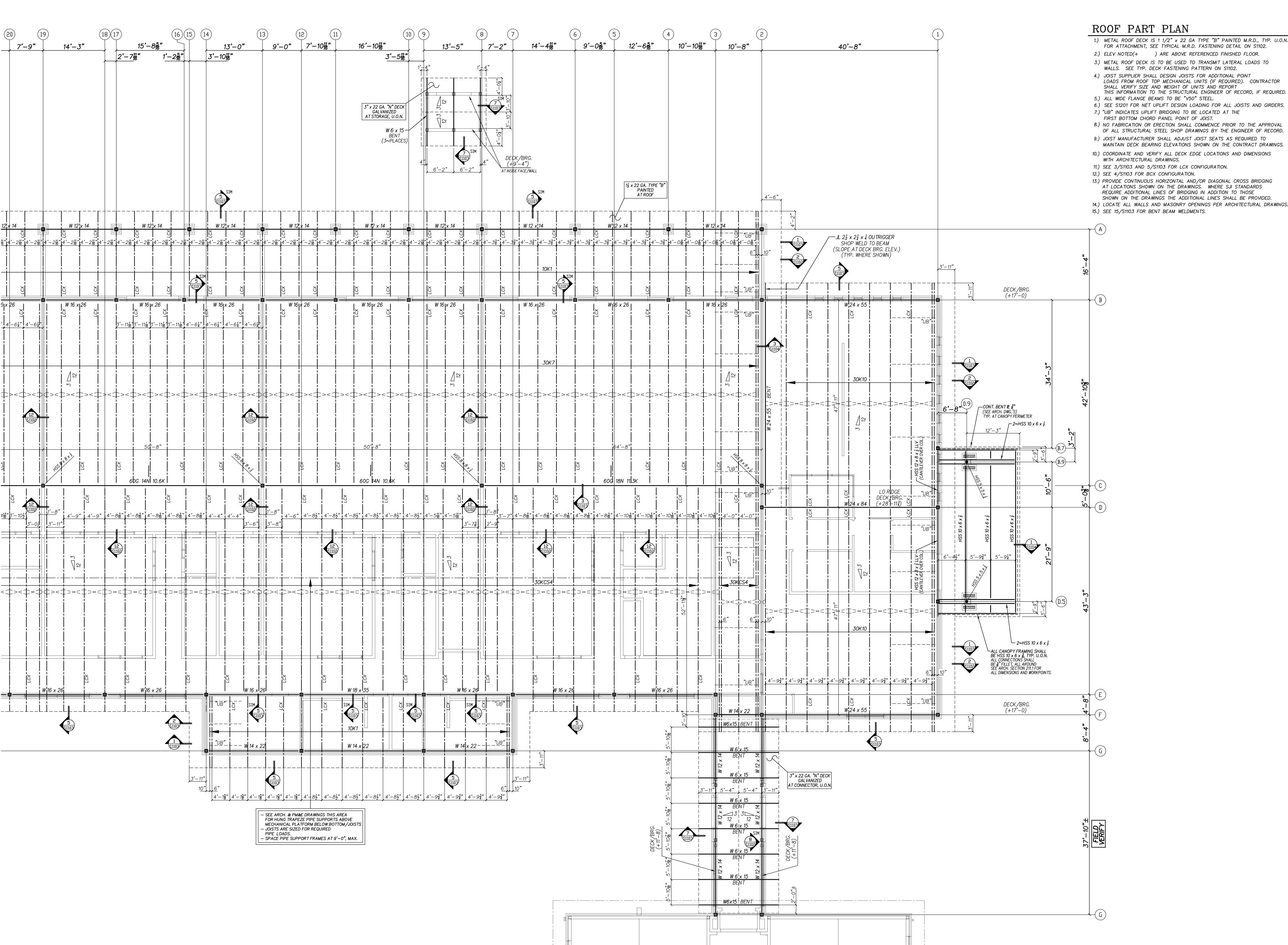
10.) REFERENCE ARCHITECTURAL AND PLUMBING DRAWINGS FOR COORDINATION OF SLOPED FLOORS AT FLOOR DRAINS, AND DEPRESSED FLOOR SLAB LOCATIONS.



 $\frac{1}{8}$ " = 1'-0"



26 FEB 2025



1.) METAL ROOF DECK IS 1 1/2" x 22 GA TYPE "B" PAINTED M.R.D., TYP. U.O.N. FOR ATTACHMENT, SEE TYPICAL M.R.D. FASTENING DETAIL ON S1102.

2.) ELEV NOTED(+) ARE ABOVE REFERENCED FINISHED FLOOR.

LOADS FROM ROOF TOP MECHANICAL UNITS (IF REQUIRED). CONTRACTOR SHALL VERIFY SIZE AND WEIGHT OF UNITS AND REPORT

THIS INFORMATION TO THE STRUCTURAL ENGINEER OF RECORD, IF REQUIRED.

7.) "UB" INDICATES UPLIFT BRIDGING TO BE LOCATED AT THE

8.) NO FABRICATION OR ERECTION SHALL COMMENCE PRIOR TO THE APPROVAL OF ALL STRUCTURAL STEEL SHOP DRAWINGS BY THE ENGINEER OF RECORD.

9.) JOIST MANUFACTURER SHALL ADJUST JOIST SEATS AS REQUIRED TO MAINTAIN DECK BEARING ELEVATIONS SHOWN ON THE CONTRACT DRAWINGS.

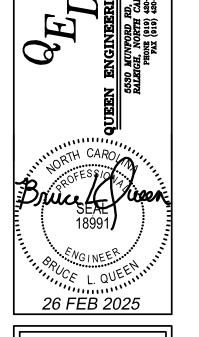
10.) COORDINATE AND VERIFY ALL DECK EDGE LOCATIONS AND DIMENSIONS

AT LOCATIONS SHOWN ON THE DRAWINGS. WHERE SJI STANDARDS REQUIRE ADDITIONAL LINES OF BRIDGING IN ADDITION TO THOSE SHOWN ON THE DRAWINGS THE ADDITIONAL LINES SHALL BE PROVIDED.

14.) LOCATE ALL WALLS AND MASONRY OPENINGS PER ARCHITECTURAL DRAWINGS.

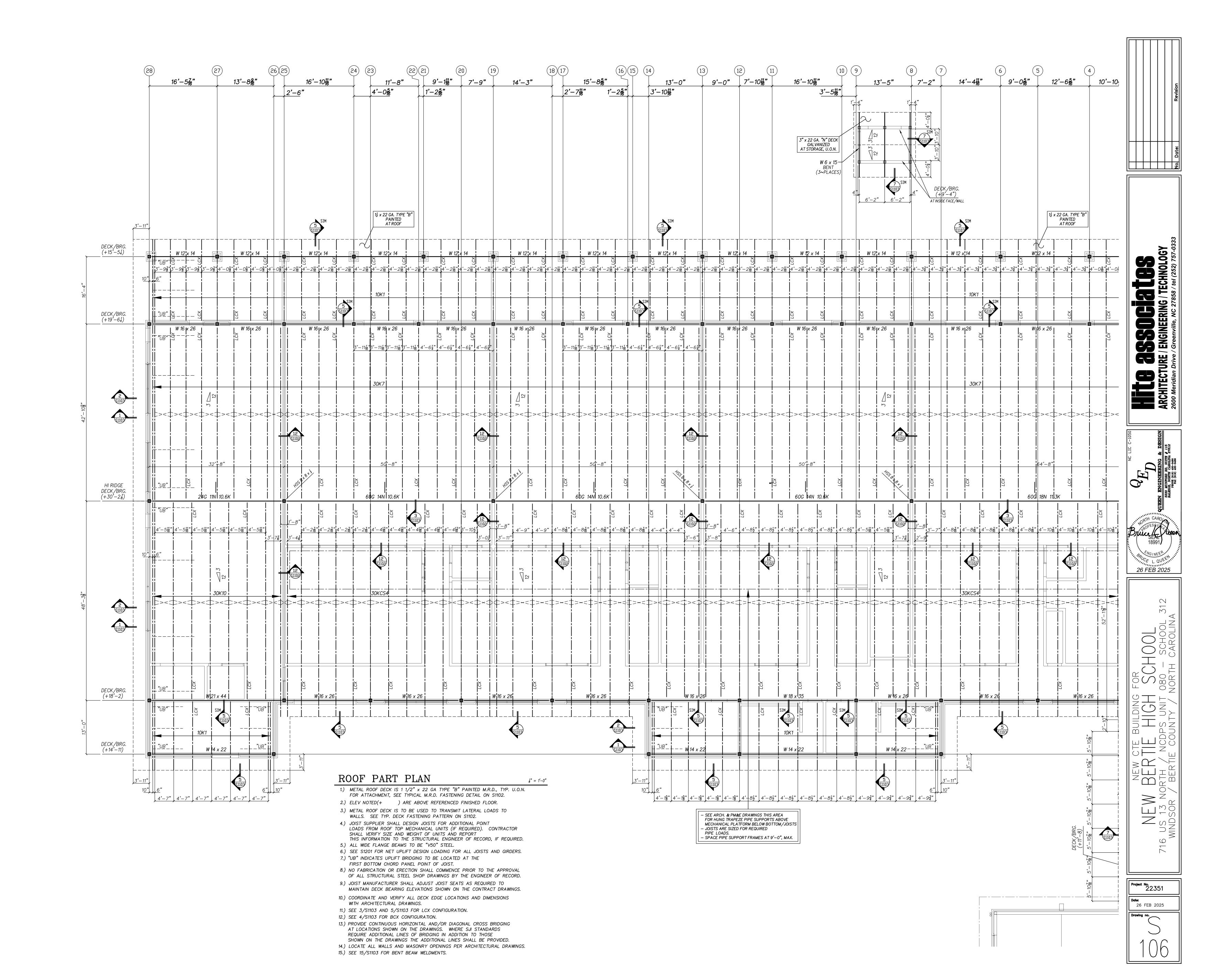
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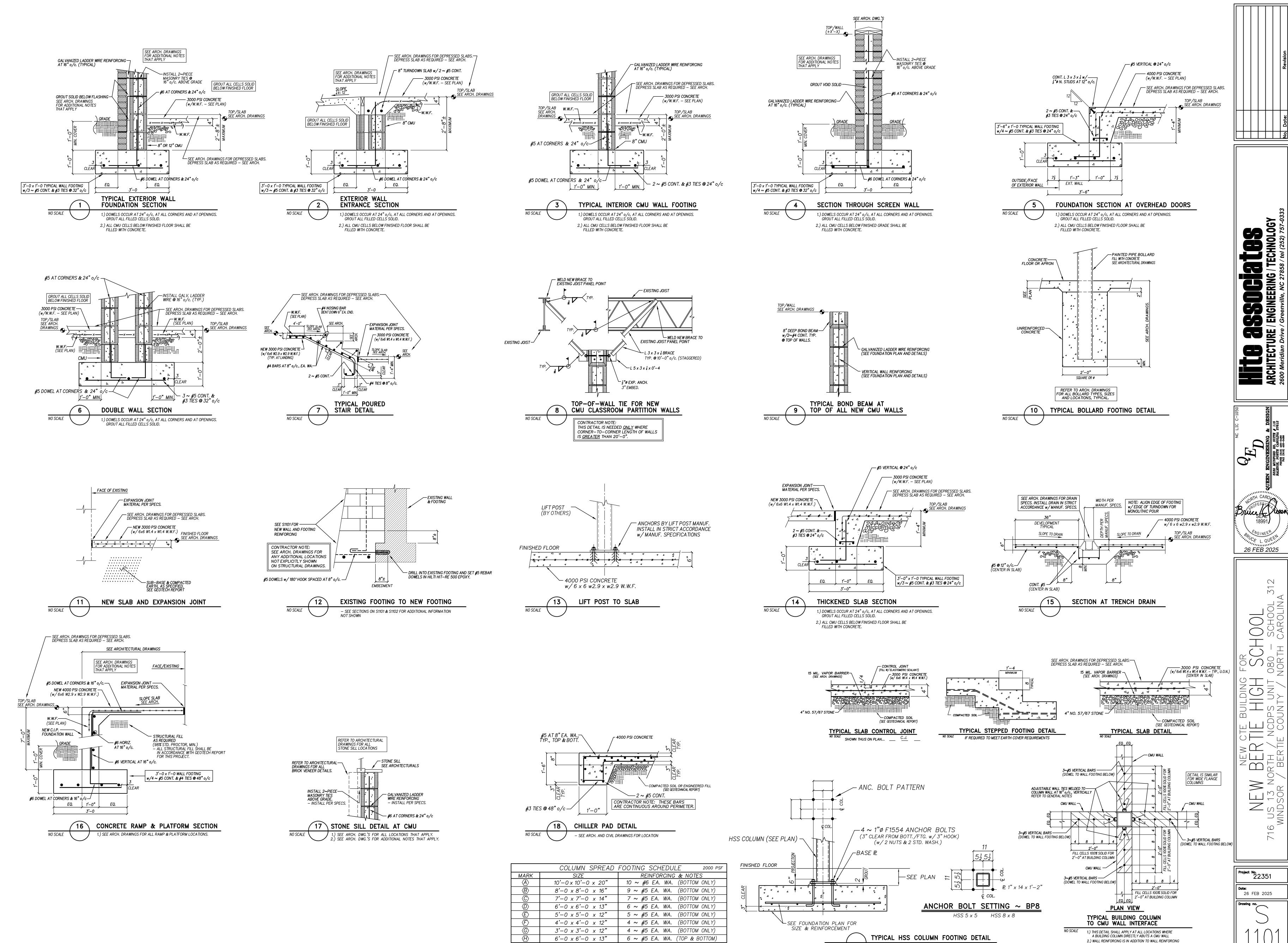
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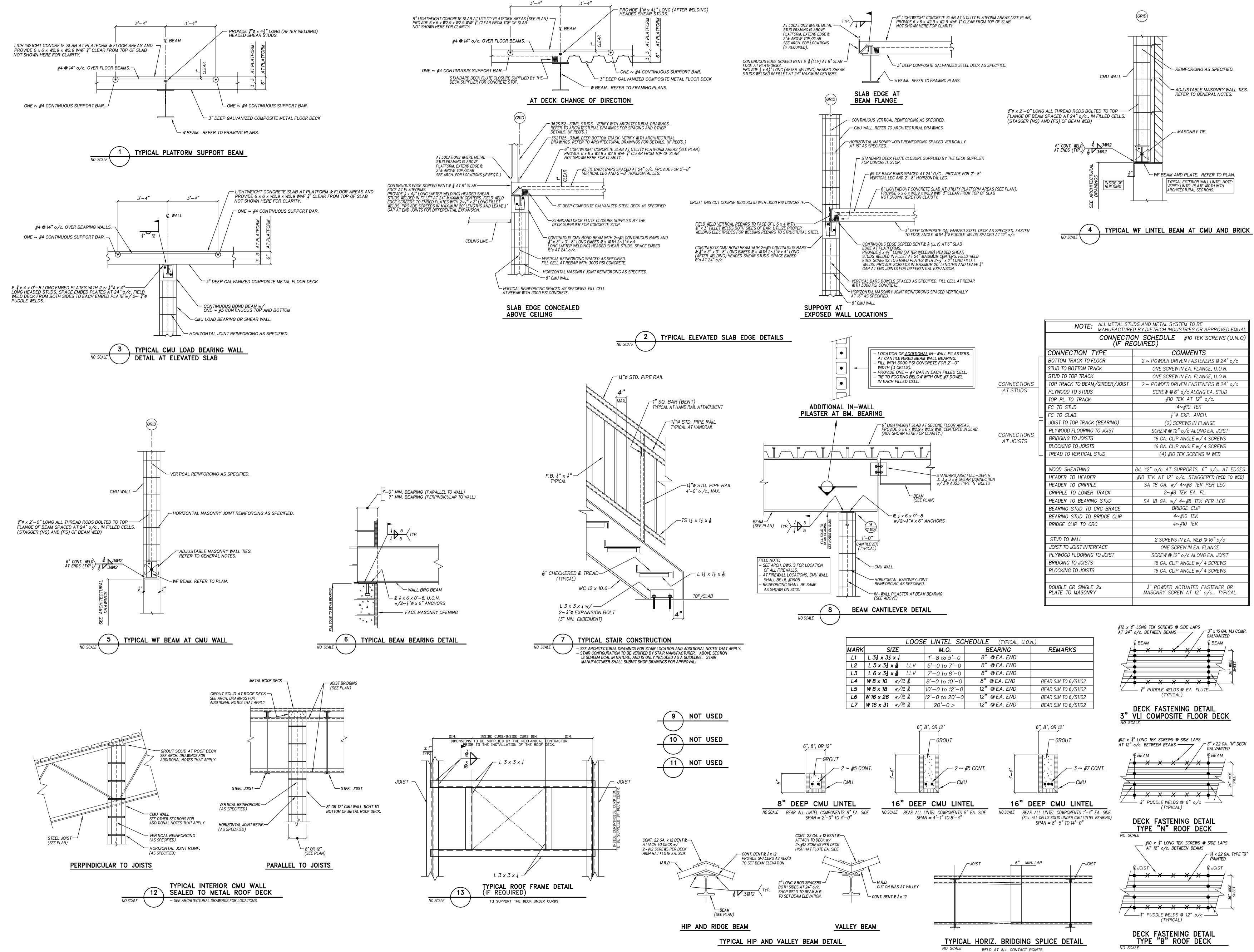
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SHOWN ON PLANS AND OTHER DETAILS THAT APPLY.

& TYPICAL ANCHOR BOLT PATTERN AS NOTED



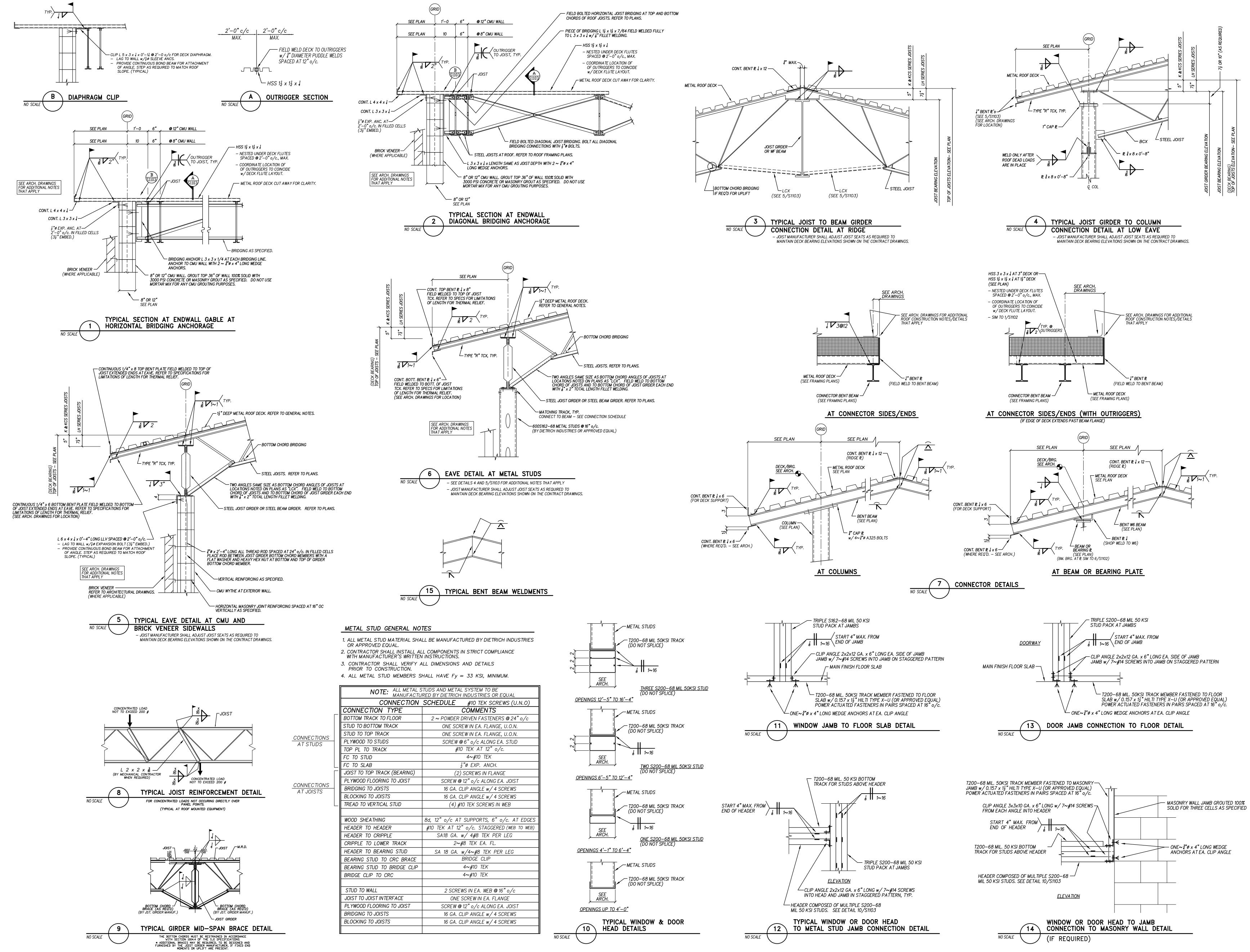
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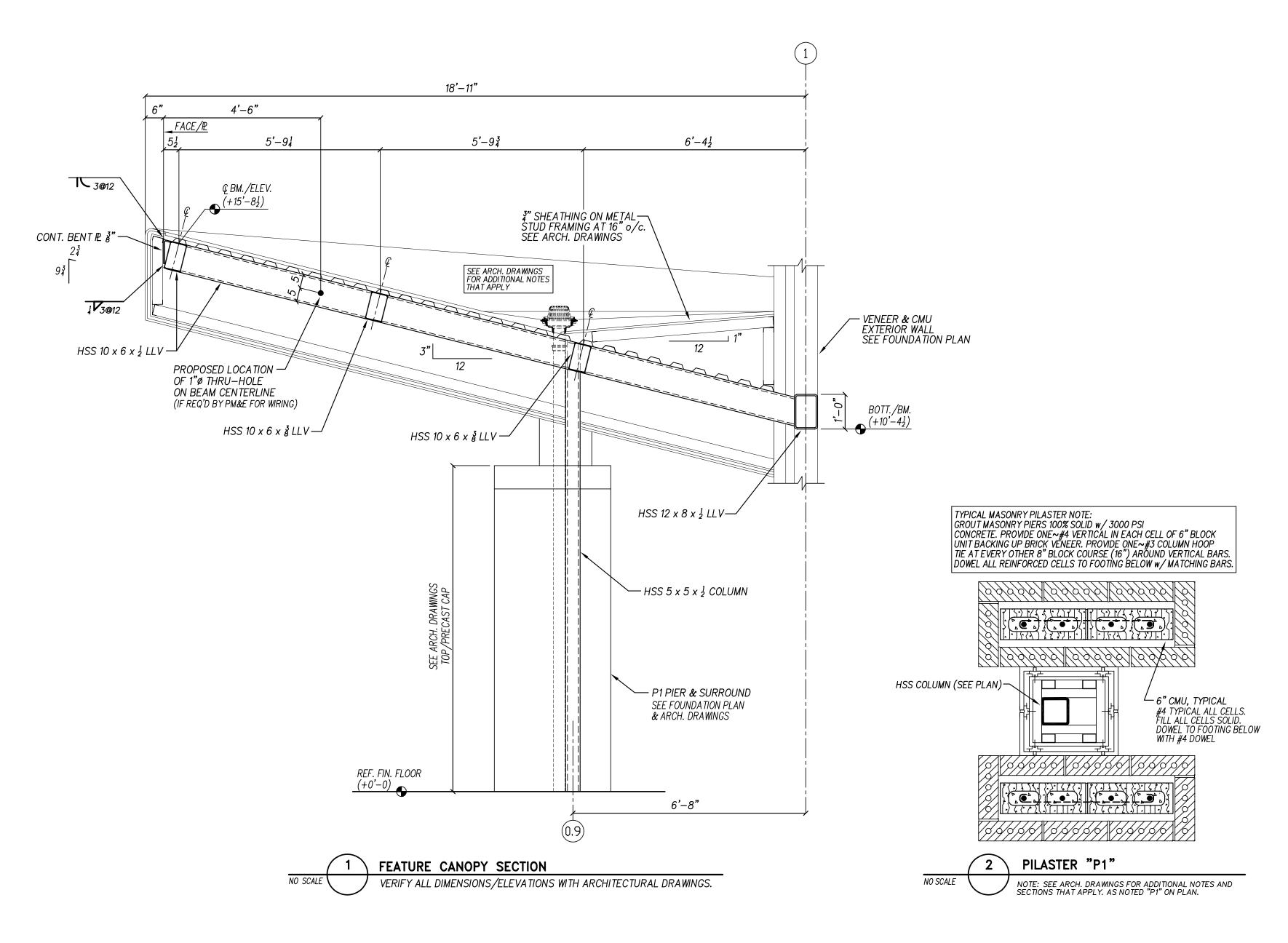


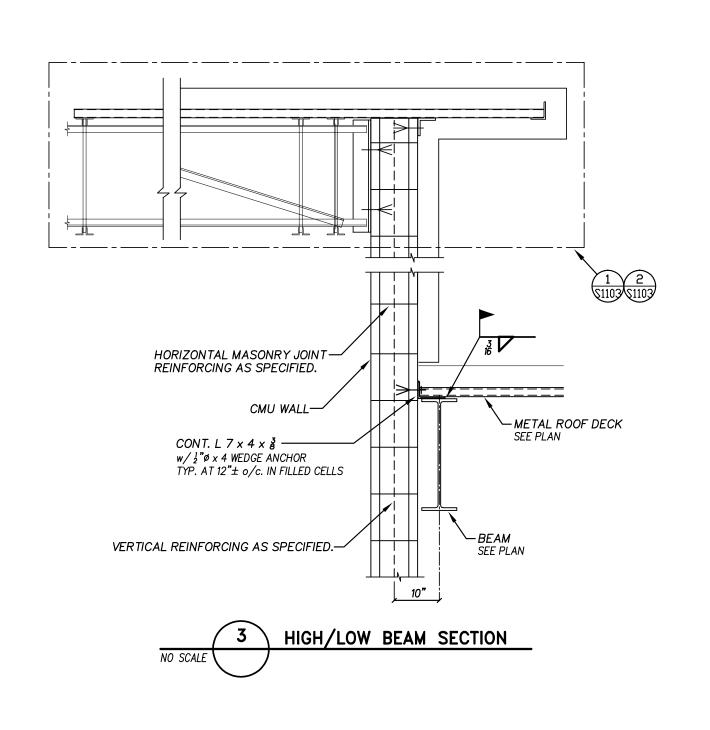
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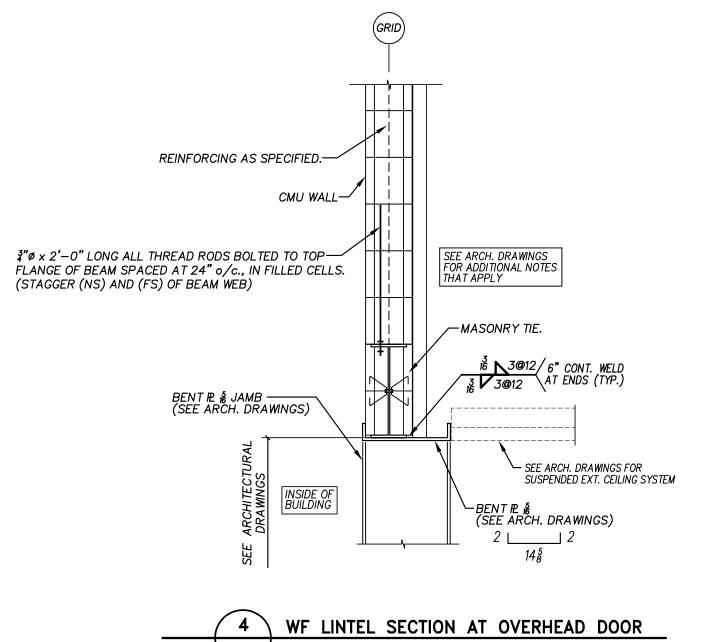
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Project No. 22351

Date: 26 FEB 2025

Drawing no. 4

- P. CONTINUOUS WALL FOOTINGS SHALL BE PLACED UPON BEARING SOIL HAT IS EITHER UNIMPROVED OR IMPROVED TO THE SAME ALLOWABLE NET BEARING VALUE AS THE CONTIGUOUS COLUMN FOOTINGS.
- 3. SITE CONDITIONS MAY DICTATE THAT THE SOIL IMPROVEMENT MEASURES BE TRANSITIONED OUT FROM THE AREAS OF UNIMPROVED OR SHALLOW IMPROVED DEPTHS. SUCH TRANSITIONING SHALL BE AS DIRECTED BY THE GEOTECHNICAL LABORATORY AT THE TIME OF
- 4. SITE PREPARATION AND PLACEMENT OF ENGINEERED COMPACTED FILL AND THE INTERMEDIATE SOIL IMPROVEMENT WORKS SHALL BE MONITORED BY THE GEOTECHNICAL LABORATORY. ALL NECESSARY PREPARATORY STRIPPING. CUTTING. PROOF ROLLING. AND FILLING AND IMPROVEMENT OPERATIONS SHALL BE SO MONITORED.
- 5. ALL FILL INSIDE THE BUILDING AND TO 10' OUTSIDE THE BUILDING INCLUDING RAMPS. STOOPS. AND STEPS SHALL BE CLEAN SELEC MATERIAL FREE OF DELETERIOUS MATERIALS SUCH AS WOOD, ROOTS, TRASH, OR OTHER EXTRANEOUS MATERIALS. PLACE FILL TO BE COMPACTED IN 9" LIFTS. MEASURED LOOSE, AND COMPACT EACH LIFT O 95% MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT AS MEASURED BY ASTM D698. COMPACT THE TOP THREE 9" LIFTS TO 100% MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT AS MEASURED BY
- 6. ALL FOOTING EXCAVATIONS SHALL BE APPROVED BY THE GEOTECHNICAL LABORATORY PRIOR TO PLACING FOOTING CONCRETE.

7. FOOTING ELEVATIONS SHALL NOT BE RAISED OR LOWERED UNLESS

- SPECIFICALLY APPROVED BY THE ARCHITECT. 8. FOOTINGS MAY BE CARRIED TO LOWER ELEVATION WHERE DIRECTED BY
- THE ARCHITECT. 9. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONAL LOCATIONS OF MASONRY WALLS AND PARTITIONS THAT REQUIRE CONTINUOUS WALL FOOTINGS AND/OR MONOLITHIC THICKENED SLAB FOOTINGS FOR
- 10. CONSTRUCTION JOINTS IN CONTINUOUS WALL FOOTINGS SHALL BE MADE MIDWAY BETWEEN COLUMNS AND AT LEAST 4' FROM THE INTERSECTION OF ANOTHER WALL FOOTING.
- 11. COLUMN FOOTINGS IN LINE WITH WALL FOOTINGS SHALL BE PLACED MONOLITHICALLY AND FLUSH TOP WITH THE CONTIGUOUS WALLS
- 12. STEPPED WALL FOOTINGS IF REQUIRED SHALL START OR TERMINATE AT LEAST 4' FROM A COLUMN FOOTING, WALL CORNER, OR WALL INTERSECTIONS.
- 13. AT LOCATIONS WHERE SLAB BLOCK OUTS ARE REQUIRED AWAITING THE STEEL COLUMN ERECTION PROVIDE FOR A SLAB TURNDOWN OF 8" MINIMUM CONCRETE THICKNESS AROUND THE BLOCKOUT TO RETAIN THE SUPPORTING SOIL UNDER THE SLAB. AT THE CONTRACTORS OPTION THIS SOIL RETAINING TURNDOWN MAY BE CONSTRUCTED WITH 8" CMU FILLED 100% SOLID WITH 3000 PSI CONCRETE.
- 14. FOUNDATIONS SHALL BE PLACED ONLY ON APPROVED NATURAL UNDISTURBED SOIL STRATA OR ON PROPERLY PLACED ENGINEERED NTROLLED COMPACTED IMPROVED FILL UNDER THE SUPERVISION OF THE GEOTECHNICAL LABORATORY.

1. CONCRETE SHALL DEVELOP THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS AT 28 DAYS:

- INTERIOR SLABS ON GRADE 3000 PSI FILL FOR MASONRY UNIT OTHER INTERIOR CONCRETE 3000 PS 4000 PS EXPOSED EXTERIOR CONCRETE SUSPENDED SLABS ON METAL DECK - 3000 PSI MASONRY CAVITY WALL FILL - 3000 KSI
- 2. CONCRETE FOR FOOTINGS AND SLABS ON GRADE SHALL BE REGULAI STONE CONCRETE.
- 3. CONCRETE FOR SLABS SUSPENDED ON COMPOSITE METAL DECK SHALL BE STRUCTURAL LIGHTWEIGHT CONCRETE WITH MAXIMUM AIR UNIT WEIGHT 4. CONCRETE FOR FILL IN CONCRETE MASONRY BLOCK CELLS, BOND
- BEAMS, LINTEL BLOCKS, AND CAVITY WALL FILL BELOW FLOOR IN EXTERIOR WALLS AND OTHER MASONRY UNITS SHALL BE FINE AGGREGATE CONCRETE MASONRY GROUT CONFORMING TO ASTM C476 OR 3000 PSI CONCRETE WITH 3/8" MAXIMUM COARSE AGGREGATE SIZE AT CONTRACTORS OPTION 3000 PSI SELF CONSOLIDATING CONCRETE ROUT MAY BE UTILIZED FOR MASONRY GROUTING PURPOSES WITH PROPER PRECAUTIONS TAKEN FOR THE HYDROSTATIC PRESSURE ISSUES.
- 5. UNLESS SELF CONSOLIDATING CONCRETE GROUT IS UTILIZED THE CONCRETE MIX FOR CONCRETE MASONRY BLOCK AND CAVITY WALL GROUT SHALL BE PROPORTIONED AT THE PLANT FOR A HIGH SLUMP OF 8" TO 1" TO FACILITATE PLACEMENT AND TO ACCOMMODATE WATER ABSORPTION BY THE MASONRY UNITS. HIGH SLUMP SHALL B ACHIEVED WHILE KEEPING THE WATER CEMENT RATIO IN THE NORMA RANGE FOR A 3000 PSI MIX. THIS WILL REQUIRE ADDITIONAL CEMEN' IN THE BATCH TO GO ALONG WITH THE WATER ADDED FOR THE SLUMP REQUIREMENT.
- 6. MORTAR MIX SHALL NOT BE ALLOWED FOR ANY BLOCK MASONRY FILL
- 7. CONCRETE TO BE PERMANENTLY EXPOSED TO WEATHER SHALL HAVE 5% (+/-1%) AIR ENTRAINMENT.
- 8. CONCRETE NOT PERMANENTLY EXPOSED TO THE WEATHER SHALL NOT HAVE AIR ADDED BY ENTRAINMENT. THIS REQUIREMENT SHALL BE VERIFIED AND REPORTED BY LABORATORY TESTS.
- 9. ALL CONCRETE WORK SHALL CONFORM TO "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE." ACI 318.
- 10. OBSERVE ALL AND STRICTLY FOLLOW ALL ACI 305 AND 306 REQUIREMENTS RESPECTIVELY FOR PROTECTION OF CONCRETE IN HOT AND COLD WEATHER.
- 11. ALL CONCRETE SLAB WORK SHALL BE PROPERLY CURED IN CONFORMANCE WITH ACI 308. EITHER WATER CURING. WATERPROOF PAPER CURING. PLASTIC SHEET. OR SPRAY—ON SEALING MATERIALS METHOD MAY B USED PROVIDED THAT THE METHOD CHOSEN HAS NO DETRIMENTAL EFFECT ON THE FINAL FINISH SPECIFIED FOR THE RESPECTIVE AREAS. THE PROPOSED CURING METHOD TO BE USED SHALL BE APPROVED BY THE ARCHITECT.
- 12. BUILDING SLABS ON GRADE SHALL BE 4" MINIMUM THICKNESS. 13. BUILDING SLABS ON COMPOSITE METAL FLOOR DECK IF REQUIRED AT THE UTILIT
- PLATFORM AREAS SHALL BE 6" MINIMUM THICKNESS NECESSARY FOR ACHIEVING THE REQUIRED 1 HOUR FIRE SEPARATION RATING. 14. PLACE 1/4" PRE-FORMED, IMPREGNATED EXPANSION JOINT FILLER
- FULL DEPTH OF SLAB ON GRADE AT ABUTTING WALL SURFACES UNLESS OTHERWISE NOTED. 15. PROVIDE CONSTRUCTION OR CONTROL JOINTS IN SLABS ON GRADE IN
- LOCATIONS AS SHOWN ON FOUNDATION PLAN OR AT OTHER LOCATIONS APPROVED OR REQUIRED BY THE ARCHITECT. BUT SPACING OF JOINTS SHALL NOT EXCEED 12' IN ANY DIRECTION. 16. THE TYPE OF JOINT USED WHETHER CONTROL JOINT OR CONSTRUCTION
- JOINT IS THE OPTION OF THE CONTRACTOR UNLESS OTHERWISE NOTED ON THE DRAWINGS 17. SAW JOINTS AT CONTROL JOINTS IN THE CONCRETE SLABS SHALL BE
- MADE AS SOON AS THE CONCRETE HAS SUFFICIENT STRENGTH TO PREVENT SPALLING OF THE JOINT DUE TO THE ACTION OF THE SAW BUT IN NO CASE GREATER THAN 4 HOURS AFTER INITIAL PLACEMENT OF THE CONCRETE.
- 18. SLAB JOINT FILLER SHALL BE OF THE TYPE COMPATIBLE WITH THE FINAL FLOOR COVERING USED. SLAB JOINTS UNDER PERMANENT PARTITIONS OR CASE WORK NEED NOT BE FILLED.
- 19. CHAMFER EXPOSED EDGES AND CORNERS OF CONCRETE 3/4" UNLESS OTHERWISE NOTED.
- 20. SEE ARCHITECTURAL DRAWINGS FOR REQUIRED FLOOR FINAL FINISHES AND PROVIDE NECESSARY SLOPES, DEPRESSIONS, AND SLAB FINISH AS REQUIRED TO ACCEPT THE SPECIFIED FINAL FINISHES.
- 21. SEE ARCHITECTURAL DRAWINGS FOR ALL POURED RAMP LOCATIONS AND DIMENSIONS, TYPICAL. SEE ARCHITECTURAL DRAWINGS FOR ALL POURED STAIR LOCATIONS AND DIMENSIONS, TYPICAL. CONSTRUCT ALL POURED RAMPS AND STAIRS IN ACCORDANCE WITH STRUCTURAL SECTIONS 7 & 16/S1101 AND ARCH. DRAWINGS.

REINFORCING STEEL

- BARS SHALL BE ROLLED FROM NEW BILLET-STEEL CONFORMING TO SPECIFICATION FOR DEFORMED BILLET—STEEL BARS FOR CONCRETE REINFORCEMENT," ASTM A615, GRADE 60.
- 2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185.
- DETAIL AND FABRICATE REINFORCING STEEL IN ACCORDANCE WITH "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," ACI 315.
- 4. REINFORCING STEEL SHALL BE IN PLACE AND REVIEWED BY THE ARCHITECT PRIOR TO PLACING CONCRETE.
- 5. PROVIDE WWF IN FLAT SHEETS. ROLLED WWF WILL NOT BE ACCEPTED ON THIS PROJECT.
- 6. LAP ALL WWF END SPLICES TWO FULL MESHES AND ALL SIDE LAP SPLICES ONE FULL MESH AND TIE OFF WITH STANDARD TIE WIRES.
- 7. PLACE ONE LAYER OF 6 x 6 W2.9 x W2.9 WWF AT 3/4" CLEAR FROM TOP OF ELEVATED SLABS ON COMPOSITE METAL DECK AT SECOND FLOOR AND UTILITY PLATFORM SLABS. TIE WWF TO REINFORCING STEEL OVER STEEL BEAMS TO PREVENT MOVEMENT WHILE PLACING THE FRESH CONCRETE. IN ADDITION PROVIDE ONE LINE OF CONTINUOUS SLAB BOLSTERS AT MID SPAN OF ALL ELEVATED SLABS TO SUPPORT WWF IN THE PROPER POSITION AND TIE OFF TO PREVENT MOVEMENT DURING CONCRETE PLACEMENT OPERATIONS.
- 8. FABRICATE REBARS IN CONTINUOUS FOOTINGS, WALLS, BOND BEAMS, AND MONOLITHIC TURNED DOWN SLABS TO LONGEST PRACTICAL
- 9. LAP HORIZONTAL REBAR SPLICES A MINIMUM OF 40 BAR DIAMETERS BUT A MINIMUM OF 24" UNLESS OTHERWISE NOTED. PLAN REBAR SPLICES TO OCCUR AT POINTS OF MINIMUM STRESS UNLESS OTHERWISE
- 10. LAP VERTICAL REBAR SPLICES INCLUDING DOWELS FROM FOOTINGS IN CMU WALLS A MINIMUM OF 60 BAR DIAMETERS. 11. BARS IN INDIVIDUAL COLUMN SPREAD FOOTINGS SHALL NOT BE
- 12. TERMINATE CONTINUOUS BARS IN WALL FOOTINGS, WALLS, BOND BEAMS AND TURNED DOWN SLABS, WITH A STANDARD 90 DEGREE HOOK AT DISCONTINUOUS ENDS, CORNERS, AND INTERSECTIONS.
- 13. AT LOCATIONS REQUIRING VERTICAL DOWELS INTO FOOTINGS AND ELEVATED SLABS, THE PLACEMENT OF THE DOWELS SHALL MATCH THE SIZE AND CLOSELY MATCH THE LOCATION OF THE VERTICAL WALL REBARS REQUIRING THE DOWELS.
- 14. ALL DOWELS SHALL TERMINATE IN THE FOOTING OR ELEVATED SLAB WITH A STANDARD ACI 90 OR 180 DEGREE HOOK AS APPROPRIATE UNLESS SPECIFICALLY SHOWN OTHERWISE. DOWELS SHALL LAP THEIR MATCHING VERTICAL REBAR 60 BAR DIAMETERS.
- 15. PROVIDE THE FOLLOWING CLEARANCES FROM REBARS TO CONCRETE FACE UNLESS OTHERWISE NOTED ON DRAWINGS:
- EARTH FORMS 3 WALL FORMS TOP OF SLAB - 3/4"
- CMU WYTHE CENTER BARS IN CENTER OF WYTHE 16. UNLESS OTHERWISE NOTED OR SPECIFIED PROVIDE #6 VERTICAL
- REINFORCING AT 24" o/c. FOR EXTERIOR MASONRY WALLS. 17 LINLESS OTHERWISE NOTED OR SPECIFIED PROVIDE #5 VERTICAL REINFORCING AT 24" o/c. FOR INTERIOR MASONRY WALLS.
- 18. REFER TO "CONCRETE AND BRICK MASONRY" SECTION BELOW FOR SPECIAL REINFORCING FOR THE 12" AND 8" CMU FIRE WALLS.
- 19. PROVIDE INDUSTRY APPROVED REBAR CENTERING DEVICES FOR HOLDING VERTICAL REINFORCING BARS SECURELY IN THE CENTER OF THE CMU
- 20. SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION.

METAL ROOF DECK

- 1. METAL ROOF DECK ON CONNECTORS AND ENTRANCE VESTIBULES SHALL BE 3" DEEP, 22 GAGE, TYPE "N", GALVANIZED STEEL ROOF DECK UNITS WITH 24" COVER, CONFORMING TO STEEL DECK INSTITUTE (SDI) STANDARDS.
- 2. METAL ROOF DECK ON OPEN WEB STEEL JOISTS SHALL BE 1 1/2" DEEP, 22 GAGE, WIDE RIB PRIMED DECK UNITS WITH 36" COVER CONFORMING TO STEEL DECK INSTITUTE (SDI)
- 3. ALL 3" DEEP METAL ROOF DECKING SHALL BE WELDED TO STEEL SUPPORTING MEMBERS WITH 3/4" DIAMETER PUDDLE WELDS AT 8" o/c. AT ALL END LAPS AND ALL INTERMEDIATE SUPPORTS, EXCEPT THAT AT THE WELD SPACING SHALL BE DECREASED TO 4" o/c. AT CONTINUOUS DECK EDGE SUPPORTS AT LOW EAVES, GABLE EDGES, AND RIDGES. A HIPS AND VALLEYS WHERE DECK UNITS ARE CUT ON A BIAS PROVIDE 2 – 3/4" DIAMETER PUDDLE WELDS PER FLUTE.
- 4. SIDE LAPS FOR ALL 3" METAL ROOF DECK UNITS SHALL BE MECHANICALLY FASTENED AT 12" MAXIMUM o/c. WITH #12 SELF DRILLING TEK SCREWS.
- 5. 1 1/2" DEEP METAL ROOF DECKING SHALL BE WELDED TO STEE SÚPPORTING MEMBERS WITH 3/4" DIAMETER PUDDLE WELDS AT 12" o/c AT ALL END LAPS AND ALL INTERMEDIATE SUPPORTS, EXCEPT THAT AT THE WELD SPACING SHALL BE DECREASED TO 6" o/c. WITHIN 12' OF LOW EAVES, RIDGES, AND GABLE ENDS. THE WELD SPACING SHALL BI DECREASED TO 6" o/c. AT CONTINUOUS DECK EDGE SUPPORTS AT LOW EAVES AND RIDGES. AT HIPS AND VALLEYS WHERE DECK UNITS ARE CUT ON A BIAS PROVIDE 2 - 3/4" DIAMETER PUDDLE WELDS PER
- 6. SIDE LAPS FOR ALL 1 1/2" METAL ROOF DECK UNITS SHALL BE MECHANICALLY FASTENED AT 12" o/c. WITH #10 SELF DRILLING TEK
- 7. ALL DECKING UNITS SHALL BE LAID OUT BY THE SUPPLIER TO A PATTERN THAT PROVIDES FOR A MINIMUM THREE SPAN CONDITION AND SHALL BE LAPPED 3" MINIMUM AT END JOINTS OVER SUPPORTS.
- 8. DECK UNITS SHALL BE SECURELY FASTENED IN STRAIGHT LINES AND TO TRUE PLANES TO ACCEPT THE SPECIFIED ROOFING SYSTEMS. UNEVEN, LOOSE, BENT, OR OTHERWISE BADLY INSTALLED DECK WILL BE REJECTED AND SHALL BE REPLACED AT NO ADDITIONAL COST IF SO DIRECTED BY THE ARCHITECT.
- 9. PROVIDE ALL NECESSARY ACCESSORIES, SUCH AS CONTINUOUS RIDGE, HIP, VALLEY, EAVE, RAKE CLOSURE PLATES, SUMP PANS, ANOTHER DECK RELATED ACCESSORIES OF PROPER SIZE AND SHAPE TO PROVIDE A COMPLETE FIRST CLASS WEATHERTIGHT JOB. MINIMUM WIDTH OF THESE CLOSURE PLATES SHALL BE 8" AND SHALL BE 22 GAGE G60 GALVANIZED MATERIAL AND SHALL BE WELDED WITH PAIRS OF 3/4" DIAMETER PUDDLE WELDS SPACED AT 6" o/c. OR FASTENED INTO PLACE
- WITH PAIRS OF #12 SDST TEK SCREWS SPACED AT 6" o/c. 10. PROPER CARE SHALL BE TAKEN BY THE ERECTOR SO AS NOT TO BURN HOLES IN DECK OR SUPPORTING MEMBERS. BURNED HOLES OR DAMAGES
- SUPPORTING MEMBERS SHALL BE CAUSE FOR REJECTION. REJECTED WORK SHALL BE REMOVED AND REPLACED AT NO ADDITIONAL COST II SO DIRECTED BY THE ARCHITECT.
- 11. TOUCH UP WELDS AND ABRASIONS WITH ZINC RICH PAINT. 12. METAL DECKING SHALL BE INSPECTED BY ARCHITECT AND ROOFING
- 13. SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION.

COMPOSITE METAL DECK

INSTALLER PRIOR TO COVERING.

- . COMPOSITE METAL DECK FOR SUSPENDED FLOOR SLABS BE 3" DEEP, 16 GAGE, GALVANIZED COMPOSITE DECK TYPE 3 VLI AS MANUFACTURED BY VULCRAFT OR AN APPROVED EQUAL WITH ALL NECESSARY CLOSURES, FILLERS, AND OTHER ACCESSORIES TO GIVE A FIRST CLASS WORKMANLIKE INSTALLATION.
- 2. ONE LINE OF ADJUSTABLE HEIGHT TEMPORARY SHORING MAY BE REQUIRED IN SOME INSTANCES AT MIDSPAN. CONSULT WITH MANUFACTURER FOR HIS REQUIREMENTS ON TEMPORARY SHORING. HEIGHT OF THE SHORING IF USED SHALL BE CAREFULLY CONTROLLED TO KEEP THE SHORING 1/4" BELOW THE LEVEL OF THE BOTTOM OF THE BARE UNLOADED DECK ŚO THAT THE DECK IS NEVER IN A "CAMBERED" POSITION. THE WEIGHT OF THE FRESH CONCRETE WILL "SETTLE" THE DECK DOWN ONTO THE SHORING MEMBER AS THE CONCRETE IS BEING
- 3. TO MINIMIZE ERESH CONCRETE LEAKAGE AT SIDE LAPS, PLAN CONCRETE PLACEMENT SEQUENCE SO THAT THE WEIGHT OF THE FRESH CONCRETE IS PLACED ON THE TOPMOST SHEET OF DECK FIRST.
- 4. DECK SHALL BE LAID OUT FOR A MINIMUM 3 1/2" DECK BEARING AT END SUPPORTS.
- 5. FASTEN SIDE LAPS AT 24" o/c. WITH #12 SDST TEK SCREWS.
- 6. WELD DECK TO STEEL SUPPORTS OR MASONRY EMBED PLATES WITH 2 · 3/4" DIAMETER PUDDLE WELDS PER FLUTE.
- 7. SUBMIT SHOP DRAWINGS OF COMPOSITE FOR APPROVAL PRIOR TO

STEEL JOISTS AND JOIST GIRDERS

ACCESSORIES.

- 1. STEEL JOISTS SHALL CONFORM TO STEEL JOIST INSTITUTE (SJI) "KCS", "LH" SERIES JOISTS AND ACCESSORIES. 2. JOIST GIRDERS SHALL CONFORM TO SJI "G" SERIES GIRDERS AND
- 3. BEARING DEPTH OF SLOPING "K" AND "KCS" SERIES JOISTS SHALL BE 5" UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS.
- 4. BEARING DEPTH OF SLOPING "LH" SERIES JOISTS SHALL BE 7 1/2" UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS.
- 5. BEARING DEPTH OF "G" SERIES GIRDERS SHALL BE 7 1/2" MINIMUM. PROVIDE DEEPER BEARING DEPTHS AS REQUIRED.
- 6. REFER TO PLANS FOR OTHER SPECIAL JOIST END DEPTH BEARING
- JOIST GIRDER SYSTEMS. 3. "K" AND "KCS" SERIES JOISTS SHALL BEAR A MINIMUM OF 2 1/2" ON STEEL SUPPORTS UNLESS NOTED OTHERWISE ON THE DRAWINGS.

7. JOIST MANUFACTURER SHALL COORDINATE AND CONFIRM WITH THE

STEEL FABRICATOR ALL SPECIAL BEARING CONDITIONS FOR JOIST AND

- 9. "LH" SERIES JOISTS SHALL BEAR A MINIMUM OF 4" ON STEEL SUPPORTS UNLESS NOTED OTHERWISE ON THE DRAWINGS. 10. "G" SERIES JOIST GIRDERS SHALL BEAR A MINIMUM OF 6" ON STEEL
- SUPPORTS UNLESS OTHERWISE NOTED. JOISTS BEARING ONLY ON ONE SIDE OF A STEEL SUPPORT SHALL BEAR A MINIMUM OF 2" PAST THE CENTERLINE OF THAT SUPPORT UNLESS OTHERWISE NOTED.
- JOIST GIRDERS BEARING ONLY ON ONE SIDE OF A STEEL SUPPORT SHALL BEAR THE FULL LENGTH OF THE SUPPORT UNLESS OTHERWISE

13. THE BOTTOM CHORD STRUTS OF JOISTS OR JOIST GIRDERS SHALL NOT

- BE RIGIDLY FASTENED TO MASONRY WALLS UNLESS SPECIFICALLY NOTED TO DO SO ON THE DRAWINGS. . BOTTOM CHORD STRUTS OF JOISTS WHERE REQUIRED SHALL BE RIGIDLY ATTACHED TO STEEL SUPPORTS DURING THE STEEL ERECTION PROCESS.
- 5. BOTTOM CHORD STRUTS OF JOIST GIRDERS SHALL NOT BE WELDED TO SUPPORTS UNTIL THE FULL DEAD LOAD OF THE ROOF DECK AND INSULATION IS IN PLACE. ROOF SUPPORTED MECHANICAL APPARATUS
- IS NOT INCLUDED IN THIS REQUIREMENT. . ALL STEEL JOISTS REQUIRED TO SJI AND OSHA BE BOLTED TO THE COLUMN OR BEAM OR JOIST GIRDER SHALL BE BOLTED WITH 2 - 3/4" DIAMETER A325 BOLTS. DO NOT UTILIZE 1/2" DIAMETER BOLTS FOR
- 17. BOLTS IN JOIST GIRDER TO COLUMN CONNECTIONS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITION AS DEFINED BY THE AISC.
- 18. BOLTS IN JOIST TO JOIST GIRDER CONNECTIONS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITION AS DEFINED BY THE AISC. 19. ADDITIONALLY THE BOLTED ENDS OF JOIST CONNECTIONS SHALL BE

FIELD WELDED IN ADDITION TO THE BOLTING REQUIREMENTS AS NOTED

- ON THE DRAWINGS. 20. PROVIDE FIELD BOLTED CONNECTIONS UTILIZING 3/4" DIAMETER A325 BOLTS FOR COMMON JOISTS TO JOIST GIRDER CONNECTIONS AS REQUIRED FOR FRECTION BY SJI AND OSHA REGULATIONS. THESE CONNECTIONS SHALL BE FIELD WELDED IN ADDITION TO THE ERECTION
- 21. THE CAPACITY OF ANY BOLTS FOUND MISSING IN THE JOIST AND JOIST GIRDER CONNECTIONS SHALL BE MADE UP BY PROPER FIELD WELDING TO PROVIDE EQUAL STRENGTH OF THE MISSING BOLTS AS
- DIRECTED BY THE INSPECTING TESTING LABORATORY. 22. BOLT TIGHTENING AND FIELD WELDING OF CONNECTIONS SHALL BE
- VERIFIED AND REPORTED BY THE TESTING LAB. 3. BOTTOM CHORDS OF JOISTS DESIGNATED ON PLANS WITH A "BCX" SHALL BE RIGIDLY ATTACHED TO THE INDICATED JOISTS AND STEEL SUPPORTS DURING THE STEEL ERECTION PROCESS WITH FIELD WELDING. THESE FIELD WELDS SHALL BE OF SIZE AND LENGTH AS REQUIRED FOR 4 KIP UNFACTORED AXIAL LOAD. THIS IS A WELDING REQUIREMENT AND DOES AFFECT THE JOIST SUPPLIER'S DESIGN OF
- 24. PROVIDE CONTINUOUS HORIZONTAL AND/OR DIAGONAL CROSS BRIDGING AT LOCATIONS SHOWN ON THE DRAWINGS. WHERE SJL STANDARDS REQUIRE ADDITIONAL LINES OF BRIDGING IN ADDITION TO THOSE SHOWN ON THE DRAWINGS THE ADDITIONAL LINES SHALL BE PROVIDED.
- 25. FIFLD BOLTED DIAGONAL BRIDGING AS REQUIRED BY SJL SHALL BE BOLTED TO JOISTS AND AT INTERSECTIONS WITH 1/2" DIAMETER A307 MACHINE BOLTS. THESE BRIDGING CONNECTIONS DO NOT REQUIRE FIELD WELDING IN ADDITION TO BOLTING. MISSING OR MISALIGNED BOLTS SHALL BE COMPENSATED FOR FULLY BY FIELD WELDING. DO NOT UTILIZE 3/8" BOLTS FOR THIS REQUIREMENT.
- PROPER ALLOWANCE FOR EXTRA BOTTOM CHORD BRIDGING WHERE REQUIRED FOR UPLIFT SHALL BE MADE BY THE JOIST FABRICATOR.

27. ALL BRIDGING SHALL BE WELDED TO TOP AND BOTTOM CHORDS AT ALL

- POINTS OF CONTACT WITH WELDS OF SUFFICIENT SIZE AND CONFIGURATION TO DEVELOP 2 KIPS MINIMUM FORCE. ANY LARGER FORCES THAT MAY BE REQUIRED BY SJI SHALL BE DESIGNATED ON THE JOIST SHOP DRAWINGS TO BE PROVIDED BY THE JOIST ERECTOR. 8. REFER TO DRAWINGS FOR ADDITIONAL FIELD WELDED DIAGONAL 21. UNLESS OTHERWISE NOTED OR SPECIFIED PROVIDE #6 VERTICAL BRIDGING AT ENDS OF BRIDGING RUNS.
- ALL JOISTS SHALL BE FIELD WELDED TO THE STEEL SUPPORT BEAM, COLUMN, OR BEARING PLATE WITH A 3/16" x 2" FILLET WELD OR EQUIVALENT FIELD WELD BOTH SIDES OF EACH JOIST SEAT AT EACH
- 30. ALL JOIST CHORD MEMBERS SHALL BE ANGLES.
- BRIDGING SHALL BE IN PLACE, AND WELDED FOR REVIEW BY THE ARCHITECT PRIOR TO INSTALLING THE METAL ROOF DECK. JOISTS AND ACCESSORIES SHALL RECEIVE ONE DIP COAT OF STANDARD GREY OXIDE PAINT WHERE PERMANENTLY EXPOSED TO VIEW. ALL PRIMERS SHALL BE COMPATIBLE WITH FIREPROOFING AND FINAL
- STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THEIR FABRICATOR FOR A NET UPLIFT OF NOT LESS THAN 33 PSF. ONE THIRD INCREASE IN ALLOWABLE STRESS MAY NOT BE UTILIZED FOR
- JOIST GIRDERS SHALL BE DESIGNED BY THE JOIST FABRICATOR WITH SELF WEIGHT OF THE GIRDER ADDED TO THE LOADS GIVEN ON THE
- PROPER CARE SHALL BE EXERCISED IN THE ERECTION AND ALL FIELD WELDING WORKS TO PREVENT DAMAGE TO THE JOIST MEMBERS. ANY JOIST FOUND TO BE DAMAGED SHALL BE REPLACED OR REPAIRED AS DIRECTED BY THE ARCHITECT.
- 6. PROVIDE SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION.

LIGHT GAGE METAL FRAMING

- 1. LIGHT GAGE METAL STUD LOCATIONS SHALL BE AS NOTED ON THE DRAWINGS.
- 2. ALL MATERIAL SHALL BE GALVANIZED AND ALL SCREWS SHALL BE SUITABLE GRADE STAINLESS STEEL OR ZINC PLATED. 3. ALL WELDS AND ABRASIONS IF ANY SHALL BE TOUCHED UP WITH ZINC
- 4. LIGHT GAGE HEADERS OVER DOOR AND WINDOW OPENINGS IN EXTERIOR WALLS SHALL BE COMPOSED OF MULTIPLE MEMBERS AS NOTED ON THE ALL MULTIPLE MEMBERS SHALL BE INTERCONNECTED SO AS TO ACT AS
- A COMPOSITE UNIT. ALL INDIVIDUAL MEMBERS COMPRISING A MULTIPLE MEMBER SHALL BE FULL LENGTH UNSPLICED MATERIAL. 6. ALL CONNECTIONS SHALL BE SCREWED OR WELDED TOGETHER BOTH SIDES WITH SUFFICIENT WELDS OR SCREWS TO SAFELY SUPPORT THE
- . ALL CONNECTIONS SHALL BE ACCOMPLISHED WITH STANDARD ACCESSORY COMPONENTS SUPPLIED FOR THAT PURPOSE.

LOADS TO BE IMPOSED ON THE CONNECTIONS.

ALL CONNECTION DESIGNS FOR THE LIGHT GAGE FRAMING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE SUBJECT TO THE APPROVAL OF THE ARCHITECT. REFER TO ARCHITECTURAL DRAWINGS FOR INTERIOR METAL STUD

PARTITION WALL REQUIREMENTS. PROVIDE BRACING FOR TOPS OF ALL

PARTITIONS TO MAIN STRUCTURE FOR PROPER PARTITION STABILITY.

PROVIDE NON-SHRINK, NON-METALLIC PRE-MIXED GROUT UNDER ALL COLUMN BASE PLATES AND BEAM BEARING PLATES. GROUT SHALL HAVE A COMPRESSIVE STRENGTH OF 7,000 PSI AT 7 DAYS.

CONCRETE AND BRICK MASONRY

- 1. CONCRETE MASONRY UNITS SHALL BE LIGHTWEIGHT OR BLENDED LIGHTWEIGHT UNITS CONFORMING TO ASTM C90 WITH A COMPRESSIVE STRENGTH OF 1900 PSI MINIMUM TESTED IN ACCORDANCE WITH ASTM = 36 KSI. Fu = 58 KSI.
- 2. BRICK VENEER UNIT SHALL BE FULLY FIRED VITRIFIED CLAY UNITS BOLTS - ASTM A325. FLAT WASHERS - ASTM F436. AS SELECTED BY THE ARCHITECT. WELDS - AWS CLASS E70 LOW HYDROGEN ELECTRODES. 3. ALL MORTAR SHALL BE TYPE "S" PER PROPORTION SPECIFICATION ASTM C270.
- 4. CONCRETE MASONRY DESIGN f'm = 1500 PSI.
- 5. WALLS SHALL BE ACCURATELY LAID OUT AND CONSTRUCTED SO THAT NO WALL VARIES MORE THAN 1/4" FROM ITS DESIGNATED LOCATION. THIS VARIATION LIMIT SHALL APPLY TO WALL PLUMBNESS AND WALL STRAIGHTNESS
- 6. ALL BED AND HEAD JOINTS SHALL BE FULLY MORTARED.
- 7. HORIZONTAL JOINT REINFORCING FOR ALL SINGLE WYTHE CONCRETE MASONRY WALLS SHALL BE HEAVY DUTY GALVANIZED TRUSS TYPE HORIZONTAL JOINT REINFORCEMENT BY HOHMAN AND BARNARD OF APPROVED EQUAL. HORIZONTAL JOINT REINFORCEMENT SHALL BE SPACED AT 16" o/c. MAXIMUM.
- 8. THE TRUSS TYPE HORIZONTAL JOINT REINFORCING SHALL HAVE 3/16" SIDE RODS AND #9 DIAGONAL CROSS RODS. LAP ALL REINFORCEMENT JOINTS A MINIMUM OF 12".
- HORIZONTAL JOINT REINFORCING FOR ALL MULTIPLE WYTHE EXTERIOR MASONRY WALLS SHALL BE GALVANIZED HEAVY DUTY TRUSS TYPI HORIZONTAL JOINT REINFORCEMENT BY HOHMAN AND BARNARD OF APPROVED EQUAL WITH 3/16" SIDE RODS AND #9 DIAGONAL CROSS RODS FOR THE CMU WYTHE WITH INTEGRAL DOUBLE EYE AND ADJUSTABLE 1/4" DIAMETER PINTLE BRICK TIE SPACED AT 24" o/c. HORIZONTALLY.
- 10. PROVIDE PREFABRICATED TEE AND CORNER PIECES AT WALL INTERSECTIONS.
- 11. ALL MASONRY WALLS SHALL BE BONDED AT CORNERS UNLESS OTHERWISE NOTED ON THE PLANS. REFER TO ARCHITECTURAL DRAWINGS FOR CORNER AND TEE WALL INTERSECTION REQUIREMENTS AT MASONRY PARTITIONS AND LOAD BEARING WALLS.
- 12. ALL CMU WALLS SHALL BE TIED TO ALL CONTACT FACES OF ALL STEEL COLUMNS WITH FIELD WELDED HEAVY DUTY ADJUSTABLE TIES AS NOTED ON THE DRAWINGS. PROVIDE MINIMUM 97 MIL GALVANIZED STAMPED STEEL ANCHORS WITH AN INTEGRAL VERTICAL SLOT TO LIMIT HORIZONTAL MOVEMENT OF THE TRIANGULAR WIRE TIE TO 1/16". DO
- NOT USE THE BENT WIRE TYPE ANCHORS TO THE COLUMN. ALL CMU WALLS SHALL BE FASTENED TO THE TOP AND BOTTOM FLANGES OF EMBEDDED STEEL BEAMS WITH THE 3/4" DIAMETER x 2'-0" LONG

ALL THREAD ROD DOWELS SPACED AT 24" o/c. OR AS NOTED ON THE

- 14. PROVIDE VERTICAL CRACK CONTROL JOINTS IN ALL MASONRY WALLS AT INTERVALS SHOWN ON THE ARCHITECTURAL DRAWINGS. CONTROL JOINT
- INTERVALS SHALL NOT EXCEED 32'. 15. UNLESS OTHERWISE NOTED LINTELS FOR OPENINGS UP TO 6'-4" IN CONCRETE MASONRY WALLS (CMU) SHALL BE BOND BEAM TYPE LINTELS OF PROPER LENGTH TO PROVIDE 16" BEARING EACH END. PROVIDE ONE - #5 CONTINUOUS TOP AND BOTTOM FOR 8" x 8" CMU BOND BEAM LINTËLS FOR OPENINGS UP TO 6'-4". PROVIDE 2 - #5 CONTINUOUS TOP AND BOTTOM FOR 12" x 8" CMU
- BOND BEAM LINTELS FOR OPENINGS UP TO 6'-4". WHERE A STEEL COLUMN OCCURS IN INTERIOR AND EXTERIOR CMU WALLS THAT PRECLUDE THE PROPER 16" MINIMUM BEARING LENGTH OF THE BOND BEAM TYPE LINTEL A STEEL BEAM AND PLATE LINTEL SHALL BE PROVIDED AND SHALL BE CONNECTED DIRECTLY TO THE STEEL COLUMN. REFER TO THE PLANS AND/OR LINTEL SCHEDULE FOR ALL
- 16. ALL DOOR, WINDOW, AND OPENING JAMBS IN CMU WALLS SHALL BE FILLED WITH 3000 PSI CONCRETE FOR 2'-0" WIDTH (3 CELLS) FROM LINTEL BEARING DOWN TO FLOOR. PROVIDE ONE - #6 BAR IN EACH

STEEL MEMBER LINTELS.

- ALL CONCRETE MASONRY UNITS BELOW FINISH MAIN FLOOR AND/OR BELOW FINISH GRADE SHALL BE 100% GROUTED SOLID WITH FINE AGGREGATE CONCRETE MASONRY GROUT CONFORMING TO ASTM C476 OF 3000 PSI CONCRETE WITH 3/8" MAXIMUM COARSE AGGREGATE SIZE. AT CONTRACTORS OPTION 3000 PSI SELF CONSOLIDATING CONCRETI GROUT MAY BE UTILIZED FOR MASONRY GROUTING PURPOSES WITH
- 18. IN ADDITION IN ALL MULTIPLE WYTHE MASONRY WALLS THE CAVITY COLLAR JOINT SPACE BETWEEN WYTHES BELOW GRADE SHALL BE GROUTED SOLID AT THE TIME THE BRICK WYTHE IS BEING LAID WITH FINE AGGREGATE CONCRETE MASONRY GROUT CONFORMING TO ASTM C476 OR 3000 PSI CONCRETE WITH 3/8" MAXIMUM COARSE AGGREGATE SIZE AT CONTRACTORS OPTION 3000 PSI SELF CONSOLIDATING CONCRETE GROUT MAY BE UTILIZED FOR MASONRY GROUTING PURPOSES WITH PROPER PRECAUTIONS TAKEN FOR THE HYDROSTATIC PRESSURE ISSUES

PROPER PRECAUTIONS TAKEN FOR THE HYDROSTATIC PRESSURE ISSUES.

- 19. UNLESS OTHERWISE NOTED PROVIDE WEEP HOLES AT 32" o/c. MAXIMUM ABOVE ALL WALL FLASHING LOCATIONS. REFER TO ARCHITÉCTURAL DRAWINGS FOR LOCATION AND DETAILS OF ALL FLASHING.
- CONTRACTOR SHALL BE RESPONSIBLE TO PROVIDE ADEQUATE TEMPORARY BRACING AND GUYING OF ALL MASONRY WALLS TO PROVIDE FOR SAFETY OF THE STRUCTURE AND WORKMEN. BRACING TO REMAIN UNTIL NO LONGER REQUIRED FOR SAFE SUPPORT OF WALLS.
- REINFORCING AT 24" o/c. FOR EXTERIOR MASONRY WALLS. 22. UNLESS OTHERWISE NOTED OR SPECIFIED PROVIDE #5 VERTICAL REINFORCING AT 24" o/c. FOR INTERIOR MASONRY WALLS. 23. UNLESS OTHERWISE NOTED ALL PROVIDE #6 VERTICAL REINFORCING SPACED AT 16" o/c. FULL HEIGHT FOR ALL FIREWALLS.

24. TERMINATE CONTINUOUS BARS IN WALL FOOTINGS, WALLS, BOND

AT DISCONTINUOUS ENDS, CORNERS, AND INTERSECTIONS.

BEAMS AND TURNED DOWN SLABS WITH A STANDARD 90° HOOK

CONTINUOUS WALL FOOTING REBARS TERMINATING IN COLUMN FOOTINGS SHALL EXTEND FULLY TO THE FAR FACE OF THE COLUMN FOOTING AND TERMINATE WITH A STANDARD 90° HOOK. 25. AT LOCATIONS REQUIRING VERTICAL WALL REINFORCING, PROVIDE MATCHING DOWELS INTO FOOTINGS AND ELEVATED SLABS. THE PLACEMENT

OF THE DOWELS SHALL MATCH THE SIZE AND CLOSELY MATCH THE

LOCATION OF THE VERTICAL WALL REBARS REQUIRING THE DOWELS.

- <u>WOOD AND PLASTICS</u> 1. ALL STRUCTURAL WOOD SHALL BE SYP #2 OR BETTER UNLESS OTHERWISE NOTED. 2. ALL TRUSSES SHALL BE DESIGNED BY AN ENGINEER LICENSED IN NORTH CAROLINA.
- 3. SUBMIT SHOP DRAWINGS FOR REVIEW BY ARCHITECT FOR APPROVAL. 4. ALL WOOD IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED OR ISOLATED BY METAL FLASHING, IF PERMITTED BY THE ARCHITECT IN WRITING. . NO STUD OR FRAMING MEMBER SHALL BE CUT FOR PIPING, DUCT WORK, ETC. WITHOUT THE PRIOR WRITTEN APPROVAL OF THE ARCHITECT OR ENGINEER OF RECORD.
- 7. ALL MASONRY VENEER WALL TIES, FRAMING ANCHORS, METAL TIES, HANGERS AND STRAPS SHALL BE TECO, KANT-SAG, SIMPSON STRONG-TIE, OR APPROVED EQUAL. B. APPLY MISCELLANEOUS FASTENERS USING ONLY FACTORY APPROVED HARDENED NAILS

6. DOUBLE HEADERS OR DOUBLE TRIMMERS SHALL BE PUT AROUND ALL STAIRWAYS

- FOR WOOD TO WOOD CONNECTIONS. 9. PROVIDE FASTENERS IN SIZES, QUANTITIES, AND SPACING REQUIRED BY FASTENER SCHEDULE IN NORTH CAROLINA STATE BUILDING CODE, LATEST RECOGNIZED EDITION. ALL EXTERIOR FASTENERS SHALL BE HOT-DIPPED GALVANIZED OR STAINLESS STEEL.
- 11. PROVIDE FURRING AND BLOCKING AS REQUIRED FOR TOILET CABINETS. CURTAINS TOWEL BARS, MIRROS, HAND RAILS, SPECIAL DECORATIVE ITEMS, AND INTERIOR TRIM. 12. ALL CONNECTIONS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS.

STRUCTURAL STEEL

- 1. STEEL WIDE FLANGE SHAPES SHALL CONFORM TO STRUCTURAL STEEL -ASTM A992, $F_V = 50$ KSI, $F_U = 65$ KSI. STEEL CHANNELS, PLATES, ANGLES, BARS, AND RODS — ASTM A36, FY PIPE COLUMNS – ASTM A53, GRADE B, Fy = 35 KSI, Fu = 60 KSI.
- TUBE COLUMNS ASTM A500, GRADE B, Fy = 46 KSI, Fu = 58 KSI. COLUMN ANCHOR RODS - ASTM F1554 GRADE 36. HEADED SHEAR STUDS - ASTM A108.
- ALL STEEL WORK SHALL CONFORM TO "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS - ALLOWABLE STRESS DESIGN" AND THE AISC CODE OF STANDARD PRACTICE.
- CONNECTION BOLTS SHALL BE 3/4" DIAMETER ASTM A325 BOLTS WITH PROPER HARDENED WASHERS AND HEX NUTS UNLESS OTHERWISE NOTED. ALL BOLTS AND INSTALLATION PROCEDURES SHALL BE IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING A325
- AND A490 BOLTS. 4. UNLESS OTHERWISE NOTED ALL CONNECTION BOLTS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITION AS DEFINED BY AISC.
- INSTALL A SUITABLE HARDENED WASHER UNDER THE HEAD OR NUT, WHICHEVER IS USED AS THE TURNED ELEMENT FOR TIGHTENING AND OVER ALL EXPOSED SLOTTED OR OVERSIZED HOLES. 6. UNLESS OTHERWISE NOTED ON THE PLANS ALL CONNECTION BOLTS

8. UNLESS OTHERWISE NOTED MASONRY SUPPORTED BEAMS BEARING

SHALL BE DESIGNED AS BEARING TYPE WITH TREADS IN THE SHEAR 7. ALL WELDING SHALL BE DONE BY WELDING OPERATORS CERTIFIED JOB. ACCORDING TO AWS D1. FOR THE WELDING POSITIONS AND WELDING EQUIPMENT BEING USED FOR MAKING THE CONNECTIONS REQUIRED FOR

PERPENDICULAR TO MASONRY WALLS SHALL BEAR A MINIMUM OF 6" ON

SOLID MASONRY AT EXPOSED 8" CMU WALLS AND 8" ON SOLID MASONRY

- AT CONCEALED 8" CMU WALLS AND 8" ON SOLID MASONRY AT 12" CMU WALLS. MASONRY SUPPORTED BEAMS AND PLATE LINTELS BEARING PARALLEL TO MASONRY WALLS SHALL BEAR A MINIMUM OF 12" ON SOLID MASONRY. 9. ALL COPES, CUTS, BLOCKS, NOTCHES SHALL HAVE SMOOTH RE-ENTRANT
- 10. RETURN ALL WELDS AT CORNERS A MINIMUM OF TWICE THE NOMINAL SIZE OF THE WELD

11. PROVIDE HOLES FOR BLOCKING AND NAILER BOLTS AS REQUIRED BY

CORNERS OF 1/2" MINIMUM RADIUS.

- ARCHITECTURAL DRAWINGS. STRUCTURAL STEEL SHALL RECEIVE ONE COAT OF GREY OXIDE PAINT OF 2 MIL DFT. SURFACE PREPARATION SHALL BE POWER TOOL CLEANING CONFORMING TO SSPC-SP3.
- CONTRACTOR SHALL BE RESPONSIBLE TO PROVIDE ADEQUATE TEMPORARY BRACING AND GUYING OF STEEL FRAMING AND LOAD BEARING WALLS TO PROVIDE FOR SAFETY OF THE STRUCTURE AND WORKMEN. BRACING TO REMAIN UNTIL NO LONGER REQUIRED FOR SAFE SUPPORT OF FRAME. 14. COLUMN CAP AND BASE PLATES SHALL BE FULLY WELDED ALL AROUND

TO COLUMN SHAFT WITH 5/16" FILLET WELDING UNLESS OTHERWISE

16. ALL COLUMN CAP PLATES SUPPORTING OR BEING CONNECTED TO OTHER

- 15. ALL COLUMNS SHALL HAVE A MINIMUM OF 4 ANCHOR RODS SET ON A
- MEMBERS SHALL BE 3/4" MINIMUM THICKNESS. 17. ALL OTHER COLUMN CAP PLATES NOT SUPPORTING OR BEING CONNECTED TO OTHER MEMBERS SHALL BE CAPPED WITH 1/2" MINIMUM THICKNESS.

18. SHEAR TAB PLATES ATTACHED TO COLUMN OR BEAMS SHALL BE

- MINIMUM THICKNESS AND SHALL BE WELDED TO THE SUPPORT MEMBER WITH 5/16" CONTINUOUS FILLET WELDS BOTH SIDES. 19. STABILIZER PLATES FOR JOIST BOTTOM CHORDS AT COLUMNS OR BEAMS SHALL BE 3/4" x 8" x 0'-8" MINIMUM AND SHALL BE SHOP WELDED
- TO COLUMN OR BEAM WITH 5/16" CONTINUOUS FILLET WELDS BOTH SIDES. STABILIZER PLATES FOR JOIST GIRDER BOTTOM CHORDS AT COLUMNS OR BEAMS SHALL BE 3/4" x 8" x 0'-8" MINIMUM AND SHALL BE SHOP WELDED TO COLUMN OR BEAM WITH 5/16" CONTINUOUS FILLET WELDS BOTH SIDES. UNLESS OTHERWISE NOTED PROVIDE 2" NOMINAL GROUT THICKNESS

UNDER COLUMN BASE PLATES TO ALLOW FOR A HEAVY HEX LEVELING

ON THE UNDERSIDE OF THE COLUMN BASE PLATE FOR PRECISION

ALL INTERMITTENT WELDS AND OTHER SEAMS ON BEAM AND PLATE

NUT AND STANDARD FLAT WASHER TO BE PLACED ON EACH ANCHOR BOLT

- LEVELING AND PLUMBING THE COLUMN. PROVIDE A STANDARD FLA WASHER UNDER A HEAVY HEX CLAMPING NUT ON TOP OF THE BASE PLATE FOR EACH ANCHOR BOLT ALL LINTEL ANGLES, BEAMS AND PLATES IN EXTERIOR WALLS SHALL BE HOT DIP GALVANIZED TO G60 STANDARDS AFTER FABRICATION
- PROVIDE 3/4" x 4 1/2" LONG HEADED FIELD WELDED SHEAR STUDS FOR ALL BEAMS SUPPORTING 3" COMPOSITE METAL DECK. SPACI HEADED SHEAR STUDS AT 12" o/c. AND PLACE STUDS IN THE STRONG

24. PROVIDE SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION.

LINTELS SHALL BE SEALED.

POSITION OF THE DECK FLUTES

AND BOTTOM

MASONRY OPENING 4'-1" TO 6'-8"

BRICK WYTHE L 6 x 4 x 3/8 LLV

LINTEL SCHEDULE FOR MASONRY OPENINGS NOT SPECIFIED OTHERWISE

EXTERIOR MASONRY WALL OPENINGS MASONRY OPENING UP TO 4'-0" 8" CMU WYTHE 8" \times 8" BOND BEAM w/2 - #5 CONTINUOUS TOP

BRICK WYTHE L 6 x 4 x 5/16 LLV 8" CMU WYTHE 8" \times 8" BOND BEAM w/2 - #5 CONTINUOUS TOP AND BOTTOM MASONRY OPENING 6'-9" TO 8'-0"

MASONRY OPENING UP TO 4'-0" BRICK WYTHE L 4 x 4 x 1/412" CMU WYTHE 12" x 8" BÓND BEAM w/ 2 - #5 CONTINUOUS TOP AND BOTTOM

8" CMU WYTHE 8" \times 8" BOND BEAM w/2 - #6 CONTINUOUS TOP

MASONRY OPENING 4'-1" TO 6'-8" BRICK WYTHE L 6 x 4 x 5/16 LLV 12" CMU WYTHE 12" x 8" BOND BEAM w/ 2 - #5 CONTINUOUS TOP

MASONRY OPENING 6'-9" TO 8'-0" BRICK WYTHE L 6 x 4 x 3/8 LLV 12" CMU WYTHE 12" x 8" BOND BEAM w/ 2 - #6 CONTINUOUS TOP AND BOTTOM

INTERIOR MASONRY WALL OPENINGS

AND BOTTOM

MASONRY OPENING UP TO 4'-0"

8" CMU WYTHE 8" x 8" BOND BEAM w/ 2 - #5 CONTINUOUS TOP AND BOTTOM MASONRY OPENING 4'-1" TO 6'-0" 8" CMU WYTHE 8" x 8" BOND BEAM w/ 2 - #5 CONTINUOUS TOP AND BOTTOM

8" CMU WYTHE W 8 x 10 w/ BOTTOM PLATE 1/4" x 7" FULL LENGTH

12" CMU WYTHE 12" x 8" BOND BEAM w/ 2 - #5 CONTINUOUS TOP

12" CMU WYTHE W 8 x 10 w/ BOTTOM PLATE 1/4" x 11" FULL LENGTH

MASONRY OPENING 4'-1" TO 6'-0" 12" CMU WYTHE 12" \times 8" BOND BEAM w/2 - #5 CONTINUOUS TOP AND BOTTOM MASONRY OPENING 6'-1" TO 8'-0"

SPECIAL INSPECTIONS 1. SPECIAL INSPECTIONS SHALL BE PERFORMED CONTINUOUSLY OR

- PERIODICALLY AS REQUIRED BY CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE (IBC BUILDING CODE) AS SCHEDULED BELOW. 2. SPECIAL INSPECTORS SHALL BE DULY CERTIFIED TO INSPECT THE WORK
- REQUIRING THE INSPECTION AS SCHEDULED. 3. SPECIAL INSPECTION NOTES:
- A) THE INSPECTOR SHALL MONITOR ALL SOIL CUTTING AND FILLING AREAS UNDER THE BUILDING AND TO 10' OUTSIDE THE BUILDING AREA IN ACCORDANCE WITH TABLE 1705.6 OF THE IBC BUILDING
- B) STEEL FABRICATION SHOPS NOT CERTIFIED BY AISC WILL REQUIRE IN SHOP INSPECTION BY THE SPECIAL INSPECTOR. AISC SHOPS MAY WAIVE THIS REQUIREMENT.
- C) ALL STRUCTURAL STEEL FABRICATION SHALL BE PERFORMED IN SHOPS WITH MINIMUM OF FIVE YEARS OF SATISFACTORY OMPLETION OF WORK SIMILAR TO WORK ON THIS PROJECT.
- WELD CERTIFICATION PAPERS FOR ALL FIELD WELDERS SHALL BE VERIFIED BY THE SPECIAL INSPECTOR. ALL FIELD WELDING OF LIGHT GAGE MEMBERS TO STRUCTURAL STEEL PARTS SHALL BE PERIODICALLY INSPECTED.
- LIGHT GAGE FRAMING SHALL BE PERIODICALLY INSPECTED. INSPECTION SHALL COVER METAL STUD SIZES, SHAPES THICKNESS, STRAIGHTNESS, SCREW SIZES AND NUMBER, STUD BEARING AGAINST TRACK WEBS. STRAP BRACING SIZE AND CONNECTION DETAILS. BRIDGING SIZE AND CONNECTION DETAILS WALL OPENING HEADER SIZE AND DETAILS, WALL ALIGNMENT AND
- PLUMBNESS, AND DETAILS OF WORKMANSHIP AND INTERCONNECTION OF LIGHT GAGE MEMBERS CONNECTED BY SCREWS OR FIELD SPECIAL INSPECTORS SHALL BE IN DIRECT COMMUNICATION WITH THE ENGINEER OF RECORD DURING THE INSPECTION PROCESS.

) FULL REPORTS OF ALL INSPECTIONS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD.

JEAD LOAD:

ROOF

CLASSROOM ADDITION | C

STATEMENT OF SPECIAL INSPECTIONS

STATEMENT OF SPECIAL INSPECTIONS

REINFORCING STEEL WELDING.

PLACEMENT OF CONCRETE.

TESTS. PERFORM SLUMP AND

TEMPERATURE OF THE CONCRETE.

IN HARDENED CONCRETE.

. INSPECTION OF BOLTS TO BE INSTALLED

. VERIFYING USE OF REQUIRED DESIGN MIX.

S. AT THE TIME FRESH CONCRETE IS SAMPLED

TO FABRICATE SPECIMENS FOR STRENGTH

INSPECTION OF CONCRETE AND SHOTCRETE

. INSPECTION FOR MAINTENANCE OF SPECIFIED

VERIFICATION OF IN-SITU CONCRETE STRENGTH,

INSPECT FORMWORK FOR SHAPE, LOCATOIN AND

1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS

ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING

. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER

DEPTH AND HAVE REACHED PROPER MATERIAL.

L. VERIFY USE OF PROPER MATERIALS, DENSITIES

5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE PERIODIC

3. PERFORM CLASSIFICATION AND TESTING OF

AND LIFT THICKNESSES DURING PLACEMENT

SUBGRADE AND VERIFY THAT SITE HAS BEEN

AND COMPACTION OF COMPACTED FILL.

COMPACTED FILL MATERIALS.

PREPARED PROPERLY.

DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.

FORMS FROM BEAMS AND STRUCTURAL SLABS.

PLACEMENT FOR PROPER APPLICATION

CURING TEMPERATURE AND TECHNIQUES.

IN CONCRETE PRIOR TO AND DURING

INSPECTION OF ANCHORS INSTALLED

INSPECTION OF REINFORCING STEEL, AND PLACEMENT.

ASSURANCE FOR WIND AND SEISMIC
THE FOLLOWING COMPONENTS (STEEL, MASONRY, DESIGNATED SEISMIC)

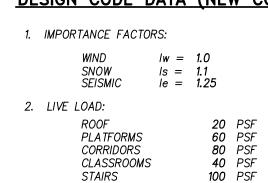
<u>DIMENSIONS</u> 1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REVIEWING ALL DIMENSIONS IN THE DRAWINGS AND ADVISING THE ARCHITECT OF ANY DIFFERENCES IN THE DIMENSIONS ON THE DRAWINGS PRIOR TO

DESIGN CODE DATA (NEW CONSTRUCTION)

24 PSF

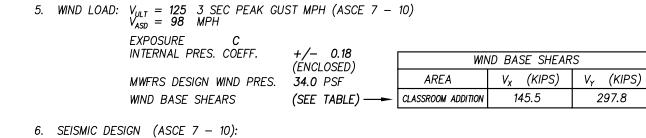
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COMMENCING CONSTRUCTION.

4. SNOW LOAD: 10.0 PSF



0.115 0.089 SEISMIC ANALYSIS RESULTS R Cs PROCEDURE COMPONENTS CONTROL V_X (KIPS) V_Y (KIPS)AREA |CATEGORY| SITE | USE D III B. BUILDING FRAME SYSTEM 4 0.036 EQUIV. LATERAL FORCE

BOLTING

ACI 318: 3.5, 7.1-7.7

ACI 318: 3.8.6, 8.1.3, 21.2.8

ACI 318: 8.1.3, 21.2.8

ACI 318: 5.9. 5..10

ACI 318: 5.11-5.13

ACI 318: 6.1.1

NOT PERMITTED AWS D1.4, ACI 318: 3.5.2

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ID MARKING

CALIBRATED TURN OF NUT

OTHER STEEL ID MARKS

WELDFILLER ID MARKINGS

SINGLE PASS FILLET 4

PROPORTIONS OF MATERIALS

FRAME BRACING

WELDMENTS

MASONRY

<u>MATERIALS</u>

MORTAR

SLUMP OF GROUT

MASONRY UNITS

MORTAR JOINTS

REINFORCEMENT

PLACEMENT OF GROUT

SIZE OF MASONRY UNITS

PREPARE GROUT PRISMS

LOCATIONS OF ANCHORS

SIZE OF REINFORCEMENT

WELDING OF REINFORCEMENT

VERIFY VERTICAL REINFORCEMENT

SIZE OF ANCHORS

LOCATIONS OF MASONRY UNITS

GRADE AND TYPE OF ANCHORS

VERIFY GROUT SPACE PRIOR TO GROU'

GRADE AND TYPE OF REINFORCEMENT

VERIFY CONNECTION TO FOUNDATION COLD WEATHER PROCEDURES (∠ 40° F

HOT WEATHER PROCEDURES (̀ \= 90° F́,

DESIGNATED SEISMIC SYSTEMS EXTERIOR WALL PANELS AND ANCHORAGE*

SPRINKLER PIPE CONNECTIONS*

MECHANICAL EQUIPMENT SUPPORTS*

DUCTWORK SUPPORTS*

SUSPENDED CEILINGS SYSTEMS AND ANCHORAGE*

MANUFACTURER'S CERTIFICATE OF COMPLIANCE.

ANCHORAGE OF EMERGENCY ELECTRICAL EQUIPMENT* PERIODIC

*VERIFY THAT THE LABEL, ANCHORAGE, OR MOUNTING CONFORMS TO THE

PLACEMENT OF

VERIFY f'm

MEMBER LOCATIONS

UNCALIBRATED TURN OF NUT

JOINT DETAILS AT EACH CONNECTION

MANUFACTURER'S CERTIFICATION

FLOOR AND ROOF DECK WELDS AND SCREWS

7. SOIL BEARING VALUE 2000 PSF (GEOTECHNICAL REPORT BY TERRACON (PROJECT NO. 72245090, DATED 20 DEC 2024)

1) MILL TEST REPORTS FOR ALL STRUCTURAL STEEL MATERIAL SHALL BE SUBMITTED FOR APPROVAL WITH THE SHOP DRAWINGS.

QUALITY ASSURANCE WIND AND SEISMIC SUBMITTALS

- 2) MILL TEST REPORTS FOR ALL STEEL JOISTS SHALL BE SUBMITTED
- WITH THE SHOP DRAWINGS FOR APPROVAL 4) OPEN WEB STEEL JOISTS SHALL BE MANUFACTURED IN AN SJI CERTIFIED
- 5) MILL TEST REPORTS FOR ALL METAL DECK AND STRUCTURAL PROPERTIES OF DECK PROFILES SHALL BE SUBMITTED WITH THE SHOP DRAWINGS FOR APPROVAL.
- 6) ALL METAL ROOF DECK SHALL BE ROLL FORMED IN A STEEL DECK INSTITUTE (SDI) CERTIFIED SHOP.
- ALL COMPOSITE METAL FLOOR DECK SHALL BE ROLL FORMED IN A STEEL DECK INSTITUTE (SDI) CERTIFIED SHOP. 6) MILL TEST REPORTS FOR ALL BOLTS AND SHEAR STUDS SHALL BE
- SUBMITTED FOR APPROVAL WITH THE SHOP DRAWINGS. 7) ALL FIELD WELDING SHALL BE PERFORMED BY WELDING PERSONNEL
- HOLDING CURRENT CERTIFICATION BY AWS FOR THE TYPES OF WELDING BEING PERFORMED. THE CERTIFICATION PAPERS SHALL BE EXAMINED
- AND APPROVED BY THE TESTING AGENCY. 8) TEST REPORTS FOR ALL CONCRETE MASONRY UNITS SHALL BE
- SUBMITTED PRIOR TO CONSTRUCTION. 9) TEST REPORTS FOR ALL MORTARS AND GROUTS SHALL BE SUBMITTED
- PRIOR TO CONSTRUCTION. 10) MILL TEST REPORTS FOR ALL REINFORCING STEEL SHALL BE SUBMITTED WITH THE SHOP DRAWINGS.

11) TEST REPORTS FOR ALL CONCRETE SHALL BE SUBMITTED PRIOR TO

CONSTRUCTION.

EXISTING CONDITIONS 1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REVIEWING ALL EXISTING JOB CONDITIONS. ANY ADVERSE EXISTING CONDITIONS AFFECTING WORK SHOWN ON THESE DRAWINGS SHALL BE BROUGHT TO

THE ATTENTION OF THE ARCHITECT FOR POSSIBLE CLARIFICATION OF RECONCILIATION.

297.8

CONSTRUCTION SAFETY 1. THESE DRAWINGS DO NOT CONTAIN THE REQUIREMENTS FOR JOB SAFETY. ALL PROVISIONS FOR SAFETY SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

| ANCHORED | WIND | 132 | 132

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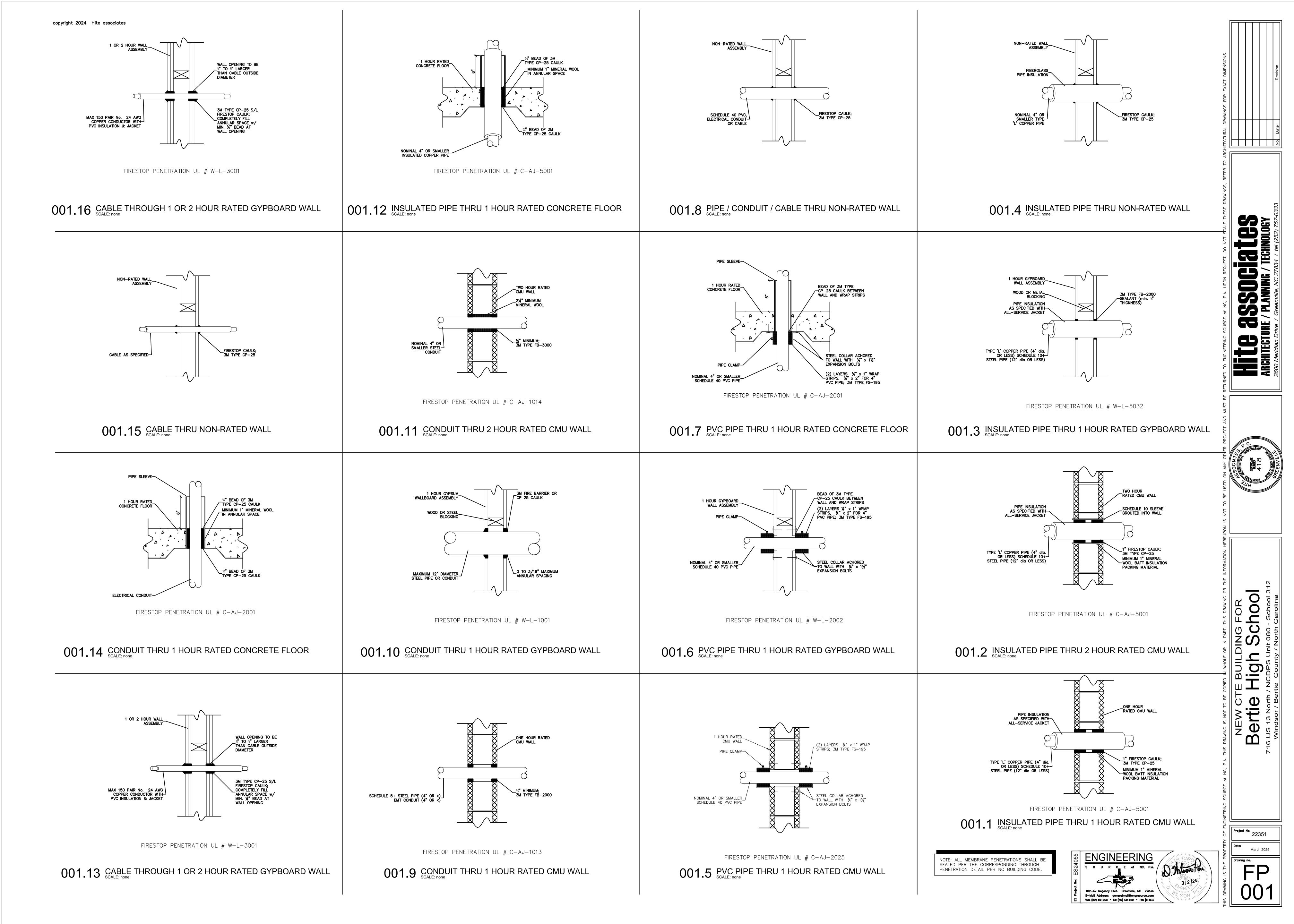
TMS 602 WIND AND SEISMIC

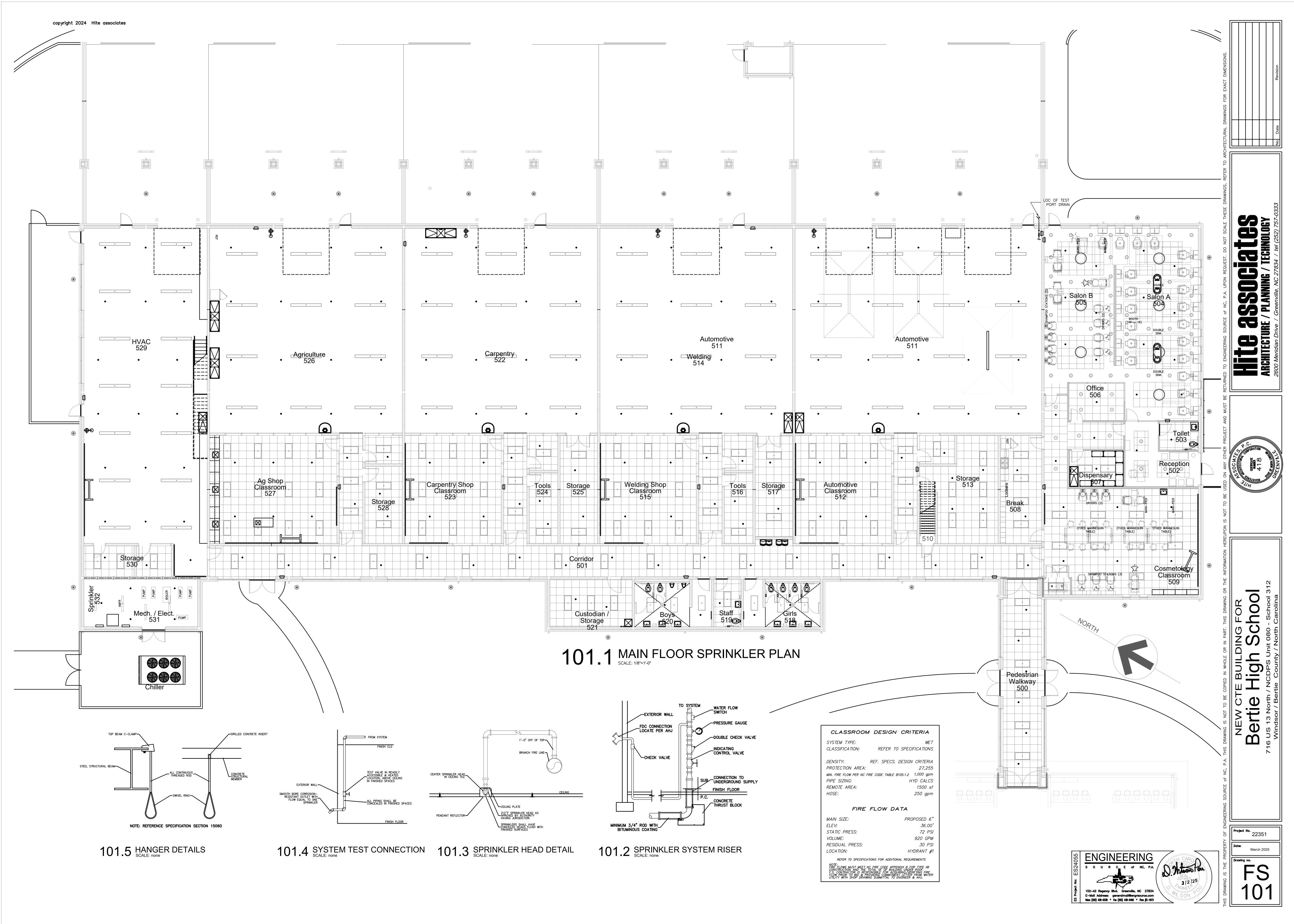
WIND AND SEISMIC

9

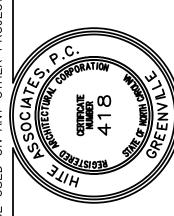
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26 FEB 2025





HITE 3880 BEANNING / TECHNOLOGY 2600 Meridian Drive / Greenville, NC 27834 / tel (252) 757-0



E BUILDING FOR
High School
NCDPS Unit 080 - School 312

Bertie High

Project No. 22351

Date:

March 2025

March 2025

Drawing no.

FS

102

102-A2 Regency Blvd. Greenville, NC 27834 E-Mail Address: generalmail@engrsource.com Voice (252) 439-0338 * Fax (252) 439-0462 * Firm **f**C-1973