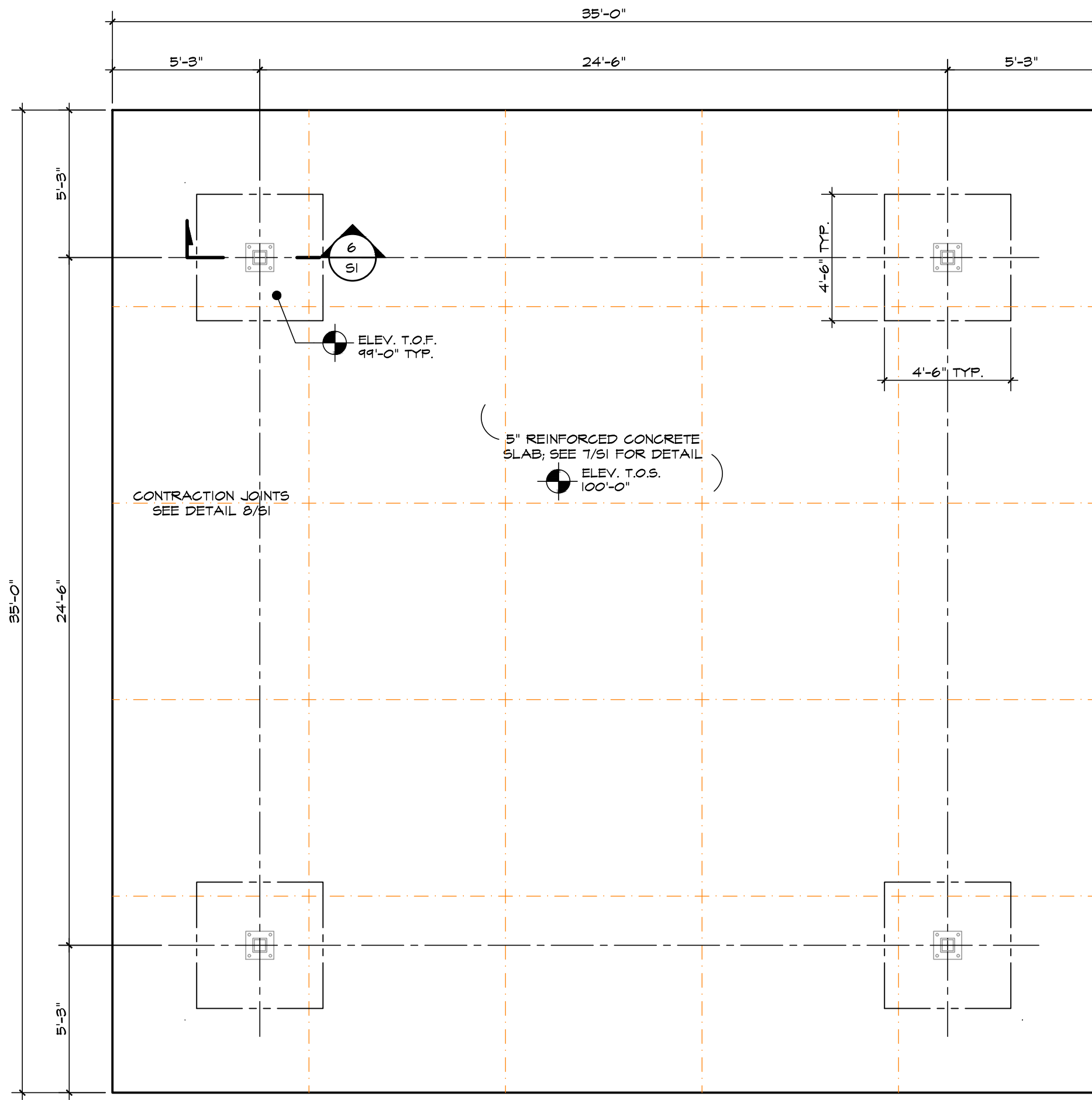
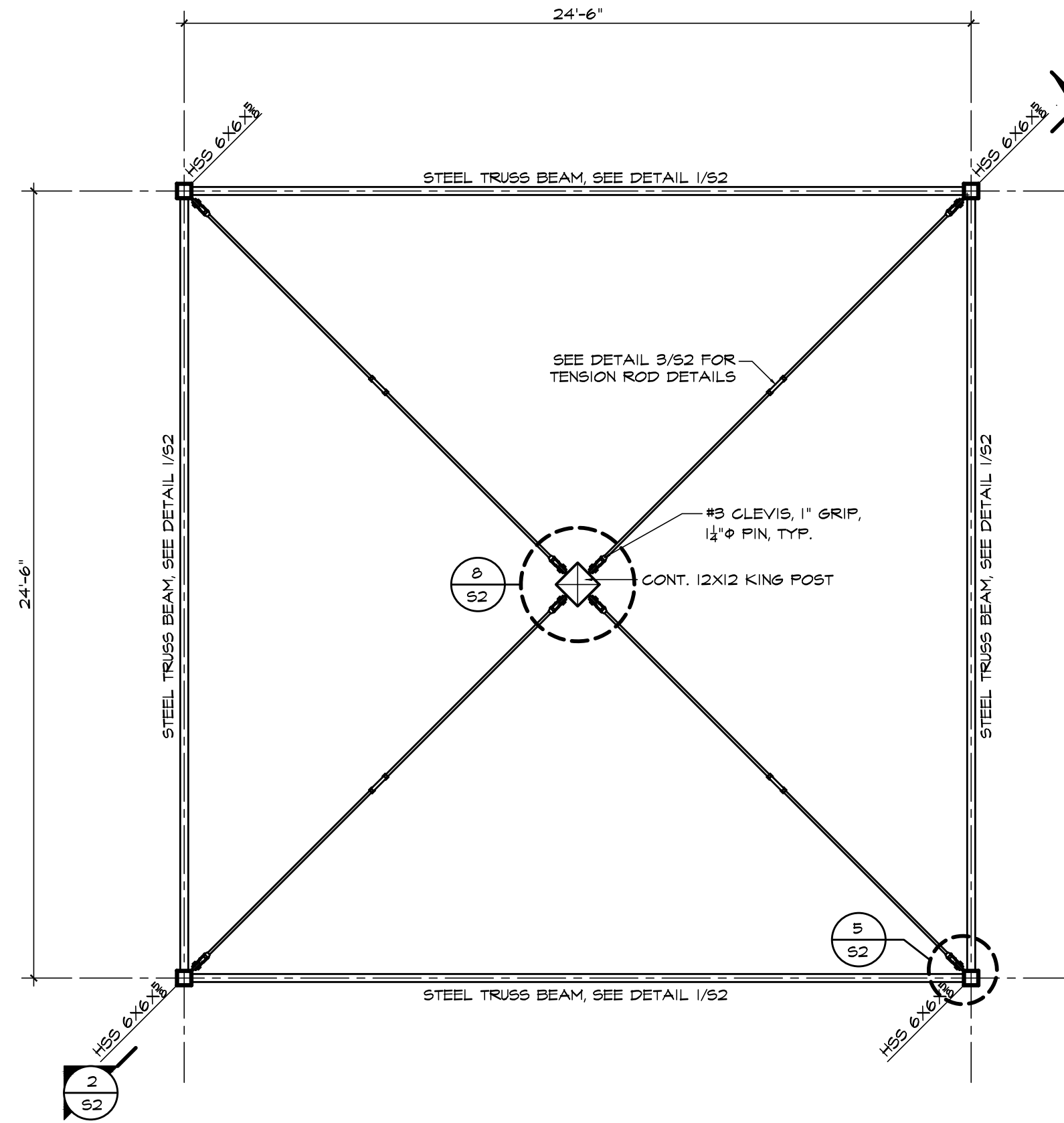
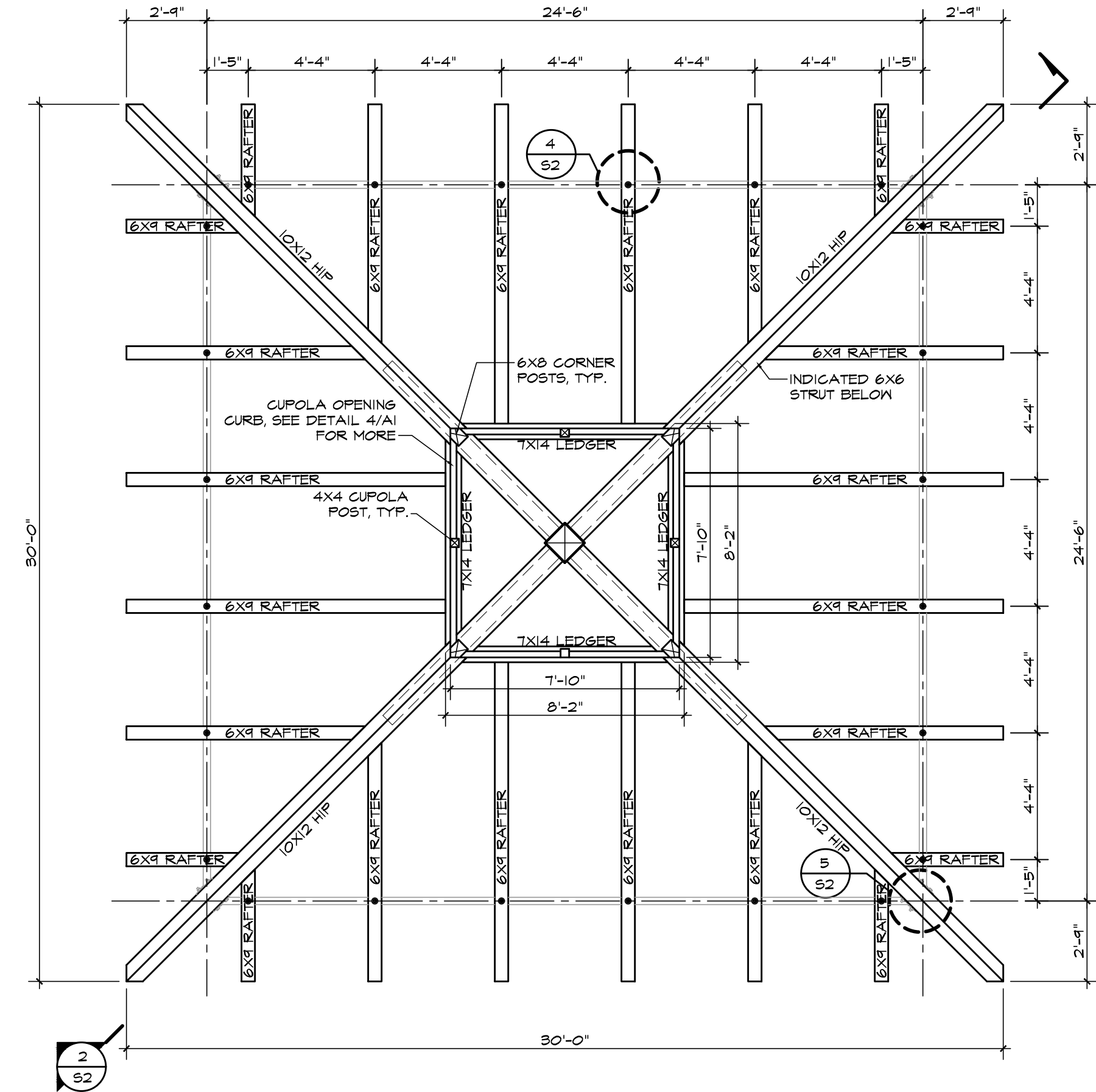




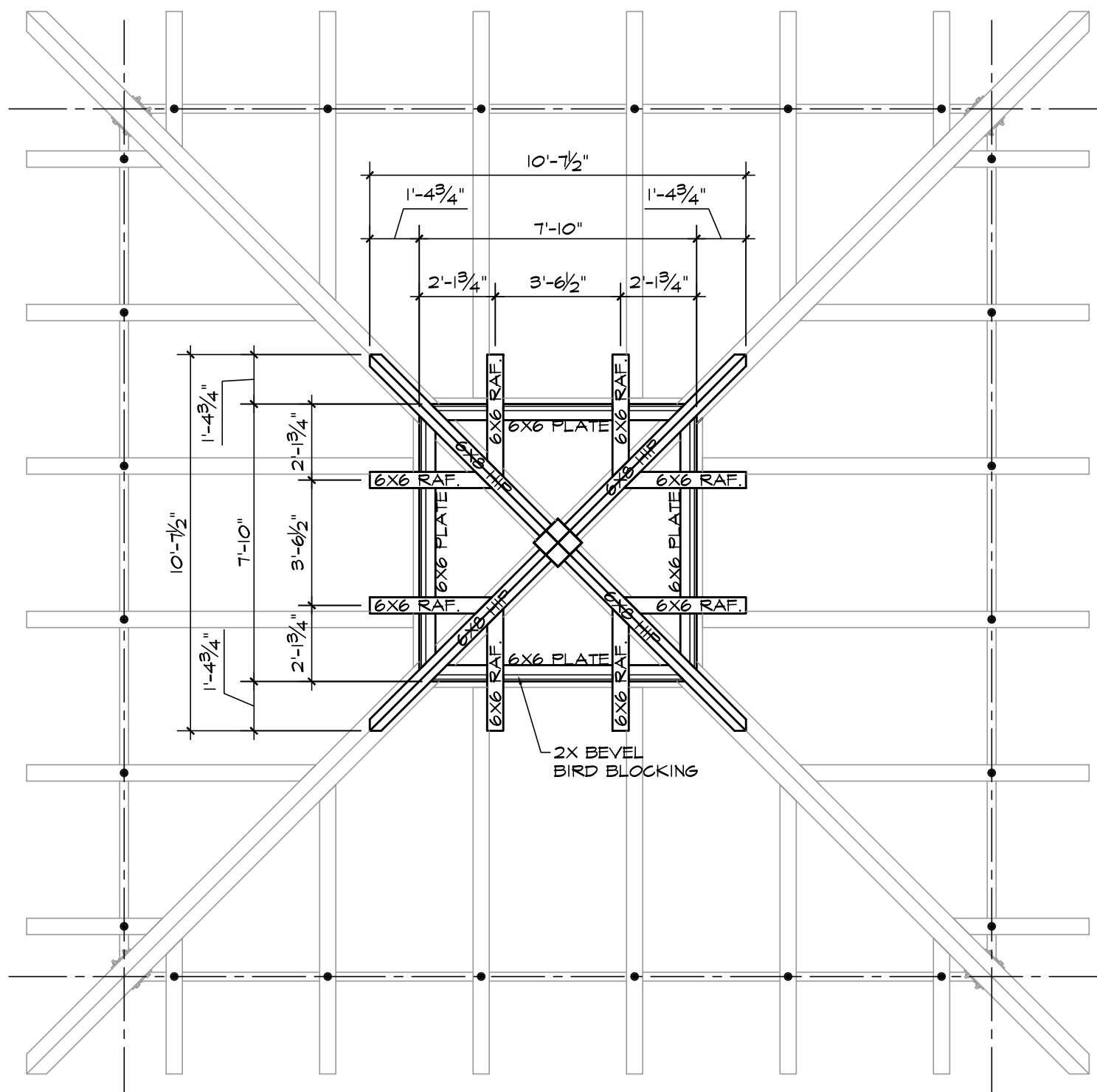
1 FOUNDATION PLAN
1/4"=1'-0"



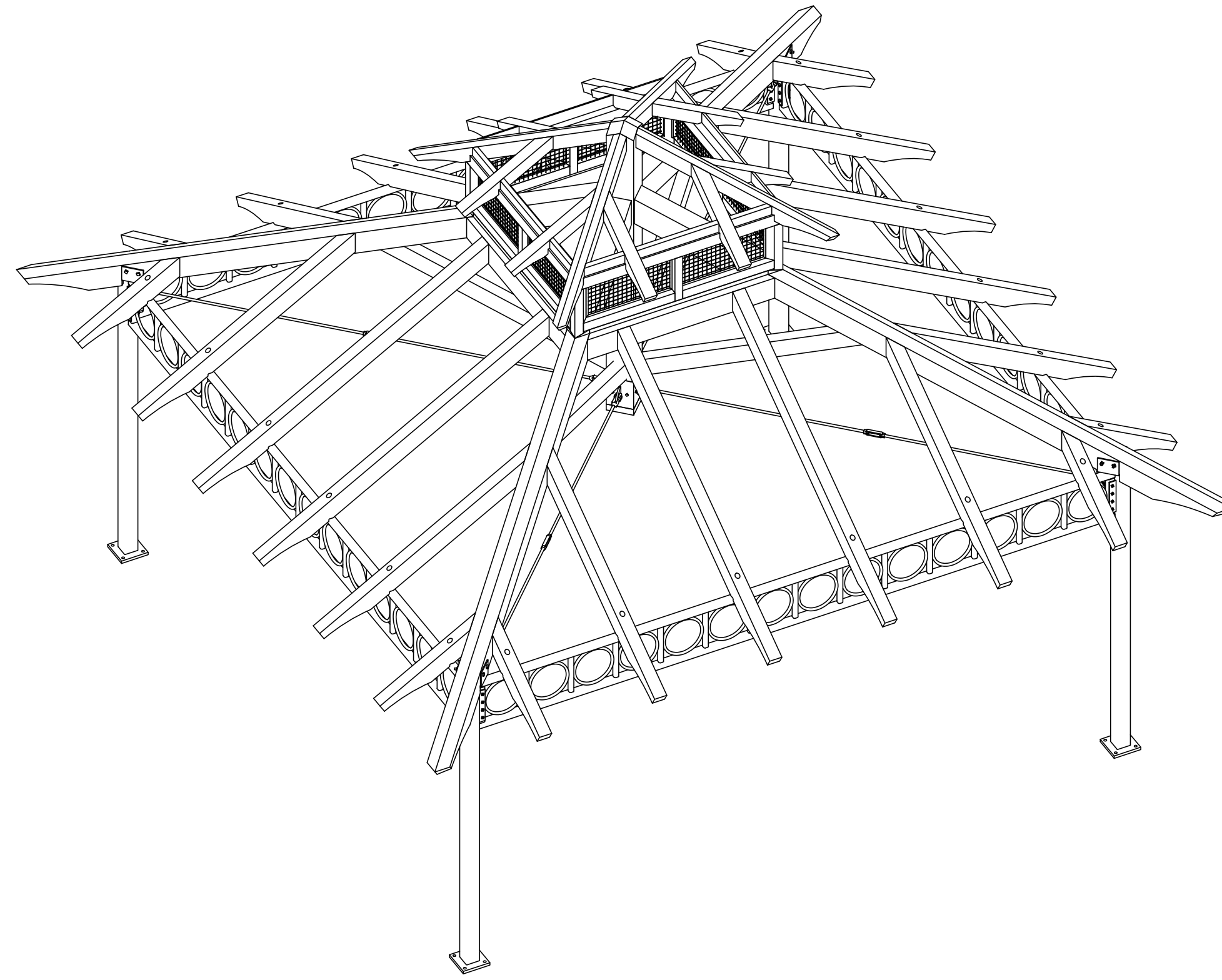
2 STEEL TRUSS/BEAM LEVEL PLAN
1/4"=1'-0"



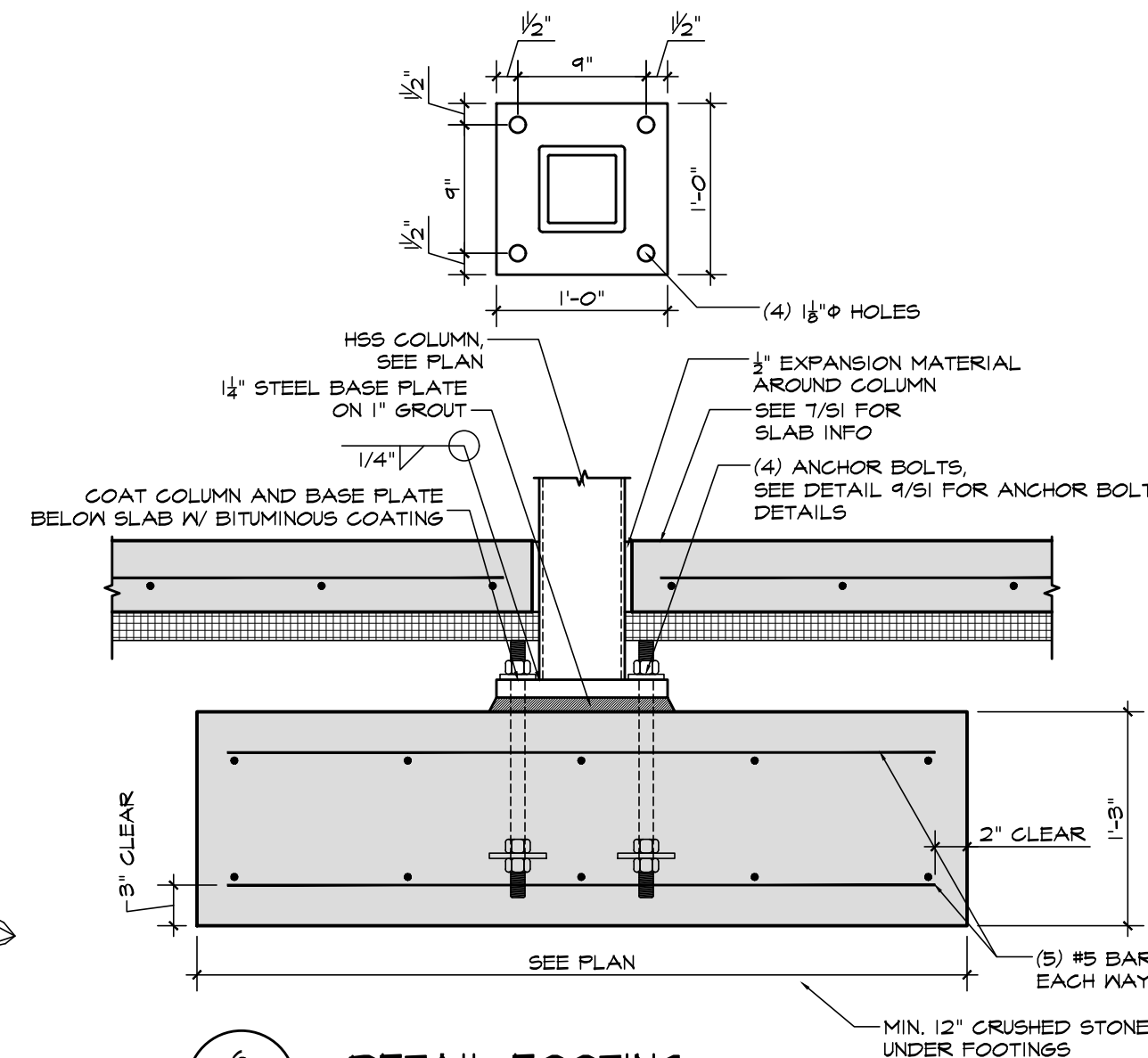
3 MAIN ROOF FRAMING PLAN
1/4"=1'-0"



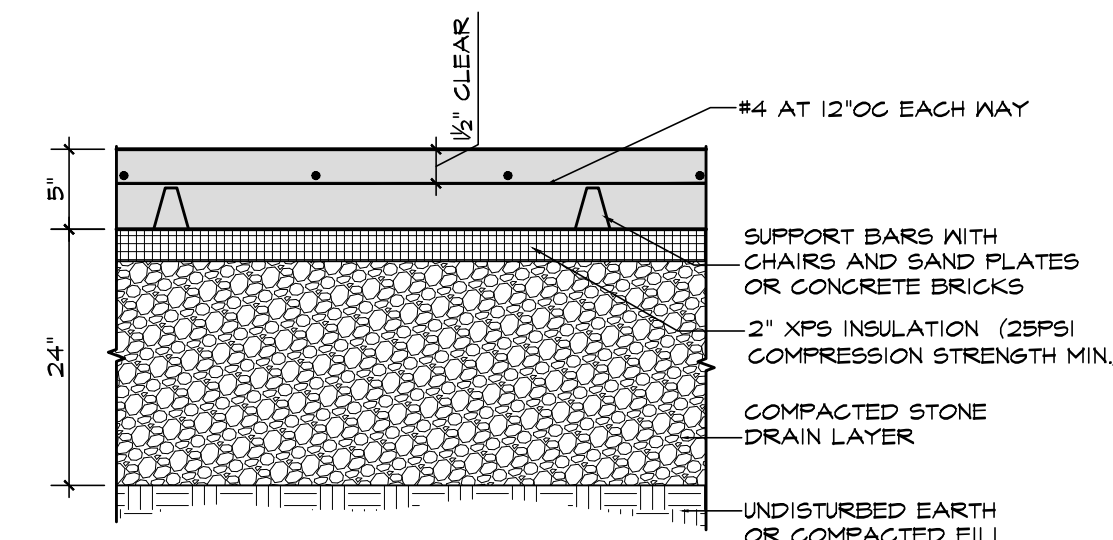
4 CUPOLA ROOF FRAMING PLAN
1/4"=1'-0"



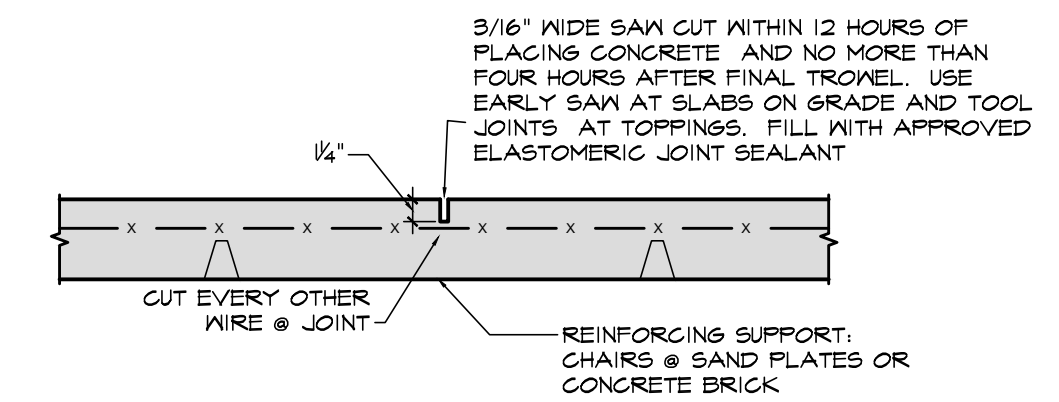
5 STEEL AND TIMBER ISOMETRIC VIEW
N.T.S.



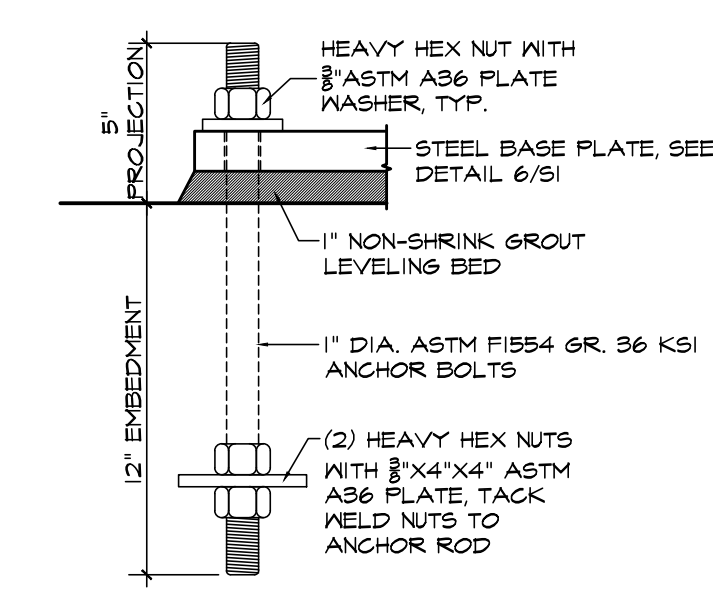
6 DETAIL FOOTING
1"=1'-0"



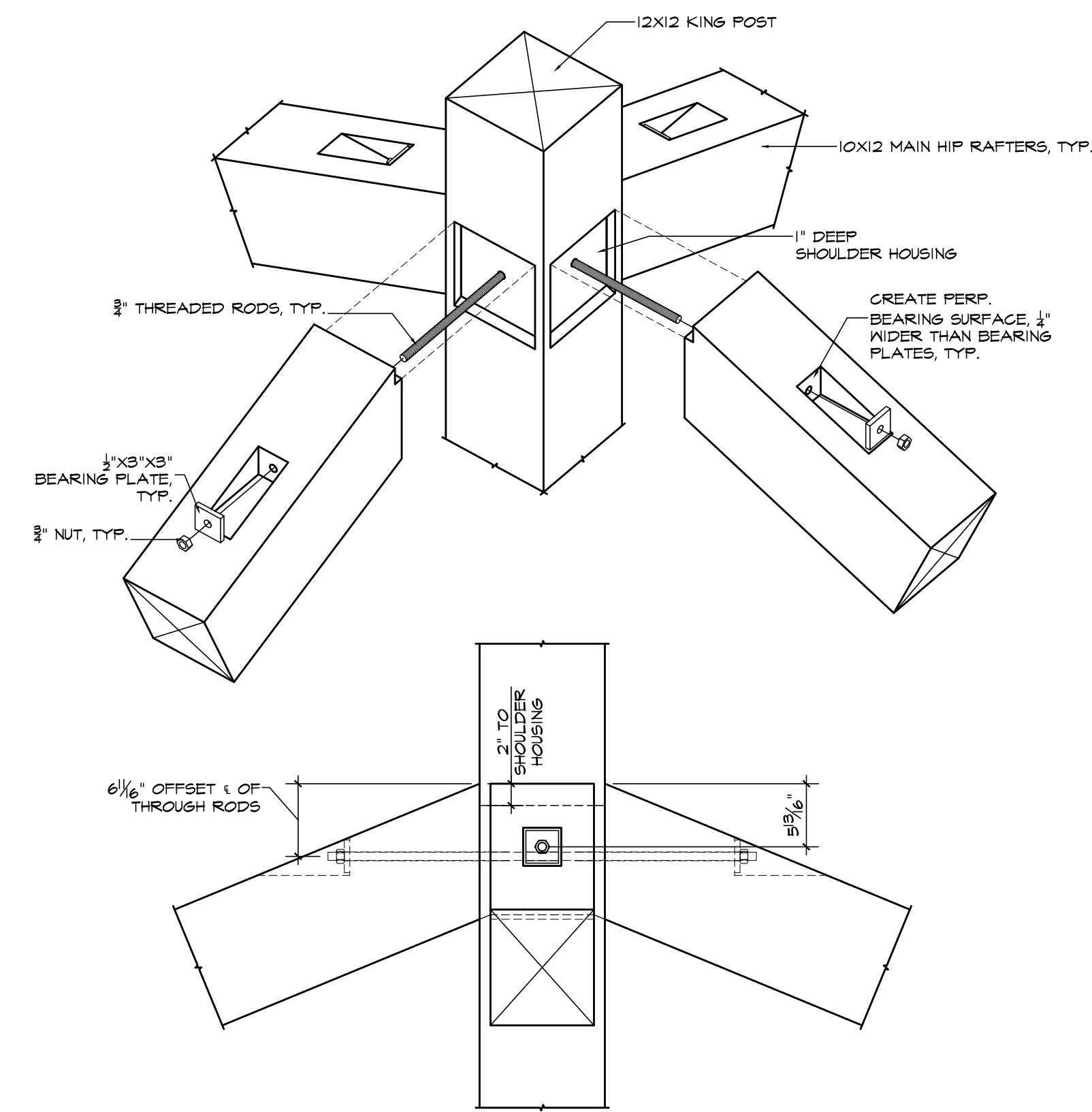
7 DETAIL @ NEW CONCRETE SLAB
1"=1'-0"



8 DETAIL @ SLAB CONTROL JOINT
1"=1'-0"



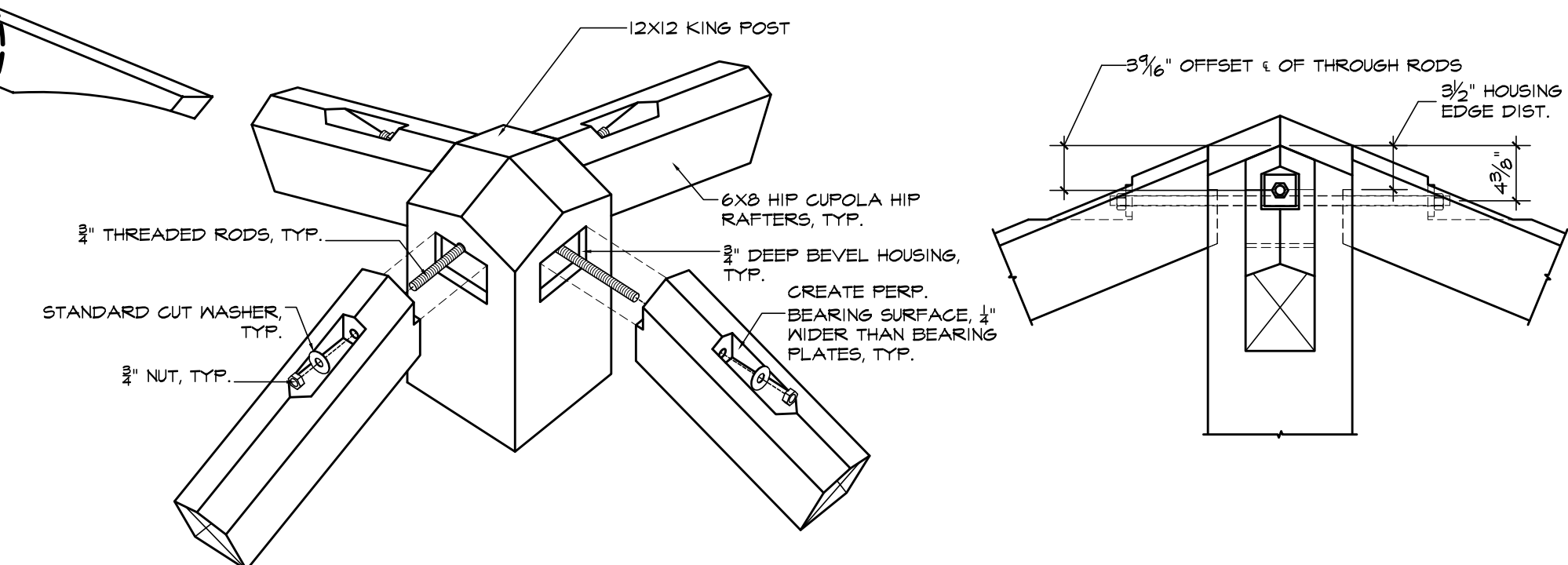
9 ANCHOR BOLT DETAIL
1"=1'-0"



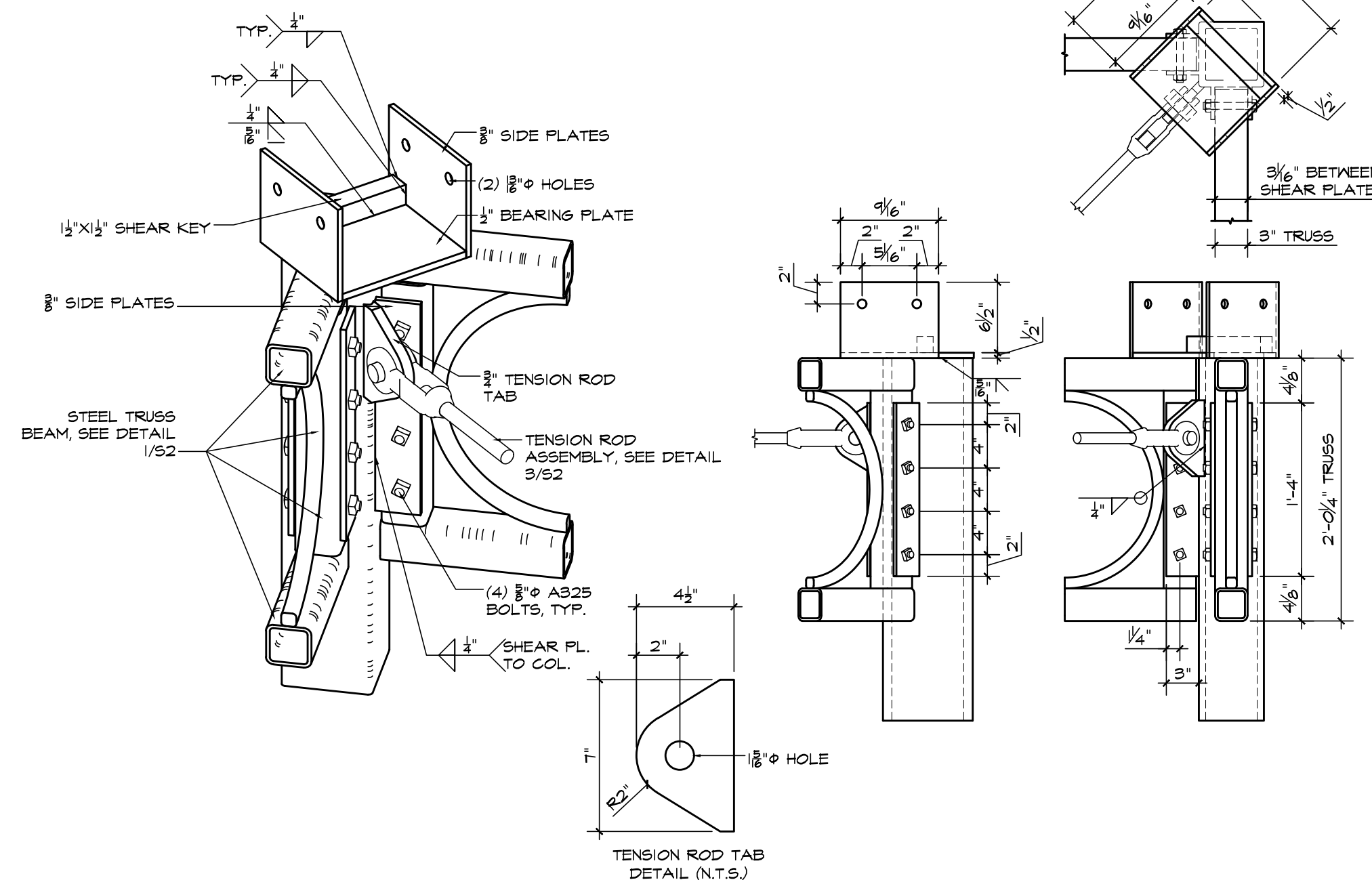
6
52

MAIN HIP CONNECTION TO KING POST

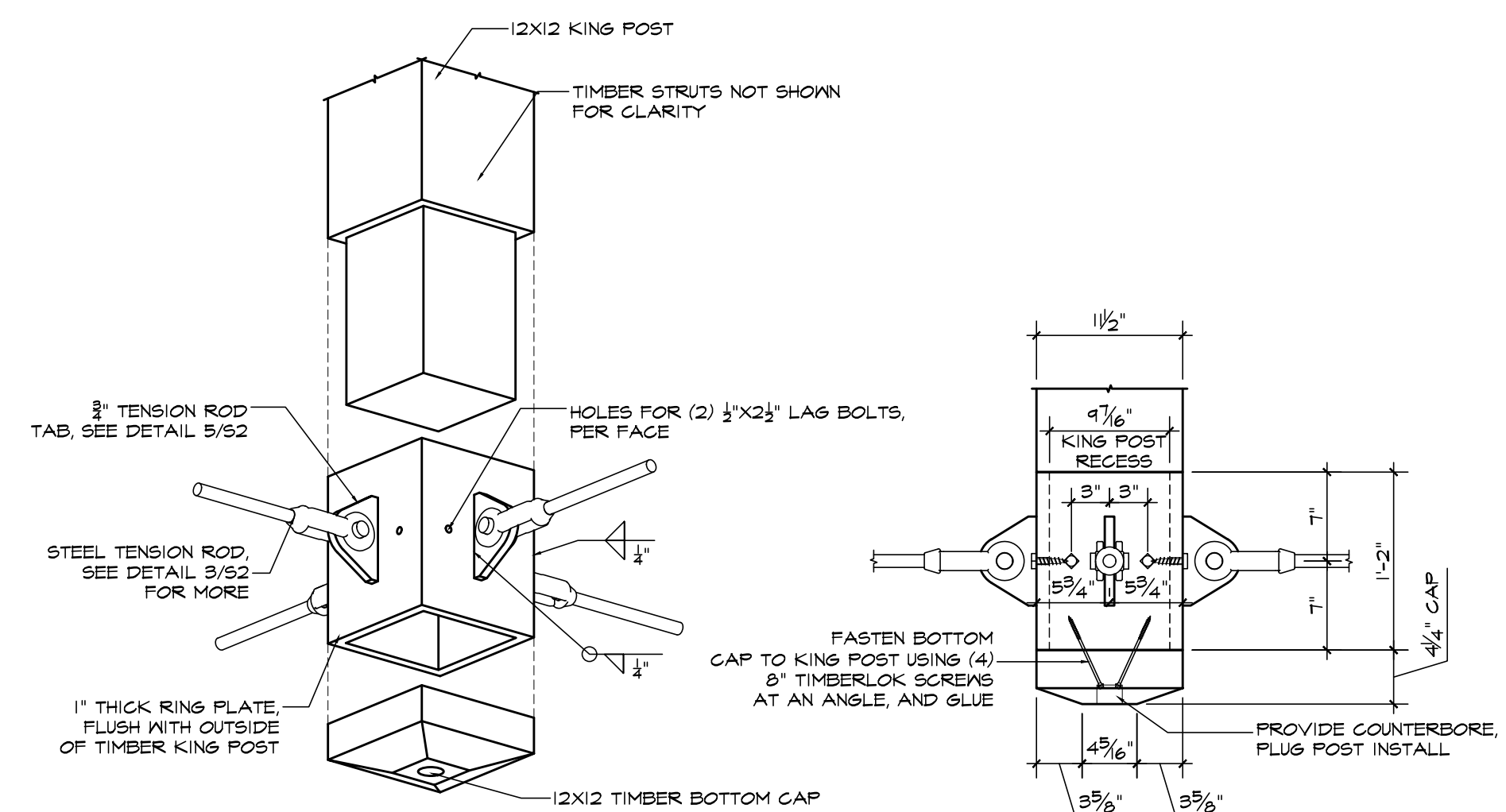
1" = 1'-0"



7
52 CUOLA HIP CONNECTION TO KING POST
1"=1'-0"



5
S2
HIP CONNECTION TO STEEL COLUMN
1"=1'-0"
TYPICAL



4 RAFTER CONN. TO STEEL TRUSS BEAM
52 1"=1'-0" TYPICAL

8
S2

TENSION ROD RING PLATE CONN. TO KING POST

1"=1'-0"

STAMP AND SIGNATURE:

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AND MAY NOT BE REPRODUCED
OR USED FOR ANY
PURPOSE WITHOUT PRIOR
WRITTEN PERMISSION

ENT PAVILION
BENNINGTON, VT
CONSTRUCTION SET

ISSUED: 3/19/26

IONS:

BY: JM

 SIGNED BY:

SECTIONS / DETAILS

ING NO.

S2

GENERAL NOTES

1. ALL STRUCTURAL WORK SHALL CONFORM TO THE PROJECT SPECIFICATIONS, DRAWINGS AND THE 2025 VERMONT FIRE & BUILDING SAFETY CODE.
2. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS AND ARCHITECTURAL, SITE, MECHANICAL, ELECTRICAL DRAWINGS AND THE APPROVED SHOP DRAWINGS.
3. IN THE CASE OF A CONFLICT BETWEEN THE DRAWINGS, SPECIFICATIONS OR NOTES ON THE DRAWINGS, THE ARCHITECT SHALL BE NOTIFIED TO RESOLVE THE DISCREPANCY.
4. THE SCOPE OF WORK FOR THE DESIGN ENGINEER FOR THIS PROJECT DOES NOT INCLUDE ANY EVALUATION OF THE SOIL CONDITIONS ON SITE. ADEQUACY OF SOIL STABILITY, BEARING, ETC, IS THE FULL RESPONSIBILITY OF OTHERS.
5. THE STRUCTURAL ELEMENTS OF THE PROJECT HAVE BEEN DESIGNED FOR THE SPECIFIED VERTICAL AND LATERAL FORCES ACTING ON THE COMPLETED BUILDING. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DESIGN AND PROVIDE ALL REQUIRED SHORING AND BRACING NEEDED DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF THE PARTIALLY-COMPLETED STRUCTURE AND FOR CONSTRUCTION LOADINGS THAT EXCEED THE SPECIFIED DESIGN LOADS.
6. DETAILS LABELED "TYPICAL DETAILS" ON THE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. THE APPLICABILITY OF THE TYPICAL DETAIL IS TO BE DETERMINED BY THE TITLE OF THE TYPICAL DETAIL. SUCH TYPICAL DETAILS SHALL APPLY WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION IN THE DRAWINGS.
7. AT ALL TIMES THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONDITIONS OF THE JOBSITE INCLUDING SAFETY OF PERSONS AND PROPERTY. THE ARCHITECT'S OR ENGINEER'S PRESENCE OR REVIEW OF WORK DOES NOT INCLUDE THE ADEQUACY OF THE CONTRACTOR'S MEANS OR METHODS OF CONSTRUCTION.
8. DO NOT SCALE THE DRAWINGS.

FOUNDATION RELATED EARTHWORK

1. FOUNDATION DESIGN IS BASED ON PRESUMPTIVE SOIL PROPERTIES FROM THE IBC FOR ASSUMED SUBGRADE CONDITIONS DESCRIBED AS "SAND, SILTY SAND, CLAYEY SAND, SILTY GRAVEL AND CLAYEY GRAVEL (SW, SP, SM, SC, GM AND GC)."
CONTRACTOR SHALL FIELD-VERIFY THAT ACTUAL SUBGRADE CONDITIONS ARE CONSISTENT WITH THESE ASSUMED SUBGRADE MATERIALS AND SHALL IMMEDIATELY CONTACT ENGINEER, PRIOR TO STARTING FOUNDATION CONSTRUCTION, IF THEY ARE NOT. THE FOUNDATIONS MAY NEED TO BE REDESIGNED IF ACTUAL SUBGRADE CONDITIONS ARE WORSE THAN THE ASSUMED CONDITIONS.
2. FOOTINGS HAVE BEEN DESIGNED FOR A MINIMUM PRESUMPTIVE LOAD-BEARING CAPACITY OF 2,000 PSF PER IBC SECTION 1806.2. CONTRACTOR SHALL NOTIFY ENGINEER IMMEDIATELY, PRIOR TO PROCEEDING WITH WORK, IF ACTUAL CONDITIONS DIFFER.
3. EXCAVATION

3.2. EXCAVATE SUBSOIL TO ACCOMMODATE FOUNDATIONS. HAND TRIM EXCAVATIONS. REMOVE LOOSE MATERIAL.

3.3. COMPACT DISTURBED LOAD BEARING SOIL IN DIRECT CONTACT WITH FOUNDATIONS TO ORIGINAL BEARING CAPACITY. PLACE A MINIMUM OF 12 INCHES OF CRUSHED STONE BENEATH FOOTINGS. WRAP STONE FULLY WITH NON-WOVEN FILTER FABRIC (MIRAFI 140N OR EQUIVALENT) OVERLAPPING 12 INCHES MINIMUM, IF CLAY SOILS ARE ENCOUNTERED IN EXCAVATIONS.

3.4. IF OVER-EXCAVATION OCCURS, REPLACE MATERIAL WITH SUITABLE WELL-DRAINED MATERIAL, IN 6 INCH LIFTS, APPROVED BY THE ENGINEER AND COMPACTED TO 95% OF MODIFIED PROCTOR. OVEREXCAVATION MAY ALSO BE REPLACED WITH LEAN CONCRETE WITH ENGINEER'S APPROVAL.

3.5. PROTECT THE SITE AND ALL CONSTRUCTION, EXISTING AND PROPOSED, FROM THE EFFECTS OF FREEZING OR FROST ACTION.
4. SUBMITTALS FOR REVIEW

4.1. SUBMIT SIEVE ANALYSIS AND STANDARD MOISTURE-DENSITY CURVE FOR EACH BACKFILL MATERIAL. RESUBMIT WHENEVER A NEW PIT OR SUBSTANTIALLY DIFFERENT MATERIAL IS USED.
5. BACKFILL AND COMPACTION

5.1. PLACE AND COMPACT BACKFILL IN EQUAL CONTINUOUS LAYERS NOT EXCEEDING 8" OF COMPACTED DEPTH FOR HAND HELD COMPACTION EQUIPMENT AND A MAXIMUM OF 12" INCHES COMPACTED DEPTH FOR VIBRATORY ROLLERS. DO NOT USE LARGE VIBRATORY ROLLERS WITHIN 5 FEET OF BASEMENT WALLS (USE HAND-HELD EQUIPMENT).

5.2. MAINTAIN +/- 1% OF OPTIMUM MOISTURE CONTENT OF BACKFILL MATERIALS TO ATTAIN COMPACTION DENSITY.

5.3. COMPACTION TESTING SHALL BE PERFORMED IN ACCORDANCE WITH ASTM D2922. TEST METHODS FOR SOIL BY NUCLEAR METHODS. MAXIMUM DENSITY SHALL BE DETERMINED BY THE MODIFIED PROCTOR METHOD, ASTM D1557.
6. BACKFILL REQUIREMENTS

6.1. FILL WITHIN BUILDING ENVELOPE WHERE OVER-EXCAVATION HAS OCCURRED:

6.1.1. MATERIAL: GRANULAR FILL

6.1.2. COMPACTION: 95% MODIFIED PROCTOR

6.1.3. TESTING: EVERY OTHER LIFT; EVERY 1,000 SF

6.2. CRUSHED STONE BELOW FOOTINGS AND SLAB-ON-GRADE:

6.2.1. MATERIAL: CRUSHED STONE

6.2.2. COMPACTION: MINIMUM 2 PASSES WITH COMPACTION EQUIPMENT
7. MATERIALS

7.1. GRANULAR FILL:

SIEVE DESIGNATION	% BY WEIGHT PASSING SIEVES
3 INCH	100
NO. 4	45-75
NO. 100	0-12
NO. 200	0-6

7.2. CRUSHED STONE:

SIEVE DESIGNATION	% BY WEIGHT PASSING SIEVES
1 INCH	100
¾ INCH	90-100
½ INCH	20-55
NO. 4	0-10
NO. 8	0-5

7.3. GEOTEXTILE FABRIC: NON-WOVEN GEOTEXTILE FABRIC, MIRAFI 140N OR EQUIVALENT.

CAST-IN-PLACE CONCRETE

1. GENERAL

1.1. CODES AND STANDARDS: COMPLY WITH THE PROVISIONS OF THE LATEST EDITIONS OF:

1.1.1. ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"

1.1.2. ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE"

1.1.3. ACI 305 "HOT WEATHER CONCRETING"

1.1.4. ACI 306 "STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING"

1.1.5. ACI 308 "STANDARD PRACTICE FOR CURING CONCRETE".

1.2. PRIOR TO PLACEMENT OF CONCRETE, SUBMIT TO ENGINEER MIX DESIGN INCLUDING TECHNICAL DATA SHEETS ON ANY ADMIXTURES TO BE USED.

1.3. CONCRETE TESTING: THE CONTRACTOR SHALL PREPARE A SET OF 4 CYLINDERS/TEST SET TO BE TESTED AT AN INDEPENDENT LABORATORY. THE CYLINDERS SHALL BE TAKEN FROM ONE CONCRETE TRUCK AND LABELED WITH DATE, TRUCK NUMBER, AND LOCATION OF CONCRETE PLACEMENT. EACH SAMPLE SHALL ALSO BE TESTED FOR SLUMP, AIR CONTENT, AND TEMPERATURE. THE CYLINDERS SHALL BE TESTED AS FOLLOWS: 1 AT 7 DAYS; 2 AT 28 DAYS; AND A FOURTH HELD FOR A 56 DAY BREAK IF REQUIRED. TEST CYLINDERS SHALL BE TAKEN AT LEAST ONCE PER FOOTING AND SLAB-ON-GRADE PLACEMENT.

1.4. FIELD TESTING SHALL BE PERFORMED BY A GRADE I ACI FIELD TESTING TECHNICIAN.

1.5. FIELD TESTING TO BE PAID FOR BY OWNER.

1.6. SUBMIT MIX DESIGN AND EITHER TRIAL MIX DESIGNS OR HISTORIC FIELD DATA FOR APPROVAL IN ACCORDANCE WITH ACI 301.

1.7. CONCRETE MIXES (PER ACI 318 REQUIREMENTS FOR CONCRETE BY EXPOSURE CLASS):

LOCATION	f _c (PSI)	DENSITY	MAX. AGG.	TARGET AIR	SLUMP	MAX. W/C	NOTES
FOOTINGS	3,500	145 PCF	¾"	6%	3'-5"	0.55	
EXTERIOR SLABS AND SIDEWALKS	4,500	145 PCF	¾"	6%	3'-5"	0.45	

1.8. TRANSIT MIX SHALL CONFORM TO ASTM C94.

1.9. NO CHLORIDE OR OTHER UNAUTHORIZED ADMIXTURES SHALL BE USED.

1.10. PLACE NO CONCRETE WHEN AMBIENT TEMPERATURE IS BELOW 40 DEGREES FAHRENHEIT OR MORE THAN 90 DEGREES FAHRENHEIT.

1.11. COMPLY WITH ACI CODES AND PLACE CONCRETE IN A CONTINUOUS OPERATION WITHIN PLANNED JOINTS OR SECTIONS. DO NOT PERMIT COLD JOINTS TO OCCUR.

1.12. CURING: BEGIN INITIAL CURING AS SOON AS FREE WATER HAS DISAPPEARED FROM EXPOSED SURFACES. CONTINUE CURING BY USE OF MOISTURE RETAINING COVER OR MEMBRANE-FORMING CURING COMPOUND. REFER TO ARCHITECTURAL SPECIFICATIONS FOR ADDITIONAL CURING FINISHES AND TREATMENT INFORMATION.

1.13. FINISHING: FLOOR SLABS

1.13.1. EXTERIOR SLABS: BROOM FINISH PERPENDICULAR TO PEDESTRIAN TRAFFIC.

1.14. UNLESS OTHERWISE NOTED, CENTER ALL FOUNDATION PIERS AND FOOTINGS ON COLUMN CENTERLINES.

2. CONCRETE REINFORCING

2.1. REINFORCING STEEL SHALL BE ASTM A615, 60 KSI YIELD GRADE, DEFORMED STEEL BARS PER ASTM A305.

2.2. WELDED STEEL WIRE FABRIC SHALL BE ASTM A185. LAP A MINIMUM OF TWO MESHES.

2.3. CHAIRS AND SPACERS SHALL BE PLACED TO ADEQUATELY SUPPORT REINFORCING DURING PLACEMENT. FOREIGN MATERIALS SUCH AS WOOD OR OTHER UNSUITABLE SUPPORTS SHALL NOT BE USED TO SUPPORT REINFORCING. SET WIRE TIES SO ENDS ARE DIRECTED INTO CONCRETE WHERE CONCRETE WILL BE EXPOSED.

2.4. CONCRETE CLEAR COVER FOR REINFORCEMENT (UNLESS SHOWN OTHERWISE):

2.4.1. BOTTOM OF FOOTINGS: 3"

2.4.2. SIDES OF FOOTINGS, GRADE BEAMS: 2"

2.5. UNLESS NOTED OTHERWISE, LAP LENGTHS SHALL BE AS FOLLOWS: 16" FOR #3, 22" FOR #4, 27" FOR #5, AND 32" FOR #6. (NOTE: THESE LAP LENGTHS ASSUME NORMAL-WEIGHT CONCRETE, F_c ≥ 3000 PSI, CLEAR COVER ≥ 1½", CENTER-TO-CENTER BAR SPACING ≥ 4 BAR DIAMETERS, NO EPOXY COVERING, AND NOT A "TOP BAR.")

2.6. DOWELS SHALL MATCH SIZE AND SPACING OF MAIN REINFORCEMENT, UNLESS OTHERWISE NOTED.

STRUCTURAL STEEL

1. SUBMITTALS FOR REVIEW

1.1. SHOP DRAWINGS: INDICATE PROFILES, SIZES, SPACING, LOCATIONS OF STRUCTURAL STEEL MEMBERS, INCLUDING OPENINGS, ATTACHMENTS, AND FASTENERS. SHOW ALL CONNECTION DETAILS. INDICATE WELDED CONNECTIONS WITH AWS A2.0 WELDING SYMBOLS.

2. SUBMITTALS FOR INFORMATION

2.1. MANUFACTURER'S MILL CERTIFICATE: CERTIFY THAT PRODUCTS MEET OR EXCEED SPECIFIED REQUIREMENTS.

2.2. WELDERS CERTIFICATES: CERTIFY WELDERS EMPLOYED ON THE WORK, VERIFYING AWS QUALIFICATION WITHIN THE PREVIOUS 12 MONTHS.

3. QUALITY ASSURANCE

3.1. FABRICATOR AND ERECTOR: COMPANY SPECIALIZING IN PERFORMING THE WORK OF THIS SECTION WITH MINIMUM FIVE YEARS EXPERIENCE.

4. MATERIALS

4.1. UNLESS NOTED OTHERWISE, STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:

4.1.1. WIDE FLANGES: ASTM A992, GRADE 50

4.1.2. ANGLES, CHANNELS, PLATE AND OTHER HOT ROLLED SHAPES: ASTM A36

4.1.3. TUBES: ASTM A500, GRADE C.

4.1.4. PIPES: ASTM A53, GRADE B.

4.2. BOLTS, NUTS, AND WASHERS:

4.2.1. BOLTS: TYPE 1, HEAVY HEX, MINIMUM ¾" DIAMETER. USE "A325" UNO (= ASTM F3125 GRADE A325 OR GRADE F1852); USE "A490" (= ASTM F3125 GRADE A490 OR GRADE F2280) WHERE NOTED.

4.2.2. NUTS: ASTM A563 HEAVY HEX.

4.2.3. WASHERS: ASTM F436 TYPE 1 HARDENED WASHERS.

4.2.4. PROVIDE BOLT ASSEMBLIES GALVANIZED TO ASTM A153 AT GALVANIZED STRUCTURAL MEMBERS.

4.3. ANCHOR BOLTS: ASTM F1554 GRADE 36, UNLESS NOTED OTHERWISE.

4.4. WELDING ELECTRODES: E70xx PER AWS D1.1 UNLESS NOTED OTHERWISE.

4.5. GROUT: ASTM C1107, FACTORY-PACKAGED, NONMETALLIC AGGREGATE, NONCORROSIVE AND NONSTAINING, SHRINKAGE-RESISTANT GROUT WITH MINIMUM COMPRESSIVE STRENGTH OF 7,000 PSI AT 28 DAYS.

5. FINISH

5.1. PREPARE STRUCTURAL COMPONENT SURFACES IN ACCORDANCE WITH SSPC SP-2 FOR ENCLOSED STEEL TO RECEIVE STANDARD PRIMER AND SSPC SP-6 FOR STEEL TO RECEIVE ZINC-RICH PRIMER.

5.2. SHOP PRIME STRUCTURAL STEEL MEMBERS. DO NOT PRIME SURFACES THAT WILL BE FIREPROOFED, FIELD WELDED, OR IN CONTACT WITH CONCRETE.

6. EXECUTION

6.1. FABRICATE AND ERECT STRUCTURAL STEEL MEMBERS IN ACCORDANCE WITH THE 2016 AISC CODE OF STANDARD PRACTICE.

6.2. INSTALL BOLTS IN ACCORDANCE WITH RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS"

6.2.1. JOINT TYPE: SNUG TIGHTENED, UNO.

6.3. WELD IN ACCORDANCE WITH AWS D1.1. MINIMUM WELD SIZES SHALL CONFORM TO AISC REQUIREMENTS FOR MATERIAL THICKNESS BUT SHALL NOT BE LESS THAN 1/4" UNLESS SPECIFICALLY NOTED TO THE CONTRARY.

6.4. ALLOW FOR ERECTION LOADS, AND FOR SUFFICIENT TEMPORARY BRACING TO MAINTAIN STRUCTURE SAFE, PLUMB, AND IN TRUE ALIGNMENT UNTIL COMPLETION OF ERECTION AND INSTALLATION OF PERMANENT BRACING.

6.5. DO NOT FIELD CUT OR ALTER STRUCTURAL MEMBERS WITHOUT APPROVAL OF ARCHITECT/ENGINEER.

6.6. AFTER ERECTION, PRIME WELDS, ABRASIONS, AND SURFACES NOT SHOP PRIMED, EXCEPT SURFACES TO BE IN CONTACT WITH CONCRETE.

6.7. GROUT UNDER BASE PLATES.
- HEAVY TIMBER FRAMING
1. SUBMITTALS FOR REVIEW

1.1. SHOP DRAWINGS: SHOW DESIGN LOADS AND CRITERIA, MATERIAL SPECIFICATIONS, LAYOUT OF TIMBER FRAME SYSTEM, FULL DIMENSIONS OF EACH MEMBER, AND LARGE-SCALE DETAILS OF JOINTS AND CONNECTIONS. SHOP DRAWINGS SHALL BEAR THE SEAL OF A REGISTERED PROFESSIONAL ENGINEER.

2. MATERIALS

2.1. SPECIES: DOUGLAS FIR

2.2. GRADE: SELECT STRUCTURAL (WWPA)

2.3. MOISTURE CONTENT: 19% MAXIMUM

2.4. DRESSING: S4S ½" UNDER STATED SIZE

2.5. OTHER: FREE OF HEART CENTER

3. GENERAL

3.1. WORK SHALL BE IN CONFORMANCE WITH THE TPEC 2-2018 CODE OF STANDARD PRACTICE FOR TIMBER FRAME STRUCTURES BY THE TIMBER FRAME ENGINEERING COUNCIL.

3.2. KEEP TIMBERS DRY DURING DELIVERY AND STORAGE. STORE TIMBERS OFF THE GROUND AND COVERED.

3.3. CUT AND STACK TIMBERS SO AS NOT TO ENCOURAGE THE GROWTH OF SAP-STAIN FUNGI, MOLD OR MILDEW. STACK TIMBERS WITH STICKERS AND SPACERS BETWEEN BUNDLES TO ALLOW AIR CIRCULATION.

3.4. PROVIDE REASONABLE PROTECTION DURING TRANSPORTATION, STORAGE, HANDLING AND ERECTION OF THE TIMBERS TO AVOID MARRING, STAINING, OR THE ACCUMULATION OF EXCESS MOISTURE, DIRT AND FOREIGN MATTER.

3.5. TIMBERS SHALL BE FABRICATED TO JOIN TIGHTLY AND IN PROPER ALIGNMENT AT THE TIME OF ASSEMBLY. SMALL (1/8" OR LESS) VARIATIONS WHERE THE FACES OF JOINTS COME TOGETHER (IN ALIGNMENT OR SEPARATION) ARE ACCEPTABLE. PROVISIONS SHALL BE MADE IN THE DETAILING OF JOINERY TO ALLOW FOR DIMENSIONAL CHANGES (JOINT SEPARATION AND ALIGNMENT) ASSOCIATED WITH TIMBER DRYING SHRINKAGE.
- BASIS OF DESIGN
1. BUILDING CODE: 2025 VERMONT FIRE & BUILDING SAFETY CODE

2. ROOF DEAD LOAD: 14 PSF ON SLOPE + WEIGHT OF FRAMING

3. ROOF LIVE LOAD: SNOW LOAD GOVERNS

4. ROOF SNOW LOAD DATA

4.1. GROUND SNOW LOAD, P_g40 PSF

4.2. FLAT ROOF SNOW LOAD, P_f40 PSF

4.3. SNOW EXPOSURE FACTOR, C_e1.0

4.4. SNOW IMPORTANCE FACTOR, I_s1.0

4.5. THERMAL FACTOR, C_t1.2

4.6. SLOPE FACTORS PER ASCE 7-16

5. WIND DESIGN DATA

5.1. BASIC DESIGN WIND SPEED, V110 MPH

5.2. ALLOWABLE STRESS DESIGN WIND SPEED, V_{asd}85 MPH

5.3. RISK CATEGORYII

5.4. WIND EXPOSUREB

5.5. INTERNAL PRESSURE COEFFICIENTSNA (OPEN BUILDING)

5.6. COMPONENTS AND CLADDING WIND PRESSURE PER ASCE 7-16

6. EARTHQUAKE DESIGN DATA

6.1. RISK CATEGORYII

6.2. SEISMIC IMPORTANCE FACTOR, I_e1.0

6.3. MAPPED SPECTRAL RESPONSE ACCELERATION, S_S0.179

6.4. MAPPED SPECTRAL RESPONSE ACCELERATION, S₁0.060

6.5. SITE CLASSD - DEFAULT

6.6. SPECTRAL RESPONSE COEFFICIENT, SDS0.191

6.7. SPECTRAL RESPONSE COEFFICIENT, SD10.096

6.8. SEISMIC DESIGN CATEGORYB

6.9. BASIC SEISMIC-FORCE-RESISTING SYSTEMSTEEL ORDINARY CANTILEVER COLUMN SYSTEMS

6.10. DESIGN BASE SHEAR4.8K

6.11. SEISMIC RESPONSE COEFFICIENT, C_S0.153

6.12. RESPONSE MODIFICATION FACTOR, R1.25

6.13. ANALYSIS PROCEDURE USED EQUIVALENT LATERAL FORCE PROCEDURE

7. GEOTECHNICAL INFORMATION

7.1. DESIGN LOAD-BEARING VALUE OF SOIL2,000 PSF (PRESUMPTIVE)

7.2. MINIMUM FROST COVER5'-0"
- STATEMENT OF SPECIAL INSPECTIONS
1. THE OWNER OR OWNER'S AGENT SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PERFORM SPECIAL INSPECTIONS DURING CONSTRUCTION OF THE TYPES OF WORK LISTED BELOW. REFER TO GENERAL NOTES FOR SOIL COMPACTION TESTING INFORMATION AND ADDITIONAL CONCRETE CYLINDER TESTING NOT LISTED BELOW.

2. COMPLY WITH IBC 2015, CHAPTER 17 "SPECIAL INSPECTIONS AND TESTS" AND IN ACCORDANCE WITH THE PROJECT MANUAL:

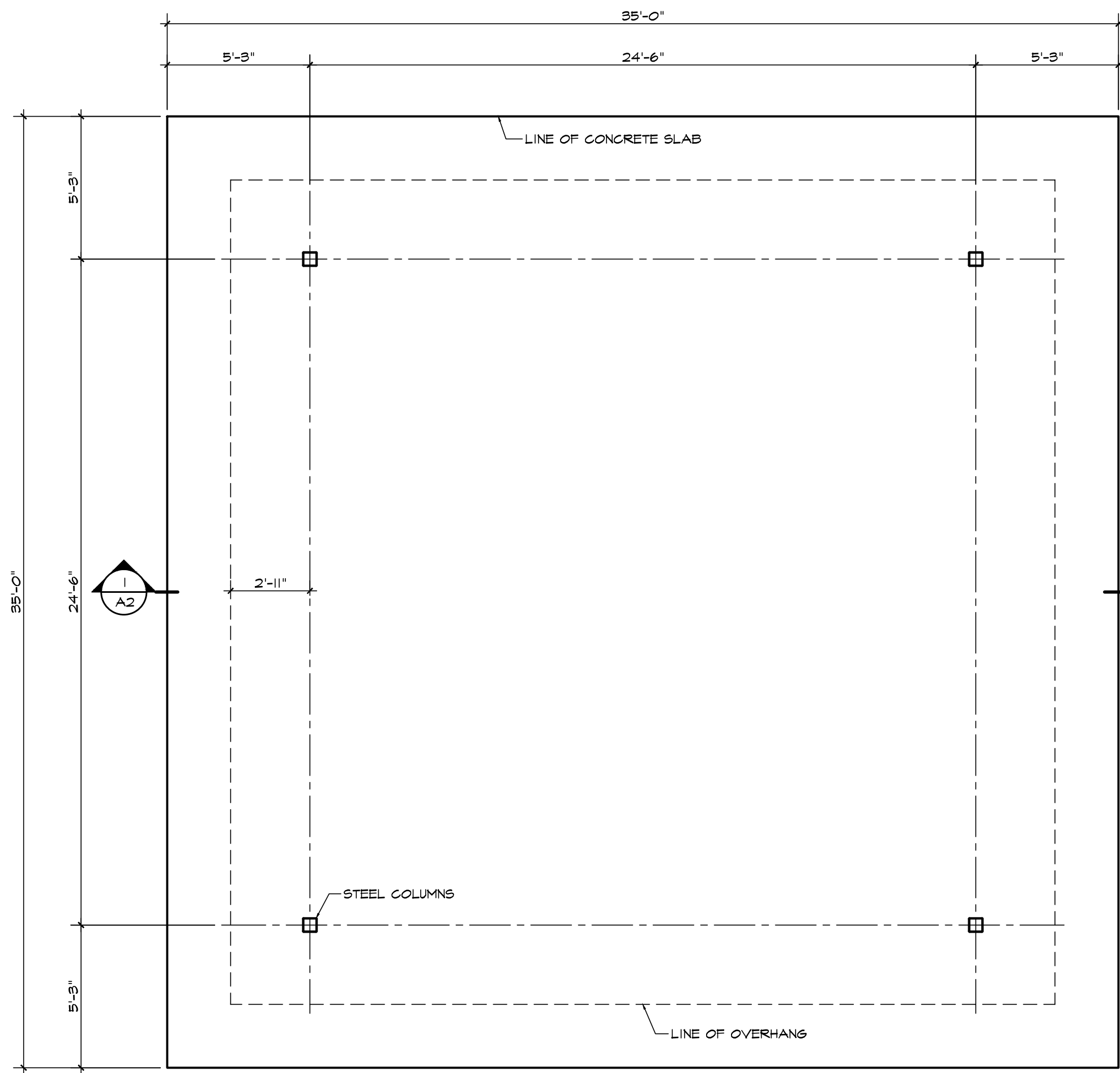
2.1. CONCRETE: INSPECTIONS NOT REQUIRED PER SECTION 1705.3, SEE CAST-IN-PLACE CONCRETE NOTES FOR ADDITIONAL TESTING.

2.2. STRUCTURAL STEEL: INSPECT FIELD BOLTING IN ACCORDANCE WITH SECTION 1705.2 (AISC 360).

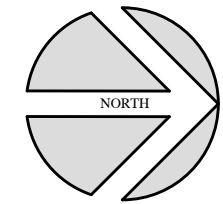
2.3. SOILS: SEE TABLE BELOW IN ACCORDANCE WITH SECTION 1705.6

2.4. ADDITIONAL SPECIAL INSPECTIONS FOR WIND RESISTANCE NOT REQUIRED PER SECTION 1705.12.

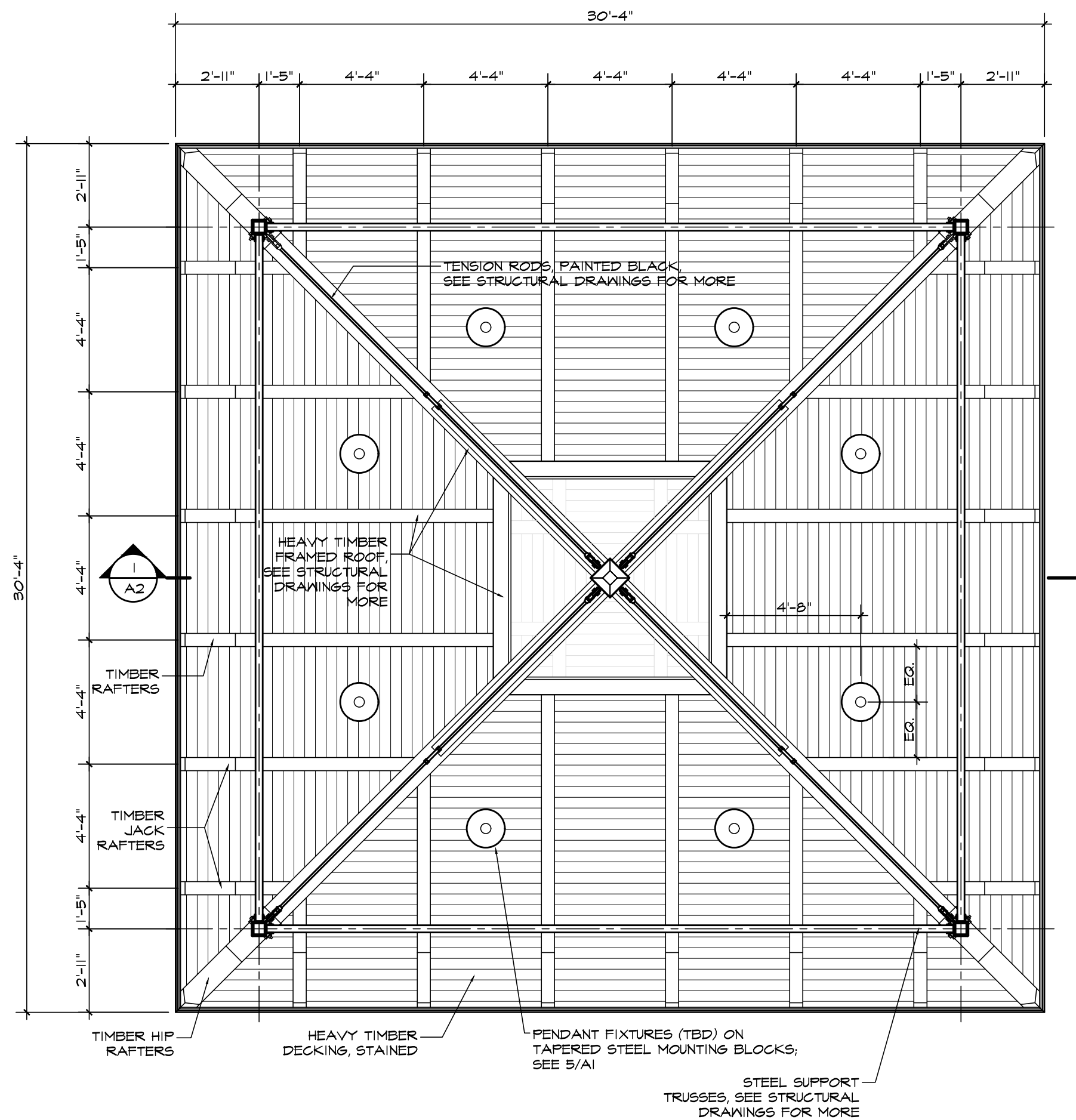
2.5. ADDITIONAL SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE NOT REQUIRED PER SECTION 1705.13.
- | IBC 1705.6 - SOILS | | |
|--|-------------------------------------|-------------------------------------|
| VERIFICATION AND INSPECTION | CONTINUOUS | PERIODIC |
| 1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY. | ☒ | ☐ |
| 2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL. | ☒ | ☐ |
| 3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS | ☐ | ☒ |
| 4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL. | ☒ | ☐ |
| 5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY. | ☒ | ☐ |
-
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- BENNINGTON MUSEUM
EVENT PAVILION
BENNINGTON, VT
CONSTRUCTION SET
- DATE ISSUED: 3/19/26
- REVISIONS:
- DRAWN BY: JM
- CHECKED BY:
- TITLE:
- STRUCTURAL
NOTES
- DRAWING NO.
- S3



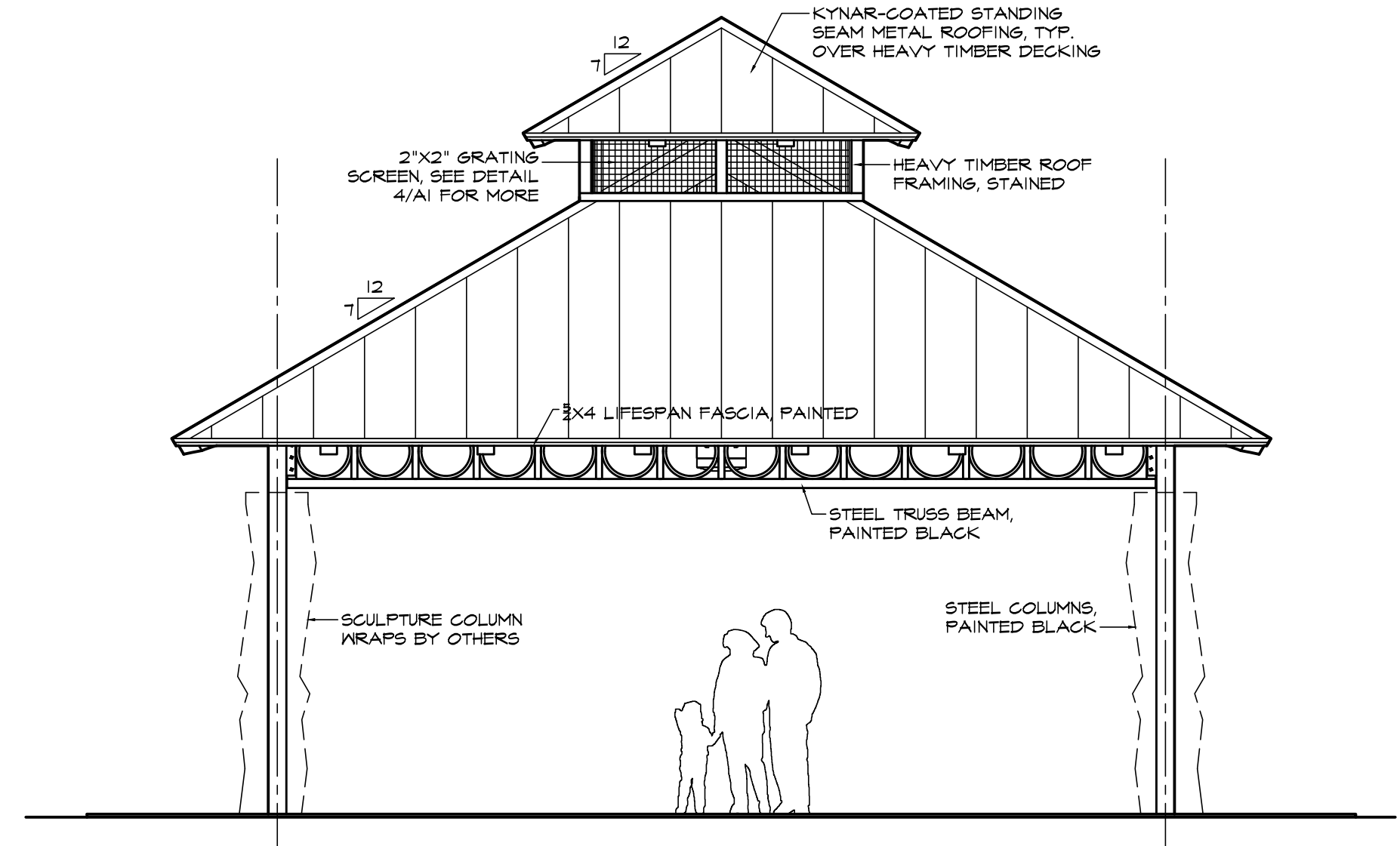
NOTE: PROVIDE BOLLARD FOR ELECTRICAL SERVICE W/ QUAD RECTANGLE & SWITCHES FOR LIGHTING IN PAVILION; SEE SITE PLAN FOR LOCATION



1 FIRST FLOOR PLAN
1/4"=1'-0"



2 REFLECTED CEILING PLAN
1/4"=1'-0"



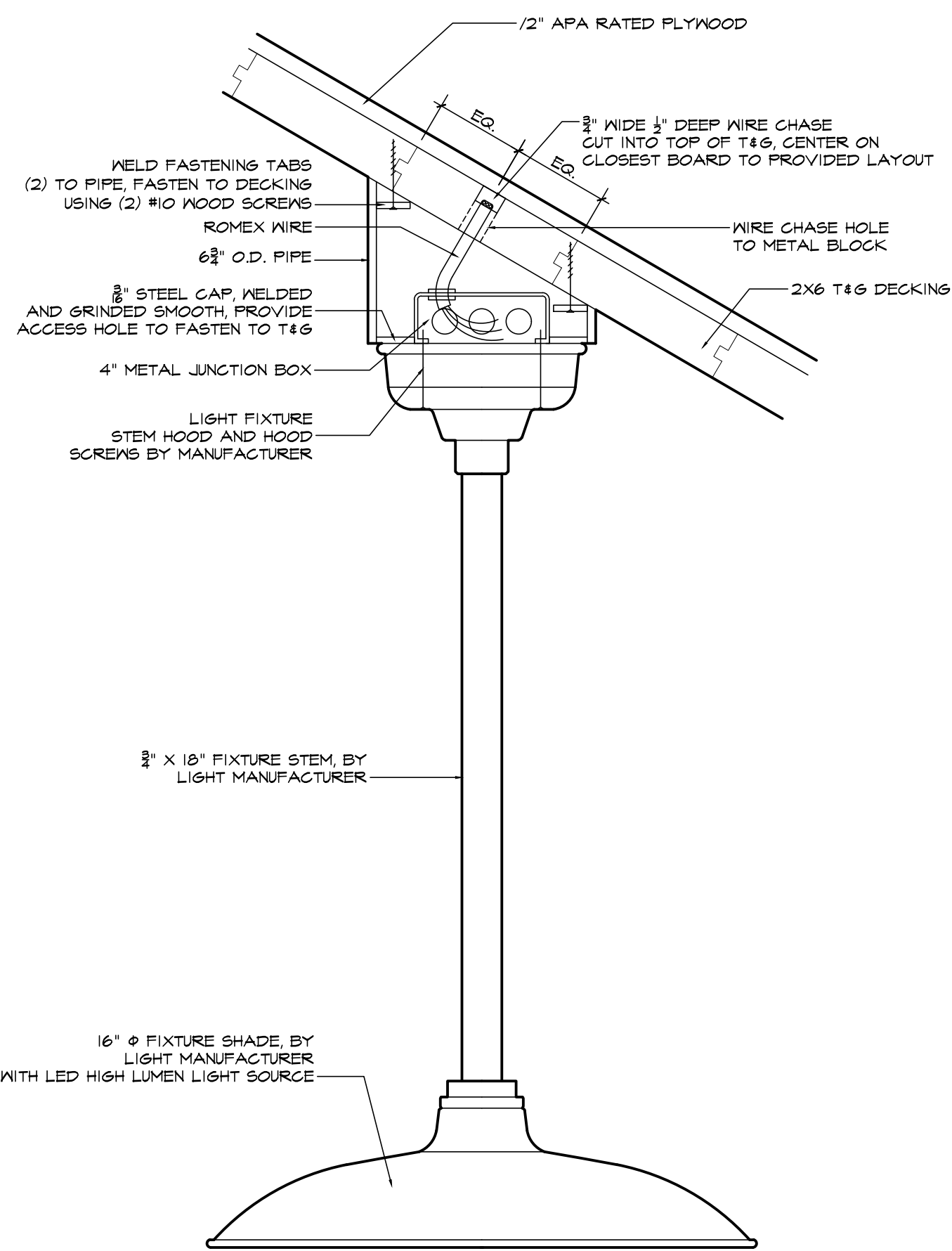
3 EAST ELEVATION
1/4"=1'-0"



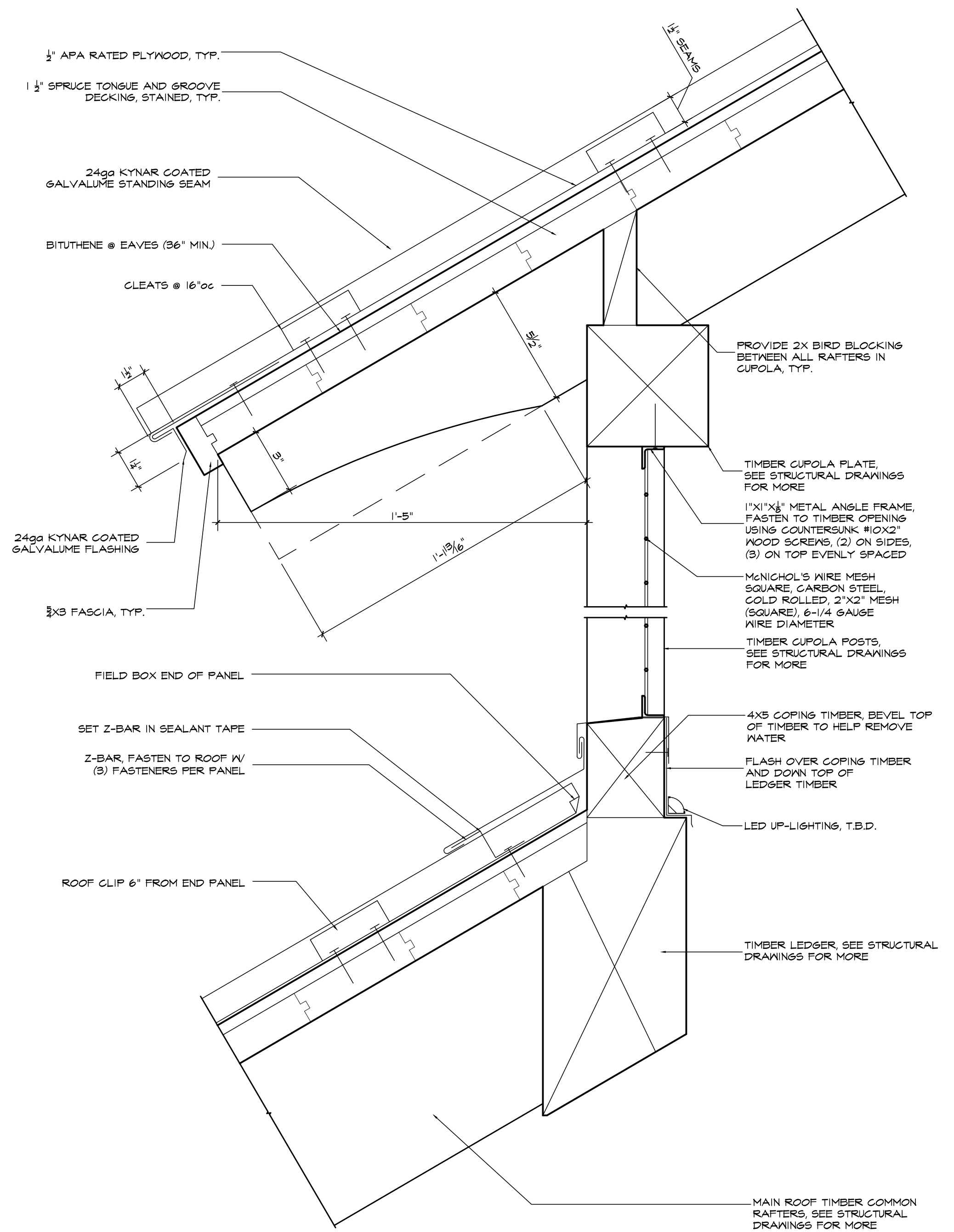
6 EXTERIOR RENDERING VIEW
NTS



7 INTERIOR RENDERING VIEW
NTS



5 SECTION @ CUPOLA
3"=1'-0"



4 SECTION @ CUPOLA
3"=1'-0"

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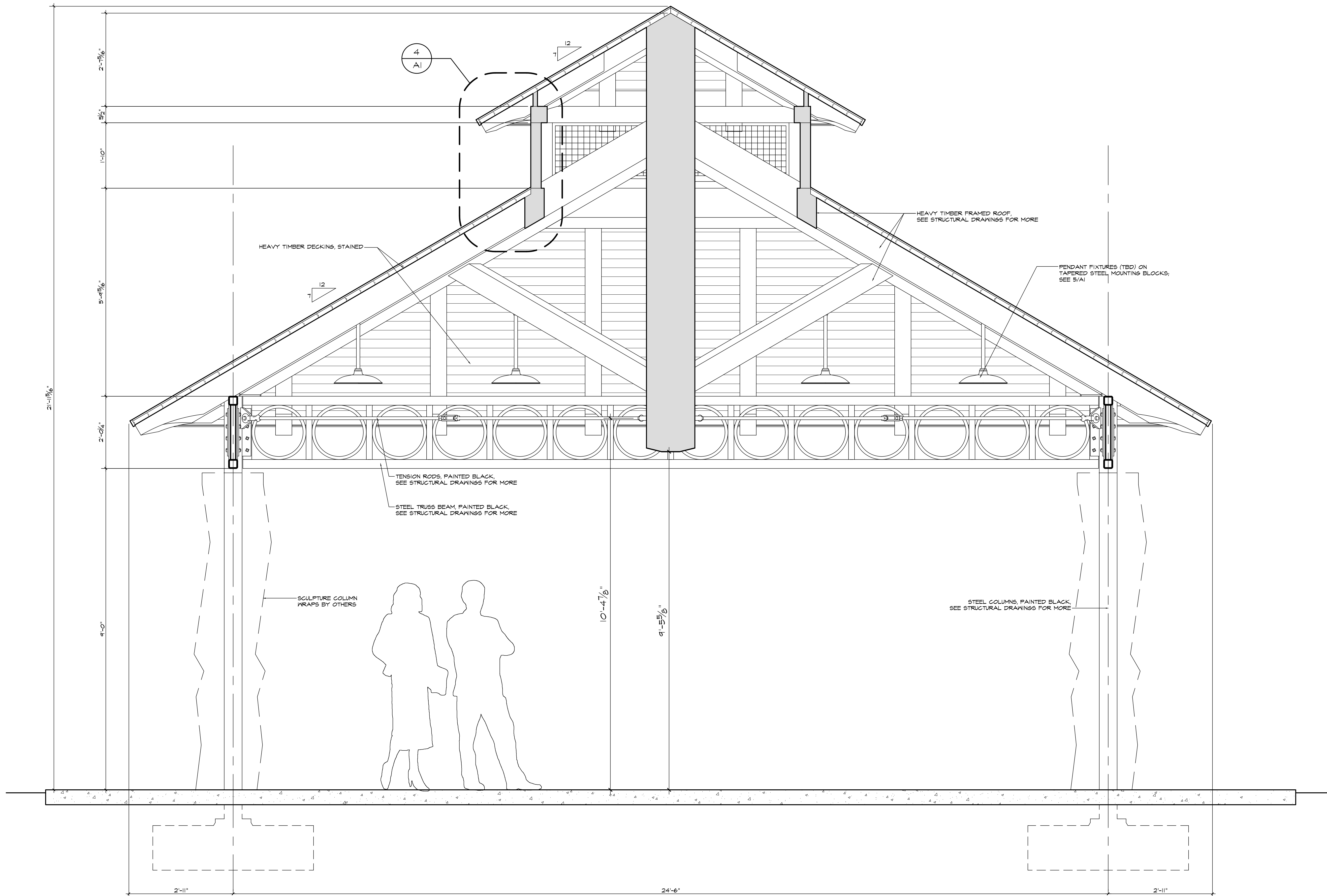
CHECKED BY:

TITLE:

FLOOR PLANS/
ELEVATIONS/
DETAILS

DRAWING NO.

A1



1
A2 BUILDING SECTION
3/4" = 1'-0"

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TITLE:

BUILDING
SECTION

DRAWING NO.

A2