

February 7th, 2019 (Rev 05)

HES PV

320 Mary Street
Victoria, BC, V9A 3V9

Attention: Mr. Dan Partridge

**Re: Structural Engineering Review for
HES-PV Fast Rack Mounting Rails and Roof Connections**

As requested Gerrits Engineering Ltd. (GEL) has completed a structural engineering review of the following components of the HES-PV Fast Rack (FR) Solar PV racking system;

1. HES-PV HD Rail System
2. HES-PV UL Rail System
3. 5/16" SS Lag Bolt Roof Connection
4. EJOT-8.0-110 Roof Connection

In our opinion, the above racking components are structurally adequate to be used on buildings designed as per either Part 4 or Part 9 of the National Building Code of Canada (sloped roofs) for rooftop solar PV applications in Canada with the limitations, spans and embedment details as per the attached tables.

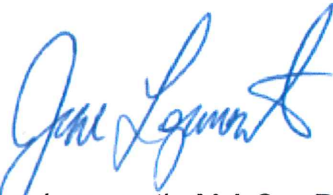
The capacity of the base building to safely support the racking system is to be determined by others.

Our review considered only the racking components outlined herein. Application of HES-PV FR racking system with alternate anchors (e.g., S-5 clamps on standing seam metal roofs) is not considered herein. Application of alternate anchors and structural adequacy of the respective roofing system/structure must be reviewed a qualified Professional Engineer.

Please contact the undersigned for any inquiry related to this document.

Sincerely,

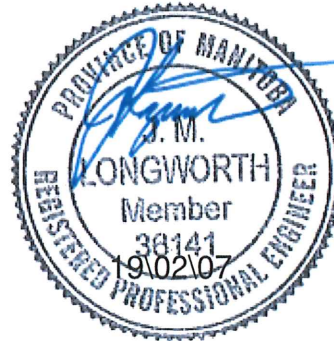
GERRITS ENGINEERING LIMITED



Jesse Longworth, M.A.Sc., P.Eng.
Senior Structural Engineer

Attachments:

1. HES PV Fast Rack HD Rail Maximum Span Table
2. HES PV Fast Rack UL Rail Maximum Span Table
3. HES PV SS Lag Screw Connection – General Arrangement Drawing No. 103499 Revision F (Sheet 3 of 6)
4. HES PV SS EJOT Hanger Bolt Connection – General Arrangement Drawing No. 103499 Revision F (Sheet 4 of 6)



HESPV FAST RACK HD RAIL MAXIMUM SPAN TABLE /m (in)

Specified Roof Snow Load S, kPa (PSF)	5/16" SS Lag Bolt w/ min. $L_t = 89\text{mm}$⁽²⁾ Mean Hourly Wind Pressure, $q_{1/50}$ /kPa (3 Seconds Wind Gust, V_{basic} /mph)			
	0.4 (84)	0.5 (94)	0.6 (103)	0.7 (112)
1.0 (21)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
1.5 (31)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
2.0 (42)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
2.5 (52)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
3.0 (63)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")

Specified Roof Snow Load S, kPa (PSF)	EJOT-8.0-110⁽³⁾ Mean Hourly Wind Pressure, $q_{1/50}$ /kPa (3 Seconds Wind Gust, V_{basic} /mph)			
	0.4 (84)	0.5 (94)	0.6 (103)	0.7 (112)
1.0 (21)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
1.5 (31)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
2.0 (42)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
2.5 (52)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
3.0 (63)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")

Notes:

- 1) Combined dead, wind and snow loads considered as defined by OBC 2012 and NBCC 2015.
- 2) Pull out strengths for lag bolts defined for SPF Grades 1/2 and seasoned wood as per CSA 086-14; Engineering Design in Wood where L_t is the threaded embedment length of the bolt. Minimum joist or truss top chord size is 2x4 (38x89).
- 3) Pull out strength for EJOT, based on 3 11/32" (85 mm) embedment depth.
- 4) Normal Importance Category buildings considered as defined in OBC 2012 for wind and snow loads.
- 5) Results are based on rough wind exposure with maximum wind exposure factor $C_e = 0.7$ or maximum mean roof height of 6m.
- 6) Modules are not to overhang from the roof edge and/or ridge.
- 7) Above tabulated spans are to be reduced half if modules are installed within 1m of a roof edge and/or ridge.
- 8) Valid only for roof slopes ranging from 7° to 45°.
- 9) Based on HD & UL Rails made from 6005A-T61 Aluminum.
- 10) HD & UL rails are not to be spliced within middle 1/3 of the span.
- 11) Maximum live load deflection of the rails limited to **$L/180$** (where **L** is the rail span).
- 12) Results are based on minimum of 3 continuous rail spans.

HESPV FAST RACK UL RAIL MAXIMUM SPAN TABLE /m (in)

Specified Roof Snow Load S, kPa (PSF)	5/16" SS Lag Bolt w/ min. $L_t = 89\text{mm}$⁽²⁾ Mean Hourly Wind Pressure, $q_{1/50}/\text{kPa}$ (3 Seconds Wind Gust, $V_{\text{basic}}/\text{mph}$)			
	0.4 (84)	0.5 (94)	0.6 (103)	0.7 (112)
1.0 (21)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
1.5 (31)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
2.0 (42)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
2.5 (52)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
3.0 (63)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")

Specified Roof Snow Load S, kPa (PSF)	EJOT-8.0-110⁽³⁾ Mean Hourly Wind Pressure, $q_{1/50}/\text{kPa}$ (3 Seconds Wind Gust, $V_{\text{basic}}/\text{mph}$)			
	0.4 (84)	0.5 (94)	0.6 (103)	0.7 (112)
1.0 (21)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
1.5 (31)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
2.0 (42)	1.83 (72")	1.83 (72")	1.22 (48")	1.22 (48")
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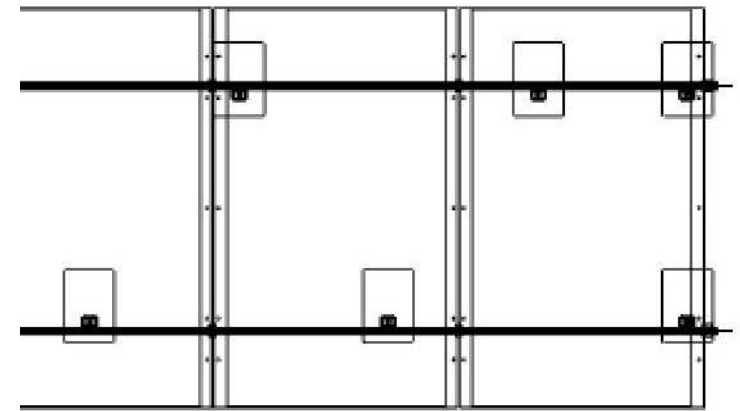
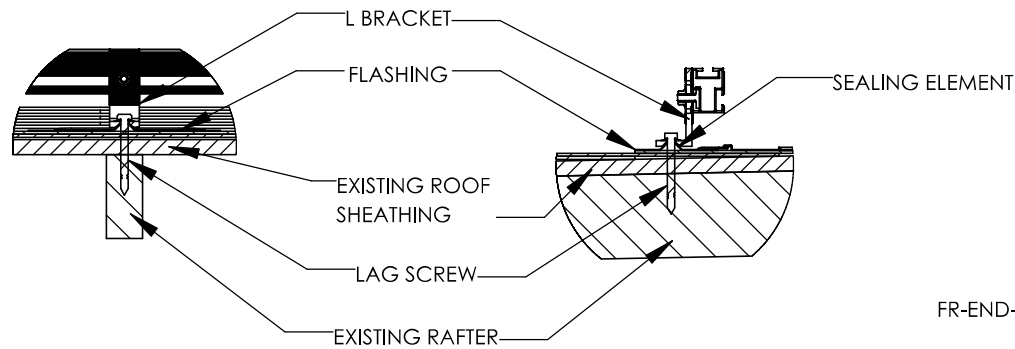
Notes:

- 1) Combined dead, wind and snow loads considered as defined by OBC 2012 and NBCC 2015.
- 2) Pull out strengths for lag bolts defined for SPF Grades 1/2 and seasoned wood as per CSA 086-14; Engineering Design in Wood where L_t is the threaded embedment length of the bolt. Minimum joist or truss top chord size is 2x4 (38x89).
- 3) Pull out strength for EJOT, based on 3 11/32" (85 mm) embedment depth.
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- 11) Maximum live load deflection of the rails limited to **$L/180$** (where **L** is the rail span).
- 12) Results are based on minimum of 3 continuous rail spans.

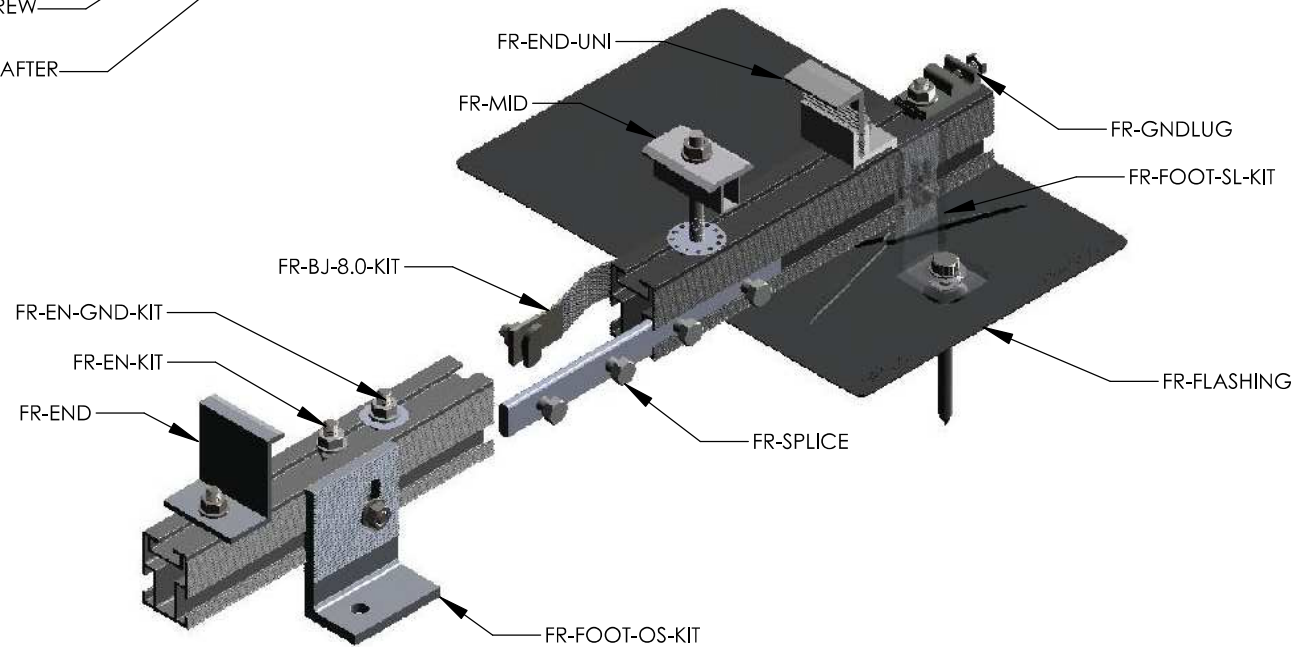
NOTES:

- BONDING WASHER TO BE USED ON EVERY MID CLAMP
- MOUNTING FEET SPACING BASED ON 24" O/C TRUSSES
- MOUNTING FEET ALTERNATE RAILS TO DITRIBUTE LOAD
- MAXIMUM SPAN 64"

FLASHING CROSS-SECTION



TYPICAL MOUNTING FOOT PLACEMENT PATTERN



H.E.S. PV

84 MORROW RD.
BARRIE, ON L4N 3V8
1-866-258-0110
WWW.HESPV.COM

PROJECT:

-

USED ON ASS'Y:

-

CLONE FROM:

-

UNLESS OTHERWISE SPECIFIED:

DIMENSIONS ARE IN INCHES

TOLERANCES:

FRACTIONAL $\pm 1/16$

ANGULAR: MACH \pm BEND \pm

TWO PLACE DECIMAL ± 0.06

THREE PLACE DECIMAL ± 0.03

INTERPRET GEOMETRIC

TOLERANCING PER:

MATERIAL:

FINISH:

DRAWN

NAME

DP

DATE

MM/DD/YY

CHECKED

-

-

ENG APPR.

MFG APPR.

Q.A.

TITLE: RESIDENTIAL LAYOUT SAMPLE

DWG. NO.

103499

REV

-

SCALE: NTS

SHEET SIZE: A

SHEET 3 OF 6

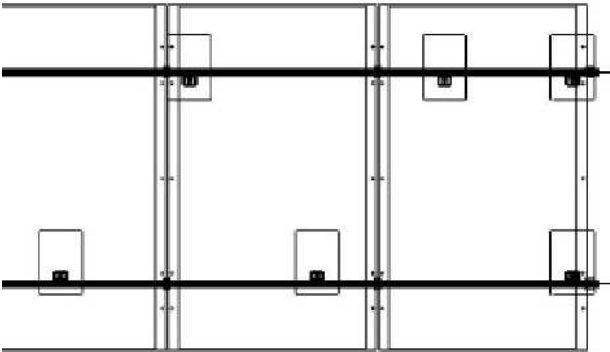
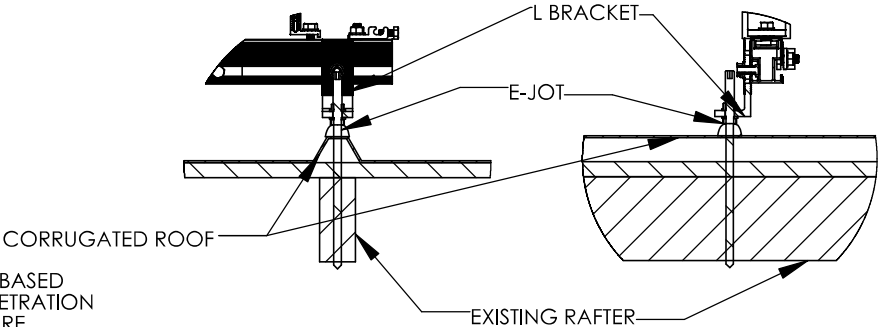
PROPRIETARY AND CONFIDENTIAL
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HOME ENERGY SOLUTIONS. ANY REPRODUCTION IN PART OR AS A WHOLE
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EJOT HANGER BOLT

NOTES:

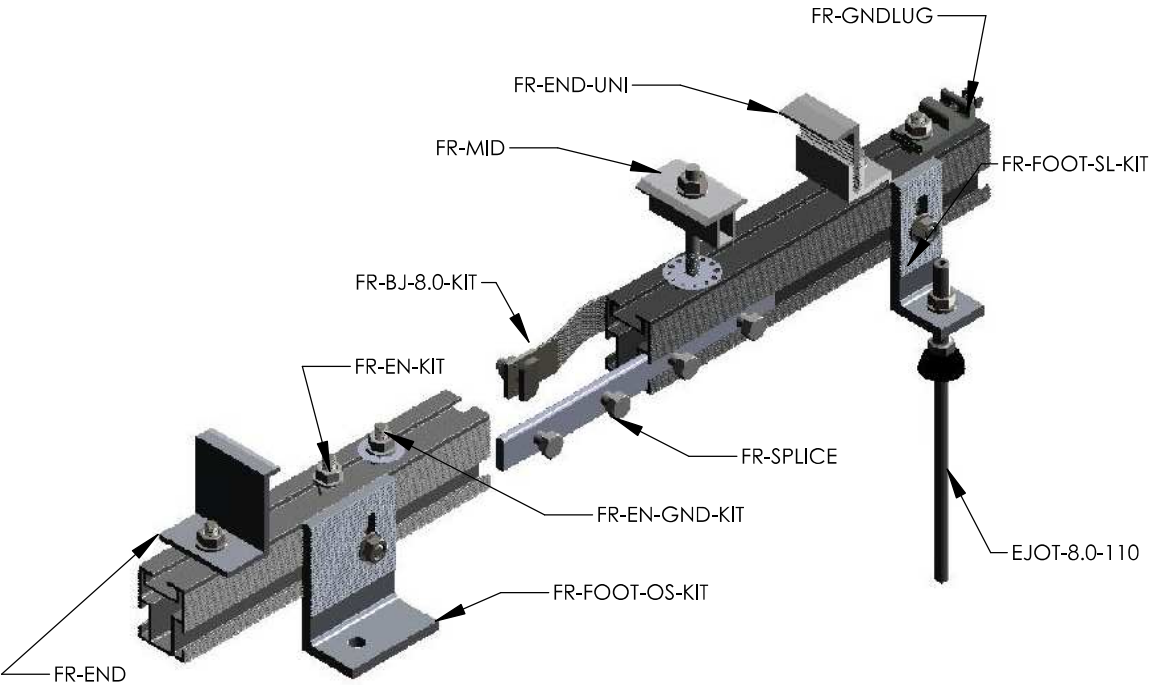
- BONDING WASHER TO BE USED ON EVERY MID CLAMP
- MOUNTING FEET SPACING BASED ON 24" O/C TRUSSES
- MOUNTING FEET ALTERNATE RAILS TO DITRIBUTE LOAD
- MAXIMUM SPAN 64"



TYPICAL MOUNTING FOOT PLACEMENT PATTERN

EFFECTIVE SCREW IN DEPTH BASED ON INCHES OF THREAD PENETRATION INTO WOODEN SUBSTRUCTURE.

Effective Screw In Depth l _{ef} in inches	M8 (lb)	M10 (lb)
1.25	396	0
1.57	495	616
1.77	555	695
1.89	591	740
1.96	616	771
2.2	690	863
2.36	740	926
2.52	789	987
2.76	863	1079
2.83	888	1111
3.15	987	1234
3.35	1050	1311
3.46	1086	1358
3.54	1111	1387
3.78	1185	1481
3.93	1234	1542
4.04	1284	1603
4.33	1358	1697
4.4	1383	1726
4.72	1481	1850



H.E.S. PV			UNLESS OTHERWISE SPECIFIED: DIMENSIONS ARE IN INCHES TOLERANCES: FRACTIONAL ± 1/16 ANGULAR: MACH± BEND ± TWO PLACE DECIMAL ± 0.06 THREE PLACE DECIMAL ± 0.03 INTERPRET GEOMETRIC TOLERANCING PER: MATERIAL: FINISH:		NAME		DATE		TITLE: RESIDENTIAL LAYOUT SAMPLE				
84 MORROW RD. BARRIE, ON L4N 3V8 1-866-258-0110 WWW.HESPV.COM		PROJECT: —			DRAWN	DP	MM/DD/YY		DWG. NO. 103499		REV —		
					CHECKED	-	-						
					ENG APPR.								
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		CLONE FROM:		-	Q.A.								