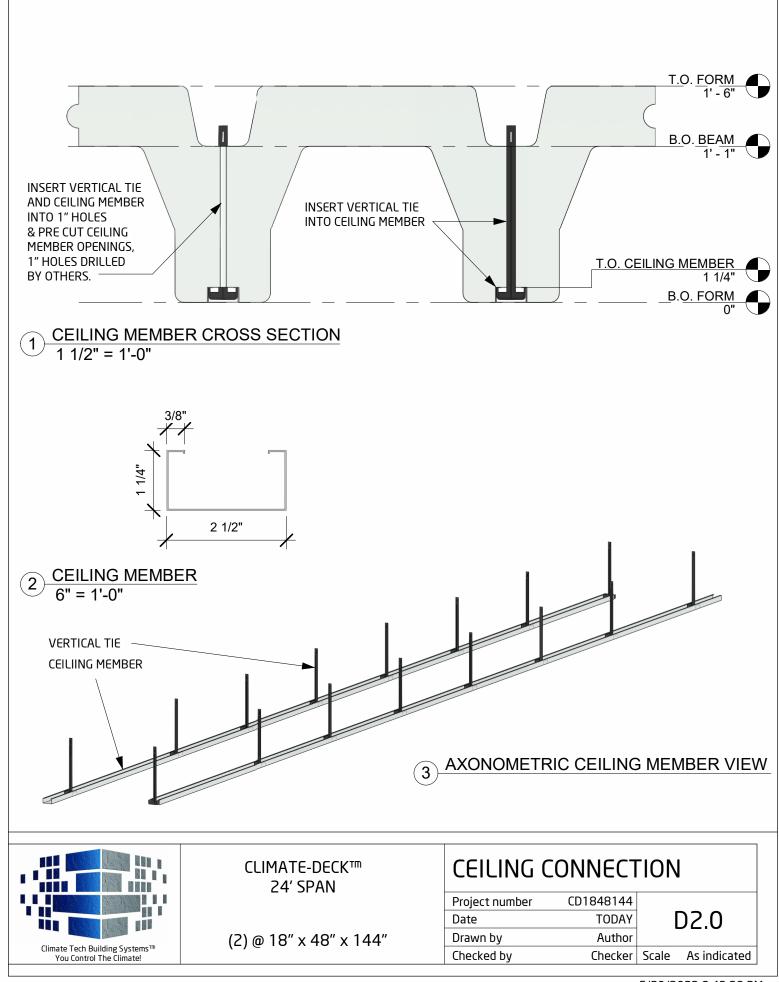


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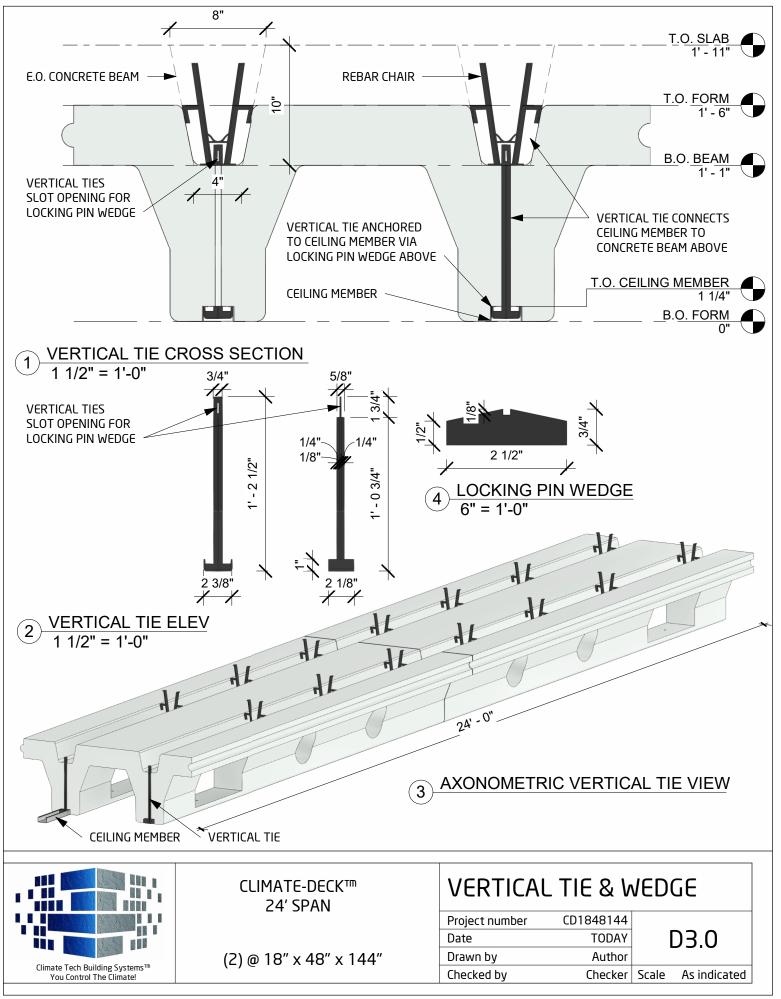
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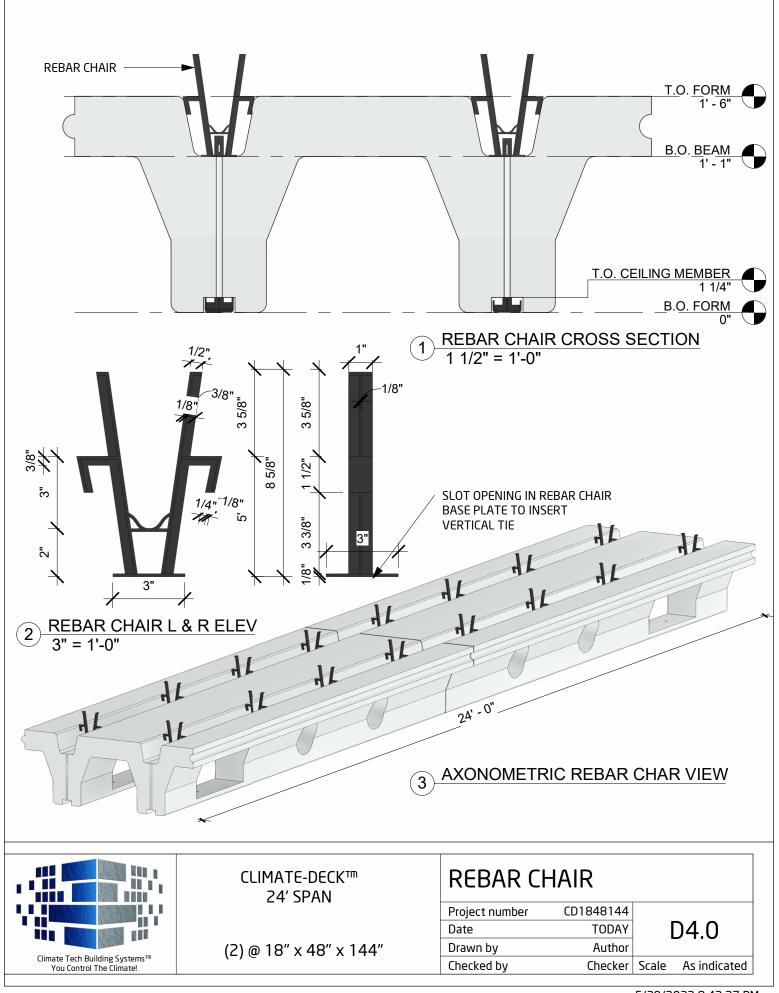
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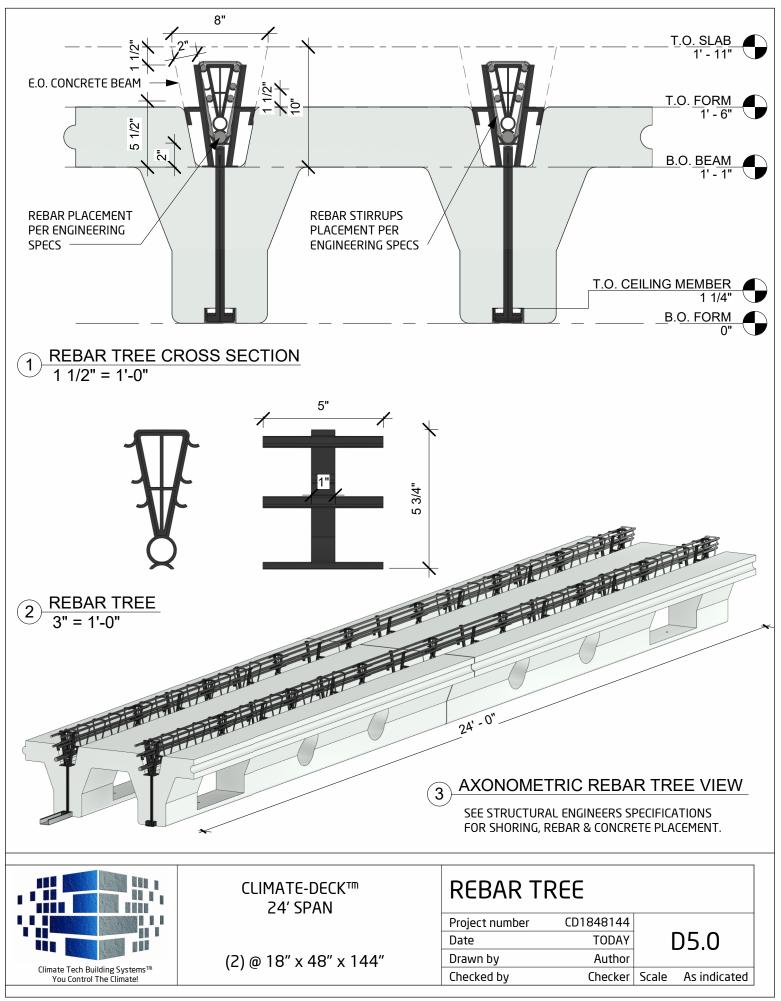
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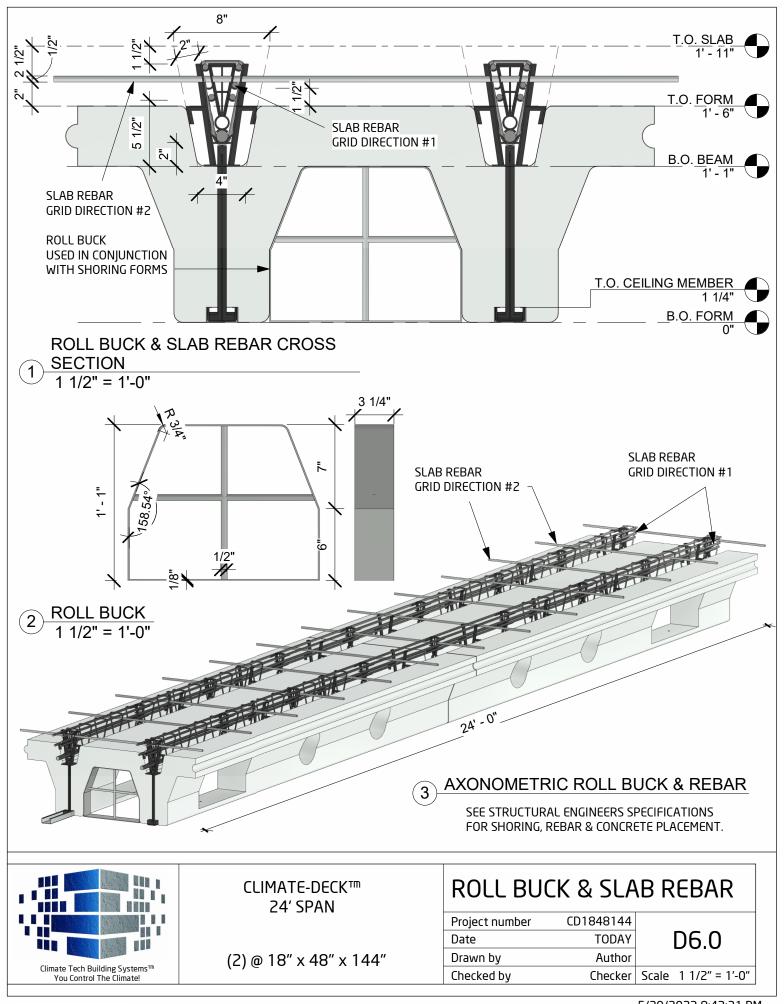
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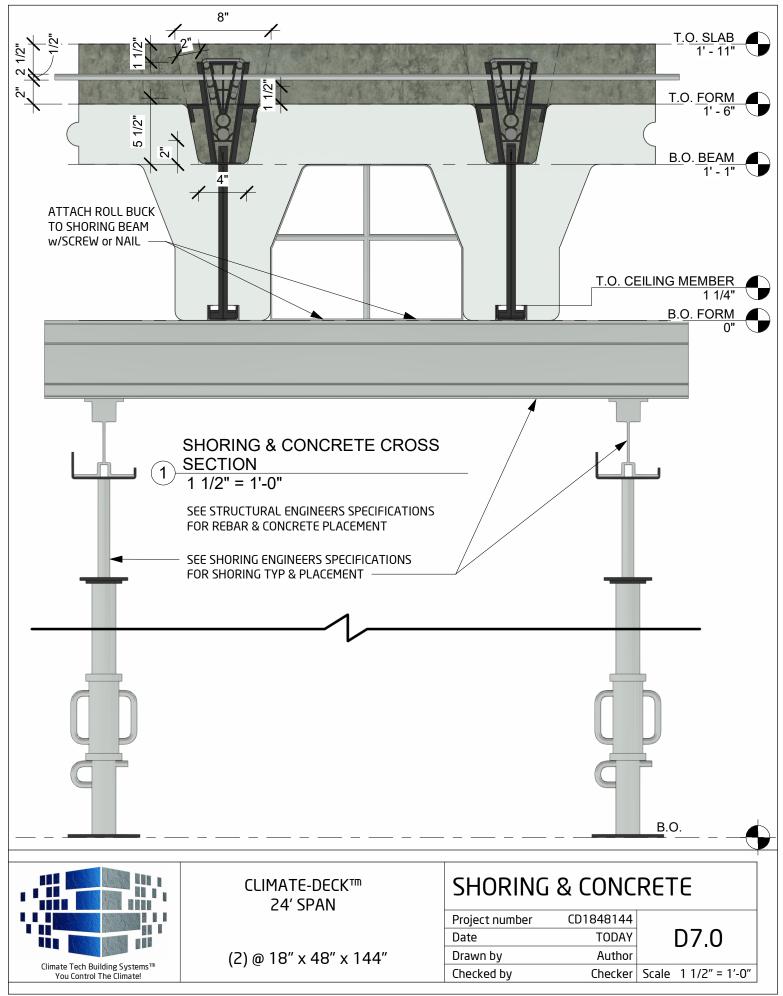
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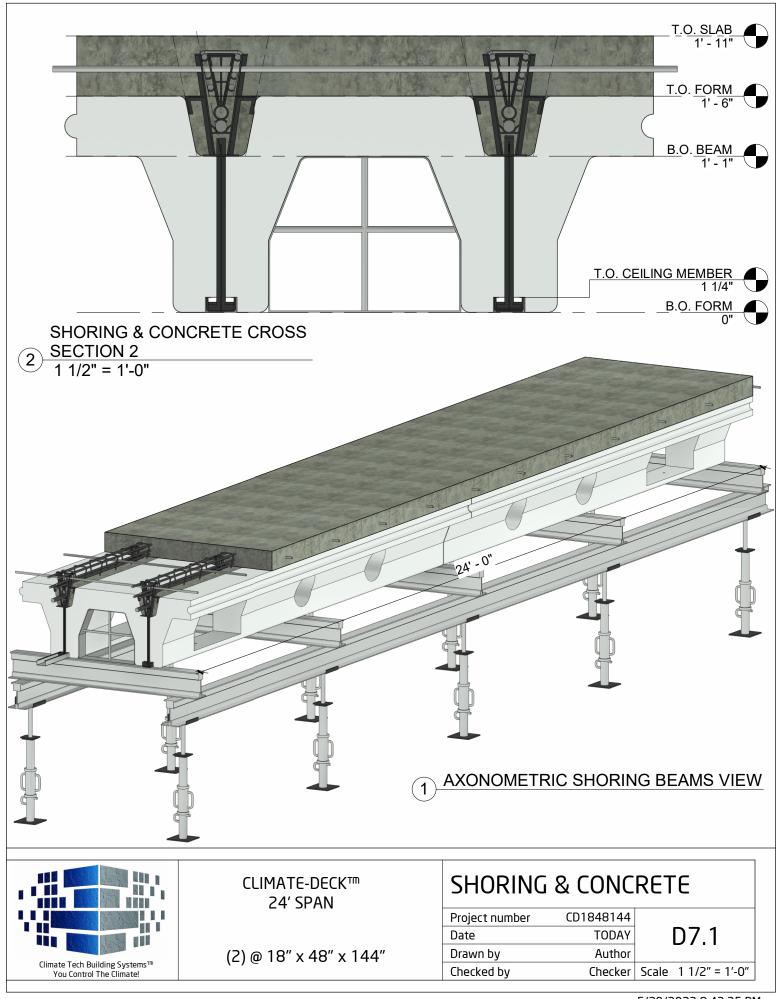
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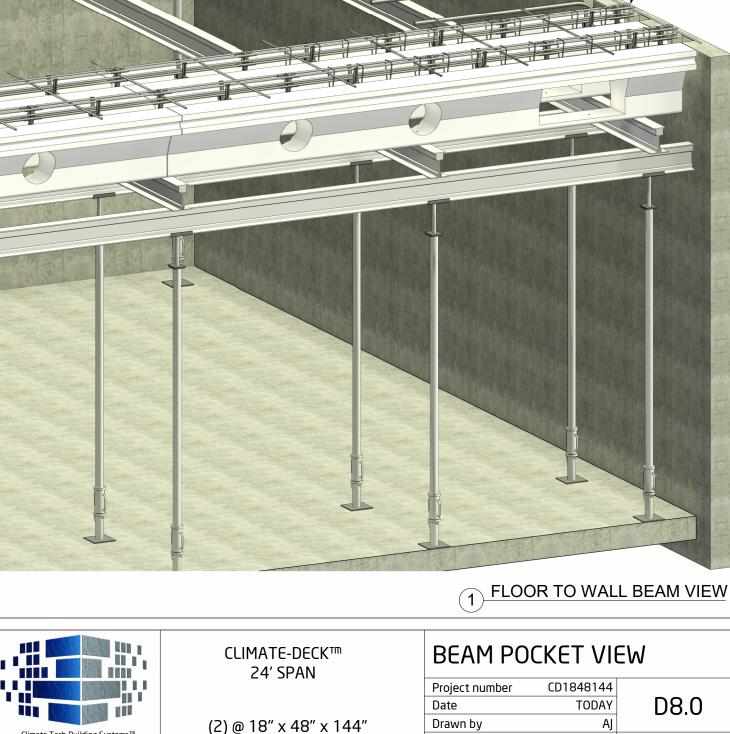
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STRIP FORMS, BREAK OUT EPS POCKET **EXPOSING #8 REBAR, BEND** REBAR DOWN INTO THE CONCRETE FORMS BEAM VOID

SET CLIMATE-DECK[™] & REBAR BEAM CAGE INTO PLACE, CONNECTING #8 BAR FROM WALL TO CONCRETE BEAM



Climate Tech Building Systems™ You Control The Climate!

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AJ Scale

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Checked by

Concrete Beam			Software copyright ENE	RCALC, INC. 1983-2020	
DESCRIPTION: typ 24' span					Site Serve LLC
CODE REFERENCES Calculations per ACI 318-14, IB	C 2018 CBC 2019 ASCE 7-1	6			
Load Combination Set : ASCE 7		•			
Material Properties					
fc = 3.0 k fr = fc ^{1/2} * 7.50 = 410.792 p				ð in	
		ear: 0.750 0.850			
ψ Density = 145.0 p λ LtWt Factor = 1.0	pcf β ₁ =	0.000		• • /	
Elastic Modulus = 3,122.0 k	si Fy - Stirrups	40.0 ksi			
fy - Main Rebar = 60.0 k	si E - Stirrups = 2 si Stirrup Bar Size #	29,000.0 ksi 3	é	• • /	
E - Main Rebar = 29,000.0 k	si of Resisting Legs Per Stirrup =	2			
Number	or resisting Legs r er otirrup -	2			
			·		
			L	4in .	
\searrow	D(D.06) L(0.08)			
*	÷		÷		*
×					×
		24.0 ft			
•	8	' w x 10" h			
Cross Section & Reinforci	na Deteile				
Span #1 Reinforcing 1-#8 at 2.0 in from Bottom, fror 2-#4 at 5.0 in from Bottom, fror Beam self weight calculated and add	n 0.0 to 24.0 ft in this span	: 10.0 in 2-#4 at 1.50 in fror	n Top, from 0.0 to 2	4.0 ft in this span	
1-#8 at 2.0 in from Bottom, fror 2-#4 at 5.0 in from Bottom, fror	n 0.0 to 24.0 ft in this span ded to loads		n Top, from 0.0 to 2	4.0 ft in this span	
1-#8 at 2.0 in from Bottom, from 2-#4 at 5.0 in from Bottom, from Beam self weight calculated and add Load for Span Number 1 Uniform Load : D = 0.030, L = 0.04 DESIGN SUMMARY	n 0.0 to 24.0 ft in this span ded to loads 0 ksf, Tributary Width = 2.0 ft	2-#4 at 1.50 in fror	n Top, from 0.0 to 2		gn OK
1-#8 at 2.0 in from Bottom, fror 2-#4 at 5.0 in from Bottom, fror Beam self weight calculated and add Load for Span Number 1 Uniform Load : D = 0.030, L = 0.04 DESIGN SUMMARY Maximum Bending Stress Ratio	n 0.0 to 24.0 ft in this span ded to loads 0 ksf, Tributary Width = 2.0 ft = 0.670 : 1	2-#4 at 1.50 in fror Maximum Deflection		Desi	
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1-#8 at 2.0 in from Bottom, fror 2-#4 at 5.0 in from Bottom, fror 2-#4 at 5.0 in from Bottom, fror Beam self weight calculated and add Load for Span Number 1 Uniform Load : D = 0.030, L = 0.04 DESIGN SUMMARY Maximum Bending Stress Ratio Section used for this span Mu : Applied Mn * Phi : Allowable Location of maximum on span Span # where maximum occurs Vertical Reactions Load Combination Overall MAXimum Overall MAXimum Overall MINimum +D+H +D+Lr+H +D+Lr+H +D+Lr+H +D+0.750Lr+0.750L+H +D+0.60W+H +D+0.750Lr+0.750L+0.450W+H	n 0.0 to 24.0 ft in this span ded to loads 0 ksf, Tributary Width = 2.0 ft = 0.670 : 1 Typical Section 19.620 k-ft 29.303 k-ft 12.022 ft Span # 1 Support 1 Support 2 2.405 2.405 0.867 0.867 1.445 1.445 2.405 2.405 1.445 1.445 2.405 2.405 1.445 1.445 2.405 2.405 1.445 1.445 2.405 2.165 2.165 2.165 2.165 2.165 1.445 1.445 2.405 2.165 2.165 2.165	2-#4 at 1.50 in fror Maximum Deflection Max Downward Transien Max Upward Transient I Max Downward Total Defle Max Upward Total Defle	nt Deflection Deflection sflection	Desi 0.625 in Ratio = 0.000 in Ratio = 1.735 in Ratio =	460 >=360. 0 <360.0 165 >=150
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