A. INTRODUCTION AND SUMMARY OF FINDINGS

This chapter examines the potential effects of the Proposed Project on the transportation system in the "Traffic Study Area" described below and in **Figure 11-1**, describing existing conditions within the Traffic Study Area and comparing future conditions in 2032 (the "Build year") both without the Proposed Project (the "No Build" condition) and with the Proposed Project (the "Build" condition).

The time periods and study intersections analyzed were developed in coordination with the City of Yonkers and their consultants. Traffic conditions were evaluated at 38 intersections for the Weekday AM and PM peak hours. Traffic conditions for 21 of these intersections were also evaluated for the Saturday midday peak hour.

The analyses did not indicate project-related pedestrian safety, pedestrian, or transit impacts at the evaluated intersections. The analyses did indicate project-related traffic impacts at 18 of the 38 evaluated intersections. Measures to mitigate those impacts are proposed. **Table 11-1** lists the intersections and time/days where project-related impacts would occur and where mitigation measures are proposed to return to the future No Build condition.

A majority of the impacted intersections would operate at an overall level of service D or better with the proposed mitigations; however, while mitigated to the No Build condition, the following intersections would operate at an overall level of service E:

- Riverdale Avenue and Prospect Street
- Nepperhan Avenue and Ashburton Avenue

The Proposed Project would result in the removal of existing on- and off-street parking at both the Chicken Island and North Broadway Sites. The Proposed Project, with the proposed changes to the residential parking requirements, is expected provide sufficient off-street parking through a combination of valet parking, self-parking, and shared parking operations to accommodate the project-generated increase in parking demand.

Table 11-1 Summary of Project-Related Traffic Impacts

	Proposed Action							
	Week	day AM	Weekda		Saturday ¹			
	Traffic	Mitigation	Traffic	Mitigation	Traffic	Mitigation		
Intersection	Impact	Provided	Impact	Provided	Impact	Provided		
Buena Vista / Prospect Street	SBLT	Yes	WBR NBTR SBLT	Yes	Not Impacted	N/A		
Nepperhan Street / Warburton Avenue / Dock Street / Manor House Square	SBLTR	Yes	SBLTR	Yes	Not Impacted	N/A		
Riverdale Avenue / Warburton Avenue / Main Street	Not Impacted	N/A	EBLTR	Yes	Not Impacted	N/A		
Riverdale Avenue / Hudson Street	Not Impacted	N/A	EBLTR	Yes	Not Impacted	N/A		
Riverdale Avenue / Prospect Street	WBL	Yes	EBLTR WBL WBT	Yes	Not Impacted	N/A		
Broadway / Hudson Street	Not Impacted	N/A	EBLR	Yes	Not Impacted	N/A		
South Broadway / Prospect Street / Nepperhan Avenue	EBTR WBL SBL	Yes	WBL NBTR SBL	Yes	EBTR	Yes		
South Broadway / Vark Street / Park Hill Avenue	Not Impacted	N/A	NBLTR	Yes				
New Main Street / Nepperhan Avenue	Not Impacted	N/A	Not Impacted	N/A	WBL NBLTR	Yes		
Waverly Street / Nepperhan Avenue	NBLR	Yes	NBLR	Yes	NBLR	Yes		
Nepperhan Avenue / Ashburton Avenue	EBL	Yes	EBL	Yes				
Nepperhan Avenue / Elm Street	EBL	Yes	EBL NBTR	Yes	EBL	Yes		
Walnut Street / Yonkers Avenue	Not Impacted	N/A	EBL	Yes				
Yonkers Avenue / Saw Mill Northbound Ramps	EBL	Yes	EBL WBT	Yes				
Yonkers Avenue / Midland Avenue (West)	SBL	Yes	Not Impacted	N/A				
Yonkers Avenue / Saw Mill Southbound Ramps	SBR	Yes	SBR	Yes				
Yonkers Avenue / Cross County Parkway On-Ramp / Midland Avenue (East)	SBTR	Yes	Not Impacted	N/A				
Hawthorne Avenue / Prospect Street	Not Impacted	N/A	WBL	Yes	Not Impacted	N/A		
Locust Hill Avenue / Ashburton Avenue	Not Impacted	N/A	NBLR	No				

Ashburton Avenue | Impacted | NA | NOLE | NOLE | NOTE | NO SB = Southbound, N/A = Not Applicable.

1. Shading indicates intersection was not evaluated during Saturday peak hour.

B. CAPACITY ANALYSIS

B.1. SIGNALIZED INTERSECTIONS

The operation of signalized intersections in the Traffic Study Area (see **Figure 11-1**) was analyzed by applying the Percentile Delay Methodology included in the Synchro 10 traffic signal software. The Percentile Delay Methodology calculates vehicle delays for five different percentile scenarios (10th, 30th, 50th, 70th, and 90th) and takes the volume weighted average of the scenarios as compared to the *Highway Capacity Manual 6th Edition* (HCM), which calculates delay for a single average scenario. The Percentile Delay Methodology was used in this analysis (versus HCM) because it includes a queue delay component to account for the effects of queues and blocking on short links and turning bays. The methodology evaluates signalized intersections for average delay per vehicle and Level of Service (LOS).

LOS is characterized for the entire intersection, each intersection approach, and/or each lane group. LOS is the only measure of effectiveness provided for the entire intersection operation. Total delay and volume-to-capacity (v/c) ratio are used to characterize LOS for a lane group. The volume-to-capacity ratio quantifies the degree to which a phase's capacity is utilized by a lane group.

LOS A describes operation with a delay of 10 seconds per vehicle or less. This level is typically assigned progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

LOS B describes operation with delay between 10 and 20 seconds per vehicle. This level is typically assigned when the progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.

LOS C describes operation with delay between 20 and 35 seconds per vehicle. This level is typically assigned when the volume-to-capacity ratio is favorable or the cycle length is moderate. Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

LOS D describes operation with delay between 35 and 55 seconds per vehicle. This level is typically assigned when the volume-to-capacity ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

LOS E describes operation with delay between 55 and 80 seconds per vehicle. This level is typically assigned when the volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

LOS F describes operation with delay exceeding 80 seconds per vehicle or a volume-to-capacity ratio greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

A lane group can incur a delay less than 80 seconds per vehicle when the volume-to-capacity ratio exceeds 1.0. This condition typically occurs when the cycle length is short,

the signal progression is favorable, or both. As a result, both the delay and volume-to-capacity ratio are considered when lane group LOS is established. A ratio of 1.0 or more indicates that an intersection is at capacity and experiences heavy congestion.

HCM's standard delay criteria for the range of service levels at signalized intersections are shown in **Table 11-2**.

Table 11-2 LOS Criteria for Signalized Intersections

	8						
	Level-of-Se	rvice (LOS) ¹					
Total Delay Per Vehicle	v/c ratio ≤ 1.0	v/c ratio > 1.0					
≤ 10.0 seconds	Α	F					
>10.0 and ≤ 20.0 seconds	В	F					
>20.0 and ≤ 35.0 seconds	С	F					
>35.0 and ≤ 55.0 seconds	D	F					
>55.0 and ≤ 80.0 seconds	E	F					
>80.0 seconds	F	F					

Note: ¹ For approach-based and intersection-wide assessments, LOS is defined solely by delay. **Source:** Transportation Research Board. *Highway Capacity Manual, 6th Edition*.

B.2. UNSIGNALIZED INTERSECTIONS

LOS for a two-way stop-controlled (TWSC) and all-way stop-controlled (AWSC) intersections is determined by the computed or measured control delay using HCM methodology. LOS is determined for each minor-street movement (or shared movement), major-street left turns at TWSC intersections, and for all movements at AWSC intersections. LOS is not defined for the intersection as a whole for TWSC intersections. HCM's standard LOS criteria for TWSC and AWSC unsignalized intersections are summarized in **Table 11-3**.

Table 11-3 LOS Criteria for Unsignalized Intersections

8						
	Level-of-Service (LOS) ¹					
Control Delay Per Vehicle	v/c ratio ≤ 1.0	v/c ratio > 1.0				
≤ 10.0 seconds	Α	F				
>10.0 and ≤ 15.0 seconds	В	F				
>15.0 and ≤ 25.0 seconds	С	F				
>25.0 and ≤ 35.0 seconds	D	F				
>35.0 and ≤ 50.0 seconds	E	F				
>50.0 seconds	F	F				

Note: ¹ For TWSC intersections, the LOS criteria apply to each lane on a given approach and to each approach on the minor street (for TWSC intersections). LOS is not calculated for major-street approaches or for the intersection as a whole.

Source: Transportation Research Board. Highway Capacity Manual, 6th Edition.

The LOS criteria for unsignalized intersections are somewhat different from the criteria used for signalized intersections. At TWSC intersections, drivers on the stop-controlled approaches need to find a break in the traffic to cross a lane or make a turn. When drivers on the stop-controlled approach are waiting in a traffic queue, this results in additional delay incurred while waiting to enter the main roadway. AWSC intersections require drivers on all approaches to stop before proceeding into the intersection.

C. 2020 EXISTING CONDITIONS

To assess potential traffic impacts associated with the Proposed Project, key intersections in the Traffic Study Area that might be affected by Project generated trips were identified. As presented in **Figure 11-1**, 38 intersections were identified for analysis during the weekday AM and PM peak hours (locations denoted with an asterisk were also identified in coordination with the City for analysis during the Saturday peak hour):

- 1. Buena Vista Avenue and Main Street*
- 2. Buena Vista Avenue and Hudson Street*
- 3. Buena Vista Avenue and Prospect Street*
- 4. Prospect Street and Hawthorne Street*
- 5. Hawthorne Avenue/Market Place and Main Street*
- 6. Hawthorne Avenue and Hudson Street*
- 7. Warburton Avenue and Ashburton Avenue
- 8. Warburton Avenue and Wells Avenue*
- 9. Warburton Avenue and Nepperhan Street*
- 10. Riverdale Avenue and Main Street*
- 11. Riverdale Avenue and Hudson Street*
- 12. Riverdale Avenue and Prospect Street*
- 13. Riverdale Avenue and Vark Street
- 14. North Broadway and Ashburton Avenue
- 15. North Broadway and Manor House Way*
- 16. North Broadway/South Broadway and New Main St/Palisade Avenue*
- 17. South Broadway and Hudson Street*
- 18. South Broadway and Nepperhan Avenue/Prospect Street*
- 19. South Broadway and Vark Street/Park Hill Avenue
- 20. Locust Hill Avenue and Palisade Avenue*
- 21. Locust Hill Avenue and Ashburton Avenue
- 22. New Main Street and Nepperhan Avenue*
- 23. Palisade Avenue and Ashburton Avenue
- 24. Palisade Avenue and Lafayette Place
- 25. Palisade Avenue/School Street and Elm Street*
- 26. School Street and Nepperhan Avenue*
- 27. Waverly Street and Nepperhan Avenue*
- 28. Nepperhan Avenue and Ashburton Avenue
- 29. Nepperhan Avenue and Copcutt Lane
- 30. Nepperhan Avenue and Elm Street*
- 31. Yonkers Avenue and Walnut Street
- 32. Yonkers Avenue and Prescott Street
- 33. Yonkers Avenue and Ashburton Avenue
- 34. Yonkers Avenue and Saw Mill River Parkway Southbound Ramps
- 35. Yonkers Avenue and Saw Mill River Parkway Northbound Ramps
- 36. Yonkers Avenue and Wasylenko Lane
- 37. Yonkers Avenue and Midland Avenue (east)
- 38. Yonkers Avenue and Midland Avenue (west)

The global pandemic of Coronavirus disease 2019 (COVID-19) resulted in atypical levels and patterns of vehicular traffic. As such, traffic volume field measurements were lower than would

otherwise be anticipated, and therefore, not representative of typical existing traffic conditions that serve as a baseline in estimating future traffic conditions.

In its recently released *Traffic Data Collection Guidance during COVID-19 Pandemic Memorandum*, NYSDOT recommends collecting turning movement count (TMC) and automatic traffic recorder (ATR) data during this time and adjusting the counts based a single factor of the collected ATR data and the historical ATR data. However, this approach does not account for changes in traffic patterns, including turning movement percentages at intersections, during the pandemic, and an adjustment of TMC using a single factor may not reflect historical or "normal" conditions.

To more accurately establish baseline traffic volumes to reflect pre-pandemic conditions, historical traffic volume data were sourced from the StreetLight Insight data platform. StreetLight Data is historical location-based service data aggregated over several months and may better reflect historical, or "normal," traffic patterns including turning movement percentages. The StreetLight data were calibrated utilizing historical traffic data from the following sources:

- NYSDOT Traffic Data Viewer Annual Average Daily Traffic (AADT) and Average Daily Traffic (ADT)
- Westchester County Department of Public Works AADT and ADT
- Buena Vista Teutonia Development Traffic Study (2017) TMC and ATR
- City TMC and ATR for nearby projects collected within the past five years

Due to limited historical data for the Saturday peak hour, AKRF also collected TMCs on Saturday, November 14, 2020 at the following intersections to validate/calibrate the Saturday StreetLight data:

- Buena Vista Avenue and Main Street
- Buena Vista Avenue and Prospect Street
- North Broadway and Ashburton Avenue
- South Broadway and Nepperhan Avenue/Prospect Street
- Palisade Avenue/School Street and Elm Street

Traffic count data are provided in **Appendix L-1**.

Based on a review of the traffic count data, the peak hours for the Traffic Study Area were determined to be as follows:

Weekday AM: 7:00–8:00 AM
Weekday PM: 5:00–6:00 PM
Saturday Midday: 1:00–2:00 PM

C.1. ROADWAY AND INTERSECTION CHARACTERISTICS

The following is a brief description of the major roadways and intersections within the Traffic Study Area.

C.1.a. Ashburton Avenue

Ashburton Avenue traverses the Traffic Study Area in an east-west direction between Alexander Street/Polychrome Street to the west and Yonkers Avenue to

the east and is maintained by the City. NYSDOT classifies Ashburton Avenue as a major collector roadway from Alexander Street to Warburton Avenue, a minor arterial roadway from Warburton Avenue to North Broadway, a principal arterial roadway from North Broadway to Palisade Avenue, and a minor arterial roadway from Palisade Avenue to Yonkers Avenue. Ashburton Avenue is also designated as NYS Route 9A ("Route 9A") between North Broadway and Saw Mill River Road. Ashburton Avenue generally provides one moving lane in each direction with additional left and right turn lanes at several intersections along its length. Two-way traffic volumes along Ashburton Avenue range from approximately 225 to 1,240 vehicles per hour (vph) within the Traffic Study Area. The pavement along Ashburton Avenue ranges from approximately 34 to 53 feet in width and was observed to be in fair to good condition. There is an overpass clearance of 12'1" where Ashburton Avenue passes underneath the Metro-North Railroad ("MNR") Hudson Line railroad tracks.

C.1.b. Buena Vista Avenue

Buena Vista Avenue is a local, City-maintained roadway that generally traverses the Traffic Study Area in a north-south direction. Buena Vista Avenue provides one moving lane in each direction with two-way traffic volumes that range from approximately 44 to 731 vph within the Traffic Study Area. The pavement of Buena Vista Avenue ranges from approximately 33 to 56 feet in width was observed to be in fair to good condition.

C.1.c. Elm Street

Elm Street traverses the Traffic Study Area in an east-west direction and is maintained by the City. NYSDOT classifies Elm Street as a principal arterial between Palisade Avenue/New School Street and Nepperhan Avenue and as a minor arterial east of Nepperhan Avenue. Between Palisade Avenue/New School Street and Nepperhan Avenue, Elm Street is a one-way eastbound street with one-way traffic volumes that range from approximately 264 to 368 vph. East of Nepperhan Avenue, Elm Street provides two-way traffic flow with one moving lane in each direction with two-way traffic volumes that range from approximately 335 to 847 vph within the Traffic Study Area. The pavement along Elm Street ranges from approximately 35 to 37 feet in width and was observed to be in good condition.

C.1.d. Hawthorne Avenue

Hawthorne Avenue is classified by NYSDOT as a major collector roadway and is maintained by the City. Hawthorne Avenue generally traverses the Traffic Study Area in a north-south direction. South of Prospect Street, Hawthorne Avenue is a one-way southbound street with one-way traffic volumes that range from approximately 69 to 171 vph within the Traffic Study Area. From Prospect Street to Hudson Street, Hawthorne Avenue is a one-way northbound street with one-way traffic volumes that range from approximately 112 to 189 vph. From Hudson Street to Main Street, Hawthorne Avenue provides two-way traffic flow with one moving lane in each direction and two-way traffic volumes that range from approximately 87 to 164 vph. The pavement along Hawthorne Avenue ranges from approximately 22 to 23 feet in width and was observed to be in fair to good condition.

C.1.e. Hudson Street

Hudson Street generally traverses the Traffic Study Area in an east-west direction between Buena Vista Avenue to the west and South Broadway to the east and is maintained by the City. NYSDOT classifies Hudson Street as a principal arterial roadway between Hawthorne Avenue and Riverdale Avenue. Between Buena Vista Avenue and Hawthorne Avenue, Hudson Street provides two-way traffic flow with one moving lane in each direction and two-way traffic volumes that range from approximately 81 to 193 vph. Between Hawthorne Avenue and Riverdale Avenue, Hudson Street is one-way eastbound with one-way traffic volumes that range from approximately 74 to 614 vph. The pavement along Hudson Street ranges from approximately 34 to 36 feet in width and was observed to be in fair to good condition.

C.1.f. Main Street

Main Street generally traverses the Traffic Study Area in an east-west direction between the Hudson River Waterfront to the west and Palisade Avenue to the east and is maintained by the City. NYSDOT classifies Main Street as a principal arterial. Between Buena Vista Avenue and Riverdale Avenue/Warburton Avenue, Main Street provides two-way traffic flow with one moving lane in each direction and two-way traffic volumes that range from approximately 97 to 545 vph. Between Riverdale Avenue and Palisade Avenue, Main Street is a one-way westbound street with one-way traffic volumes that range from approximately 206 to 447 vph. The pavement along Main Street ranges from approximately 12 to 45 feet in width and was observed to be in good condition. There is an overpass clearance of 10'10" where Main Street passes underneath the MNR Hudson Line railroad tracks.

C.1.g. New Main Street

New Main Street is classified by NYSDOT as a principal arterial roadway and is maintained by the City. New Main Street generally traverses the Traffic Study Area in a north-south direction. South of Nepperhan Avenue, New Main Street provides two-way traffic flow with one moving lane in each direction and two-way traffic volumes that range from approximately 681 to 726 vph within the Traffic Study Area. Between Nepperhan Avenue and Palisade Avenue, New Main Street is a one-way northbound street with one-way traffic volumes that range from approximately 338 to 509 vph. The pavement along New Main Street ranges from approximately 34 to 40 feet in width and was observed to be in good condition. The east side of New Main Street at its approach to Palisade Avenue is designated and signed as a taxi stand.

C.1.h. Nepperhan Avenue

Nepperhan Avenue generally traverses the Traffic Study Area in a north-south direction and is maintained by NYSDOT. NYSDOT classifies Nepperhan Avenue as a principal arterial roadway. Nepperhan Avenue generally provides two to three moving lanes in each direction with additional left and right turn lanes at several intersections along its length. Two-way traffic volumes along Nepperhan Avenue range from approximately 622 to 3,280 vph within the Traffic Study Area. The pavement along Nepperhan Avenue ranges from approximately 62 to 103 feet in width and was observed to be in good condition.

C.1.i. New School Street

New School Street is classified by NYSDOT as a principal arterial roadway and is maintained by the City. New School Street generally traverses the Traffic Study Area in a north-south direction. South of Nepperhan Avenue, New School Street is a one-way southbound street with one-way traffic volumes that range from approximately 33 to 77 vph within the Traffic Study Area. Between Nepperhan Avenue and Palisade Avenue/Elm Street, New School Street provides two-way traffic flow with one moving lane in each direction and two-way traffic volumes that range from approximately 290 to 532 vph. The pavement along New School Street ranges from approximately 18 to 48 feet in width and was observed to be in good condition.

C.1.j. Riverdale Avenue

Riverdale Avenue generally traverses the Traffic Study Area in a north-south direction and is maintained by NYSDOT. NYSDOT classifies Riverdale Avenue as a principal arterial roadway. Riverdale Avenue provides two moving lanes in each direction with left and right turn lanes at several intersections along its length. Two-way traffic volumes along Riverdale Avenue range from approximately 449 to 1,510 vph within the Traffic Study Area. The pavement along Riverdale Avenue ranges from approximately 77 to 82 feet in width and was observed to be in fair to good condition. Riverdale Avenue has a posted speed limit of 30 mph.

C.1.k. Route 9/Broadway

Broadway is classified by NYSDOT as a principal arterial roadway and is maintained by the City. Broadway is also designated as NYS Routes 9 and 9A ("Routes 9 and 9A") south of Ashburton Avenue and as Route 9 north of Ashburton Avenue. Broadway traverses the Traffic Study Area in a north-south direction and is designated as South Broadway south of Main Street and as North Broadway north of Main Street. Between Vark Street and Hudson Street, South Broadway provides two-way traffic flow with one moving lane in each direction and two-way traffic volumes that range from approximately 384 to 1,094 vph. Between Hudson Street and Wells Avenue, South/North Broadway is a one-way northbound street with one-way traffic volumes that range from approximately 361 to 768 vph. Between Wells Avenue and Ashburton Avenue, North Broadway generally provides two-way traffic flow with one moving lane in each direction and two-way traffic volumes that range from approximately 611 to 711 vph. The pavement along Broadway ranges from approximately 20 to 66 feet in width and was observed to be in fair to good condition.

C.1.l. Palisade Avenue

Palisade Avenue generally traverses the Traffic Study Area in a north-south direction and is maintained by the City. NYSDOT classifies Palisade Avenue as a principal arterial roadway from New Main Street to Ashburton Avenue and a major collector roadway north of Ashburton Avenue. Between New Main Street and New School Street/Elm Street, Palisade Avenue is a one-way eastbound street with one-way traffic volumes that range from approximately 229 to 469 vph. Between New School Street/Elm Street and Ashburton Avenue, Palisade Avenue provides two-way traffic flow with one moving lane in each direction and two-

way traffic volumes that range from approximately 340 to 670 vph. North of Ashburton Avenue, Palisade Avenue is a one-way northbound street with one-way traffic volumes that range from approximately 218 to 364 vph within the Traffic Study Area. The pavement along Palisade Avenue ranges from approximately 25 to 39 feet in width and was observed to be in fair to good condition.

C.1.m. Prospect Street

Prospect Street is a local, City-maintained roadway that generally traverses the Traffic Study Area in an east-west direction. Prospect Street provides one to two moving lanes in each direction with left and right turn lanes at its intersections with Riverdale Avenue and South Broadway. Prospect Street has two-way traffic volumes that range from approximately 314 to 1,501 vph within the Traffic Study Area. The pavement of Prospect Avenue ranges from approximately 47 to 62 feet in width was observed to be in fair to good condition.

C.1.n. Yonkers Avenue

Yonkers Avenue generally traverses the Traffic Study Area in an east-west direction and is maintained by NYSDOT. NYSDOT classifies Yonkers Avenue as a principal arterial roadway. Yonkers Avenue provides two moving lanes in each direction with left and right turn lanes at several intersections along its length. Two-way traffic volumes along Yonkers Avenue range from approximately 1,771 to 3,581 vph within the Traffic Study Area. The pavement along Yonkers Avenue ranges from approximately 49 to 73 feet in width and was observed to be in good condition.

C.2. INTERSECTION LEVEL OF SERVICE CONDITIONS

Traffic volumes for the peak hours under current (2020) conditions (the "2020 Existing Condition") are presented in **Figures 11-2a**, **11-2b**, **and 11-3**. Traffic operating conditions at each Traffic Study Area intersection were analyzed using the Synchro 10 Percentile delay (for signalized intersections) and the HCM (for unsignalized intersections) methodology, (see **Appendix L-2** for Synchro 10 outputs for all Traffic Study Area intersections) to compute delays, v/c ratios, and LOS as described in Section B of this chapter.

As shown in **Table 11-4** at the end of this chapter, of the 189 Traffic Study Area intersection lane groups/approaches analyzed for the weekday AM and PM peak hours and 97 Traffic Study Area intersection lane groups/approaches analyzed for the Saturday peak hour, 86 percent (162 lane groups), 87 percent (164 lane groups), and 93 percent (90 lane groups) were determined to be operating at LOS D or better under the 2020 Existing Condition during the weekday AM, weekday PM, and Saturday peak hours, respectively. LOS D operations during peak hours are generally considered to be acceptable operating conditions for signalized and unsignalized intersections. The following 25 intersections and lane groups were operating at LOS E and/or F in the existing condition:

• Riverdale Avenue and Prospect Street

- Northbound through/right turn movement Weekday AM peak hour: LOS E
- Southbound left turn movement Weekday AM peak hour: LOS E, Weekday PM peak hour: LOS F, Saturday peak hour: LOS E

- Broadway and Hudson Street
 - Eastbound left turn/right turn movement Weekday PM peak hour: LOS F
- South Broadway and Prospect Street/Nepperhan Avenue
 - Northbound through/right turn movement Weekday PM peak hour: LOS E
 - Southbound left turn movement Weekday PM peak hour: LOS F
- New Main Street and Nepperhan Avenue
 - Westbound left turn movement Weekday PM peak hour: LOS F
 - Northbound left turn/through/right turn movement Weekday AM peak hour: LOS E, Weekday PM peak hour: LOS F
- Waverly Street and Nepperhan Avenue
 - Northbound left turn/right turn movement Weekday AM peak hour: LOS F, Weekday PM peak hour: LOS F
- Nepperhan Avenue and Ashburton Avenue
 - Eastbound left turn movement Weekday AM peak hour: LOS E, Weekday PM peak hour: LOS F
 - Westbound left turn movement Weekday AM peak hour: LOS E, Weekday PM peak hour: LOS E
 - Westbound through/right turn movement Weekday AM peak hour: LOS E
 - Northbound left turn movement Weekday AM peak hour: LOS E, Weekday PM peak hour: LOS E
 - Southbound left turn movement Weekday AM peak hour: LOS F, Weekday PM peak hour: LOS F
- Nepperhan Avenue and Elm Street
 - Eastbound left turn movement Weekday PM peak hour: LOS E
 - Westbound left turn/through/right turn movement Weekday AM peak hour: LOS F
- Walnut Street and Yonkers Avenue
 - Northbound left turn/through/right turn movement Weekday AM peak hour: LOS F
 - Southbound left turn/through/right turn movement Weekday AM peak hour: LOS F
 - Intersection Weekday AM peak hour: LOS F
- Prescott Street and Yonkers Avenue
 - Northbound left turn/through/right turn movement Weekday AM peak hour: LOS E

- Ashburton Avenue and Yonkers Avenue
 - Northbound left turn/right turn movement Weekday AM peak hour: LOS E, Weekday PM peak hour: LOS E
 - Southbound left turn movement Weekday AM peak hour: LOS E, Weekday PM peak hour: LOS E
 - Southbound left turn/right turn movement Weekday PM peak hour: LOS E
- Yonkers Avenue and Midland Avenue (West)
 - Southbound left turn movement Weekday AM peak hour: LOS E
- Yonkers Avenue and Saw Mill River Parkway Southbound Ramps
 - Southbound right turn movement Weekday PM peak hour: LOS E
- Yonkers Avenue and Midland Avenue (East)
 - Eastbound left turn movement Weekday AM peak hour: LOS E

C.3. PARKING CONDITIONS

This section describes available on-street and off-street parking adjacent to the Project Sites.

C.3.a. Teutonia Site

Along Buena Vista Avenue, on-street parking is provided on the east side of the roadway only. At the corner of Buena Vista Avenue and Hudson Street, across the street from the Teutonia Site, is the City's Buena Vista Parking Garage, which provides 598 public parking spaces.

C.3.b. Chicken Island Site

Metered on-street parking is provided along New Main Street, Palisade Avenue, James Street, and Henry Herz Street while free on-street parking is provided on the west side of New School Street. Nepperhan Avenue, Ann Street, and John Street do not provide on-street parking. The City's Government Center Garage, located at the northwest corner of Nepperhan Avenue and New Main Street, provides 543 public parking spaces available to the public with access on both Nepperhan Avenue and New Main Street.

Within the Chicken Island Site, there are two privately owned surface parking lots. These lots are leased to the City of Yonkers:

- Engine Place Lot provides 77 surface lot parking spaces at the northeast corner of James Street and John Street with access on James Street. The Engine Place Lot is available for general public parking.
- Getty Square Lot provides 287 surface lot parking spaces at the northwest corner of New School Street and Nepperhan Avenue with access on Henry Herz Street. The Getty Square Lot is not available for general public parking.

C.3.c. North Broadway Site

On-street parking is provided along the west side of Locust Hill Avenue, Overlook Terrace, and Baldwin Place with metered on-street parking along North Broadway.

C.4. CITY OF YONKERS COMPLETE STREET POLICY

The City of Yonkers Complete Streets Policy (Section 103-129 of the Yonkers City Code) seeks to improve the City's interconnected network of transportation facilities. This policy incorporates active transportation into the planning, design, and operation of future City streets projects. Active transportation (walking, bicycling, and public transportation) improves public health, reduces traffic congestion, enhances air quality, and supports local economic development. Complete streets are streets that are planned, designed, operated, and maintained to enable safe access for all users, and upon which pedestrians, bicyclists, transit users, persons with disabilities, and motorists of all ages and abilities are able to safely move along and across.

Per the Complete Streets Policy, the incorporation of bicycle, pedestrian and transit facilities shall be mandated in all street construction, reconstruction, rehabilitation, and pavement maintenance projects undertaken by or on behalf of the City, unless the City Engineer determines exceptional conditions exist to not implement Complete Street design elements.

C.5. PEDESTRIAN AND BICYCLE CONDITIONS

This section describes pedestrian and non-automobile infrastructure within two blocks of the Project Sites.

C.5.a. Teutonia Site

Buena Vista Avenue provides sidewalks on both sides of the roadway. At present, the sidewalk along the frontage of the Teutonia Site is blocked by fencing installed for the remediation project undertaken by the Site's previous owner. Sidewalks are provided on both sides of Hudson Street, Prospect Street, and Hawthorne Avenue.

Along Prospect Street, crosswalks are provided on all approaches between Buena Vista Avenue and Riverdale Avenue except on the west leg of the Prospect Street/Hawthorne Avenue intersection. Along Hudson Street between Buena Vista Avenue and Riverdale Avenue, crosswalks are provided on the south intersection leg at Buena Vista Avenue, the north intersection leg at Hawthorne Avenue, and on each leg of the Riverdale Avenue intersection.

Within the two-block area of the Teutonia Site, there a mix of ADA and non-ADA compliant pedestrian ramps.

There are no bicycle facilities adjacent to the Teutonia Site.

C.5.b. Chicken Island Site

Adjacent to and within the Chicken Island Site, Nepperhan Avenue, New Main Street, New School Street, Palisade Avenue, Anne Street, John Street, James Street, and Henry Herz Street provide sidewalks on both sides of the roadway.

Pedestrian elements provided at intersections adjacent to the Chicken Island Site are summarized below:

- Nepperhan Avenue / New School Street crosswalks are provided on all intersection approaches however ADA pedestrian curb ramps are not provided.
- Nepperhan Avenue / New Main Street crosswalks are provided on all intersection approaches except for the west leg of the intersection. ADAcompliant pedestrian ramps are only provided on the northeast and northwest corners.
- Palisade Avenue / New Main Street / Getty Square crosswalks are provided on all intersection approaches. Pedestrian ramps are present but are in poor condition and do not provide tactile dome warning strips.
- Palisade Avenue / Locust Hill Avenue / James Street crosswalks are
 provided on the east and south legs of the intersections. Pedestrian ramps are
 present but are in poor condition and don't provide tactile dome warning
 strips.
- Palisade Avenue / New School Street / Elm Street -crosswalks are provided on all approaches except for the east leg of the intersection. ADA-compliant pedestrian curb ramps are provided on each corner of the intersection.
- John Street / New School Street a crosswalk is provided on the southern leg of the intersection as well as pedestrian crossing warning signage. However, there are no ADA-compliant pedestrian ramps.

There are no bicycle facilities adjacent to the proposed Chicken Island Site.

C.5.c. North Broadway Site

Sidewalks are provided on both sides of North Broadway, Locust Hill Avenue, Overlook Terrace, and Baldwin Place. At the North Broadway/Manor House Square intersection there are faded crosswalks with some corners providing pedestrian ramps without tactile dome warning strips. At the Overlook Terrace and Baldwin Place intersections on Locust Hill Avenue, there are no crosswalks or ADA pedestrian ramps provided.

There are no bicycle facilities adjacent to the proposed North Broadway Site.

C.6. PUBLIC TRANSPORTATION

Extensive public bus and rail service is offered in the Traffic Study Area. The Westchester County Bee-Line Bus System operates the following bus routes within the Traffic Study Area: Routes 1, 1C, 1T, 1W, 1X, 2, 3, 4, 5, 6, 7, 8, 9, 25, 30, 32, and 78 (see **Figure 11-4**). These bus routes offer direct service to several Westchester County municipalities and the Bronx. These bus routes also provide direct connections to other regional bus and rail services including New York City bus and subway lines, Hudson Link (Rockland County) bus lines, and CTtransit (Connecticut) bus lines. **Table 11-5** summarizes the frequency of each bus line, the presence of bus stops within two blocks of the Project Sites, and passenger amenities.

The Metropolitan Transportation Authority operates the BxM3 bus route, which has stops along South Broadway at Vark Street/Park Hill Avenue, Prospect Street/Nepperhan

Avenue, and New Main Street, within the Traffic Study Area and provides express bus service between Yonkers and Midtown Manhattan.

Table 11-5 Westchester County Bee Line Bus Service

			· · · · · · · · · · · · · · · · · · ·	ester county B	ee Line Bus Service
			us Stops Located w o Blocks of Project		
	Headways during				
Bus Route	Peak Periods ¹	Teutonia	Chicken Island	North Broadway	Amenities
1, 1C, 1T,	AM: 1 to 5 minutes				
1W, 1X	PM: 5 to 10 minutes	<u>i</u> _			
2	AM: 2 to15 minutes PM: 10 to 16 minutes		Х	Х	Bus Shelters provided at Getty Square
3	AM: 15 minutes PM: 10 to 30 minutes		Х	Х	Bus Shelters provided at Getty Square
4	AM: 12 to 14 minutes PM: 10 to 15 minutes				
5	AM: 5 to 20 minutes PM: 15 to 30 minutes		Х	Х	Bus Shelters provided at Getty Square
6	AM: 2 to 18 minutes PM: 15 minutes	Х		Х	
7	AM: 2 to 15 minutes PM: 15 minutes	Х	Х	Х	Bus Shelters provided at Getty Square
8	AM: 4 to 20 minutes PM: 15 to 20 minutes		Х	Х	
9	1 bus during AM 1 bus during PM	Х	Х		
25	AM: 20 minutes PM: 20 minutes	X		Х	
30	AM: 8 to 18 minutes PM: 20 minutes			X	
32	AM: 30 to 50 minutes PM: 45 minutes	X	Х	Х	Bus Shelters provided at Getty Square
78	AM: 35 minutes PM: 35 minutes		Х	X	Bus Shelters provided at Getty Square

The Metropolitan Transportation Authority's MNR offers commuter rail service to Yonkers via its Hudson Line with headways varying from 10 to 30 minutes during peak hours. The Yonkers MNR station is located at 5 Buena Vista Avenue at the western terminus of Dock Street, approximately 0.13 miles from the Teutonia Site. Amtrak intercity rail service is also provided at the Yonkers MNR station with Amtrak operating its Adirondack, Ethan Allen Express, Maple Leaf, and Empire Service routes with service to northern and western New York State, Montreal and Toronto, Canada.

D. CRASH HISTORY AND SAFETY ASSESSMENT

Crash data for all 38 Traffic Study Area intersections were obtained from NYSDOT for the three-year time period between February 1, 2017 and January 31, 2020 to determine crash trends that may be addressed with Complete Street designs. The data obtained quantify the total number of reportable crashes (involving fatality, injury, or more than \$1,000 in property damage), fatalities, and injuries during the three-year period, and a yearly breakdown of vehicular crashes with pedestrians and bicycles at each intersection.

During the three-year period, 517 total crashes, three fatalities, 460 injuries, 86 pedestrian crashes, and seven bicyclist crashes occurred at Traffic Study Area intersections.

Table 11-6 lists total crashes by Traffic Study Area intersection during the three-year period, and a breakdown of pedestrian and bicycle crashes by year and location.

Table 11-6 Crash Summary

Interse	ection	Vehicular Crashes Pedestrian and Bicycle								iiiiiiai y				
litterst	Cuon	All Cra	ashes b			63		Pedes	strian Cr			cle Cra		Pedestrian
North- South Roadway	East-West Roadway	2017– 2018	2018– 2019	2019– 2020	All Crashes – 12-Month Rolling Maximum	Total Fatalities	Total Injuries	2017– 2018	2018– 2019	2019– 2020	2017– 2018	2018– 2019	2019– 2020	and Bicycle Crashes – 12-Month Rolling Maximum ¹
Buena Vista Avenue	Main Street	9	0	1	9	0	10	1	0	1	0	0	0	1
Buena Vista Avenue	Hudson Street	0	1	0	1	0	0	0	0	0	0	0	0	0
Buena Vista Avenue	Prospect Street	2	1	4	4	0	3	1	1	2	0	0	0	3
Hawthorne Avenue	Prospect Street	0	2	3	4	0	3	0	1	0	0	0	0	1
Hawthorne Avenue	Main Street	3	2	1	3	0	4	0	0	0	0	0	0	0
Hawthorne Avenue	Hudson Street	2	1	0	2	0	3	1	0	0	0	0	0	1
Warburton Avenue	Ashburton Avenue	1	4	2	5	0	4	0	0	0	0	0	0	0
Warburton Avenue	Wells Avenue	2	2	2	4	0	3	0	0	0	0	0	0	0
Warburton Avenue	Dock Street	1	1	2	2	0	2	1	0	1	0	0	0	1
Warburton Avenue / Riverdale Avenue	Main Street	2	5	3	5	0	5	1	1	1	0	1	0	3
Riverdale Avenue	Hudson Street	1	1	3	3	0	4	0	0	1	0	0	0	1
Riverdale Avenue	Prospect Street	12	18	25	25	0	43	2	2	5	0	3	0	6
Riverdale Avenue	Vark Street	7	2	5	7	0	13	0	1	1	1	0	0	2
North Broadway	Ashburton Avenue	7	4	7	7	0	18	3	0	1	0	0	0	3
North Broadway	Manor House Square	0	3	1	3	0	2	0	0	0	0	0	0	0
North Broadway	Main Street	0	1	3	3	0	6	0	0	1	0	0	0	1
South Broadway	Hudson Street	5	1	0	6	0	7	3	0	0	0	0	0	3
South Broadway	Nepperhan Avenue	8	8	2	11	0	15	3	2	0	0	0	0	3
South Broadway	Vark Street	0	1	1	1	0	0	0	0	0	0	0	0	0
Locust Hill Avenue	Palisade Avenue	3	8	5	9	0	11	1	3	2	0	0	0	4
Locust Hill Avenue	Ashburton Avenue	0	1	1	1	0	1	0	0	0	0	0	0	0
New Main Street Palisade	Nepperhan Avenue	10	4	9	11	0	21	1	1	1	0	0	0	2
Avenue	Ashburton Avenue	2	2	1	4	0	3	0	0	0	0	0	0	0
Palisade Avenue	Lafayette Place	2	2	2	3	0	3	0	0	0	0	0	0	0
Palisade Avenue / New School Street	Elm Street	5	8	4	9	0	16	3	3	0	0	0	0	4

Table 11-6 (cont'd) Crash Summary

Interse	ection		Vehicular Crashes			Pedestrian and Bicycle Crashes								
		All Cra	ashes by	y Year	All			Pedes	strian Cı	ashes	Bicy	cle Cra	shes	Pedestrian
North- South Roadway	East-West Roadway	2017– 2018	2018– 2019	2019– 2020	Crashes – 12-Month Rolling Maximum	Total Fatalities	Total Injuries	2017– 2018	2018– 2019	2019– 2020	2017– 2018	2018– 2019	2019– 2020	and Bicycle Crashes – 12-Month Rolling Maximum ¹
New School Street	Nepperhan Avenue	5	9	5	11	0	19	0	2	0	1	0	0	2
Waverly Street	Nepperhan Avenue	1	0	0	1	0	1	0	0	0	0	0	0	0
Nepperhan Avenue	Avenue	10	9	10	13	1	29	3	2	2	0	0	0	3
Copcutt Lane	Nepperhan Avenue	1	0	1	1	0	1	0	0	0	1	0	0	1
Nepperhan Avenue	Elm Street	10	12	21	21	0	49	2	2	5	0	0	0	5
Walnut Street	Yonkers Avenue	10	17	17	18	0	43	0	4	3	0	0	0	4
Prescott Street	Yonkers Avenue	2	10	9	10	0	16	1	1	0	0	0	0	1
Ashburton Avenue	Yonkers Avenue	5	11	9	12	0	23	0	0	0	0	0	0	0
Saw Mill River Parkway SB Ramps	Yonkers Avenue	0	0	2	2	0	1	0	0	0	0	0	0	0
Saw Mill River Parkway NB Ramps	Yonkers Avenue	1	6	5	8	0	6	0	0	0	0	0	0	0
Fox Terrace / Wasylenko Lane	Yonkers Avenue	5	6	3	7	0	21	0	1	0	0	0	0	1
Yonkers Avenue	Midland Avenue (West)	4	8	6	9	0	11	0	0	0	0	0	0	0
Yonkers Avenue	Midland Avenue (East)	7	8	4	9	0	24	0	0	1	0	0	0	1
Oak Street	Yonkers Avenue	2	3	9	9	2	16	1	2	1	0	0	0	2

Note:

Bold, gray shaded intersections are high crash locations, defined as having ten or more crashes in a 12-month period.

Bold, blue striped intersections are high injury or fatality locations, defined as having 20 or more injuries during the three-year period or one or more crashes, excluding high crash locations.

Source: Crash data from NYSDOT for February 2017 through January 2020

D.1. INTERSECTION CRASHES

As shown in **Table 11-6**, a rolling total of crash data identifies the following nine locations having ten or more crashes in a 12-month period:

- Riverdale Avenue and Prospect Street
- South Broadway and Nepperhan Avenue
- New Main Street and Nepperhan Avenue
- New School Street and Nepperhan Avenue
- Nepperhan Avenue and Ashburton Avenue

¹ The 12-month rolling maximum is the maximum number of crashes occurring in any consecutive 12-month period in the three-year crash history period.

- Nepperhan Avenue and Elm Street
- Walnut Street and Yonkers Avenue
- Prescott Street and Yonkers Avenue
- Ashburton Avenue and Yonkers Avenue

Additionally, the following three locations have are identified as high injury or fatality locations (excluding high crash locations):

- Fox Terrace / Wasylenko Lane and Yonkers Avenue
- Midland Avenue (East) and Yonkers Avenue
- Oak Street and Yonkers Avenue

The high crash, high injury, and fatality locations are discussed in detail in this section, including a summary of crash types, severity, and trends that could be addressed with safety improvement measures.

D.1.a. Riverdale Avenue and Prospect Street

As shown in **Table 11-6**, during the three-year period, 55 crashes occurred at the Riverdale Avenue and Prospect Street intersection, resulting in 43 injuries including three serious injuries.

As shown in **Table 11-7**, the predominant crash type at the intersection is a rear end collision with pedestrian crashes secondary. In addition, dark-road lighted conditions (20 percent of the total crashes) and wet road surface conditions (25 percent of total crashes) were common contributing environmental conditions. Sixty-nine percent of the crashes at the intersection were attributed to driver error.

Table 11-7
Riverdale Avenue and Prospect Street Crash Types

· · · · · · · · · · · · · · · · · · ·					
Crash Type	Number	Percentage			
Rear End	12	22%			
Right Turn	7	13%			
Left Turn	8	15%			
Sideswipe	1	2%			
Right Angle	4	7%			
Overtaking	8	15%			
Fixed Object	0	0%			
Head On	0	0%			
Pedestrian	9	16%			
Bicycle	3	5%			
Other/Unknown	3	5%			
Total	55	-			
Source: NYSDOT, February 1, 2017 through January 31, 2020 crash data.					

D.1.a.i Potential Safety Improvements

- Install yellow retroreflective signal backplates to improve signal visibility
- Add leading pedestrian intervals for pedestrian crossings

- Install lane line extensions through the intersection to delineate space for left turning vehicles
- Improve roadway lighting at the intersection

D.1.b. South Broadway and Nepperhan Avenue

As shown in **Table 11-6**, during the three-year period, 18 crashes occurred at the South Broadway and Nepperhan Avenue intersection, resulting in 15 injuries including two serious injuries.

As shown in **Table 11-8**, the predominant crash type at the intersection is a pedestrian collision with rear end and left turn crashes secondary. In addition, dark-road lighted conditions (17 percent of the total crashes) and wet road surface conditions (17 percent of total crashes) were common contributing environmental conditions. Fifty percent of the crashes at the intersection were attributed to driver error.

Table 11-8 South Broadway and Nepperhan Avenue Crash Types

Crash Type	Number	Percentage
Rear End	4	22%
Right Turn	0	0%
Left Turn	4	22%
Sideswipe	0	0%
Right Angle	0	0%
Overtaking	3	17%
Fixed Object	0	0%
Head On	0	0%
Pedestrian	5	28%
Bicycle	0	0%
Other/Unknown	2	11%
Total	18	-
Source: NYSDOT, February 1, 2017	through January 31,	2020 crash data.

D.1.b.i Potential Safety Improvements

- Add leading pedestrian intervals for pedestrian crossings
- Install left turn flashing yellow arrow signals for permitted left turns with supplemental traffic signs with text "Left Turn Yield on Flashing Yellow Arrow"
- Install lane line extensions through the intersection to delineate space for left turning vehicles
- Install yellow retroreflective signal backplates to improve signal visibility
- Improve roadway lighting at the intersection

D.1.c. New Main Street and Nepperhan Avenue

As shown in **Table 11-6**, during the three-year period, 23 crashes occurred at the New Main Street and Nepperhan Avenue intersection, resulting in 21 injuries including one serious injury.

As shown in **Table 11-9**, the predominant crash type at the intersection is a rear end collision with overtaking, fixed object, and pedestrian crashes secondary. In addition, dark-road lighted conditions (26 percent of the total crashes) and wet road surface conditions (35 percent of total crashes) were common contributing environmental conditions. Seventy percent of the crashes at the intersection were attributed to driver error.

Table 11-9 New Main Street and Nepperhan Avenue Crash Types

1 (e) William Street und 1 (epperman 11) ende Stush 1 jpe						
Crash Type	Number	Percentage				
Rear End	7	30%				
Right Turn	2	9%				
Left Turn	2	9%				
Sideswipe	0	0%				
Right Angle	0	0%				
Overtaking	3	13%				
Fixed Object	3	13%				
Head On	0	0%				
Pedestrian	3	13%				
Bicycle	0	0%				
Other/Unknown	3	13%				
Total	23	-				
Source: NYSDOT, February 1, 2017 through January 31, 2020 crash data.						

D.1.c.i Potential Safety Improvements

- Install yellow retroreflective signal backplates to improve signal visibility
- Add a crosswalk at the Nepperhan Avenue west leg
- Add leading pedestrian intervals for pedestrian crossings
- Improve roadway lighting at the intersection

D.1.d. Nepperhan Avenue New School Street

As shown in **Table 11-6**, during the three-year period, 19 crashes occurred at the Nepperhan Avenue and New School Street intersection, resulting in 19 injuries including one serious injury.

As shown in **Table 11-10**, the predominant crash type at the intersection is a rear end collision with right turn and pedestrian crashes secondary. In addition, dark-road lighted conditions (26 percent of the total crashes) were common contributing environmental conditions. Sixty-three percent of the crashes at the intersection were attributed to driver error.

Table 11-10 Nepperhan Avenue and New School Street Crash Types

Crash Type	Number	Percentage			
Rear End	10	52%			
Right Turn	2	11%			
Left Turn	0	0%			
Sideswipe	0	0%			
Right Angle	0	0%			
Overtaking	1	5%			
Fixed Object	0	0%			
Head On	0	0%			
Pedestrian	2	11%			
Bicycle	1	5%			
Other/Unknown	3	16%			
Total	19	-			
Source: NYSDOT, February 1, 2017 through January 31, 2020 crash data.					

D.1.d.i Potential Safety Improvements

- Install yellow retroreflective signal backplates to improve signal visibility
- Add leading pedestrian intervals for pedestrian crossings
- Improve roadway lighting at the intersection

D.1.e. Nepperhan Avenue and Ashburton Avenue

As shown in **Table 11-6**, during the three-year period, 29 crashes occurred at the Nepperhan Avenue and Ashburton Avenue intersection, resulting in one fatality and 29 injuries including two serious injuries.

As shown in **Table 11-11**, the predominant crash types at the intersection are rear end, left turn, and pedestrian collisions. In addition, dark-road lighted conditions (34 percent of the total crashes) and wet road surface conditions (21 percent of total crashes) were common contributing environmental conditions. Seventy-six percent of the crashes at the intersection were attributed to driver error.

Table 11-11 Nepperhan Avenue and Ashburton Avenue Crash Types

Crash Type	Number	Percentage
Rear End	7	25%
Right Turn	1	3%
Left Turn	7	25%
Sideswipe	1	3%
Right Angle	2	7%
Overtaking	1	3%
Fixed Object	1	3%
Head On	1	3%
Pedestrian	7	25%
Bicycle	0	0%
Other/Unknown	1	3%
Total	29	-
Source: NYSDOT, February 1, 2017 through	h January 31, 2020 cr	ash data.

D.1.e.i Potential Safety Improvements

- Install yellow retroreflective signal backplates to improve signal visibility
- Install left turn flashing yellow arrow signals for permitted left turns with supplemental traffic signs with text "Left Turn Yield on Flashing Yellow Arrow"
- Install lane line extensions through the intersection to delineate space for left turning vehicles
- Add leading pedestrian intervals for pedestrian crossings
- Improve roadway lighting at the intersection

D.1.f. Nepperhan Avenue and Elm Street

As shown in **Table 11-6**, during the three-year period, 43 crashes occurred at the Nepperhan Avenue and Elm Street intersection, resulting in 49 injuries including five serious injuries.

As shown in **Table 11-12**, the predominant crash type at the intersection is a rear end collision with pedestrian and overtaking crashes secondary. In addition, darkroad lighted conditions (23 percent of the total crashes) and wet road surface conditions (35 percent of total crashes) were common contributing environmental conditions. Sixty-five percent of the crashes at the intersection were attributed to driver error.

Table 11-12 Nepperhan Avenue and Elm Street Crash Types

11							
Crash Type	Number	Percentage					
Rear End	18	42%					
Right Turn	1	2%					
Left Turn	2	5%					
Sideswipe	2	5%					
Right Angle	0	0%					
Overtaking	8	18%					
Fixed Object	1	2%					
Head On	0	0%					
Pedestrian	9	21%					
Bicycle	0	0%					
Other/Unknown	2	5%					
Total	43	-					
Source: NYSDOT, February 1, 2017 through January 31, 2020 crash data.							

D.1.f.i Potential Safety Improvements

- Install yellow retroreflective signal backplates to improve signal visibility
- Add leading pedestrian intervals for pedestrian crossings
- Extend the Nepperhan Avenue south leg median to provide a pedestrian refuge space
- Improve roadway lighting at the intersection

D.1.g. Yonkers Avenue and Walnut Street

As shown in **Table 11-6**, during the three-year period, 44 crashes occurred at the Yonkers Avenue and Walnut Street intersection, resulting in 43 injuries including three serious injuries.

As shown in **Table 11-13**, the predominant crash type at the intersection is a rear end collision with left turn crashes secondary. In addition, dark-road lighted conditions (34 percent of the total crashes) and wet road surface conditions (25 percent of total crashes) were common contributing environmental conditions. Seventy-five percent of the crashes at the intersection were attributed to driver error.

Table 11-13 Yonkers Avenue and Walnut Street Crash Types

Tonkers rivenue and wanted Street Grash Types			
Crash Type	Number	Percentage	
Rear End	13	30%	
Right Turn	3	7%	
Left Turn	10	23%	
Sideswipe	1	2%	
Right Angle	1	2%	
Overtaking	7	16%	
Fixed Object	0	0%	
Head On	0	0%	
Pedestrian	7	16%	
Bicycle	0	0%	
Other/Unknown	2	4%	
Total	44	-	
Source: NYSDOT, February 1, 2017 through January 31, 2020 crash data.			

D.1.g.i Potential Safety Improvements

- Install yellow retroreflective signal backplates to improve signal visibility
- Install left turn flashing yellow arrow signals for permitted left turns with supplemental traffic signs with text "Left Turn Yield on Flashing Yellow Arrow"
- Install lane line extensions through the intersection to delineate space for left turning vehicles
- Improve roadway lighting at the intersection

D.1.h. Yonkers Avenue and Prescott Street

As shown in **Table 11-6**, during the three-year period, 21 crashes occurred at the Yonkers Avenue and Prescott Street intersection, resulting in 16 injuries.

As shown in **Table 11-14**, the predominant crash type at the intersection is a rear end collision with overtaking crashes secondary. In addition, dark-road lighted conditions (19 percent of the total crashes) and wet road surface conditions (14 percent of total crashes) were common contributing environmental conditions. Eighty-one percent of the crashes at the intersection were attributed to driver error.

Table 11-14 Yonkers Avenue and Prescott Street Crash Types

Tonkers Avenue and Trescott Street Crash Types				
Crash Type	Number	Percentage		
Rear End	6	28%		
Right Turn	2	10%		
Left Turn	2	10%		
Sideswipe	2	10%		
Right Angle	2	10%		
Overtaking	5	23%		
Fixed Object	0	0%		
Head On	0	0%		
Pedestrian	2	10%		
Bicycle	0	0%		
Other/Unknown	0	0%		
Total	21	-		
Source: NYSDOT, February 1, 2017 through January 31, 2020 crash data.				

D.1.h.i Potential Safety Improvements

- Install yellow retroreflective signal backplates to improve signal visibility
- Move driveway at the northeast corner to Prescott Street and move westbound stop bar closer to the intersection
- Improve roadway lighting at the intersection

D.1.i. Yonkers Avenue and Ashburton Street

As shown in **Table 11-6**, during the three-year period, 25 crashes occurred at the Yonkers Avenue and Ashburton Street intersection, resulting in 23 injuries.

As shown in **Table 11-15**, the predominant crash type at the intersection is a rear end collision with left turn and overtaking crashes secondary. In addition, darkroad lighted conditions (52 percent of the total crashes) were common contributing environmental conditions. Seventy-six percent of the crashes at the intersection were attributed to driver error.

Table 11-15 Yonkers Avenue and Ashburton Street Crash Types

Tomicis ilvenue unu rismourton street erusii rypes				
Crash Type	Number	Percentage		
Rear End	14	56%		
Right Turn	0	0%		
Left Turn	4	16%		
Sideswipe	0	0%		
Right Angle	0	0%		
Overtaking	3	12%		
Fixed Object	0	0%		
Head On	1	4%		
Pedestrian	0	0%		
Bicycle	0	0%		
Other/Unknown	3	12%		
Total	25	-		
Source: NYSDOT, February 1, 2017 through January 31, 2020 crash data.				

D.1.i.i Potential Safety Improvements

- Install yellow retroreflective signal backplates to improve signal visibility
- Add striping or hardscaping at the intersection to restrict eastbound left turn and northbound through movements
- Consolidate driveways at the 220 Yonkers Avenue property to provide one driveway past the eastbound stop bar
- Improve roadway lighting at the intersection

D.1.j. Yonkers Avenue and Fox Terrace/Wasylenko Lane

As shown in **Table 11-6**, during the three-year period, 14 crashes occurred at the Yonkers Avenue and Fox Terrace/Wasylenko Lane intersection, resulting in 21 injuries including two serious injuries.

As shown in **Table 11-16**, the predominant crash type at the intersection is a rear end collision with fixed object crashes secondary. In addition, dark-road lighted conditions (57 percent of the total crashes) and wet road surface conditions (36 percent of total crashes) were common contributing environmental conditions. Ninety-three percent of the crashes at the intersection were attributed to driver error.

Table 11-16 Yonkers Avenue and Fox Terr/Wasylenko Ln Crash Types

Crash Type	Number	Percentage	
Rear End	8	57%	
Right Turn	0	0%	
Left Turn	0	0%	
Sideswipe	1	7%	
Right Angle	0	0%	
Overtaking	1	7%	
Fixed Object	2	15%	
Head On	1	7%	
Pedestrian	1	7%	
Bicycle	0	0%	
Other/Unknown	0	0%	
Total	14	-	
Source: NYSDOT, February 1, 2017 through January 31, 2020 crash data.			

D.1.j.i Potential Safety Improvements

- Install yellow retroreflective signal backplates to improve signal visibility
- Improve roadway lighting at the intersection

D.1.k. Yonkers Avenue and Midland Avenue (East)

As shown in **Table 11-6**, during the three-year period, 19 crashes occurred at the Yonkers Avenue and Midland Avenue (East) intersection, resulting in 24 injuries including one serious injury.

As shown in **Table 11-17**, the predominant crash type at the intersection is a left collision with rear end crashes secondary. In addition, dark-road lighted conditions (32 percent of the total crashes) and wet road surface conditions (32 percent of total crashes) were common contributing environmental conditions. Sixty-three percent of the crashes at the intersection were attributed to driver error.

Table 11-17 Yonkers Avenue and Midland Avenue (East) Crash Types

Crash Type	Number	Percentage		
Rear End	5	26%		
Right Turn	2	11%		
Left Turn	7	37%		
Sideswipe	0	0%		
Right Angle	1	5%		
Overtaking	0	0%		
Fixed Object	2	11%		
Head On	0	0%		
Pedestrian	1	5%		
Bicycle	0	0%		
Other/Unknown	1	5%		
Total	19	-		
Source: NYSDOT, February 1, 2017 through January 31, 2020 crash data.				

D.1.k.i Potential Safety Improvements

- Install lane line extensions through the intersection to delineate space for left turning vehicles
- Install left turn flashing yellow arrow signals for permitted left turns with supplemental traffic signs with text "Left Turn Yield on Flashing Yellow Arrow"
- Install yellow retroreflective signal backplates to improve signal visibility
- Improve roadway lighting at the intersection

D.1.l. Yonkers Avenue and Oak Street

As shown in **Table 11-6**, during the three-year period, 14 crashes occurred at the Yonkers Avenue and Oak Street intersection, resulting in two fatalities involving pedestrians crossing Yonkers Avenue and 16 injuries including one serious injury.

As shown in **Table 11-18**, the predominant crash types at the intersection are rear end and pedestrian collisions with overtaking crashes secondary. In addition, dark-road lighted conditions (14 percent of the total crashes) and wet road surface conditions (36 percent of total crashes) were common contributing environmental conditions. Fifty-seven percent of the crashes at the intersection were attributed to driver error.

Table 11-18 Yonkers Avenue and Oak Street Crash Types

Crash Type	Number	Percentage	
Rear End	4	29%	
Right Turn	0	0%	
Left Turn	2	14%	
Sideswipe	0	0%	
Right Angle	0	0%	
Overtaking	3	21%	
Fixed Object	0	0%	
Head On	0	0%	
Pedestrian	4	29%	
Bicycle	0	0%	
Other/Unknown	1	7%	
Total	14	-	
Source: NYSDOT, February 1, 2017 through January 31, 2020 crash data.			

D.1.l.i Potential Safety Improvements

- Install High-intensity Activated Crosswalk (HAWK) signal, also known as a pedestrian hybrid beacon, and curb extensions into the parking lane on both sides of Yonkers Avenue, to provide a protected pedestrian crossing
- Add a median barrier along a section Yonkers Avenue to discourage pedestrians from crossing midblock
- Add wayfinding signs to direct pedestrians to pedestrian bridge
- Improve roadway lighting at the intersection

E. NO BUILD CONDITION

The future without the Proposed Project, or "No Build" condition, is an interim scenario that establishes a future baseline condition that would be expected to occur in 2032 (the Build year) without the Proposed Project. No Build condition traffic volumes ("2032 No Build Volumes") were ascertained using the following methodology:

- Increase the 2020 Existing Condition traffic volumes by 0.5 percent per year from 2020 (existing year) to 2032 for background growth, resulting in an overall compounded growth rate of 6.17 percent; and
- Manually add trips from pending developments ("No Build projects") located in the vicinity of the Proposed Project Sites.

The No Build project list was developed in coordination with the City and is summarized in **Table 11-19**, identifying developments included in the background growth factor and developments with discrete trips added to the No Build condition traffic network. Discrete trips generated by these developments were pulled from published traffic studies prepared for the No Action projects and provided by the City of Yonkers.

Table 11-19 No Build Projects

Project Name	Units / Size	Туре	Location	Manually add trips	Include in Background Rate
Avalon Bay	606	Multifamily	79-91 Alexander Street	Х	
Extell	1,395	Multifamily	Alexander Street	X	
Altman Lighting/ Rose	440	Multifamily	57 Alexander Street	Х	
Ludlow (1)	214	Multifamily	70 Pier Street	Х	
Ludiow (1)	3,125 sf	Retail	50 Downing Street	^	
Ludlow (2)	178	Multifamily	150 Downing Street	Х	
Ludiow (2)	10,770 sf	Retail/Restaurant	ס	^	
Alma Realty	128	Multifamily	70 Jackson Street		X
St. Joseph's Housing	80	Supportive and affordable housing	School Street		Х
Hudson Regency	126	Multifamily	86 Buena Vista	Х	
Westhab	113	Affordable	Locust Hill Avenue		Х
Westhab II	63	Supportive and affordable housing	227 Elm Street		Х
Lionsgate Studios	110,000 sf	Studio and Backlot (60k sf and 50k sf)	iDark		Х
9-11 Riverdale	29	Multifamily	9-11 Riverdale Avenue		Х
Conifer	146	Multifamily	Ravine Avenue and Gold	Х	
320 Nepperhan		Self Storage 320 Nepperhan			Х
Riverdale Self Storage		Self Storage	Riverdale Avenue		Х
St. Denis School		Convert former parochial school to public school	McLean Avenue		х
Charter School for Educational Excellence	400	High School for 400 Students	Warburton and Lamartine		Х
222 Lake Avenue		Unknown	222 Lake Avenue		Х
44 Hudson Street	300	Multifamily	Multifamily 44 Hudson Street		Х
Adira Rehab / St. John Hospital		Zone Change Medical Office / Dialysis			Х
Chicken Island Brewery		Brew pub; restaurant	57 Alexander Street		X
The Plant	Unknown	Unknown	Glenwood Power Plant		Х
70 Ashburton Avenue	70	Multifamily	70 Ashburton Avenue		X
10 St. Casimir Avenue	68 beds	Men shelter/day prog			Х

Based on available information, there are no major roadway improvements scheduled through 2032 that would affect traffic patterns along the Traffic Study Area roadways.

E.1. INTERSECTION LEVEL OF SERVICE CONDITIONS

Traffic volumes for the No Build condition for the peak hours analyzed are shown in **Figures 11-5a, 11-5b, and 11-6**. **Table 11-20** (at the end of this chapter) compares 2020 Existing and 2032 No Build LOS Conditions for the Traffic Study Area intersections for the Weekday AM, Weekday PM, and Saturday peak hours. Synchro 10 outputs for the No Build condition are provided in **Appendix L-3**.

Under the No Build condition, the following notable changes in LOS would occur at the Traffic Study Area intersections:

- Buena Vista Avenue and Main Street
 - Southbound left turn/through/right turn movement Weekday PM peak hour: deteriorate from LOS C to LOS E
- Buena Vista Avenue and Hudson Street
 - Southbound left turn/through movement Weekday PM peak hour: deteriorate from LOS C to LOS F

- Buena Vista Avenue and Prospect Street
 - Southbound left turn/through movement Weekday AM peak hour: deteriorate from LOS C to LOS F, Weekday PM peak hour: deteriorate from LOS D to LOS F
- Nepperhan Street and Warburton Avenue and Dock Street/Manor House Square
 - Southbound left turn/through/right turn movement Weekday PM peak hour: deteriorate from LOS C to LOS E
- Riverdale Avenue and Prospect Street
 - Westbound left turn movement Weekday AM peak hour: deteriorate from LOS
 C to LOS F, Weekday PM peak hour: deteriorate from LOS D to LOS F
 - Northbound left turn movement Weekday PM peak hour: deteriorate from LOS D to LOS E
 - Northbound through/right turn movement Weekday AM peak hour: deteriorate from LOS E to LOS F, Weekday PM peak hour: deteriorate from LOS D to LOS F
 - Southbound left turn movement Weekday AM peak hour: deteriorate from LOS
 E to LOS F, Saturday peak hour: deteriorate from LOS E to LOS F
 - Intersection Weekday AM peak hour: deteriorate from LOS D to LOS E,
 Weekday PM peak hour: deteriorate from LOS D to LOS E
- Riverdale Avenue and Vark Street
 - Eastbound left turn/through/right turn movement Weekday PM peak hour: deteriorate from LOS D to LOS E
- Broadway and Hudson Street
 - Northbound through movement Weekday PM peak hour: deteriorate from LOS D to LOS E
- South Broadway and Prospect Street/Nepperhan Avenue
 - Eastbound through/right turn movement Weekday AM peak hour: deteriorate from LOS D to LOS F, Weekday PM peak hour: deteriorate from LOS D to LOS F
 - Westbound left turn movement Weekday PM peak hour: deteriorate from LOS C to LOS F
 - Intersection Weekday PM peak hour: deteriorate from LOS D to LOS E
- New Main Street and Nepperhan Avenue
 - Westbound left turn movement Saturday peak hour: deteriorate from LOS D to LOS E
 - Northbound left turn/through/right turn movement Weekday AM peak hour: deteriorate from LOS E to LOS F
- Nepperhan Avenue and Ashburton Avenue
 - Eastbound through movement Weekday AM peak hour: deteriorate from LOS
 C to LOS E, Weekday PM peak hour: deteriorate from LOS C to LOS E
 - Westbound through/right turn movement Weekday AM peak hour: deteriorate from LOS E to LOS F, Weekday PM peak hour: deteriorate from LOS D to LOS F

- Intersection Weekday AM peak hour: deteriorate from LOS D to LOS E,
 Weekday PM peak hour: deteriorate from LOS D to LOS F
- Nepperhan Avenue and Elm Street
 - Northbound through/right turn movement –Weekday PM peak hour: deteriorate from LOS D to LOS E
 - Intersection Weekday AM peak hour: deteriorate from LOS D to LOS E
- Ashburton Avenue and Yonkers Avenue
 - Southbound left turn movement Weekday AM peak hour: deteriorate from LOS
 E to LOS F, Weekday PM peak hour: deteriorate from LOS E to LOS F
 - Southbound left turn/right turn movement Weekday AM peak hour: deteriorate from LOS D to LOS E, Weekday PM peak hour: deteriorate from LOS E to LOS F
- Yonkers Avenue and Midland Avenue (West)
 - Southbound left turn movement Weekday AM peak hour: deteriorate from LOS
 E to LOS F, Weekday PM peak hour: deteriorate from LOS D to LOS F
- Yonkers Avenue and Saw Mill River Parkway Southbound Ramps
 - Southbound right turn movement Weekday AM peak hour: deteriorate from LOS C to LOS E, Weekday PM peak hour: deteriorate from LOS E to LOS F
- Yonkers Avenue and Midland Avenue (East)
 - Southbound through/right turn movement Weekday AM peak hour: deteriorate from LOS D to LOS E, Weekday PM peak hour: deteriorate from LOS D to LOS E
- Hawthorne Avenue and Prospect Street
 - Westbound left turn movement Weekday PM peak hour: deteriorate from LOS C to LOS F

E.2. PARKING CONDITIONS

Off-street parking facilities are proposed for most of the No Build projects shown in **Table 11-19** and therefore, no significant changes to parking conditions within the Traffic Study Area are expected in the No Build condition.

E.3. PEDESTRIAN AND BICYCLE CONDITIONS

The future pedestrian and bicycle conditions would be similar to the existing conditions with the exception of the proposed Yonkers Greenway. The Yonkers Greenway would provide a 2.4-mile long north-south on-street and off-street greenway along South Broadway between the Yonkers MNR Station and the City border with the Bronx. Within the Downtown area, bicycle facilities that are being considered are bicycle lanes and a cycle track. The Greenway is being implemented by the City and Groundworks Hudson Valley and funded through a combination of grant money from NYSDOT, public funds from the City, and private donations.

E.4. PUBLIC TRANSPORTATION

No significant changes in public transportation conditions are expected under the No Build condition. While an increase in public transit ridership is expected with the No Build

projects, it is the policy of the transit agencies (MNR and Westchester County) to adjust their operating schedules to reflect demand as needed.

F. BUILD CONDITION

F.1. PROJECT DESCRIPTION

The Proposed Project would consist of multiple mixed-use buildings across three Project Sites within a half-mile (5- to 10-minute walk) of the Yonkers MNR Station. A description of the proposed development at each of the Project Sites is presented below.

F.1.a. Teutonia Project

The Teutonia Project would include approximately 906 multi-family residential units, approximately 10,000 gsf of street level retail, and approximately 956 parking spaces. The Teutonia Project would include two residential towers on top of a mixed-use podium. Approximately 956 parking spaces would be constructed in a combination of above- and below-grade structures. The parking garage would be accessed from two driveways along Buena Vista Avenue. The Teutonia Project's northern driveway would be signalized as part of the Buena Vista Avenue/Hudson Street intersection and coordinated with the traffic signal at Buena Vista Avenue and Main Street. The City would need access to the property for maintaining signal equipment (i.e., traffic lights, detectors, etc.).

F.1.b. Chicken Island Project

The Chicken Island Project would include approximately 2,000 multi-family residential units, approximately 70,000 gsf of street-level retail, 17,000 gsf of office space, and approximately 2,200 parking spaces. The Chicken Island Site is bounded by New Main Street, Palisade Avenue, New School Street, and Nepperhan Avenue. There are multiple garages accessed via New School Street or via internal roadways.

F.1.c. North Broadway Project

The North Broadway Project includes approximately 650 residential units, approximately 15,000 gsf of retail space, approximately 13,000 gsf of office space and approximately 676 parking spaces. The parking garage would be accessed from Overlook Terrace and would have approximately 676 parking spaces. The Build condition traffic analysis utilizes the previous, larger land use program for the North Broadway Project with 650 residential units, 17,000 gsf of retail space, and 21,000 gsf of office space, which results in a conservative trip generation estimate and traffic analysis results.

F.2. PROJECT TRIP GENERATION

Table 11-21 summarizes the land use program at each of the Project Sites.

To project the number of trips that would be generated by the Proposed Project, data from the *Institute of Transportation Engineers (ITE) Trip Generation Manual, 10th Edition* were examined and, where appropriate, adjusted to reflect the urban environment, mixeduse development, pedestrian and bicycle activity, and the proximity of the Project Sites to transit.

Table 11-21 Development Program

Project Site	Residential (units)	Retail (ksf)	Office (ksf)
Teutonia	906	10	0
Chicken Island	2,000	70	17
North Broadway ¹	650	17	21
TOTAL	3,556	102	38

Note: ¹ The previous, larger land use program is utilized for the North Broadway site, which results in a conservative trip generation estimate. The updated North Broadway Project includes 650 residential units, 15,000 gsf of retail space, and 13,000 gsf of office space.

Trip reductions to capture proximity to transit (25 percent reduction) and pedestrian/bicycle activity (five percent reduction) were developed using 2012–2016 5-year American Community Survey Census Data for residents and workers in the Traffic Study Area and in the City.

Trip reductions to capture internal trips between complimentary land uses were calculated for the North Broadway and Chicken Island Projects using the *ITE Trip Generation Handbook (3rd Edition)* and *National Cooperative Highway Research Program (NCHRP) Report 684: Enhancing Internal Trip Capture Estimation for Mixed-Use Developments* methodology. The trip reduction to capture internalized trips ranged from a three to six percent reduction during the Weekday AM peak hour and from 15 to 17 percent reduction during the Weekday PM and Saturday peak hours. A summary of the trip generation and reductions are shown in **Table 11-22**, at the end of this chapter. Back-up information for the trip generation estimates is presented in **Appendix L-4**.

The Proposed Project would result in 883, 1,199, and 1,178 total vehicle trips during the Weekday AM, Weekday PM, and Saturday peak hours, respectively.

F.3. PROJECT TRIP DISTRIBUTION AND ASSESSMENT

Figures 11-7, 11-8, and 11-9 present the assumed trip distribution percentages for the Chicken Island Site, Teutonia Site, and North Broadway Site, respectively. Figures 11-10, 11-11, 11-12, and 11-13 present the trip assignments for each Project Site and a cumulative total of the three Project Sites. Trip assignments were based on trip assignments prepared for previous studies conducted in the area and existing travel patterns.

F.4. INTERSECTION LEVEL OF SERVICE CONDITIONS

Project generated vehicle trips were added to the No Build condition traffic volumes to estimate the Build condition traffic volumes. **Figures 11-14 and 11-15** show the Build condition traffic volumes for the Weekday AM, Weekday PM, and Saturday peak hours. **Table 11-23**, at the end of this chapter, compares the No Build condition and Build LOS conditions for the Traffic Study Area intersections. Synchro 10 outputs for the Build condition are provided in **Appendix L-5**.

Typically, for robust, downtown high-density urban environments, LOS D conditions are expected and represent an active downtown area where there is a lot of activity but vehicles aren't traveling at high speeds. For this analysis, traffic impacts are: (1) a change in LOS D or better to LOS E or F; (2) a change from LOS E to LOS F; or (3) an increase

of 10 percent or greater in traffic volumes for LOS F. The impact criteria are applied to the approach/lane group LOS for signalized intersections and approach/movement group LOS for unsignalized intersections.

Under the Build condition, absent additional improvements beyond those specified above (i.e., new streets within Chicken Island and signalization of Teutonia's northern driveway), the Proposed Project would result in impacts at the following locations, as shown in **Table 11-23**:

- Buena Vista Avenue and Prospect Street
 - Westbound right turn movement Weekday PM peak hour
 - Northbound through/right turn movement Weekday PM peak hour
 - Southbound left turn/through movement Weekday AM peak hour, Weekday PM peak hour
- Nepperhan Street and Warburton Avenue and Dock Street/Manor House Square
 - Southbound left turn/through/right turn movement Weekday AM peak hour,
 Weekday PM peak hour
 - Intersection Weekday PM peak hour
- Riverdale Avenue/Warburton Avenue and Main Street
 - Eastbound left turn/through/right turn movement Weekday PM peak hour
- Riverdale Avenue and Hudson Street
 - Eastbound left turn/through/right turn movement Weekday PM peak hour
- Riverdale Avenue and Prospect Street
 - Eastbound left turn/through/right turn movement Weekday PM peak hour
 - Westbound left turn movement Weekday AM peak hour, Weekday PM peak hour
 - Westbound through movement Weekday PM peak hour
 - Intersection Weekday AM peak hour, Weekday PM peak hour
- Broadway and Hudson Street
 - Eastbound left turn/right turn movement Weekday PM peak hour
- South Broadway and Prospect Street/Nepperhan Avenue
 - Eastbound through/right turn movement Weekday AM peak hour, Saturday peak hour
 - Westbound left turn movement Weekday AM peak hour, Weekday PM peak hour
 - Northbound through/right turn movement Weekday PM peak hour
 - Southbound left turn movement Weekday AM peak hour, Weekday PM peak hour
 - Intersection Weekday PM peak hour
- South Broadway and Vark Street/Park Hill Avenue

- Northbound left turn/through/right turn movement Weekday PM peak hour
- Intersection Weekday PM peak hour
- New Main Street and Nepperhan Avenue
 - Westbound left turn movement Saturday peak hour
 - Northbound left turn/through/right turn movement Saturday peak hour
- Waverly Street and Nepperhan Avenue
 - Northbound left turn/right turn movement Weekday AM peak hour, Weekday PM peak hour, Saturday peak hour
- Nepperhan Avenue and Ashburton Avenue
 - Eastbound left turn movement Weekday AM peak hour, Weekday PM peak hour
- Nepperhan Avenue and Elm Street
 - Eastbound left turn movement Weekday AM peak hour, Weekday PM peak hour, Saturday peak hour
 - Northbound through/right turn movement Weekday PM peak hour
 - Intersection Weekday PM peak hour
- Walnut Street and Yonkers Avenue
 - Eastbound left turn movement Weekday PM peak hour
- Yonkers Avenue and Saw Mill Northbound Ramps
 - Eastbound left turn movement Weekday AM peak hour, Weekday PM peak hour
 - Westbound through movement Weekday PM peak hour
- Yonkers Avenue and Midland Avenue (West)
 - Southbound left turn movement Weekday AM peak hour
- Yonkers Avenue and Saw Mill Southbound Ramps
 - Southbound right turn movement Weekday AM peak hour, Weekday PM peak hour
- Yonkers Avenue and Cross County Parkway On-Ramp/Midland Avenue (East)
 - Southbound through/right turn movement Weekday AM Peak hour
 - Intersection Weekday AM Peak hour
- Hawthorne Avenue and Prospect Street
 - Westbound left turn movement Weekday PM peak hour
- Locust Hill Avenue and Ashburton Avenue
 - Northbound left turn/right turn movement Weekday PM peak hour

F.5. PARKING CONDITIONS

F.5.a. Proposed Amendments to Parking Requirements and Parking Supply

The Applicant proposes the following amendments to the parking requirements of the Zoning Ordinance:

• Amend the parking ratio for "apartments" to 1 per dwelling unit within 0.5 miles of a train station and 1 per dwelling unit plus 0.33 per bedroom beyond 0.5 mile (from one per unit within 0.25 mile of a train station and 1 per unit plus 0.33 per bedroom beyond 0.25 mile)

It is noted that the Teutonia and North Broadway Sites are within ¼ mile of the Yonkers Train Station and thus are currently subject to the 1 space per unit apartment parking requirement. The proposed Zoning Amendments would allow attended or valet parking to satisfy the minimum off-street parking requirements.

The Project's proposed residential parking rates are similar to those applied in urban environments in Westchester County. **Table 11-24** compares the Project's proposed parking requirements with those in the City of New Rochelle and the City of White Plains, both of which include developments within a ½-mile of a MNR train station.

Table 11-24 Parking Requirement Comparison

		<u> </u>	
Land Use	Proposed Amended Rates	City of New Rochelle ¹	City of White Plains ¹
Residential	1 space/unit ²	1 space/unit	1 space/unit
Retail / Commercial	1 space per 300 sf	1 space per 400 sf	1.65 spaces per 500 sf
Office	1 space per 500 sf	1 space per 500 sf	1 space per 500 sf

Notes:

sf = square feet

1 Within the Central Parking Area. Allows payments in lieu of providing off-street parking

² Expanded from ¼-mile to ½-mile of train station

The proposed one parking space per dwelling unit is also supported by recent parking trends at similar developments located near train stations in downtown environments. A residential parking demand study was conducted in November 2021 which compared the number of residential units to the overnight parking demand and residential parking permits issues at four locations:

- Sawyer Place (Yonkers, NY)
- Hudson Park South (Yonkers, NY)
- Hudson Park North (Yonkers, NY)
- 360 Huguenot Street (New Rochelle, NY)

The study indicated that the existing parking demand to residential ratio is 0.94 parking spaces per dwelling unit, supporting the proposed one parking space per dwelling unit parking rate. **Appendix L-6** provides the detailed parking study which includes the surveyed locations and national trends.

Tables 11-25, 11-26, and 11-27 present the parking that would be required and provided within the proposed amended rates for each of the three Project Sites. As shown, the parking supply, provided with a combination of self-park and valet

parking operations, would exceed the proposed requirement at each Project Site with the exception of Chicken Island, which would have a parking deficiency of approximately 87 parking spaces once fully built. This parking shortfall would be offset by allowing shared parking between complimentary land uses (i.e., a parked residential vehicle leaving during the morning will free a parking space for an incoming retail or office vehicle). The parking study provided in **Appendix L-6** provides a shared parking analysis for Chicken Island.

Table 11-25 Chicken Island Site Parking Requirements

Dhasa	Land Use	C:		Annual Darking Sumply Drawided
Phase		Size	Parking Spaces Required	Approx. Parking Supply Provided
	Residential	650 units	650	
Phase 1	Retail	39 ksf	130	
i ilase i	Office	17ksf	34	
		Total	814	903
	Residential	425 units	425	
Phase 2	Retail	10 ksf	33	
Pilase 2	Office	0 ksf	0	
		Total	458	403
	Residential	425 units	425	
Phase 3	Retail	6 ksf	20	
Pilase 3	Office	0 ksf	0	
		Total	445	604
	Residential	250 units	250	
Phase 4	Retail	10 ksf	33	
Pilase 4	Office	0 ksf	0	
		Total	284	-
	Residential	250 units	250	
Dhasa F	Retail	5 ksf	17	
Phase 5	Office	0 ksf	0	
		Total	267	270
	Residential	2,000 units	2,000	
Total	Retail	70 ksf	233	
iotai	Office	17 ksf	34	
		Total	2,267	2,180

Table 11-26
Teutonia Site Parking Requirements

Phase	Land Use	Size	Parking Spaces Required	Approx. Parking Supply Provided
	Residential	510 units	510	
Phase 1	Retail	5 ksf	17	
		Total	527	556
	Residential	396 units	396	
Phase 2	Retail	5 ksf	17	
		Total	413	400
	Residential	906 units	906	
Total	Retail	10 ksf	34	
		Total	940	956

Table 11-27
North Broadway Site Parking Requirements

			·	2100 1 W1 1111 5 110 4 111 0 1 1 0 1
Phase	Land Use	Size	Parking Spaces Required	Approx. Parking Supply Provided
	Residential	300 units	300	
Phase 1	Retail	15 ksf	50	
Phase i	Office	13 ksf	26	
		Total	376	380
	Residential	350 units	350	
Phase 2	Retail	2 ksf	7	
Priase 2	Office	8 ksf	16	
		Total	373	370
	Residential	650 units	650	
Total	Retail	17 ksf	57	
iolai	Office	21 ksf	42	
		Total	749	750

F.5.b. Changes to Existing Parking Supply

While all three Project Sites would provide off-street parking, existing off-street and/or on street parking facilities would be affected by the Chicken Island Project and North Broadway Project.

F.5.b.i Chicken Island Site

The 18 on-street parking spaces on Henry Herz Street would be removed; however, on-street parking facilities would be provided on the internal roadways.

In addition, the two existing, privately owned off-street surface lots would be eliminated. This includes the 287-parking space Getty Square lot and the 77-parking space Engine Place lot. It should be noted that the 2020 City of Yonkers Parking Needs Assessment identifies the City's preferred method of accommodating this displaced parking as construction of a new parking structure on the Cacace Justice Center site and relocation of City staff parking from the Government Center garage to the new garage, which would allow for the displaced Chicken Island Site parking to be accommodated in the Government Center garage (see **Appendix H-4**).

F.5.b.ii North Broadway Site

On-street parking along Overlook Terrace would be removed to facilitate vehicle access to the North Broadway Project parking garage. On-street parking would be removed on Baldwin Place to facilitate loading dock operations.

F.6. PEDESTRIAN AND BICYCLE CONDITIONS

This section summarizes improvements to the pedestrian and bicycle environment included as part of the Proposed Project.

F.6.a. Teutonia Site

The Teutonia Project would provide a sidewalk along the project frontage, addressing the existing sidewalk gap that exists along Buena Vista Avenue between Hudson Street and Prospect Street.

In addition, with the signalization of the Buena Vista/Hudson Street/Teutonia Project northern driveway, this intersection would be upgraded to provide ADA-compliant pedestrian ramps and new crosswalks.

F.6.b. Chicken Island Site

The Chicken Island Project would provide sidewalks adjacent to the property frontages along Nepperhan Avenue, James Street, Palisade Avenue, and New School Street. Sidewalks would also be provided on both sides of the internal roadways (Centre Street and John Street). Crosswalks internal to the Site would be located at the Centre Street/John Street intersection.

The pedestrian curb ramps and crosswalks would be upgraded to provide ADA compliant pedestrian ramps, including tactile dome warning strips, and restriped crosswalks at the Nepperhan Avenue/New School Street, Palisade Avenue/James Street, Palisade Avenue/Centre Street, and John Street/New School Street intersections.

F.6.c. North Broadway Site

A new pedestrian stairway would be constructed to provide a direct pedestrian connection between North Broadway and Overlook Terrace/ Locust Hill. ADA access between North Broadway and Overlook Terrace would be provided via an elevator located within the North Broadway Site buildings adjacent to the stairway.

Sidewalks along Overlook Terrace would be reconstructed, and new ADA-compliant pedestrian ramps and crosswalks would be constructed at the Locust Hill Avenue/Overlook Terrace intersection.

Bike parking would be accommodated at each Project Site within buildings and/or parking garages.

F.7. PUBLIC TRANSPORTATION

F.7.a. MNR

It is anticipated that the Proposed Project would increase Metro-North Commuter Railroad ridership by 145 passengers during each of the weekday AM and weekday PM peak hours (see **Appendix L-7**). As such, significant adverse impacts to MNR service are not anticipated.

F.7.b. Westchester County Bee Line Buses

An increase in the Westchester County Bee Line bus ridership is expected with the Proposed Project. However, transit ridership would be distributed among several Traffic Study Area bus routes and therefore a dramatic increase in ridership at any one bus line is not anticipated. Therefore, a significant adverse impact to Bee Line bus service is not anticipated.

F.7.c. Project Site Shuttle

While the Chicken Island Site is within walking distance (½ mile) of the Yonkers MNR Station, it is anticipated that a shuttle would be provided between the Chicken Island Site and the Station as an amenity to residents and visitors of the Chicken Island Site. It should be noted any trip reduction credits associated with this shuttle were not applied in the traffic analysis.

F.8. INTERNAL TRAFFIC CIRCULATION

F.8.a. Teutonia Site

Vehicular traffic would access the Teutonia Project via two driveways located on Buena Vista Avenue between Hudson Street and Prospect Street. The southern driveway would provide access to the at-grade and podium level parking levels and the northern driveway would provide access to the subsurface parking levels. The garage is anticipated to be self-parking but may include attended service based on the final garage configuration.

Two loading docks are provided in the at-grade parking level accessed via the proposed southern driveway.

F.8.b. Chicken Island Site

The Chicken Island Site would be accessed via New School Street, Ann Street, and Palisade Avenue. The Chicken Island Site would include two internal roadways: 1) Centre Street that would provide north-south circulation and both on-street parking and dedicated drop-off zones and 2) John Street that would provide east-west circulation. There would be one internal intersection at Centre Street and John Street.

Three parking garages would be provided on the Chicken Island Site:

- A garage accessed via John Street between Centre Street and New School Street is anticipated to be fully attended.
- In phase 2/3 of the Chicken Island Project, a garage accessed via New School Street between Nepperhan Avenue and John Street, anticipated to be a combination of self-parking and valet parking.
- In phase 5 of the Chicken Island Project, a garage accessed via John Street east of New School Street, anticipated to be fully automated.

A valet drop off-area would be provided at the Ann Street/James Street/Centre Street intersection.

There would be three loading docks provided at the Chicken Island Site:

- Palisade Avenue between Centre Street and New School Street
- New School Street between Nepperhan Avenue and the phase 2/3 garage driveway
- John Street between New School Street and the phase 5 garage driveway

F.8.c. North Broadway Site

The North Broadway Site parking garage would be accessed via Overlook Terrace and would be fully attended. The loading dock would be located and accessed via Baldwin Place at Bell Place.

F.9. SIGHT DISTANCE

Sight distances at the proposed garage entrances and Project Site access points would be acceptable and meet the standards of the American Associations of State and Highway Transportations Officials as illustrated in **Appendix L-8**.

G. TRAFFIC MITIGATION

For the impacted Traffic Study Area intersections and lane groups identified in **Table 11-1**, mitigation measures, such as signal installation or retiming and roadway restriping, were examined as potential improvements the City of Yonkers can implement as a means to improve traffic operating conditions and mitigate the potential impacts of the Proposed Project to return to No Build condition without exceeding the impact criteria thresholds. It should be noted the City has previously determined that projects within the downtown zoning districts will pay a mitigation fee, based on the number of projected development trips, and the actual implementation of mitigation measures will be the responsibility of the City.

A discussion of the proposed mitigation measures is provided in this section.

G.1. MITIGATION MEASURES

Table 11-28 summarizes proposed mitigation measures that address the potential impacts of the Proposed Project. With the implementation of these mitigation measures, which are subject to review and approval by the City and/or NYSDOT, Project-related significant adverse traffic impacts would be fully mitigated with the following exceptions:

• Locust Hill Avenue and Ashburton Avenue

During the Weekday PM peak hour, the northbound approach would not warrant a signal. While currently not proposed as a mitigation, the northbound left-turn could be prohibited and vehicles would be rerouted to Palisade Avenue via Lafayette, which would allow left-turning vehicles to turn onto Ashburton at a signalized intersection.

Table 11-28 Proposed Mitigation Measures

Intersection		Weekday AM		Weekday PM		Saturday
Buena Vista / Prospect Street	SBLT	Signalize intersection Add NBR 50-foot pocket Add SBL 150-foot pocket	WBR NBTR SBLT	Signalize intersection Add NBR 50-foot pocket Add SBL 150-foot pocket	SBLT	Signalize intersection Add NBR 50-foot pocket Add SBL 150-foot pocket
Nepperhan Street / Warburton Avenue / Dock Street / Manor House Square		Adjust cycle length to 90s Reduce Phase 3 by 2s Increase Phase 1 by 2s	SBLTR	Adjust cycle length to 90s Reduce Phase 3 by 2s Increase Phase 1 by 2s	Not Impacted	N/A
Riverdale Avenue / Warburton Avenue / Main Street	Not I	N/A	EBLTR	Reduce Phase 2/5 by 2s Increase Phase 4 by 2s	Not Impacted	N/A
Riverdale Avenue / Hudson Street	Not Impacted	N/A	EBLTR	Reduce Phase 1/5 by 2s Increase Phase 3 by 2s	Not Impacted	N/A

Table 11-28 (cont'd) Proposed Mitigation Measures

Intersection		Weekday AM		Weekday PM	poseu	Saturday Saturday
intersection	Add NBR 250-foot pocket Relocate EB bus stop to far side			Add NBR 250-foot pocket		Saturday
Riverdale Avenue / Prospect Street	WBL		EBLTR WBL WBT	Relocate EB bus stop to far side Relocate WB bus stop to far side Add NBR overlap phase Reduce Phase 1/2 by 4s Reduce Phase 5 by 3s Reduce Phase 6 by 5s Reduce Phase 7 by 4s Increase Phase 3 by 8s Increase Phase 8 by 4s	Not Impacted	N/A
Broadway / Hudson Street	Not Impacted	N/A	EBLR	Add EBR 75-foot pocket Relocate EB bus stop to Broadway/Main	Not Impacted	N/A
South Broadway / Prospect Street / Nepperhan Avenue	EBTR WBL SBL	Add NBR 200-foot pocket Relocate NB bus stop to far side Reduce Phase 3 by 3s Reduce Phase 5 by 3s Increase Phase 1 by 1s Increase Phase 2 by 2s Increase Phase 6 by 6s Adjust signal offset to 16s	WBL NBTR SBL	Add NBR 200-foot pocket Relocate NB bus stop to far side Reduce Phase 1 by 1s Reduce Phase 3 by 2s Reduce Phase 5 by 3s Increase Phase 2 by 3s Increase Phase 6 by 5s Adjust signal offset to 18s	EBTR	Add NBR 200-foot pocket Relocate NB bus stop to far side Reduce Phase 2 by 5s Increase Phase 1 by 5s
South Broadway / Vark Street / Park Hill Avenue	Not Impacted	N/A	NBLTR	Add NBR 50-foot pocket		
New Main Street / Nepperhan Avenue	Not Impacted	N/A	Not Impacted	N/A	WBL NBLTR	Add NBR 200-foot pocket Add NBR overlap phase Reduce Phase 2/6 by 2s Increase Phase 1/5 by 2s
Waverly Street / Nepperhan Avenue	NBLR	Signalize intersection	NBLR	Signalize intersection	NBLR	Signalize intersection
Nepperhan Avenue / Ashburton Avenue	EBL	Allow EBL permitted turns Allow WBL permitted turns	EBL	Allow EBL permitted turns Allow WBL permitted turns		
Nepperhan Avenue / Elm Street	EBL	Add WBR 50-foot pocket Add WBR overlap phase Reduce Phase 1 by 4s Increase Phase 2 by 4s	EBL NBTR	Add WBR 50-foot pocket Add WBR overlap phase Reduce Phase 1 by 4s Increase Phase 2 by 4s	EBL	Add WBR 50-foot pocket Add WBR overlap phase Reduce Phase 1 by 4s Increase Phase 2 by 4s
Walnut Street / Yonkers Avenue	Not Impacted	N/A	EBL	Reduce Phase 2/6 by 3s Increase Phase 1/5 by 3s		
Yonkers Avenue / Saw Mill Northbound Ramps	EBL	Reduce Phase 2 by 4s Increase Phase 1 by 4s Adjust signal offset to 101s	EBL WBT	Reduce Phase 2 by 8s Reduce Phase 3 by 4s Increase Phase 1 by 12s Increase Phase 6 by 4s Adjust signal offset to 101s		
Yonkers Avenue / Midland Avenue (West)	SBL	Reduce Phase 2 by 3s Increase Phase 1 by 3s	Not Impacted	N/A		
Yonkers Avenue / Saw Mill Southbound Ramps	SBR	Signalize intersection	SBR	Signalize intersection		
Yonkers Avenue / Cross County Parkway On-Ramp / Midland Avenue (East)	SBTR	Reduce Phase 1 by 5s Increase Phase 2 by 5s	Not Impacted	N/A		
Hawthorne Avenue / Prospect Street Locust Hill Avenue	Not Impacted	N/A	WBL	Signalize intersection	Not Impacted	N/A
/ Ashburton Avenue	Not Impacted	N/A	NBLR	Unmitigated		

Notes: L = Left Turn, T = Through, R = Right Turn, EB = Eastbound, WB = Westbound, NB = Northbound, SB = Southbound, N/A = Not Applicable, s = seconds

G.2. LEVEL OF SERVICE CONDITIONS

Table 11-29, at the end of this chapter, presents a comparison of the No Build condition, Build condition, and Build condition with mitigation for the impacted locations. Synchro 10 outputs for the Build condition with mitigation condition are provided in **Appendix L-9**.

A majority of the impacted intersections would operate at an overall level of service D or better with the proposed mitigations; however, while mitigated to the No Build condition, the following intersections would operate at an overall level of service E:

- Riverdale Avenue and Prospect Street
- Nepperhan Avenue and Ashburton Avenue

H. TRAFFIC MITIGATION PHASING

In order to mitigate impacts of the Proposed Project prior to various phases of development being completed, the traffic mitigation measures listed in **Table 11-28** would be implemented by phase based on the project-generated vehicle trips in each phase of the Teutonia, Chicken Island, and North Broadway Projects listed in Chapter 15, "Construction" (see **Table 15-1**).

Trip generation for the development of each Project Site was conducted for all five anticipated phases and added to the No Build traffic condition traffic volumes grown to the construction phase year to estimate the Construction Phases 1 through 5 traffic volumes. Synchro 10 outputs for Construction Phases 1 through 5 are provided in **Appendix L-9.**

Using the impact criteria identified above (see Section F.4), the traffic mitigation measures would be implemented during the phase in which the project-generated vehicle trips and analysis results would exceed the impact criteria. Because impacts at the same intersection movement would be triggered during different phases for each analyzed time period, the mitigation measures at an intersection would be implemented at the earliest phase in which the impact criteria are exceeded. The traffic mitigation measures and phasing are presented in **Table 11-30**.

Table 11-30 Traffic Mitigation Phasing

Intersection	Weekday AM	Weekday PM	Saturday	Implementation Phase
Buena Vista / Prospect Street	Signalize intersection Add NBR 50-foot pocket Add SBL 150-foot pocket	Signalize intersection Add NBR 50-foot pocket Add SBL 150-foot pocket	Signalize intersection Add NBR 50-foot pocket Add SBL 150-foot pocket	3
Nepperhan Street / Warburton Avenue / Dock Street / Manor House Square		Adjust cycle length to 90s Reduce Phase 3 by 2s Increase Phase 1 by 2s	N/A	2
Riverdale Avenue / Warburton Avenue / Main Street		Reduce Phase 2/5 by 2s Increase Phase 4 by 2s	N/A	4
Riverdale Avenue / Hudson Street	N/A	Reduce Phase 1/5 by 2s Increase Phase 3 by 2s	N/A	2
Riverdale Avenue / Prospect Street	Add NBR 250-foot pocket Relocate EB bus stop to far side Relocate WB bus stop to far side Add NBR overlap phase Reduce Phase 1/2 by 3s Reduce Phase 5 by 1s Reduce Phase 6 by 5s Reduce Phase 7 by 2s Increase Phase 3 by 6s Increase Phase 8 by 8s	Add NBR 250-foot pocket Relocate EB bus stop to far side Relocate WB bus stop to far side Add NBR overlap phase Reduce Phase 1/2 by 4s Reduce Phase 5 by 3s Reduce Phase 6 by 5s Reduce Phase 7 by 4s Increase Phase 3 by 8s Increase Phase 8 by 4s	N/A	3

Table 11-30 (cont'd) Traffic Mitigation Phasing

				igation Phasing
Intersection	Weekday AM	Weekday PM	Saturday	Implementation Phase
Broadway / Hudson Street	N/A	Add EBR 75-foot pocket Relocate EB bus stop to Broadway/Main	N/A	2
South Broadway / Prospect Street / Nepperhan Avenue	Add NBR 200-foot pocket Relocate NB bus stop to far side Reduce Phase 3 by 3s Reduce Phase 5 by 3s Increase Phase 1 by 1s Increase Phase 2 by 2s Increase Phase 6 by 6s Adjust signal offset to 16s	Add NBR 200-foot pocket Relocate NB bus stop to far side Reduce Phase 1 by 1s Reduce Phase 3 by 2s Reduce Phase 5 by 3s Increase Phase 2 by 3s Increase Phase 6 by 5s Adjust signal offset to 18s	Add NBR 200-foot pocket Relocate NB bus stop to far side Reduce Phase 2 by 5s Increase Phase 1 by 5s	1
South Broadway / Vark Street / Park Hill Avenue	N/A	Add NBR 50-foot pocket		3
New Main Street / Nepperhan Avenue	N/A	N/A	Add NBR 200-foot pocket Add NBR overlap phase Reduce Phase 2/6 by 2s Increase Phase 1/5 by 2s	2
Waverly Street / Nepperhan Avenue	Signalize intersection	Signalize intersection	Signalize intersection	1
Nepperhan Avenue / Ashburton Avenue	Allow EBL permitted turns Allow WBL permitted turns	Allow EBL permitted turns Allow WBL permitted turns		2
Nepperhan Avenue / Elm Street	Add WBR 50-foot pocket Add WBR overlap phase Reduce Phase 1 by 4s Increase Phase 2 by 4s	Add WBR 50-foot pocket Add WBR overlap phase Reduce Phase 1 by 4s Increase Phase 2 by 4s	Add WBR 50-foot pocket Add WBR overlap phase Reduce Phase 1 by 4s Increase Phase 2 by 4s	1
Walnut Street / Yonkers Avenue	N/A	Reduce Phase 2/6 by 3s Increase Phase 1/5 by 3s		3
Yonkers Avenue / Saw Mill Northbound Ramps	Reduce Phase 2 by 4s Increase Phase 1 by 4s Adjust signal offset to 101s	Reduce Phase 2 by 8s Reduce Phase 3 by 4s Increase Phase 1 by 12s Increase Phase 6 by 4s Adjust signal offset to 101s		3
Yonkers Avenue / Midland Avenue (West)	Reduce Phase 2 by 3s Increase Phase 1 by 3s	N/A		3
Yonkers Avenue / Saw Mill Southbound Ramps	Signalize intersection	Signalize intersection		3
Yonkers Avenue / Cross County Parkway On-Ramp / Midland Avenue (East)	Reduce Phase 1 by 5s Increase Phase 2 by 5s	N/A		4
Hawthorne Avenue / Prospect Street	N/A	Signalize intersection	N/A	3

H.1. POST CONSTRUCTION MONITORING

The intersection analysis and associated mitigation measures are based on vehicle trip estimates anticipated to be generated by the Proposed Project. In order to ensure sufficient mitigation measures are identified and implemented by the City of Yonkers, post construction traffic monitoring will be conducted by the Applicant to determine the adequacy of the mitigation measures and to determine if the City should consider implementing additional strategies.

Following full occupancy of each construction phase, Weekday AM, Weekday PM, and Saturday peak period driveway counts will be collected at each of the Project Site

driveways. Prior to data collection, a data collection protocol will be submitted to the City for approval.

Following each data collection period, a memorandum will be submitted to the City presenting a comparison of the driveway counts to the trip generation estimates presented in this study. If the driveway peak hour counts exceed the trip generation estimates, the City may request that the Applicant perform additional traffic analyses at the impacted study intersections to determine if additional improvements should be implemented by the City using the traffic mitigation funds previously remitted by the Applicant. Any future analysis will be coordinated and approved by the City and could include collecting intersection peak hour traffic turning movement counts and conducting peak hour intersection operations analyses to identify additional improvements.

Table 11-4 2020 Existing Conditions Level of Service Analysis

			Weekday A	AM			Weekday	PM		020 Laisting Co	Saturda		
Approach		Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS
			1		enue & M	ain Street (Signali	zed)			•		•	
Main Street	Eastbound	LTR	0.30	13.8	В	LTR	0.57	18.3	В	LTR	0.16	12.3	В
Main Street	Westbound	LTR	0.33	14.7	В	LTR	0.55	18.8	В	LTR	0.21	12.9	В
Buena Vista Avenue	Northbound	LTR	0.58	20.5	С	LTR	0.44	18.0	В	LTR	0.29	15.3	В
Bueria vista Averiue	Southbound	LTR	0.45	17.5	В	LTR	0.76	27.2	С	LTR	0.13	12.3	В
		Interse	ction	17.2 B		Interse	ction	21.4	С	Intersed	ction	13.6	В
			2: B	uena Vista Avenu	ie & Huds	son Street (Unsign	alized)						
Hudson Street	Westbound	LR	0.09	9.2	Α	LR	0.10	9.7	Α	LR	0.09	8.2	Α
Buena Vista Avenue	Northbound	TR	0.45	11.7	В	TR	0.34	10.5	В	TR	0.20	8.5	Α
Buella Vista Aveilue	Southbound	LT	0.54	12.8	В	LT	0.79	22.7	С	LT	0.19	8.4	Α
			3: Bı	uena Vista Avenu	e & Pros	pect Street (Unsign	nalized)						
Prospect Street	Westbound	L	0.04	9.9	Α		0.20	11.9	В	L	0.04	9.0	Α
Flospect Street		R	0.36	11.6	В	R	0.24	10.8	В	R	0.14	8.3	Α
Buena Vista Avenue	Northbound	TR	0.42	11.7	В	TR	0.35	11.3	В	TR	0.16	8.2	Α
Bueria vista Averiue	Southbound	LT	0.60	16.4	С	LT	0.81	27.8	D	LT	0.26	9.3	Α
			4:	Hawthorne Aven	ue & Mai	n Street (Unsignal	ized)						
Main Street	Eastbound	TR	-	-	-	TR	-	-	-	TR	-	•	-
Main Sueet	Westbound	LT	0.02	7.6	Α	LT	-	0.0	Α	LT	0.03	7.5	Α
Hawthorne Avenue	Northbound	LR	0.07	10.0	В	LR	0.19	13.9	В	LR	0.06	9.9	Α
Market Place	Southbound	LR	0.02	9.8	Α	LR	0.04	11.3	В	LR	0.02	9.8	Α
			5: H	lawthorne Avenu	e & Huds	on Street (Unsigna	alized)						
Hudson Street	Eastbound	LT	-	-	-	LT	-	-	-	LT	-	-	-
Hawthorne Avenue	Northbound	LTR	0.17	10.2	В	LTR	0.30	11.7	В	LTR	0.15	9.7	Α
nawinome Avenue	Southbound	LR	0.07	9.7	Α	LR	0.13	11.4	В	LR	0.05	9.1	Α
			6: W	arburton Avenue	& Ashbu	ırton Avenue (Sign	nalized)						
	Eastbound	LTR	0.20	15.3	В	LTR	0.25	21.7	С				
Ashburton Avenue	Westbound	L	0.11	21.1	С	Ш	0.10	24.2	С				
	Westbould	TR	0.23	16.5	В	TR	0.21	20.2	С				
	Northbound	LTR	0.71	38.9	D	LTR	0.80	40.8	D				
Warburton Avenue	Southbound	L	0.30	17.2	В	L	0.28	14.2	В				
	Southbourid	TR	0.30	17.3	В	TR	0.29	14.7	В				
		Interse		23.9	С	Interse		26.4	С				
			7:	: Warburton Aven	ue & We	lls Avenue (Signali	zed)						
	Eastbound	LTR	0.06	0.5	Α	LTR	0.34	11.3	В	LTR	0.01	0.0	А
Wells Avenue	Westbound	L	0.67	32.4	С	L	0.74	35.4	D	L	0.61	25.9	С
		TR	0.35	24.6	С	TR	0.16	21.8	С	TR	0.10	17.8	В
Warburton Avenue	Northbound	Т	0.42	17.9	В	T	0.29	15.8	В	T	0.24	12.0	В
vvaibuitori Averiue	Southbound	TR	0.53	19.4	В	TR	0.48	18.6	В	TR	0.27	11.7	В
		Interse	ction	23.1	С	Interse	ction	23.5	С	Intersed	ction	17.5	В

Table 11-4 (cont'd) 2020 Existing Conditions Level of Service Analysis

									2	2020 Existing Conditions Level of Service Analysi					
Approach			Weekday .				Weekday				Saturda				
Арргоасп		Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS		
		8: Nep	perhan Street 8		ue & Do	ck Street/Manor He	ouse Square (Si								
Nepperhan Street	Northeastbound	LR	0.20	8.2	Α	LR	0.36	14.9	В	LR	0.06	0.3	Α		
	Northbound	L	0.32	18.8	В	L	0.53	25.8	С	L	0.13	14.5	В		
Warburton Avenue		TR	0.73	24.3	С	TR	0.52	15.7	В	TR	0.37	14.0	В		
	Southbound	LTR	0.78	26.9	С	LTR	0.81	32.7	С	LTR	0.63	21.2	С		
		Intersec		24.4	С	Interse		26.2	С	Intersed	ction	18.2	В		
					1	enue & Main Street		1			+				
	Eastbound	LTR	0.36	12.2	В	LTR	0.81	40.3	D	LTR	0.29	9.8	A		
Main Street	Westbound	L	0.25	28.3	С	L	0.74	45.8	D	L	0.64	38.3	D		
	Westbound	TR	0.34	18.1	В	TR	0.60	27.7	С	TR	0.33	19.5	В		
Riverdale Avenue	Northbound	L	0.08	10.7	В	L	0.10	11.8	В	L	0.02	10.0	Α		
Niverdale Avende	Northboaria	Т	0.24	11.1	В	Т	0.12	10.5	В	T	0.10	10.1	В		
Warburton Avenue	Southbound	TR	0.57	27.7	С	TR	0.66	30.6	С	TR	0.49	26.3	С		
		Intersec	tion	19.9	В	Interse	ection	30.7	С	Intersed	ction	23.7	С		
	_		1(0: Riverdale Aven	ue & Hud	Ison Street (Signa		-							
Hudson Street	Eastbound	LTR	0.54	36.6	D	LTR	0.83	52.7	D	LTR	0.46	42.8	D		
	Northbound	TR	0.41	19.1	В	TR	0.39	12.7	В	TR	0.23	5.8	Α		
Riverdale Avenue	Couthbound	L	0.27	10.1	В	L	0.49	18.8	В	L	0.28	5.5	Α		
	Southbound	Т	0.32	9.7	Α	Т	0.34	10.3	В	T	0.21	3.6	Α		
	·	Intersec	tion	16.6	В	Interse	ection	19.9	В	Intersed	ction	7.4	Α		
			11	: Riverdale Avenu	ie & Pros	pect Street (Signa	alized)								
	Eastbound	LTR	0.36	29.2	С	LTR	0.48	31.9	С	LTR	0.14	27.2	С		
Dog on a st Otro at		L	0.88	31.1	С	L	0.90	35.7	D	L	0.34	5.1	Α		
Prospect Street	Westbound	Т	0.50	9.2	Α	Т	0.71	13.2	В	T	0.28	4.3	Α		
		R	0.22	1.2	Α	R	0.18	0.7	Α	R	0.16	0.5	Α		
	N. a.i.	L	0.38	42.8	D	L	0.41	42.2	D	L	0.13	28.9	С		
	Northbound	TR	1.15dr	73.2	Е	TR	0.91dr	49.3	D	TR	0.61	36.8	D		
Riverdale Avenue		L	0.67	60.0	Е	L	0.90	127.2	F	L	0.81	59.4	Е		
	Southbound	Т	0.45	33.0	С	Т	0.46	33.1	С	T	0.26	30.1	С		
		R	0.07	0.3	Α	R	0.08	0.3	Α	R	0.04	0.1	Α		
		Intersec	tion	39.8	D	Interse	ection	39.1	D	Intersed	ction	27.3	С		
				12: Riverdale Ave	nue & Va	rk Street (Signaliz		•				•			
	Eastbound	LTR	0.40	27.6	С	LTR	0.64	51.7	D						
Vark Street	Westbound	LTR	0.15	18.7	В	LTR	0.48	40.6	D						
		L	0.12	22.5	С	L	0.09	11.7	В						
	Northbound	TR	0.80	40.8	D	TR	0.42	19.1	В						
Riverdale Avenue		L	0.59	45.2	D	L	0.42	18.6	В						
	Southbound	TR	0.64	27.8	C	TR	0.44	14.6	В						
		Intersec		33.3	C	Interse		21.6	С		1				
						rton Avenue (Sign									
	Eastbound	LTR	0.37	24.4	C	LTR	0.46	21.3	С						
Ashburton Avenue	Westbound	LTR	0.49	15.9	В	LTR	0.52	14.5	В						
	Northbound	LTR	0.68	39.2	D	LTR	0.69	39.3	D						
North Broadway	Totalboard		0.50	23.6	C	1	0.50	23.1	C						
Holai Bioddway	Southbound	TR	0.37	20.0	В	TR	0.25	17.8	В						
	Coddisound	Intersec		24.9	C	Interse		23.8	C		ļ.				
		I Intersect			_	ouse Square (Unsi		20.0							
Manor House Square	Eastbound	ı	0.37	14.7	В	Juse oquale (Olisi	0.39	14.6	В	I	0.16	11.4	В		
North Broadway	Northbound	- -	-	-	-	<u> </u>	-	-		T	- 0.10		-		
Not in Dioauway	riortibouriu	<u> </u>	_	_	_	l l				ı	<u> </u>	<u>-</u>	-		

Table 11-4 (cont'd) 2020 Existing Conditions Level of Service Analysis

									2	020 Existing C			Analysis
Approach			Weekday				Weekday				Saturda		
Арргоасп		Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS
			15a: North	n Broadway & Mair	n Street (Getty Square West	t) (Signalized)						
Main Street	Westbound	TR	0.29	0.6	Α	TR	0.49	2.1	Α	TR	0.42	2.7	Α
North Broadway	Northbound	LT	0.43	20.6	С	LT	0.47	22.0	С	LT	0.34	18.1	В
•		Interse		11.3	В	Interse		10.6	В	Interse	ction	8.6	Α
			15b: Palisad	e Avenue & New N	lain Stre	et (Getty Square E	ast) (Signalized)					
New Main Street	Westbound	TR	0.53	25.0	С	TR	0.69	29.5	С	TR	0.54	25.2	С
Palisade Avenue	Northbound	LT	0.21	22.9	С	LT	0.61	30.7	С	LT	0.37	25.3	С
	·	Interse	ction	24.5	С	Interse	ction	30.0	С	Interse	ction	25.2	С
			16: South	Broadway/North B	roadway	& Hudson Street ((Unsignalized)						
Hudson Street	Eastbound	LR	0.49	12.2	В	LR	0.98	54.7	F	LR	0.55	13.4	В
South Broadway	Northbound	Т	0.39	11.1	В	Т	0.82	30.6	D	Т	0.43	11.8	В
•			17: South E	Broadway & Prosp	ect Stree	t/Nepperhan Aven	ue (Signalized)		•				•
D 101 1		L	0.16	19.5	В	L	0.18	22.0	С	L	0.17	17.4	В
Prospect Street	Eastbound	TR	0.79	36.4	D	TR	0.81	47.8	D	TR	0.60	23.6	С
		L	0.65	16.0	В	L	0.81	32.6	С	L	0.15	5.9	Α
Nepperhan Avenue	Westbound	T	0.57	9.4	Α	Т	0.64	12.0	В	Т	0.36	6.0	Α
**		R	0.15	1.5	Α	R	0.34	1.7	Α	R	0.10	0.3	Α
		L	0.29	31.4	С	L	0.25	30.2	C	L	0.07	27.6	C
	Northbound	TR	0.71	43.2	D	TR	0.88	57.1	E	TR	0.44	33.8	C
South Broadway		1	0.30	33.8	C	1	1.29	205.8	F	1	0.29	32.4	C
	Southbound	TR	0.21	24.5	C	TR	0.25	19.3	В	TR	0.14	25.5	C
		Interse		24.2	C	Interse		41.1	D	Interse		18.6	В
		interes				/Park Hill Avenue (1110100	0.1011	10.0	
Vark Street	Eastbound	LTR	0.69	40.7	D	LTR	0.69	42.0	D				
Park Hill Avenue	Westbound	LTR	0.71	43.2	D	LTR	0.69	44.3	D				
	Northbound	LTR	0.63	26.4	C	LTR	0.88	41.6	D				
South Broadway	Southbound	LTR	0.78	33.6	C	LTR	0.74	31.8	C				
	Codtribodila	Interse		34.9	C	Interse		39.3	D				
		Interse				R Palisade Avenue		00.0					
Locust Hill Avenue	Eastbound	LTR	0.32	9.2	A	LTR	0.60	13.3	В	LTR	0.35	9.4	Α
Locust I IIII Avenue	Northbound	TR	0.08	7.9	A	TR	0.10	8.5	A	TR	0.08	7.8	A
Palisade Avenue	Southbound	LT	0.08	8.6	A	IT.	0.10	9.0	A	LT	0.10	8.3	A
	Southbound	LI				l ∟ ∟ ∟ rhan Avenue (Sign		9.0	А	LI	0.10	0.3	A
		T 1	0.03	4.6	_	man Avenue (Sign		4.7	Λ .	1	0.05	1.8	Ι Λ
	Eastbound	TR		10.2	A B	TR	0.04 0.69	13.4	A B	TR	0.05	6.1	A
Nannarhan Avanua		IK	0.49 0.51	27.9		IR	1.06	106.5	F	IR	0.41	38.9	A
Nepperhan Avenue	\A/a ath a coad	<u> </u>			C	<u> </u>			•	<u>L</u>			D
	Westbound	R	0.59 0.44	9.1 10.2	A	R	0.60 0.42	12.0 12.8	В	R	0.32 0.42	8.5	A
Now Main Street	Northbound	LTR	1.00	77.7	В	LTR		88.2	B	LTR	0.42	10.4 48.0	В
New Main Street	Northbound				E		1.04		•				D
		Interse		22.4	C	Interse		33.1	С	Interse	Cuon	18.7	В
		1 .			_	rton Avenue (Sign							
	F-645	L +	0.17	4.8	A	<u> </u>	0.11	4.1	A				
A = - A	Eastbound		0.42	16.7	В		0.52	17.7	В				
Ashburton Avenue		R	0.14	4.4	A	R	0.15	4.8	A				
	Westbound	L	0.31	4.6	A	L	0.44	4.7	A				
		TR	0.56	8.7	A	TR	0.48	6.8	A				
Palisade Avenue	Northbound	LT	0.67	43.4	D	LT	0.56	39.8	D				
	112111122	R	0.26	8.7	A	R	0.22	8.0	A				
		Interse	ction	15.3	В	Interse	ction	13.8	В				

Table 11-4 (cont'd) 2020 Existing Conditions Level of Service Analysis

			Weekday A	A N/I			Weekday	DM		020 Existing Co			
Approach		Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	Saturda v/c Ratio	Delay (sec)	LOS
		Lane Group				ade Avenue (Sign		Delay (Sec)	LUS	Lane Group	V/C Ratio	Delay (Sec)	LUS
Palisade Avenue	Eastbound	LTR	0.35	17.2	B	LTR	0.59	21.6	С	LTR	0.40	17.7	В
New School Street	Northbound	TR	0.34	12.5	В	TR	0.39	10.5	В	TR	0.40	8.2	A
Palisade Avenue	Southbound	LT	0.50	16.9	В	LT	0.49	16.0	В	LT	0.18	12.4	В
Fallsade Aveilde	Southbound	Intersec		15.4	В	Interse		17.0	В	Interse		13.6	В
		IIILEISEC				& Nepperhan Aven			Ь	IIILEISE	CHOTI	13.0	В
		1	0.38	26.5	C		0.27	21.8	С	ı	0.25	9.2	Α
	Eastbound	TR	0.57	16.3	В	TR	0.74	12.9	В	TR	0.45	13.9	B
Nepperhan Avenue		111	0.14	11.1	В	1	0.34	15.9	В	1	0.14	11.0	В
Neppeman Avenue	Westbound	<u> </u>	0.74	30.9	C		0.75	31.0	C	<u>_</u>	0.52	26.1	C
	VVCStbourid	R	0.29	3.9	A	R	0.20	4.2	A	R	0.17	4.5	A
New School Street	Southbound	LTR	0.24	24.9	C	LTR	0.52	27.6	C	LTR	0.22	11.0	В
New School Street	Oddinbound	Intersec		23.1	C	Interse		21.8	C	Interse		18.6	В
		Intersec				an Avenue (Unsign		21.0	U	Interse	Stion	10.0	
	Eastbound	TR		-	-	TR	-	_	_	TR	_	_	
Nepperhan Avenue		111	-	0.0	A	111	0.01	21.4	C	I	0.00	12.8	В
Neppernan / Wende	Westbound	<u> </u>	_	-	-	T	-	Z 1. T	-	T	-	12.0	-
Waverly Street	Northbound	LR	0.89	76.1	F	LR	1.17	181.0	F	LR	0.44	22.1	С
waveny easer	Hortinodila	LIT			& Δshh	urton Avenue (Sig		101.0	•	LIX	0.11	<i>LL</i> .1	
			0.54	67.9	E	l l	0.60	82.6	F				
	Eastbound		0.74	30.2	C	T	0.78	27.9	С				
Ashburton Avenue	Lastbourid	R	0.07	1.3	A	R	0.05	0.4	A				
/ toribartori / tvorido		17	0.54	57.9	E		0.75	73.1	E				
	Westbound	TR	0.89	55.1	E	TR	0.87	51.2	D				
		1	0.57	59.6	E		0.51	55.9	E				
	Northbound	TR	0.62	31.4	C	TR	0.44	30.7	C				
Nepperhan Avenue		1	1.32	241.7	F	1	0.86	88.7	F				
. торрошан, топас	Southbound	T	0.32	29.5	C	T	0.33	29.8	C				
		R	0.14	4.2	A	R	0.13	3.8	A				
		Intersec		48.0	D	Interse		41.7	D		·I		
				: Nepperhan Aver	nue & Co	pcutt Lane (Signal	ized)	ı				·I	
	Eastbound	Т	0.58	26.5	С	T	0.37	23.2	С				
Nepperhan Avenue	Westbound	Т	0.35	23.1	С	Т	0.39	23.2	C				
Copcutt Lane	Southbound	LR	0.32	27.5	C	LR	0.31	22.8	С				
		Intersec		25.3	C	Interse		23.2	C		·I		
					_	Im Street (Signaliz							
			0.72	47.0	D	l	0.82	57.4	Е	I	0.59	35.8	D
Elm Street	Eastbound	TR	0.28	27.9	C	TR	0.25	27.1	C	TR	0.14	19.4	В
	Westbound	LR	1.23	150.9	F	LR	0.54	24.3	C	LR	0.38	15.5	В
	Northbound	TR	0.75	32.5	С	TR	0.87	37.7	D	TR	0.60	29.5	C
Nepperhan Avenue		L	0.39	17.0	В	L	0.41	18.1	В	L	0.17	12.2	В
	Southbound	T	0.48	14.8	В	T	0.53	15.1	В	Ť	0.39	13.9	В
	l	<u>'</u>	ction	43.4	D	Interse		28.3	C	Interse		21.8	C

Table 11-4 (cont'd) 2020 Existing Conditions Level of Service Analysis

			Weekday A	Λ M			Weekday	DM	- 1	020 Existing Co	Saturday		11141 9 515
Approach		Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS
		Lane Group				s Avenue (Signaliz		Delay (Sec)	LUS	Lane Group	V/C Ratio	Delay (Sec)	LUS
	1	Ι ι				S Avenue (Signaliz		22.2					
	Eastbound	L TD	0.31	11.2	В	L	0.67	23.3	C				
Yonkers Avenue		TR	0.43	17.7	В	TR	0.71	24.5	С				
	Westbound	L_	0.22	18.2	В	L	0.56	26.4	С				
		TR	0.55	34.9	С	TR	0.61	38.0	D				
Walnut Street	Northbound	LTR	1.15	123.6	F	LTR	0.45	30.4	С				
Walliat Street	Southbound	LTR	1.67	347.1	F	LTR	0.69	47.2	D				
		Interse	ction	86.8	F	Intersed	ction	31.5	С				
			2	9: Prescott Street	& Yonke	rs Avenue (Signali	zed)						
	- u .	L	0.04	13.6	В	L,	0.23	9.4	Α				
	Eastbound	TR	0.69	26.5	С	TR	0.78	17.5	В				
Yonkers Avenue		1	0.27	13.7	В	1	0.39	29.8	C				
	Westbound	TR				TR							
Prescott Street	Northbound												
Frescoll Street	IR 0.48 7.0 A IR 0.54 6.5 A												
		interse						13.7	В				
						Yonkers Avenue (T .					
	Eastbound	TR	0.74	8.5	Α	TR	0.65	5.1	Α				
Yonkers Avenue	Westbound	T	0.61	26.3	С	T	0.69	27.6	С				
	Westbourid	R	0.36	0.6	Α	R	0.34	0.5	Α				
Driveway	Northbound	LR	0.19	56.1	Е	LR	0.16	55.2	Е				
A - l- l	0	L	0.93	68.3	Е	L	0.84	57.3	Е				
Ashburton Avenue	Southbound	LR	0.78	38.3	D	LR	0.85	57.7	Е				
		Interse		22.3	С	Intersed		22.3	С		-1		
						II NB Ramps (Sign							
		1	0.69	28.1	C	I ND Kamps (olgii	0.52	24.7	С				
	Eastbound	<u> </u>	0.51	5.1	A	<u>L</u>	0.50	5.6	A				
Yonkers Avenue		<u> </u>	0.75	40.4		<u>'</u>	0.78	42.6					
	Westbound	<u> </u>			D				D				
		R	0.14	0.2	Α	R	0.10	0.1	Α				
Saw Mill Northbound Ramps	Southbound	L	0.28	45.7	D	L	0.24	45.0	D				
	334.1334.14	R	0.46	22.9	С	R	0.47	23.7	С				
		Interse		21.7	С	Intersed		23.0	С				
					Terrace 3	& Wasylenko Lane						<u>. </u>	
Vankare Avanua	Eastbound	LT	0.45	3.0	Α	LT	0.43	1.6	Α				
Yonkers Avenue	Westbound	TR	0.43	6.8	Α	TR	0.43	6.9	Α				
Fox Terrace	Southbound	LR	0.00	0.0	Α	LR	0.00	0.0	Α				
Wasylenko Lane	Southwestbound	LR	0.02	42.0	D	LR	0.20	52.7	D				
		Interse		4.9	A	Intersed		4.6	A		1		
		intorso				Avenue West (Sign		7.0	, , ,				
		1	0.40	16.1	В	I I	0.34	14.8	В				
Midland Avenue	Westbound	R	0.39			L D	0.35	6.2					
		K +		6.1	A	R			A				
	Northbound	<u> </u>	0.72	25.8	С	· ·	0.73	26.4	С				
Yonkers Avenue		R	0.17	5.7	Α	R	0.22	6.2	Α				
i ormore / Worldo	Southbound	L	0.91	66.2	E	L	0.80	49.9	D				
	Coddibodila	T	0.56	17.5	В	T	0.56	17.0	В				
		Interse	ction	23.2	O	Intersed	ction	21.3	С				

Table 11-4 (cont'd) 2020 Existing Conditions Level of Service Analysis

									<u> </u>	020 Existing Co	namons Lev	ei of Service A	<u> Maiy</u>
Annroach			Weekday A	AM			Weekday	PM			Saturday		
Approach		Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	L
		34:	Yonkers Avenu	ue and Saw Mill R	ver Park	way Southbound R	amps (Unsigna	alized)					
	Eastbound	Т	-	-	-	T	-	-	-				
Yonkers Avenue	Eastboulld	R	-	-	-	R	-	-	-				
	Westbound	Т	-	-	-	Т	-	-	-				
Saw Mill SB Off-Ramp	Southbound	R	0.50	24.7	С	R	0.74	40.3	Е				
		35: Yonl	kers Avenue an	d Cross County P	arkway (n-Ramp/Midland A	Avenue (East) (Signalized)					
	Eastbound	L	0.90	63.4	Е	L	0.62	40.7	D				
Midland Avenue	Eastbourid	TR	0.18	9.3	Α	TR	0.16	11.7	В				
	Westbound	LTR	0.12	15.7	В	LTR	0.13	12.5	В				
	Northbound	L	0.24	11.9	В	L	0.26	12.0	В				
Yonkers Avenue	Northbourid	TR	0.42	13.3	В	TR	0.44	13.3	В				
ronkers Avenue	Southbound	L	0.07	18.8	В	L	0.20	18.7	В				П
	Southbound	TR	1.01	52.2	D	TR	0.96	39.9	D				
		Intersed	ction	37.8	D	Intersec	ction	28.2	С				
			36: I	Hawthorne Avenu	e & Pros	ect Street (Unsign	alized)						
	Eastbound	LTR	0.27	10.7	В	LTR	0.41	12.2	В	LTR	0.15	9.5	T
Prospect Street	VA/ a a tha a consid	L	0.26	14.9	В	L	0.52	24.0	С	L	0.13	11.2	
·	Westbound	TR	0.40	11.3	В	TR	0.51	12.5	В	TR	0.25	9.9	
			37: Lo	cust Hill Avenue	& Ashbu	ton Avenue (Unsig	nalized)						
A a la la conta na Accanacia	Eastbound	TR	-	-	-	TR	-	-	-				
Ashburton Avenue	Westbound	LT	0.05	8.6	Α	LT	0.07	8.9	Α				
Locust Hill Avenue	Northbound	LR	0.19	15.0	С	LR	0.16	17.8	С				T
			38: Palis	sade Avenue & La	fayette P	ace/Walsh Road (S	Signalized)		-				
Lafayette Place	Eastbound	LTR	0.10	7.3	Α	LTR	0.06	7.0	Α				
Walsh Road	Westbound	LTR	0.10	5.2	Α	LTR	0.14	6.3	Α				
Delia ede Assessa	Northbound	TR	0.44	12.6	В	TR	0.42	12.0	В				
Palisade Avenue	Southbound	LT	0.44	13.2	В	LT	0.54	14.7	В				T
		Intersed	ction	11.8	В	Intersec	ction	12.4	В				
			50: L	ocust Hill Avenue	& Overlo	ok Terrace (Unsig	nalized)	•					
Overlook Terrace	Eastbound	LR	-	0.0	Α	LR	-	0.0	Α	LR	-	0.0	
	Northbound	LT	-	0.0	Α	LT	-	0.0	Α	LT	-	0.0	+
Locust Hill Avenue	Southbound	TR	_	_	l _	TR	_	_	<u> </u>	TR	_	-	\top

Notes:
v/c = volume to capacity, LOS = Level of Service
L = Left Turn, T = Through, R = Right Turn, dr = defacto right- turn lane

Table 11-20 2020 Existing and 2032 No Action Conditions Level of Service Analysis

										1								1					el of Ser	vice Ana	uysis
					Weeko								Week	day PM							Satu				
Approach			2020 Exi	<u> </u>			2032 No A			_	2020 Exi		T		2032 No	Action			2020 Exi		_		2032 No A		
		Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	1.08	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS
		Group	Itatio	(000)	100	Group	rtutio	(000)				ve & Main			Ratio	Boildy (000)		Croup	Ratio	(000)		Огоир	rtatio	(000)	1200
	EB	LTR	0.30	13.8	В	LTR	0.31	14.0	В	LTR	0.57	18.3	В	LTR	0.61	19.2	В	LTR	0.16	12.3	В	LTR	0.17	12.4	В
Main St	WB	LTR	0.33	14.7	В	LTR	0.36	15.1	В	LTR	0.55	18.8	В	LTR	0.59	19.8	В	LTR	0.21	12.9	В	LTR	0.23	13.1	В
	NB	LTR	0.58	20.5	C	LTR	0.73	26.3	C	LTR	0.44	18.0	В	LTR	0.86	38.0	D	LTR	0.29	15.3	В	LTR	0.63	22.0	C
Buena Vista Ave	SB	LTR	0.45	17.5	В	LTR	0.71	24.8	C	LTR	0.76	27.2	C	LTR	1.04	70.0	E	LTR	0.13	12.3	В	LTR	0.40	12.4	В
		Interse		17.2	В	Interse		22.0	C	Interse		21.4	C	Interse		41.6	D	Interse		13.6	В	Inters		17.6	В
										2: Buena \	/ista Ave	& Hudson	St (Un:	signalized)		•									
Hudson St	WB	LR	0.09	9.2	Α	LR	0.11	10.1	В	LR	0.10	9.7	À	LR	0.12	10.9	В	LR	0.09	8.2	Α	LR	0.11	9.4	Α
D	NB	TR	0.45	11.7	В	TR	0.59	15.1	С	TR	0.34	10.5	В	TR	0.70	19.3	С	TR	0.20	8.5	Α	TR	0.48	12.0	В
Buena Vista Ave	SB	LT	0.54	12.8	В	LT	0.81	24.6	С	LT	0.79	22.7	С	LT	1.08	77.5	F	LT	0.19	8.4	Α	LT	0.45	11.3	В
			•						•	3: Buena V	ista Ave 8	& Prospec	t St (Un	signalized				•	•	•			•		
Droopoot St	WB	L	0.04	9.9	Α	L	0.27	13.4	В	L	0.20	11.9	В	L	0.38	16.1	С	L	0.04	9.0	Α	L	0.20	11.6	В
Prospect St	WB	R	0.36	11.6	В	R	0.54	16.7	С	R	0.24	10.8	В	R	0.61	19.5	С	R	0.14	8.3	Α	R	0.41	12.6	В
Buena Vista Ave	NB	TR	0.42	11.7	В	TR	0.65	19.6	С	TR	0.35	11.3	В	TR	0.86	31.4	D	TR	0.16	8.2	Α	TR	0.42	12.4	В
Duella Vista Ave	SB	LT	0.60	16.4	С	LT	1.03	70.6	F	LT	0.81	27.8	D	LT	1.28	168.8	F	LT	0.26	9.3	Α	LT	0.62	17.4	С
										4: Hawtl	norne Ave	& Main S	t (Unsiç	nalized)											
Main St	EB	TR	-	-	-	TR	-	-	-	TR	-	-	-	TR	-	-	-	TR	-	-	-	TR	-		
	WB	LT	0.02	7.6	Α	LT	0.02	7.6	Α	LT	-	0.0	Α	LT	-	0.0	Α	LT	0.03	7.5	Α	LT	0.03	7.5	Α
Hawthorne Ave	NB	LR	0.07	10.0	В	LR	0.07	10.1	В	LR	0.19	13.9	В	LR	0.21	14.5	В	LR	0.06	9.9	Α	LR	0.07	10.1	В
Market Pl	SB	LR	0.02	9.8	Α	LR	0.02	9.8	Α	LR	0.04	11.3	В	LR	0.04	11.7	В	LR	0.02	9.8	Α	LR	0.02	9.9	Α
			1	1	1 1			1	1	5: Hawtho	rne Ave 8	& Hudson	St (Uns	ignalized)	ı		T		1	T			1		
Hudson St	EB	LT	- 0.47	-	-	LT	- 0.40	- 40.4	-	LT	-	- 44.7	-	LI	-	-	-	LT	- 0.45	-	-	LT	- 0.40	- 0.7	-
Hawthorne Ave	NB	LTR	0.17	10.2	В	LTR	0.18	10.4	В	LTR	0.30	11.7	В	LTR	0.32	12.0	В	LTR	0.15	9.7	A	LTR	0.16	9.7	A
	SB	LR	0.07	9.7	Α	LR	0.08	9.8	Α	LR	0.13	11.4	В	LR	0.15	11.7	В	LR	0.05	9.1	Α	LR	0.06	9.2	Α
	ED.	LTD	0.00	45.0		LTD	0.50	20.4		6: Warburt			_ ,	,	0.00	25.2	_	1							
Ashburtan Ava	EB	LTR	0.20	15.3 21.1	B C	LTR	0.59	28.4 26.4	C	LTR	0.25 0.10	21.7 24.2	C	LTR	0.66 0.19	35.3 30.9	D C								+
Ashburton Ave	WB	TR	0.11	16.5	В	L TR	0.19 0.35	23.6	C	L TR	0.10	20.2	C	TR	0.19	32.9	C								
	NB	LTR	0.23	38.9	D	LTR	0.33	40.3	D	LTR	0.80	40.8	D	LTR	0.85	43.6	D								+
Warburton Ave		LIK	0.71	17.2	В	LIN	0.70	16.2	В	LIK	0.80	14.2	В	LIK	0.30	13.1	В								+
Warburton Ave	SB	TR	0.30	17.2	В	TR	0.33	16.5	В	TR	0.29	14.7	В	TR	0.33	13.6	В								1
		Interse		23.9	C	Interse		27.1	C	Interse		26.4	C	Interse		31.4	C								+
				_0.0					J			& Wells				• • • • • • • • • • • • • • • • • • • •									
	EB	LTR	0.06	0.5	Α	LTR	0.07	0.6	Α	LTR	0.34	11.3	В	LTR	0.36	12.5	В	LTR	0.01	0.0	Α	LTR	0.01	0.0	Α
Wells Ave	WR	L	0.67	32.4	C	L	0.71	34.2	C	L	0.74	35.4	D	L	0.78	38.4	D	L	0.61	25.9	C	L	0.64	27.1	C
	WB	TR	0.35	24.6	C	TR	0.37	25.0	C	TR	0.16	21.8	C	TR	0.17	22.1	С	TR	0.10	17.8	В	TR	0.11	17.8	В
\A/	NB	Т	0.42	17.9	В	T	0.47	18.9	В	Т	0.29	15.8	В	T	0.34	16.6	В	Т	0.24	12.0	В	Т	0.30	12.6	В
Warburton Ave	SB	TR	0.53	19.4	В	TR	0.60	21.0	С	TR	0.48	18.6	В	TR	0.54	20.0	В	TR	0.27	11.7	В	TR	0.33	12.5	В
		Interse	ection	23.1	С	Interse	ction	24.2	С	Interse		23.5	С	Interse		24.9	С	Interse	ection	17.5	В	Inters	ection	17.9	В
								8: Ne	pperhai	n St & Wark	ourton Av	e & Dock	St/Mand	r House So	q (Signal	ized)									
Nepperhan St	NEB	LR	0.20	8.2	Α	LR	0.24	9.9	Α	LR	0.36	14.9	В	LR	0.41	16.9	В	LR	0.06	0.3	Α	LR	0.09	1.5	Α
	NB	L	0.32	18.8	В	L	0.40	21.5	С	L	0.53	25.8	С	L	0.67	34.8	С	L	0.13	14.5	В	L	0.18	15.5	В
Warburton Ave		TR	0.73	24.3	С	TR	0.81	28.5	С	TR	0.52	15.7	В	TR	0.59	18.1	В	TR	0.37	14.0	В	TR	0.43	15.2	В
	SB	26.9	0.78	26.9	С	LTR	0.86	39.6	D	LTR	0.81	32.7	С	LTR	0.90	56.5	E	LTR	0.63	21.2	С	LTR	0.71	25.2	С
		Interse	ection	24.4	С	Interse	ction	32.8	С	Interse	ection	26.2	С	Interse	ection	41.0	D	Interse	ection	18.2	В	Inters	ection	20.9	С

Table 11-20 (cont'd)
2020 Existing and 2032 No Action Conditions
Level of Service Analysis

				,	Wook	day AM				I			Weekd	av PM			1				9:	turday	Level of Sei	VICC IXII	41y 515
			2020 Existi		VVCCK	uay Airi	2032 No A	ction			2020 Exi		Vecku	ay i ivi	2032 No	Action			2020 Exi	stina	- 08	lturuay	2032 No A	ction	-
Approach		Lane	ZOZO ZXIOU	Delay		Lane	2002 110 7	Delay		Lane	ZOZO ZXI	Delay		Lane	2002 110 1	Delay		Lane	v/c	Delay		Lane	2002 110 7	Delay	$\overline{}$
		Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS		Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS
	•							· · · · ·	9: I	Riverdal	e Ave/Warbur	ton Ave & I	Main S	t (Signa	alized)			•	•	· · · ·				, ,	
	EB	LTR	0.36	12.2	В	LTR	0.39	12.5	В	LTR	0.81	40.3	D	LTR	0.88	52.2	D	LTR	0.29	9.8	Α	LTR	0.31	10.5	В
Main St	WB	L	0.25	28.3	С	L	0.27	28.7	С	L	0.74	45.8	D	L	0.79	50.5	D	L	0.64	38.3	D	L	0.69	40.6	D
	WD	TR	0.34	18.1	В	TR	0.36	19.1	В	TR	0.60	27.7	С	TR	0.64	29.1	С	TR	0.33	19.5	В	TR	0.35	20.2	С
Riverdale Ave	NB	L	0.08	10.7	В	L	0.09	11.1	В	L	0.10	11.8	В	L	0.11	12.5	В	L	0.02	10.0	Α	L	0.03	10.2	В
		T	0.24	11.1	В	T	0.26	12.1	В		0.12	10.5	В	T	0.15	10.7	В	<u>T</u>	0.10	10.1	В		0.13	10.4	В
Warburton Ave	SB	TR	0.57	27.7	С	TR	0.63	29.2	C	TR	0.66	30.6	C	TR	0.74	33.4	C	TR	0.49	26.3	C	TR	0.57	28.1	C
		Inter	section	19.9	В	Inte	ersection	21.0	С		ersection	30.7	C		ersection	34.0	С	Inters	ection	23.7	С	Inte	ersection	24.9	С
		LTD	0.54	00.0	T 5	LED	0.57	07.4			iverdale Ave				0.05	540		LTD	0.40	40.0			0.40	44.0	
Hudson St	EB	LTR	0.54	36.6	D	LTR	0.57	37.4 20.1	D	LTR	0.83	52.7	D	LTR	0.85	54.0	D	LTR	0.46	42.8	D	LTR	0.49	44.3	D
Riverdale Ave	NB	TR	0.41 0.27	19.1 10.1	B B	TR	0.45 0.31	20.1 11.4	C B	TR	0.39 0.49	12.7 18.8	B	TR	0.45 0.57	15.6 24.8	B	TR	0.23	5.8 5.5	A	TR	0.26 0.32	6.5 6.5	A
Riverdale Ave	SB		0.27	9.7	A		0.36	10.3	В		0.49	10.3	В	౼	0.38	11.4	В	<u>L</u>	0.26	3.6	A	T	0.32	3.9	A
	1	Inter	rsection	16.6	В	Inte	ersection	17.4	В	Int	ersection	19.9	В	Inte	ersection	22.1	С	Interse		7.4	A	Inte	ersection	7.9	A
		IIICI	SCOLIOIT	10.0		IIIC	CISCULOTI	17.7			verdale Ave 8					22.1	U	IIICIS	COLIOIT	7.7		III	CISCOLIOII	1.5	
	EB	LTR	0.36	29.2	С	LTR	0.58	34.0	С	LTR	0.48	31.9	C	LTR	0.88	50.4	D	LTR	0.14	27.2	С	LTR	0.34	29.5	С
		1	0.88	31.1	C		1.20	125.9	F	1	0.90	35.7	D	1	1.34	189.6	F		0.34	5.1	A	1	0.51	8.8	A
Prospect St	WB	T	0.50	9.2	A	T	0.66	11.0	В	Ť	0.71	13.2	В	T	0.93	28.4	С	T	0.28	4.3	A	T	0.49	6.9	A
		R	0.22	1.2	Α	R	0.24	1.4	Α	R	0.18	0.7	Α	R	0.21	0.9	Α	R	0.16	0.5	Α	R	0.18	0.5	Α
	ND	L	0.38	42.8	D	L	0.56	46.4	D	L	0.41	42.2	D	L	0.65	55.4	Е	L	0.13	28.9	С	L	0.30	31.6	С
	NB	TR	1.15dr	73.2	Е	TR	1.27dr	104.3	F	TR	0.91dr	49.3	D	TR	0.99dr	85.7	F	TR	0.61	36.8	D	TR	0.89dr	39.7	D
Riverdale Ave		L	0.67	60.0	Е	L	0.82	146.0	F	L	0.90	127.2	F	L	1.00	157.6	F	L	0.81	59.4	Е	L	0.92	123.8	F
	SB	Т	0.45	33.0	С	Т	0.50	33.9	С	Т	0.46	33.1	С	Т	0.51	34.0	С	Т	0.26	30.1	С	Т	0.31	30.8	С
		R	0.07	0.3	Α	R	0.08	0.3	Α	R	0.08	0.3	Α	R	0.09	0.3	Α	R	0.04	0.1	Α	R	0.05	0.2	Α
		Inter	section	39.8	D	Inte	ersection	65.1	Е	_	ersection	39.1	D		ersection	71.9	E	Interse	ection	27.3	С	Inte	ersection	35.1	D
							1				Riverdale Ave		· -			T -			ı			1			
Vark St	EB	LTR	0.40	27.6	C	LTR	0.43	28.6	С	LTR	0.64	51.7	D	LTR	0.70	55.9	E								
	WB	LTR	0.15	18.7	В	LTR	0.16	19.2	В	LTR	0.48	40.6	D	LTR	0.52	42.0	D								
	NB	L TD	0.12	22.5	С	TR	0.13	22.7	С	TR	0.09	11.7	В	TR	0.11	12.4	В								
Riverdale Ave		TR	0.80 0.59	40.8 45.2	D D	IK	0.91 0.66	49.6 50.2	D D	IK	0.42 0.42	19.1 18.6	B	IK	0.50 0.51	20.5 19.8	C B								
	SB	TR	0.59	27.8	С	TR	0.00	29.7	С	TR	0.42	14.6	В	TR	0.52	14.0	В								
	1		section	33.3	C		ersection	37.9	D		ersection	21.6	С		ersection	22.0	С								
		IIICI	30011011	00.0		IIIC	CISCULOTI	01.0			n Broadway &					22.0	U								
	EB	LTR	0.37	24.4	С	LTR	0.71	24.9	С	LTR	0.46	21.3	C	LTR	0.80	28.1	С								
Ashburton Ave	WB	LTR	0.49	15.9	В	LTR	0.67	20.8	C		0.52	14.5		LTR	0.89	29.8	C								
	NB	LTR	0.68	39.2	D	LTR	0.71	39.9	D	LTR	0.69	39.3		LTR	0.73	40.4	D								
North Broadway		L	0.50	23.6	C	L	0.56	24.5	C	L	0.50	23.1	C	L	0.55	24.0	С								
1	SB	TR	0.37	20.0	В	TR	0.41	19.8	В	TR	0.25	17.8	В	TR	0.33	17.2	В								
	•	Inter	section	24.9	С		ersection	26.1	С		ersection	23.8	С		ersection	29.3	С								
									14: N	orth Bro	oadway & Mar	or House S	Square	(Unsig	nalized)										
Manor House Sq		L	0.37	14.7	В	L	0.41	15.9	С	L	0.39	14.6	В	L	0.43	15.7	С	L	0.16	11.4	В	L	0.18	11.8	В
North Broadway	NB	T	-	-	-	T	-	-	-	T	-	-	-	T	-	-	-	T	-	-	-	T	-	-	-

Table 11-20 (cont'd) 2020 Existing and 2032 No Action Conditions Level of Service Analysis

				-	Week	day AM							Weekr	lay PM							Sa	turdav	Level of Se	. , 100 1 111	11,515
			2020 Exist		TTOOK	<u>uuy 7</u>	2032 No A	ction			2020 Exi			<u>,</u>	2032 No	Action			2020 Exi	istina		l	2032 No A	Action	
Approach		Lane		Delay		Lane		Delay		Lane		Delay		Lane		Delay		Lane	v/c	Delay		Lane		Delay	
		Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS
								15	a: No	rth Broa	dway & Main	St (Getty S	quare	West) (Signalized)										
Main St	WB	TR	0.29	0.6	Α	TR	0.31	0.6	Α	TR	0.49	2.1	Α	TR	0.52	2.1	Α	TR	0.42	2.7	Α	TR	0.45	2.8	Α
North Broadway	NB	LT	0.43	20.6	С	LT	0.46	21.5	С	LT	0.47	22.0	С	LT	0.51	23.0	С	LT	0.34	18.1	В	LT	0.37	19.0	В
`		Inter	section	11.3	В	Inte	ersection	11.9	В	In	tersection	10.6	В	Int	ersection	11.1	В	Interse	ection	8.6	Α	Int	ersection	9.1	Α
						_			: Pali		ve & New Maii	n St (Getty	Squar	e East)	(Signalized)							5.		-	
New Main St	WB	TR	0.53	25.0	С	TR	0.56	25.7	С	TR	0.69	29.5	С	TR	0.74	31.2	С	TR	0.54	25.2	С	TR	0.58	26.0	С
Palisade Ave	NB	LT	0.21	22.9	С	LT	0.22	23.2	С	LT	0.61	30.7	С	LT	0.65	31.9	С	LT	0.37	25.3	С	LT	0.40	25.7	С
		Inter	section	24.5	С	Inte	ersection	25.2	С	Int	ersection	30.0	С	Int	ersection	31.5	С	Interse	ection	25.2	С	Int	ersection	25.9	С
								16:	Sout	h Broad	lway/North Br	oadway & F	Hudso	n St (Ur	nsignalized)										
Hudson St	EB	LR	0.49	12.2	В	LR	0.54	13.3	В	LR	0.98	54.7	F	LR	1.08	85.5	F	LR	0.55	13.4	В	LR	0.61	15.1	С
South Broadway	NB	Т	0.39	11.1	В	T	0.43	11.8	В	Т	0.82	30.6	D	Т	0.91	38.2	Е	T	0.43	11.8	В	Т	0.48	12.8	В
										ıth Broa	dway & Prosp			n Ave (S											
Prospect St	EB	L	0.16	19.5	В	L	0.20	23.0	С	L	0.18	22.0	С	L	0.21	26.0	С	L	0.17	17.4	В	L	0.22	20.6	С
1 103pcct ot	LD	TR	0.79	36.4	D	TR	0.93	82.3	F	TR	0.81	47.8	D	TR	0.97	89.9	F	TR	0.60	23.6	С	TR	0.76	38.2	D
		L	0.65	16.0	В	L	0.82	30.8	С	L	0.81	32.6	С	L	1.04	80.1	F	L	0.15	5.9	Α	L	0.19	6.9	Α
Nepperhan Ave	WB	T	0.57	9.4	Α	Т	0.68	11.8	В	Т	0.64	12.0	В	Т	0.78	15.5	В	T	0.36	6.0	Α	Т	0.49	6.9	Α
		R	0.15	1.5	Α	R	0.16	1.6	Α	R	0.34	1.7	Α	R	0.35	1.7	Α	R	0.10	0.3	Α	R	0.11	0.5	Α
	NB	L	0.29	31.4	С	L	0.34	32.3	С	L	0.25	30.2	С	L	0.34	32.2	С	L	0.07	27.6	С	L	0.13	28.7	С
South Broadway		TR	0.71	43.2	D	TR	0.76	45.8	D	TR	0.88	57.1	E	TR	0.94	66.8	E	TR	0.44	33.8	С	TR	0.48	34.6	С
Journ Broadway	SB	L	0.30	33.8	С	L	0.35	35.6	D	L	1.29	205.8	F	L	1.60	335.2	F	<u>L</u>	0.29	32.4	С	L	0.33	33.3	С
		TR	0.21	24.5	С	TR	0.24	24.7	С	TR	0.25	19.3	В	TR	0.29	20.2	С	TR	0.14	25.5	С	TR	0.18	24.3	С
		Inter	section	24.2	С	Inte	ersection	41.4	D		ersection	41.1	D	Int	0.000.0	66.3	Е	Interse	ection	18.6	В	Int	ersection	24.5	С
	1				1 _				-		roadway & Va	1	-1		•							1		1	
Vark St	EB	LTR	0.69	40.7	D	LTR	0.74	44.0	D	LTR	0.69	42.0	D	LTR	0.75	45.8	D								
Park Hill Ave	WB	LTR	0.71	43.2	D	LTR	0.76	47.4	D	LTR	0.69	44.3	D	LTR	0.75	48.5	D								
South Broadway	NB	LTR	0.63	26.4	С	LTR	0.67	27.8	C	LTR	0.88	41.6	D	LTR	0.95	53.2	D								
,	SB	LTR	0.78	33.6	С	LTR	0.84	38.4	D	LTR	0.74	31.8	C	LTR	0.81	36.0	D								
		inter	section	34.9	С	inte	ersection	38.3	D		tersection	39.3	D A		ersection	46.0	D								
L a quad I IIII A	LED	LTD	0.00	0.0	Ι .	1.70	0.04				Locust Hill A					1445		LTD	0.05	0.4	Ι Λ	1.70	0.00	0.7	
Locust Hill Ave	EB	LTR	0.32	9.2	A	LTR	0.34	9.5	A	LTR	0.60	13.3	В	LTR	0.64	14.5	В	LTR	0.35	9.4	A	LTR	0.38	9.7	A
Palisade Ave	NB SB	TR	0.08	7.9	A	TR	0.09	8.0	A	TR	0.10 0.11	8.5 9.0	A	TR	0.11	8.7	A	TR	0.08	7.8	A	TR	0.08	7.9	A
	SR	LT	0.15	8.6	Α	LT	0.16	8.7	Α	LT 20: No.			A A	LT	0.12	9.1	Α	LT	0.10	8.3	Α	LT	0.10	8.4	Α
			0.00	4.0	Α		0.04	0.0	Ι Δ	20: Ne	w Main St & N			ignalize				1	0.05	1 4 0	Α.		0.00	4.5	
	EB	L	0.03	4.6	A	TR	0.04	6.6	A	L	0.04	4.7	A	L	0.05	6.2	A	L TR	0.05	1.8	A	TR	0.06	4.5	A
Monnorhan Assa		TR	0.49	10.2	В	IK	0.58	13.0 38.6	В	TR	0.69	13.4	B	TR	0.82	15.9	B	IK I	0.41	6.1 38.9	A	IK	0.52	8.4	A
Nepperhan Ave	WE		0.51	27.9	C	<u>L</u>	0.61		D	T	1.06	106.5		L T	1.18	142.5	•	<u>L</u>	0.68		D	T	0.84	59.1	E
	WB	T	0.59 0.44	9.1	A	T	0.68	11.8 12.9	В	Т	0.60 0.42	12.0	В	T	0.71	15.3 15.7	В	T	0.32	8.5	A	l D	0.42	8.6 10.3	A
New Main St	NB	R LTR	1.00	10.2 77.7	В	R LTR	0.47 1.06	93.3	B	R LTR	1.04	12.8	B	R LTR	0.44 1.11		B	R LTR	0.42 0.80	10.4 48.0	В	R LTR	0.45 0.85	51.9	B D
ivew iviain St	INR			22.4	E C		1100	26.8	C			88.2 33.1	C			108.0 39.8	D			18.7	D B			21.3	С
		inter	section	ZZ. 4	U	Inte	ersection	∠0.ŏ	U	l in	ersection	აპ. I	U	Inte	ersection	აყ.გ	U	Interse	ะบบบก	ΙŎ./	В	Int	ersection	∠1.3	U

Table 11-20 (cont'd)
2020 Existing and 2032 No Action Conditions
Level of Service Analysis

					Wook	day AM				I		,	Weeko	lav DN	1						9.	aturday	Level of Se	VICC AII	a1 y 515
			2020 Existi		VVCCN	day Aivi	2032 No A	ction			2020 Exi		VVCCK	ayrı	2032 No	Action			2020 Exi	stina	- 36	luruay	2032 No A	ction	
Approach		Lano	ZUZU EXIST	Delay		Lane	2032 110 7	Delay		Lane	ZUZU EXI	Delay		Lane		Delay		Lane	V/C	Delay		Lane	2032 110 7	Delay	\dashv
	Lane Group v/c Ra			(sec)	LOS	Group	v/c Ratio		LOS	Group	v/c Ratio	(sec)	LOS	Grou		(sec)	LOS		Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS
	Group v/c				· ·		•				isade Ave & A	Ashburton A						•	1		· ·		<u> </u>		
		L	0.17	4.8	Α	L	0.21	6.8	Α	L	0.11	4.1	A	L	0.16	6.8	Α								
	EB	T	0.42	16.7	В	Т	0.67	21.4	С	Т	0.52	17.7	В	Т	0.75	24.5	С								
Ashburton Ave		R	0.14	4.4	Α	R	0.15	6.0	Α	R	0.15	4.8	Α	R	0.16	7.2	Α								
	\A/D	L	0.31	4.6	Α	L	0.41	6.5	Α	L	0.44	4.7	Α	L	0.59	17.6	В								
	VVD	TR	0.56	8.7	Α	TR	0.68	10.3	В	TR	0.48	6.8	Α	TR	0.77	10.2	В								
Palisade Ave	NB	LT	0.67	43.4	D	LT	0.70	44.9	D	LT	0.56	39.8	D	LT	0.59	40.5	D								
ralisaue Ave	IND	R	0.26	8.7	Α	R	0.28	9.6	Α	R	0.22	8.0	Α	R	0.23	7.8	Α								
		Inter	section	15.3	В	Inte	ersection	17.5	В		ersection	13.8	В		ntersection	18.5	В								
											w School St 8	Palisade A													
Palisade Ave	EB	LTR	0.35	17.2	В	LTR	0.37	17.5	В	LTR	0.59	21.6	С			22.5	С	LTR	0.40	17.7	В	LTR	0.43	18.1	В
New School St	NB	TR	0.34	12.5	В	TR	0.37	12.8	В	TR	0.22	10.5	В	TR	0.24	10.8	В	TR	0.18	8.2	Α	TR	0.19	8.4	Α
Palisade Ave	SB	LT	0.50	16.9	В	LT	0.56	18.5	В	LT	0.49	16.0	В	LT	0.52	16.6	В	LT	0.22	12.4	В	LT	0.23	12.5	В
		Inter	section	15.4	В	Inte	ersection	16.1	В		ersection	17.0	В		ntersection	17.7	В	Inters	ection	13.6	В	Int	ersection	13.9	В
				1	_	1	-		1	w Scho	ol St & Maple		-1	Ave (S		1			1	+	1	1	1		!
	EB	L	0.38	26.5	С	L	0.40	28.4	С	L	0.27	21.8	С	L	0.28	22.8	С	L	0.25	9.2	Α	L	0.30	18.1	В
		TR	0.57	16.3	В	TR	0.66	18.0	В	TR	0.74	12.9	В	TR	0.87	18.0	В	TR	0.45	13.9	В	TR	0.55	13.6	В
Nepperhan Ave		<u>L</u>	0.14	11.1	В	L L	0.16	11.4	В	L L	0.34	15.9	В	L_	0.36	16.6	В	<u> </u>	0.14	11.0	В	<u> </u>	0.18	11.4	В
	WB	T	0.74	30.9	С	T	0.84	34.5	С	T	0.75	31.0	С	T	0.86	35.4	D	T	0.52	26.1	С	T	0.63	28.2	С
		R	0.29	3.9	A	R	0.31	4.8	A	R	0.20	4.2	A	R	0.22	5.3	Α	R	0.17	4.5	A	R	0.18	4.4	A
New School St	SB	LTR	0.24	24.9	С	LTR	0.25	25.3	C	LTR	0.52	27.6	С	LTR		28.7	С	LTR	0.22	11.0	В	LTR	0.23	11.0	В
		Inter	section	23.1	С	Inte	ersection	25.6	С		ersection	21.8	С		ntersection	25.8	С	Inters	ection	18.6	В	Int	ersection	19.9	В
	1 =5			1	1			I	I		erly St & Nep	perhan Av	e (Uns		zed)				1	1	1		T		
	EB	TR	-	-	-	TR	-	-	-	TR	-	-	-	TR	-	- 07.0	-	TR	-	- 40.0	-	TR	-	- 440	 -
Nepperhan Ave	WB	T T	-	0.0	Α	T L	-	0.0	Α	L T	0.01	21.4	С	L T	0.01	27.2	D	L T	0.00	12.8	В	누	0.00	14.9	В
Moverly Ct	NB	LR	0.89	76.1	F	LR	1 21	- 185.4	- F	LR	1.17	181.0	F	LR	1.78	450.2	- F	LR	0.44	22.1	-	LR	0.60	34.6	- D
Waverly St	IND	LK	0.69	70.1	Г	LK	1.21	100.4								450.2	Г	LK	0.44	22.1	С	LK	0.60	34.0	
		1	0.54	67.9		1	0.66	77.3		zo: Nepp	perhan Ave &		Ave	Signa	0.71	85.2	F								
	EB	T	0.54 0.74	30.2	E C	T	1.01	63.8	E	T	0.60 0.78	82.6 27.9	C	ㅜ	1.06	73.3	E								+
Ashburton Ave	ED	R	0.74	1.3	A	R	0.07	03.8	A	R	0.76	0.4	A	R	0.05	0.2	A								+
Ashbulton Ave		L	0.54	57.9	E	L	0.07	59.2	E	I.	0.03	73.1	E	ı	0.79	77.6	E								+-+
	WB	TR	0.89	55.1	E	TR	1.04	86.4	F	TR	0.73	51.2	D	TR	1.26	164.3	F								_
		1	0.57	59.6	E	I	0.60	61.4	Ė	1111	0.51	55.9	E	111	0.53	56.8	E								
	NB	TR	0.62	31.4	C	TR	0.67	32.6	C	TR	0.44	30.7	С	TR	0.48	31.6	С								
Nepperhan Ave		L	1.32	241.7	F	L	1.73	410.1	F	111	0.86	88.7	F	111	0.93	101.8	F								
. 100001110117110	SB	T	0.32	29.5	C	T	0.36	30.5	С	T	0.33	29.8	С	T	0.36	30.4	С								
		R	0.14	4.2	A		0.16	4.2	A	R	0.13	3.8	A	R	0.16	4.0	A								
	1		section	48.0	D		ersection	69.4	E		ersection	41.7	D		ntersection	81.2	F		1						
											pperhan Ave						•								
	EB	Т	0.58	26.5	С	Т	0.62	27.4	С	T T	0.37	23.2	C	T	0.40	23.6	С								
Nepperhan Ave	WB	T	0.35	23.1	C	Ť	0.38	23.4	C	Ť	0.39	23.2	C	Ť	0.42	23.6	C								
Copcutt Ln	SB	LR	0.32	27.5	C		0.34	27.7	C	LR	0.31	22.8	C	LR		23.6	C								
<u> </u>			section	25.3	С		ersection	25.9	С		ersection	23.2	С		ntersection	23.6	С								

Table 11-20 (cont'd)
2020 Existing and 2032 No Action Conditions
Level of Service Analysis

				,	Week	day AM							Neekd	av PM							Sa	turday	Level of Se	1100 1111	<u> </u>
_			2020 Existi		TTOOK	day Am	2032 No A	ction			2020 Ex		roona	wy 1 141	2032 No	Action			2020 Exi	stina		turuuy	2032 No A	Action	-
Approach		Lane		Delay		Lane		Delay		Lane		Delay		Lane		Delay		Lane	v/c	Delay		Lane		Delay	\neg
		Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS		Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS
					ı		<u>.</u>		ı		Nepperhan A	ve & Elm St							1			•			
		L	0.72	47.0	D	L	0.75	50.2	D	L	0.82	57.4	E	L	0.87	63.5	Е	L	0.59	35.8	D	L	0.64	40.7	D
Elm St	EB	TR	0.28	27.9	C	TR	0.30	29.0	C	TR	0.25	27.1	C	TR	0.27	27.7	С	TR	0.14	19.4	В	TR	0.15	21.7	C
	WB	LR	1.23	150.9	F	LR	1.41	227.2	F	LR	0.54	24.3	C	LR	0.67	31.0	C	LR	0.38	15.5	В	LR	0.49	21.1	C
	NB	TR	0.75	32.5	С	TR	0.83	35.2	D	TR	0.87	37.7	D	TR	1.02	59.2	E	TR	0.60	29.5	C	TR	0.70	31.0	C
Nepperhan Ave		1	0.39	17.0	В	1	0.42	18.6	В	1	0.41	18.1	В	1	0.43	19.5	В		0.17	12.2	В	1	0.20	12.2	В
140ppointail 7440	SB	T	0.48	14.8	В	T	0.53	15.3	В	Ť	0.53	15.1	В	Ť	0.60	16.2	В	_	0.39	13.9	В	Ť	0.45	14.3	В
		Inter	rsection	43.4	D		ersection	55.3	E	Int	ersection	28.3	C	Inte	ersection	38.8	D	Inters		21.8	C	Inte	ersection	23.4	C
		IIICI	300001	70.7		1110	CISCOLOTI	33.3			Walnut St & Y				CISCOLIOIT	30.0		IIICIS	COLIOIT	21.0	U	1110	CISCOLIOII	20.7	\dashv
		1	0.31	11.2	В		0.40	13.3	В	Z0.	0.67	23.3	C	alizeu)	0.81	42.4	D								
	EB	TR	0.43	17.7	В	TR	0.40	18.9	В	TR	0.67	23.3	С	TR	0.83	29.3	С								
Yonkers Ave		1 17				I IX			_	ı ıĸ				117											
	WB	L TD	0.22	18.2	В	TD	0.28	18.0	В	L	0.56	26.4	С	TD	0.68	34.4	С								
		TR	0.55	34.9	С	TR	0.65	38.5	D	TR	0.61	38.0	D	TR	0.70	40.5	D								
Walnut St	NB	LTR	1.15	123.6	F	LTR	1.33	196.7	F	LTR	0.45	30.4	С	LTR	0.61	37.4	D								
	SB	LTR	1.67	347.1	F	LTR	1.84	421.6	F	LTR	0.69	47.2	D	LTR	0.75	51.5	D								
		Inter	section	86.8	F	Int	ersection	108.7	F		ersection	31.5	С		ersection	36.4	D								
				1		1	,		1	29: P	rescott St &		e (Sigr	nalized)		•			1					Ī	
	EB	L	0.04	13.6	В	L	0.04	13.0	В	L	0.23	9.4	Α	L	0.28	9.2	Α								
Yonkers Ave		TR	0.69	26.5	С	TR	0.77	27.6	С	TR	0.78	17.5	В	TR	0.89	20.2	С								
TOTIKETS AVE	WB	L	0.27	13.7	В	L	0.31	22.7	С	L	0.39	29.8	С	L	0.44	39.3	D								
	VVD	TR	0.48	7.0	Α	TR	0.55	6.4	Α	TR	0.54	6.5	Α	TR	0.60	5.8	Α								
Prescott St	NB	LTR	0.81	55.9	Е	LTR	0.86	60.8	Е	LTR	0.24	35.8	D	LTR	0.26	36.1	D								
		Inter	section	21.9	O	Inte	ersection	22.7	С	Int	ersection	13.7	В	Inte	ersection	15.0	В								
									30: l	Drivewa	y/Ashburton	Ave & Yonk	ers Av	e (Sign	alized)										
	EB	TR	0.74	8.5	Α	TR	0.82	10.1	В	TR	0.65	5.1	Α	ŤR	0.76	8.0	Α								
Yonkers Ave	MD	T	0.61	26.3	С	Т	0.70	27.9	С	Т	0.69	27.6	С	Т	0.79	30.3	С								
	WB	R	0.36	0.6	Α	R	0.41	0.6	Α	R	0.34	0.5	Α	R	0.47	1.0	Α								
Driveway	NB	LR	0.19	56.1	Е	LR	0.20	56.6	Е	LR	0.16	55.2	Е	LR	0.17	55.7	Е								
-	0.0	L	0.93	68.3	Е	L	1.12	116.8	F	L	0.84	57.3	Е	L	1.00	82.6	F								
Ashburton Ave	SB	LR	0.78	38.3	D	LR	0.97	64.1	Е	LR	0.85	57.7	Е	LR	0.99	81.4	F								
		Inter	section	22.3	С		ersection	32.2	С		ersection	22.3	С	Inte	ersection	28.8	С								
									_		ers Ave & Sa						-								
		L	0.69	28.1	С		0.91	43.4	D		0.52	24.7	C		0.73	34.3	С								
	EB		0.51	5.1	A	T	0.57	5.6	A	T	0.50	5.6	A	-	0.56	5.9	A								
Yonkers Ave		T	0.75	40.4	D	Ť	0.82	42.2	D	Ť	0.78	42.6	D	Ť	0.88	45.8	D								
	WB	R	0.73	0.2	A		0.02	0.2	A	R	0.70	0.1	A		0.00	0.1	A								
Saw Mill NB		1	0.14	45.7	D	1	0.30	46.2	D	1	0.10	45.0	D	1	0.11	45.3	D								
Ramps	SB	R	0.26	22.9	С	R	0.56	26.5	C	R	0.24	23.7	С	R	0.28	29.4	С								
ramps	L		rsection	21.7	С		ersection	24.6	C		ersection	23.0	C		ersection	25.9	C								
		iiitei	3CCIIOI I	41.1	U	1110	CI 3CUIUII				ve & Fox Ter					20.8	U								
	ГВ	IТ	0.45	2.0	۸	1.7	0.50									17	1 1								
Yonkers Ave	EB	LT	0.45	3.0	A		0.50	3.5	A	LT	0.43	1.6	A	LT	0.48	1.7	A								
	WB	TR	0.43	6.8	Α		0.47	7.4	Α	TR	0.43	6.9	Α	TR	0.48	7.9	A								
Fox Terrace	SB	LR	0.00	0.0	Α	LR	0.00	0.0	Α	LR	0.00	0.0	Α	LR	0.00	0.0	Α								
Wasylenko Ln	SWB		0.02	42.0	D	LR	0.02	42.0	D	LR	0.20	52.7	D	LR	0.22	52.9	D								
		Inter	section	4.9	Α	Inte	ersection	5.4	Α	Int	ersection	4.6	Α	Inte	ersection	5.1	Α								

Table 11-20 (cont'd) 2020 Existing and 2032 No Action Conditions **Level of Service Analysis**

					Week	day AM							Weekd	ay PM							Sa	aturday	Level of Se		•
Approach			2020 Existi				2032 No A	ction			2020 Exi				2032 No	Action			2020 Exi	isting			2032 No A	Action	
Арргоасп		Lane		Delay		Lane		Delay		Lane		Delay		Lane		Delay		Lane	v/c	Delay		Lane		Delay	
		Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)		Group		(sec)		Group		(sec)	LOS	Group	Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS
			0.40	10.4			0.40	45.0		33: Yonk	ers Ave & Mi			Signaliz		440									
Midland Ave	WB	L	0.40 0.39	16.1	В	L	0.43 0.42	15.9	В	L	0.34	14.8 6.2	В	<u>L</u>	0.37 0.40	14.3	В								
		R		6.1	A	R		6.1	A	R	0.35		A	R	0.40	6.1	A								
	NB	R	0.72 0.17	25.8 5.7	C	I D	0.78 0.18	27.3 6.0	C	T	0.73 0.22	26.4 6.2	C	R	0.60	28.4 6.4	C A								
Yonkers Ave		I I	0.17	66.2	A E	R	1.15	134.0	A	R L	0.80	49.9	A D	K	1.05	103.8	F								
	SB	T	0.56	17.5	В	T	0.62	18.0	В	T	0.56	17.0	В	T	0.62	16.0	В								
	1	Inte	section	23.2	C	Inte	ersection	30.9	С	Inte	ersection	21.3	C	ı Int	ersection	26.5	C								
		IIIC	300001	20.2	U	III									mps (Unsigna		U								
		Т	_	_	Τ_	Т	_		-	T	-	-		T		_	T _								
Yonkers Ave	EB	R		-	-	R	-	_	<u>-</u>	R	<u>-</u>	_	 -	R	_	_	+-								
	WB	T	-	_	-	T	-	-	_	T	-	-	-	T	_	_	_								
Saw Mill SB Off-			0.50	04.7	_	-	0.75	44.0	_		0.74	40.0	_	-	4.05	470.4	_								
Ramp	SB	R	0.50	24.7	С	R	0.75	44.2	E	R	0.74	40.3	E	R	1.25	176.1	F								
							35:	: Yonkers A	ve ar	nd Cross	County Parl	way On-Ra	amp/Mi	dland A	Ave (East) (Si	gnalized)									
	EB	L	0.90	63.4	Е	L	0.95	72.9	Е	L	0.62	40.7	D	L	0.67	42.8	D								
Midland Ave		TR	0.18	9.3	Α	TR	0.19	9.3	Α	TR	0.16	11.7	В	TR	0.16	11.5	В								
	WB	LTR	0.12	15.7	В	LTR	0.12	15.4	В	LTR	0.13	12.5	В	LTR	0.14	12.2	В								
	NB	L	0.24	11.9	В	L	0.25	12.2	В	L	0.26	12.0	В	L	0.28	12.7	В								
Yonkers Ave	IND	TR	0.42	13.3	В	TR	0.45	13.8	В	TR	0.44	13.3	В	TR	0.49	14.0	В								
TOTIKOTS / WC	SB	L	0.07	18.8	В	L	0.08	17.8	В	L	0.20	18.7	В	L	0.23	19.1	В								
		TR	1.01	52.2	D	TR	1.09	78.0	Е	TR	0.96	39.9	D	TR	1.05	62.1	Е								
		Inte	section	37.8	D	Inte	ersection	51.4	D		ersection	28.2	С		ersection	38.8	D								
											thorne Ave 8														
D	EB	LTR	0.27	10.7	В		0.41	12.1		LTR	0.41	12.2	В	LTR	0.66	16.8	С	LTR	0.15	9.5		LTR	0.25	10.5	В
Prospect St	WB		0.26	14.9	В	L	0.42	23.6	С	L	0.52	24.0	С	L	1.14	159.1	F		0.13	11.2	В		0.24	15.7	С
		TR	0.40	11.3	В	TR	0.59	14.2	В		0.51	12.5	B Ava (III	TR	0.77	19.9	С	TR	0.25	9.9	Α	TR	0.51	12.8	В
	ГР	TD				TR	T			TR	t Hill Ave & A		AVE (U		izea)										
Ashburton Ave	EB WB	TR LT	0.05	8.6	- A	LT	0.06	9.3	- A	LT	0.07	8.9	- A	TR LT	0.09	9.6	- A								
Locust Hill Ave	NB	LR	0.03	15.0	C	LR	0.00	20.7	C	LR	0.07	17.8	C	LR	0.09	29.1	D								
Locust I IIII AVE	ND	LIX	0.18	13.0	U	LIX	0.29	20.1			e Ave & Lafa					۷۶.۱	D								
Lafayette PI	EB	LTR	0.10	7.3	Α	LTR	0.10	7.4	A	LTR	0.06	7.0	A	LTR	0.06	6.9	Α								
Walsh Rd	WB	LTR	0.10	5.2	A	LTR	0.10	5.4	A	LTR	0.00	6.3	A	LTR	0.00	6.5	A								
	NB	TR	0.10	12.6	В	TR	0.10	13.0	В	TR	0.14	12.0	В	TR	0.13	12.4	В								
Palisade Ave	SB	LT	0.44	13.2	В	LT	0.47	13.6	В	LT	0.54	14.7	В	LT	0.58	15.4	В								
			section	11.8	В		ersection	12.2	В		ersection	12.4	В		ersection	12.9	В								
											ill Avenue &														
Overlook Terr	EB	LR	-	0.0	Α	LR	-	0.0	Α	LR	-	0.0	A	LR	-	0.0	Α	LR	_	0.0	Α	LR	_	0.0	Α
	NB	LT	-	0.0	Α	LT	-	0.0	Α	LT	-	0.0	Α	LT	-	0.0	Α	LT	-	0.0	Α	LT	-	0.0	Α
Locust Hill Ave	SB	TR	-	_	-	TR	-	-	-	TR	_	-	-	TR	-	-	_	TR	-	-	-	TR	-	-	-
				I											l		1						l .	1	

Notes:
EB = Eastbound, WB = Westbound, NB = Northbound, SB = Southbound, NEB = Northeastbound, SWB = Southwestbound v/c = volume to capacity, LOS = Level of Service
L = Left Turn, T = Through, R = Right Turn, dr = defacto right- turn lane
= LOS E, = LOS F

Table 11-1022

														Yo	nkers A	MS De	velopme	ent Site T	rip Ger	eration	Summary
				ITF	E Data ²									1	Trip Gene	ration					
Building		opment		ITE Land Use		Peak	ITE Trip	Total			Total		Interna	ıl Trips ⁶	Transi	it Trips	Ped & E	ike Trips	Final A Tri	ps³	Total External
Component	Si	ize ¹	#	Name	Independent Variable	Hour	Rate	Trips	% In	% Out	In	Out	In	Out	In	Out	In	Out	In	Out	Trips
Chicken Island Site	е																				
				'		AM	0.32	633	0.21		133	500	3	6	33	125	7	25	90	344	434
Residential ⁴	2000	Units	222	Multi-Family Housing (High-Rise)	Dwelling Units	PM	0.36	715	0.62		443	272	57	29	111	68	22	14	253	161	414
		ı <u> </u>				Sat	0.36	720	0.55		396	324	58	27	99	81	20	16	219	200	419
		<u> </u>		'		AM	0.94	66	0.62	0.38	41	25	7	4	10	6	2	1	22	14	36
Retail	70	Ksf	820	Shopping Center	1,000 SF Gross Floor Area	PM	5.96	417	0.48		200	217	34	60	50	54	10	11	106	92	198
		i l			[Sat	6.67	467	0.52	0.48	243	224	25	59	61	56	12	11	145	98	243
		i		'		AM	2.47	42	0.86	0.14	36	6	2	2	9	2	2	0	23	2	25
Office ⁵	17	Ksf	710	General Office Building	1,000 SF Gross Floor Area	PM	4.94	84	0.18	0.82	15	69	13	15	2	17	0	3	0	34	34
		i l			^	Sat	0.53	9	0.54		5	4	4	1	1	1	0	0	0	2	2
,								L		M Trips	210	531	12	12	52	133	11	26	135	360	495
										M Trips	658	558	104	104	163	139	32	28	359	287	646
								•		ay Trips	644	552	87	87	161	138	32	27	364	300	664
Teutonia Site												-	-		•	1	·				<u></u>
				T		AM	0.32	294	0.21	0.79	62	232	-	_	16	58	3	12	43	162	205
Residential ⁴	906	Units	222	Multi-Family Housing (High-Rise)	Dwelling Units	PM	0.37	333	0.62		206	127	-	-	52	32	10	6	144	89	233
1 (00)45				Walta alling 10 a.c	2	Sat	0.36	326	0.55		179	147	-	_	45	37	9	7	125	103	228
				+		AM	0.94	9	0.62	0.38	6	3	_	<u> </u>	2	1	0	0	4	2	6
Retail	10	Ksf	820	Shopping Center	1,000 SF Gross Floor Area		9.90	99	0.48		48	51	-		12	13	2	3	34	35	69
i totali	'		020	Onopping Conto	1,000 01 01000 1 1001 7 1101	Sat	10.00	100	0.52		52	48	_	 	13	12	3	2	36	34	70
<u> </u>					<u> </u>	Oat	10.00	100		M Trips	68	235	_	-	18	59	3	12	47	164	211
										M Trips	254	178	-	 	64	45	12	9	178	124	302
										ay Trips	231	195	<u> </u>	 	58	49	12	9	161	137	298
North Broadway Si	i+o ⁷							<u>`</u>	Jaturuu	iy iiipə	4 3 i	100	-		30	70	14	5	101	101	250
North Broadway of	T T	Г		T		AM	0.33	214	0.21	0.79	45	169	1	3	11	42	2	8	31	116	147
Residential ⁴	650	Units	222	Multi-Family Housing (High-Rise)	Dwelling Units	PM	0.33	243	0.62		151	92	21	11	38	23	8	5	84	53	137
Nesideriliai	050	Ullita	ZZZ	Wulli-Fairing Flousing (Flight-1430)	Dwelling Office	Sat	0.36	234	0.62		129	105	19	11	32	26	6	5	72	63	135
	 			+	 	AM	0.36	16	0.62		10	6	4	3	3	20	1	0	2	1	3
Retail	17	Ksf	820	Shopping Center	1,000 SF Gross Floor Area		8.59	146	0.62	0.52	70	76	13	22	3 18		4	4	35	31	66
Relaii	17	1/21	020	Shopping Center	1,000 SF G1055 F1001 A164		9.00			0.52						19	4	4			
	\vdash			 		Sat	2.19	153	0.52	0.48	80	73	9	20	20 10	18			47	31	78 27
O#:5]	1/-5	740	Company Office Divilding	4 000 OF Compa Floor Area	AM		46			40	6	3	7		2	2	0 4	25	2	
Office ⁵	21	Ksf	710	General Office Building	1,000 SF Gross Floor Area		4.19	88	0.18		16	72	6	•	4	18	<u>'</u>		5	43	48
	<u> </u>					Sat	0.53	11	0.54		6	5	4	1	2	1	0	0	0	3	3
										M Trips	95	181	8	8	24	46	5	8	58	119	177
										M Trips	237	240	40	40	60	60	13	13	124	127	251
										ay Trips	215	183	32	32	54	45	10	9	119	97	216
										M Trips	373	947	20	20	94	238	19	46	240	643	883
											1,149	976	144	144	287	244	57	50	661	538	1,199
								Total S	<u>Saturda</u>	ay Trips	1,090	930	119	119	273	232	54	45	644	534	1,178

Notes: Ksf = 1,000 square feet

¹ Development sizes and uses are preliminary and are subject to change.

Rates shown are roadway peak hour rates from the Institute of Transportation Engineers (ITE) Trip Generation Manual, 10th Edition

Final Adjusted Trips are calculated by subtracting internal, transit, and pedestrian and bicycle trips from the Total Trips

Generator peak hour rates were utilized for the Residential land use, all peak hours per direction from NYSDOT.

⁵ Generator peak hour rates were utilized for the Office PM peak hour due to more conservative estimate compared to the roadway peak hour rate.
6 Saturday internal trips were calculated using PM peak hour internalization factors, based on similar travel patterns and connectivity for the two peak hours.
7 The previous, larger land use program is utilized for the North Broadway site, which results in a conservative trip generation estimate. The updated North Broadway Project includes 650 residential units, 15,000 gsf of retail space, and 13,000 gsf of office space.
= Fitted curve rates were used for land uses and peak hours where at least 20 data points are available and R² ≥ 0.75. =

Table 11-<u>1123</u>
2032 No Action and 2032 With Action Conditions
Level of Service Analysis

										T													el of Ser	rice Ana	11y515
					Week	day AM							Week	day PM							Satu				
Approach			2032 No A				2032 With			_	2032 No A		1		2032 With				2032 No A				2032 With	r	
		Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay	
		Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS		Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS
			0.04	440	-		1 000 1	40.7				ve & Main			0.70	07.7			0.47	10.4	-		1 0 00	10.4	
Main St	EB	LTR	0.31	14.0	В	LTR	0.36	18.7	В	LTR	0.61	19.2	В	LTR	0.70	27.7	С	LTR	0.17	12.4	В	LTR	0.20	16.4	В
	WB	LTR	0.36	15.1	В	LTR	0.43	20.7	С	LTR	0.59	19.8	В	LTR	0.76	35.5	D	LTR	0.23	13.1	В	LTR	0.30	18.1	В
Buena Vista Ave	NB	LTR	0.73	26.3	C	LTR	0.65	12.3	В	LTR	0.86	38.0	D	LTR	0.74	15.6	В	LTR	0.63	22.0	С	LTR	0.56	10.2	В
	SB	LTR	0.71	24.8 22.0	C	LTR	0.62	19.5 17.1	B	LTR	1.04	70.0	E	LTR	0.90	47.6	D	LTR	0.40	12.4	В	LTR	0.36	16.4	B
		Interse	ection	22.0	С	Inters				Interse		41.6	D	Interse		33.1	С	Interse	ection	17.6	В	inters	ection	13.5	В
	ED				-	LTD	+	2: Buena \	-1	ve & Huase	on St (No	Action Uns	signaliz									LTD	0.07	24.4	\perp
Hudson St	EB WB	LR	0.11	10.1	В	LTR	0.32 0.37	31.3 27.3	C	LR	0.12	10.9	В	LTR LTR	0.25 0.42	30.6 32.8	C	LR	0.11	9.4	Ι Λ	LTR LTR	0.27	31.1 33.9	C
		TR	0.11	10.1 15.1	С	<u>С</u> В		10.6	В	TR	0.12	19.3	С	LTR	0.42	12.5	В	TR	0.11	12.0	A B		0.44	10.7	
Buena Vista Ave	NB SB	LT	0.59	24.6	C	LTR	0.43 0.49	9.1	A	LT		77.5	F	LTR	0.52	14.8	В	LT	0.48		В	LTR LTR	0.39	7.0	B A
	SD	LI	0.61	24.0	10	Inters		11.8	B	LI	1.08	77.5	Г	Interse		15.6	В	LI	0.45	11.3	D		ection	12.7	B
						IIILEIS	ection	11.0	Ь	2: Buona \	lista Avo	2 Prospec	+ C+ /I Ir	nsignalized		15.0	Ь					IIILEIS	ection	12.7	Ь
	1	1	0.27	13.4	В		0.28	13.9	В	J. Buella v	0.38	16.1	C	ISIGITATIZEO	0.40	16.9	С		0.20	11.6	В	1	0.22	12.4	В
Prospect St	WB	R	0.27	16.7	C	R	0.62	19.1	С	R	0.30	19.5	C	R	0.40	35.9	E	R	0.20	12.6	В	R	0.62	19.0	С
	NB	TR	0.65	19.6	C	TR	0.02	21.6	C	TR	0.86	31.4	D	TR	1.02	51.0	F	TR	0.41	12.4	В	TR	0.53	15.8	C
Buena Vista Ave	SB	LT	1.03	70.6	F	LT	1.25	155.0	F	LT	1.28	168.8	F	I T	1.54	282.8	Ė	LT	0.42	17.4	C	LT	0.83	32.3	D
	OB		1.00	70.0			1.20	100.0	•			& Main S		gnalized)	1.04	202.0	•		0.02	17.4			0.00	02.0	
	EB	TR	_	_	T - T	TR		_	_	TR	-	- William 5		TR	_	_	1 _ 1	TR	_	_	_	TR	_	T -	Т_
Main St	WB	LT	0.02	7.6	A	LT	0.02	7.6	A	LT	_	0.0	A	LT	_	0.0	Α	LT	0.03	7.5	A	LT	0.03	7.5	Α
Hawthorne Ave	NB	LR	0.07	10.1	В	LR	0.02	10.2	В	LR	0.21	14.5	В	LR	0.22	15.0	C	LR	0.07	10.1	В	LR	0.07	10.2	В
Market Pl	SB	LR	0.02	9.8	A	LR	0.02	9.9	В	LR	0.04	11.7	В	LR	0.04	11.9	В	LR	0.02	9.9	A	LR	0.02	10.1	В
		,	0.02	0.0			0.02	0.0						signalized)	0.0				0.02	0.0		,	0.02		
Hudson St	EB	LT	_	_	1 - 1	LT	<u> </u>	_	-	LT	-	-	- -	LT	_	_	1 - 1	LT	_	_	—	LT	<u> </u>		1_
	NB	LTR	0.18	10.4	В	LTR	0.20	11.1	В	LTR	0.32	12.0	В	LTR	0.39	13.4	В	LTR	0.16	9.7	Α	LTR	0.21	10.5	В
Hawthorne Ave	SB	LR	0.08	9.8	A	LR	0.09	10.1	В	LR	0.15	11.7	В	LR	0.15	12.2	В	LR	0.06	9.2	Α	LR	0.06	9.3	A
										6: Warbur	ton Ave &	Ashburto	n Ave (Signalized)								· !	-	
	EB	LTR	0.59	28.4	С	LTR	0.60	29.2	С	LTR	0.66	35.3	D	LTR	0.67	35.9	D								
Ashburton Ave	\A/D	L	0.19	26.4	С	L	0.20	26.6	С	L	0.19	30.9	С	L	0.23	31.4	С								
	WB	TR	0.35	23.6	С	TR	0.36	23.8	С	TR	0.59	32.9	С	TR	0.60	32.7	С								
	NB	LTR	0.76	40.3	D	LTR	0.77	40.6	D	LTR	0.85	43.6	D	LTR	0.86	44.6	D								
Warburton Ave	SB	L	0.32	16.2	В	L	0.33	15.9	В	L	0.30	13.1	В	L	0.31	13.1	В								
	28	TR	0.33	16.5	В	TR	0.34	16.2	В	TR	0.33	13.6	В	TR	0.35	13.7	В								
		Interse	ection	27.1	С	Inters	ection	27.3	С	Interse	ection	31.4	С	Interse	ection	31.7	С								
										7: Wark	ourton Ave	& Wells A	ve (Si	gnalized)											
	EB	LTR	0.07	0.6	Α	LTR	0.07	0.6	Α	LTR	0.36	12.5	В	LTR	0.36	12.6	В	LTR	0.01	0.0	Α	LTR	0.01	0.0	Α
Wells Ave	WB	L	0.71	34.2	С	L	0.72	34.5	С	L	0.78	38.4	D	L	0.81	40.8	D	L	0.64	27.1	С	L	0.67	28.4	С
		TR	0.37	25.0	С	TR	0.37	24.9	С	TR	0.17	22.1	С	TR	0.17	22.2	С	TR	0.11	17.8	В	TR	0.11	17.8	В
Warburton Ave	NB	T	0.47	18.9	В	T	0.48	19.3	В	Т	0.34	16.6	В	T	0.36	16.8	В	T	0.30	12.6	В	T	0.31	12.8	В
.vaibaitoii/tvc	SB	TR	0.60	21.0	С	TR	0.61	21.4	С	TR	0.54	20.0	В	TR	0.57	20.7	С	TR	0.33	12.5	В	TR	0.35	12.8	В
		Interse	ection	24.2	С	Inters	ection	24.5	С	Interse	ection	24.9	С	Interse	ection	26.0	С	Interse	ection	17.9	В	Inters	ection	18.5	В

Table 11-23 (cont'd)
2032 No Action and 2032 With Action Conditions
Level of Service Analysis

				V	Veekday AM				I			Week	day PN	П						Satu	ırday	Level of Se	1 1100 1111	11,515
			2032 No A			2032 With	Action			2032 No A			iday i ii	2032 With	1 Action			2032 No A	Action	Juli	l	2032 With	Action	
Approach	1	Lane		Delay	Lane		Delav		Lane		Delay		Lane				Lane		Delay		Lane		Delay	\top
		Group	v/c Ratio	(sec)	LOS Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	Delay (sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS
							8: Ne	pperh	an St &	Warburton A	Ave & Dock	St/M	anor H	ouse Sq (Sigı										
Nepperhan St	NEB	LR	0.24	9.9	A LR	0.26	11.2	В	LR	0.41	16.9	В	LR	0.43	17.6	В	LR	0.09	1.5	Α	LR	0.13	3.0	Α
Warburton	NB	L	0.40	21.5	C L	0.41	21.9	С	L	0.67	34.8	С	L	0.71	39.1	D	L	0.18	15.5	В	L	0.20	15.8	В
Ave		TR	0.81	28.5	C TR	0.83	30.2	С	TR	0.59	18.1	В	TR	0.61	18.8	В	TR	0.43	15.2	В	TR	0.45	15.6	В
7.00	SB	LTR	0.86	39.6	D LTR	0.93	69.4	Е	LTR	0.90	56.5	Е	LTR	1.01	86.7	F	LTR	0.71	25.2	С		0.75	28.7	С
		Inte	rsection	32.8	C In	tersection	49.0	D		ersection	41.0	D		ersection	58.7	Е	Inte	rsection	20.9	С	Int	ersection	23.3	С
				I		T	T			lale Ave/War					-		T 1		1		T		1	
	EB	LTR	0.39	12.5	B LTR	0.43	16.0		LTR	0.88	52.2	D	LTR	0.92	58.4	Е	LTR	0.31	10.5		LTR	0.32	11.2	В
Main St	WB	L	0.27	28.7	C L	0.27	28.8	С		0.79	50.5	D	_ <u>L</u>	0.79	50.1	D	L L	0.69	40.6	D	L	0.69	41.0	D
		TR	0.36	19.1	B TR	0.38	19.0	В	TR	0.64	29.1	С	TR	0.65	29.2	C	TR	0.35	20.2	С	1	0.36	19.9	В
Riverdale Ave	NB	L T	0.09	11.1	B L	0.09	11.1	В	느	0.11	12.5	В	L	0.12	12.8	В	L T	0.03	10.2	В	L	0.03	10.3	В
Warburton		- 1	0.26	12.1	B T	0.26	12.1	В	T	0.15	10.7	В	T	0.15	10.8	В		0.13	10.4	В	l I	0.13	10.5	В
Ave	SB	TR	0.63	29.2	C TR	0.64	29.4	С	TR	0.74	33.4	С	TR	0.78	34.9	С	TR	0.57	28.1	С	TR	0.60	28.6	С
		Inte	rsection	21.0	C In	tersection	21.4	С		ersection	34.0	С		ersection	35.5	D	Inte	rsection	24.9	С	Int	ersection	25.2	С
					 			1 _		Riverdale A							1			T _			T	
Hudson St	EB	LTR	0.57	37.4	D LTR	0.71	43.9		LTR	0.85	54.0	D	LTR	0.88	57.2	E	LTR	0.49	44.3		LTR	0.62	50.1	D
.	NB	TR	0.45	20.1	C TR	0.47	21.3	С	TR	0.45	15.6	В	TR	0.49	18.1	В	TR	0.26	6.5	Α	TR	0.30	7.5	Α
Riverdale Ave	SB	L	0.31	11.4	B L	0.33	13.0	В	L	0.57	24.8	С	L T	0.63	27.6	<u>C</u>	L	0.32	6.5	A	L	0.37	9.0	A
	ļ	Into	0.36	10.3 17.4		0.37	11.2 20.1	B	l	0.38 ersection	11.4 22.1	B C	ı	0.40 ersection	12.3 24.8	B C	ı	0.24	3.9 7.9	A	l	0.26	5.5 11.2	A B
		inte	rsection	17.4	Б П	tersection	20.1	U		Riverdale A					24.0	C	me	rsection	7.9	А	IIII	ersection	11.2	<u> </u>
	EB	LTR	0.58	34.0	C LTR	0.70	37.8	П	LTR	0.88	50.4	D	LTR	0.97	64.0	Е	LTR	0.34	29.6	С	LTR	0.42	31.0	С
		LIIX	1.20	125.9	F	1.51	260.5	F	LIIN	1.34	189.6	F	LIK	B	313.8	Ē	LIIN	0.54	8.8	A	LIIX	0.42	13.7	В
Prospect St	WB	T	0.66	11.0	B T	0.70	11.3	В	T	0.93	28.4	С	Ť	1.08	71.3	Ė	T	0.49	6.9	A	T	0.63	8.6	A
	***	R	0.24	1.4	AR	0.24	1.3	A	R	0.33	0.9	A	R	0.21	0.8	A	R	0.18	0.5	A	R	0.18	0.5	A
		i	0.56	46.4	D L	0.57	46.3	D	ı.\	0.65	55.4	E	L	0.67	57.0	E	i	0.30	31.6	C	L	0.31	31.8	C
	NB	TR	1.27dr	104.3	F TR	1.31dr	109.0	F	TR	0.99dr	85.7	F	TR	1.09dr	110.7	F	TR	0.89dr	39.7	D		0.98dr	43.0	D
Riverdale Ave		L	0.82	146.0	F L	0.84	173.9	F	L	1.00	157.6	F	L	1.05	169.2	F	L	0.92	123.8	F	L	0.94	128.4	F
	SB	Т	0.50	33.9	C T	0.50	33.9	С	Т	0.51	34.0	С	Т	0.51	34.0	С	Т	0.31	30.8	С	Т	0.31	30.8	С
		R	0.08	0.3	A R	0.08	0.3	Α	R	0.09	0.3	Α	R	0.09	0.3	Α	R	0.05	0.2	Α	R	0.05	0.2	Α
		Inte	rsection	65.1	E In	tersection	87.7	F	Inte	ersection	71.9	Е	Int	ersection	104.5	F	Inte	rsection	35.1	D	Int	ersection	35.7	D
										2: Riverdale		St (S												
Vark St	EB	LTR	0.43	28.6	C LTR	0.43	28.6		LTR	0.70	55.9	Е	LTR	0.70	55.9	Е								
vaik Ot	WB	LTR	0.16	19.2	B LTR	0.16	19.2	В	LTR	0.52	42.0		LTR	0.52	42.0	D								
	NB	L	0.13	22.7	C L	0.13	22.7	С	L	0.11	12.4	В	L	0.12	12.8	В								
Riverdale Ave		TR	0.91	49.6	D TR	0.93	53.0	D	TR	0.50	20.5	С	TR	0.54	21.2	С								
	SB	L	0.66	50.2	D L	0.66	46.3	D	L	0.51	19.8	В	L	0.54	19.8	В								
		TR	0.72	29.7	C TR	0.78	31.1	C	TR	0.52	14.0	В	TR	0.56	14.2	В								
		Inte	rsection	37.9	D In	tersection	39.5	D	Inte	ersection	22.0	С	Int	ersection	22.1	С								

Table 11-23 (cont'd)
2032 No Action and 2032 With Action Conditions
Level of Service Analysis

																						Level of Se	rvice Ana	aiysis
				Veekday								Wee	kday Pl							Satı	ırday			
Approach		2032 No A				2032 With		1		2032 No				2032 With	n Action	1	,	2032 No A		-		2032 With		
7.66.000	Lane	/a Datia	Delay		ne	/- D-4:-	Delay		Lane	uda Dadia	Delay		Lane	uda Datia	D-1 ()		Lane	/- D-4:-	Delay		Lane	uda Dadia	Delay	
	Group	v/c Ratio	(sec)	LOS Gr	oup v/c	c Ratio	(sec)	LU3	Group	v/c Ratio	(sec)		Group		Delay (sec)	LU3	Group	v/c Ratio	(sec)	LU3	Group	v/c Ratio	(sec)	LOS
I ED	LTR	0.71	24.9	C L	ъ 1	0.72	25.3	С	LTR	orth Broadw 0.80	28.1	_	LTR	0.82	29.9									
Ashburton Ave EB	LTR	0.71	20.8	CL		0.72	22.7		LTR	0.89	29.8	C	LTR	0.62	42.7	C D							—	
NB		0.87	39.9	D L		0.72	41.2	D	LTR	0.89	40.4	D	LTR	0.97	41.1	D							\vdash	
North Broadway	LIN	0.71	24.5	C		0.74	26.4	C	LIK	0.75	24.0	C	LIK	0.75	27.8	С								
SB	TR	0.30	19.8			0.42	19.8	В	TR	0.33	17.2	В	TR	0.03	17.2	В								
100	_	ersection	26.1	C	Intersec		27.1	C		ersection	29.3	C		tersection	34.2	С	l							
	IIIC	rocotion	20.1		IIICIGC	,000011	21.1					_		Jnsignalized)	04.2	U								
Manor House	Ι.									·			-	· ·			I . I			1_	1 .			1_
Sq EB	L	0.41	15.9	С	- '	0.44	16.4	С	L	0.43	15.7	С	L	0.45	16.1	С		0.18	11.8	В	L	0.19	12.0	В
North Broadway NB	Т	-	-	-		-	-	-	Т	-	-	-	Т	-	-	-	Т	-	-	-	Т	-	-	-
				•	•			15a: N	lorth B	roadway & N	/lain St (Get	ty Sq	uare We	est) (Signalize	ed)									
Main St WE		0.31	0.6			0.33	0.6	Α	TR	0.52	2.1	Α	TR	0.54	1.7	Α	TR	0.45	2.8	Α	TR	0.46	2.6	Α
North Broadway NB	LT	0.46	21.5	CL	T	0.46	21.5	С	LT	0.51	23.0	С	LT	0.51	23.0	С	LT	0.37	19.0	В	LT	0.37	19.0	В
`	Inte	ersection	11.9	В	Intersec	ection	11.6	В		ersection	11.1	В		tersection	10.8	В	Inte	rsection	9.1	Α	In	tersection	8.9	Α
								_						ast) (Signaliz										
New Main St WE		0.56	25.7			0.63	27.5	С	TR	0.74	31.2	С	TR	0.85	38.2	D	TR	0.58	26.0	С		0.69	29.4	С
Palisade Ave NB	_	0.22	23.2	C L		0.28	24.1	С	LT	0.65	31.9	С	LT	0.75	36.1	D	LT	0.40	25.7	С		0.49	27.5	С
	Inte	ersection	25.2	С	Intersec	ection	26.6	С		ersection	31.5	С		tersection	37.3	D	Inte	rsection	25.9	С	In	tersection	28.7	С
	T											/ & H	udson S	t (Unsignaliz			1					1		-
Hudson St EB	LR	0.54	13.3	B L	R	0.65	16.1	С	LR	1.08	85.5	F	LR	1.21	135.8	F	LR	0.61	15.1	С	LR	0.73	20.0	С
South NB	Т	0.43	11.8	В	r (0.46	12.7	В	Т	0.91	38.2	Е	Т	1.00	48.6	Е	Т	0.48	12.8	В	Т	0.55	14.6	В
Broadway								17: 6:	outh Di	roodway ⁹ F	roopoot St/	Nonn	orbon A	ve (Signalize	4/									
	T 1	0.20	23.0	С		0.20	25.3	C	1 1	0.21	26.0	C	lerrian A	0.21	26.3	С		0.22	20.6	С		0.24	20.8	С
Prospect St EB	TR	0.20	82.3	F 1		1.00	89.6	F	TR	0.21	89.9	F	TR	1.05	87.8	F	TR	0.76	38.2	D	TR	0.83	65.5	E
	1	0.82	30.8	С		1.08	84.8	F	1	1.04	80.1	F	111	1.21	138.6	F	1	0.19	6.9	A	1	0.35	10.8	В
Nepperhan Ave WE	T	0.68	11.8			0.74	12.6	В	T	0.78	15.5	В	T	0.89	24.8	С	T	0.49	6.9	A	T	0.60	8.5	A
	R	0.16	1.6			0.16	1.3	A	R	0.35	1.7	A	R	0.36	1.6	A	R	0.11	0.5	A	R	0.11	0.9	Α
	1	0.34	32.3	С		0.35	32.4	C	L	0.34	32.2	C	L	0.38	33.1	С	L	0.13	28.7	C	L	0.16	29.0	C
South	TR	0.76	45.8			0.83	51.7	D	TR	0.94	66.8	Е	TR	1.11	110.5	F	TR	0.48	34.6	С	TR	0.64	39.5	D
Broadway SB	L	0.35	35.6	D	_	0.69	57.0	Е	L	1.60	335.2	F	L	3.37	1117.7	F	L	0.33	33.3	С	L	0.57	43.9	D
36	TR	0.24	24.7	C 1	R	0.24	24.6	С	TR	0.29	20.2	С	TR	0.29	20.2	С	TR	0.18	24.3	С	TR	0.18	24.1	С
	Inte	ersection	41.4	D	Intersec	ection	51.4	D	Int	ersection	66.3	Е	Int	ersection	121.5	F	Inte	rsection	24.5	С	In	tersection	36.0	D
								18	3: Soutl	n Broadway	& Vark St/P	ark F	lill Ave	(Signalized)										
Vark St EB		0.74	44.0	D L		0.75	45.6		LTR	0.75	45.8	_	LTR	0.75		D								
Park Hill Ave WE		0.76	47.4			0.78	49.3		LTR	0.75	48.5	D	LTR	0.75	49.0	D								
South NB		0.67	27.8			0.71	29.2		LTR	0.95	53.2	D	LTR	1.08	87.5	F								
Broadway SB	_	0.84	38.4			0.94	50.1		LTR	0.81	36.0		LTR		47.3	D								
	Inte	ersection	38.3	D	Intersec	ection	43.5	D		ersection	46.0	D		ersection	62.5	Е								
	T	1		1 . 1	_		1							Unsignalized)		1 -	· '			T -	T	T		T _
Locust Hill Ave EB		0.34	9.5	A L		0.47	11.7	_	LTR	0.64	14.5	_	LTR	0.90	33.4	D		0.38	9.7		LTR	0.59	13.9	B
Palisade Ave NB	TR	0.09	8.0			0.11	8.6	A	TR	0.11	8.7	A	TR	0.14	9.8	Α	TR	0.08	7.9	A		0.11	8.7	A
Palisade Ave SB	LT	0.16	8.7	AL	1 '	0.32	10.5	В	LT	0.12	9.1	Α	LT	0.33	11.9	В	LT	0.10	8.4	Α	LT	0.25	10.1	В

Table 11-23 (cont'd)
2032 No Action and 2032 With Action Conditions
Level of Service Analysis

				V	Veekd	ay AM							Weekda	ay PN	Λ						Satı	ırday	LCVCI OI SC	. , , , , , , , , , , , , , , , , , , ,	
Annroach		2032 No Action					2032 With	Action			2032 No	Action			2032 With	Action			2032 No A	Action			2032 With	Action	
Approach		Lane		Delay		Lane		Delay		Lane		Delay	L	ane				Lane		Delay		Lane		Delay	
		Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS G	roup	v/c Ratio	Delay (sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS
										20: ا	New Main St	& Nepperh	an Ave	(Sign	alized)										
	ЕВ	L		6.6	Α	L	0.09	6.6	Α	L	0.05	6.2	Α	L	0.19	9.6	Α	L	0.06	4.5	Α	L	0.18	6.3	Α
	CD	TR	0.58	13.0	В	TR	0.64	14.7	В	TR	0.82	15.9	В	TR	0.89	19.2	В	TR	0.52	8.4	Α	TR	0.59	11.6	В
Nepperhan Ave		L	0.61	38.6	D	L	0.72	47.0	D	L	1.18	142.5	F	L	1.25	161.7	F	L	0.84	59.1	Е	L	0.98	80.6	F
	WB	Т	0.68	11.8	В	Т	0.76	16.2	В	Т	0.71	15.3		T	0.82	21.7	С	Т	0.42	8.6	Α	Т	0.52	11.7	В
		R	0.47	12.9	В	R	0.54	16.7	В	R	0.44	15.7		R	0.56	19.7	В	R	0.45	10.3	В	R	0.56	14.2	В
New Main St	NB	LTR	1.06	93.3	F	LTR	1.08	99.2	F	LTR	1.11	108.0		LTR	1.16	127.5	F	LTR	0.85	51.9	D	LTR	0.90	57.9	Е
		Inte	rsection	26.8	С	Int	ersection	29.8	С		ersection	39.8	D		ersection	46.1	D	Inte	ersection	21.3	С	Int	ersection	25.6	С
										21: I	Palisade Ave		on Ave	(Sign											
		L	0.21	6.8	Α	L	0.22	7.1	Α	L	0.16	6.8	Α	L	0.17	7.1	Α								
	EB	Т	0.67	21.4	С	Т	0.69	22.4	С	Т	0.75	24.5	С	Т	0.77	26.3	С								
Ashburton Ave		R	0.15	6.0	Α	R	0.15	6.3	Α	R	0.16	7.2	Α	R	0.18	7.6	Α								
	WB	L	0.41	6.5	Α	L	0.45	8.2	Α	L	0.59	17.6	В	L	0.67	22.6	С								
	***	TR	0.68	10.3	В	TR	0.70	10.8	В	TR	0.77	10.2		TR	0.81	12.4	В								
Palisade Ave	NB	LT	0.70	44.9	D	LT	0.73	46.2	D	LT	0.59	40.5		LT	0.62	41.5	D								Į.
T dilloddo 7 tV o		R	0.28	9.6	Α	R	0.30	10.3	В	R	0.23	7.8	Α	R	0.25	7.5	Α								
		Inte	rsection	17.5	В	Int	ersection	18.5	В		ersection	18.5	В		ersection	20.4	С								
					1		T				New School					,				T			1		
Palisade Ave	EB		0.37	17.5	В	LTR	0.55	20.3	С	LTR	0.62	22.5		LTR	0.81	30.5	С		0.43	18.1	_	LTR	0.59	21.0	С
New School St		TR	0.37	12.8	В	TR	0.51	13.9	В	TR	0.24	10.8		TR	0.36	10.7	В	TR	0.19	8.4	Α	TR	0.30	7.7	Α
Palisade Ave	SB	LT	0.56	18.5	В	LT	0.76	29.2	С	LT	0.52	16.6		LT	0.60	18.5	В	LT	0.23	12.5	В	LT	0.29	13.3	В
		Inte	ersection	16.1	В	Int	ersection	20.3	С		ersection	17.7	В		ersection	21.5	С	Inte	ersection	13.9	В	In	ersection	14.9	В
										New Sc				n Av	e (Signalized)		1			1			· · · · · · · · · · · · · · · · · · ·		
	EB	<u>_</u> _	0.40	28.4	С	L	0.48	34.0	С	<u> </u>	0.28	22.8	С		0.50	35.7	D	<u>L</u>	0.30	18.1	В	<u>L</u>	0.52	38.5	D
I	<u> </u>	TR	0.66	18.0	В	TR	0.71	18.3	В	TR	0.87	18.0		TR	0.91	21.1	С	TR	0.55	13.6	В	TR	0.59	13.9	В
Nepperhan Ave		<u> </u>	0.16	11.4	В	L	0.17	11.8	В	L +	0.36	16.6	В	L T	0.37	17.3	В	L +	0.18	11.4	В	L +	0.18	11.6	В
	WB	T	0.84	34.5	C	T	0.87	35.9	D		0.86	35.4	D	T	0.94	43.1	D	T	0.63	28.2	C		0.70	29.8	С
N 0 1 101	0.0	R	0.31	4.8	A	R	0.39	5.2	A	R	0.22	5.3		R	0.44	6.4	A	R	0.18	4.4	A	R	0.41	3.9	A
New School St	SB	LTR	0.25	25.3	С	LTR	0.68	35.3	D	LTR	0.55	28.7		LTR	0.89	48.6	D	LTR	0.23	11.0	В	LTR	0.58	28.6	С
		inte	rsection	25.6	С	int	ersection	27.2	С		ersection	25.8	C		ersection	32.2	С	inte	ersection	19.9	В	l in	ersection	22.0	С
	LED	TO			1	T0					Vaverly St &	Neppernan				1	ı	T-0	1			T-0			
Nonnorber A	EB	TR	-	-	-	TR	-	-	-	TR	- 0.01	- 27.4		TR	0.01	- 20.0	- D	TR	- 0.00	14.0	- В	TR	0.00	16.7	-
Nepperhan Ave	WB		-	0.0	Α	L T		0.0	Α	T L	0.01	27.1	D	T		30.9	+ = -	<u> </u>	0.00	14.9		<u> </u>	0.00	16.7	С
Waverly St	NB	LR	1.21	185.4	-	I	1.71	403.4	-	LR	1.78	450.2	- F	I R	3.47	1235.8	- E	LR	0.60	34.6	- D	ID	1.09	143.9	-
waveny St	IND	LK	1.21	100.4	Г	LK	1.71	403.4	г	LK	1.70	430.2	Г	LK	3.47	1235.0	Г	LK	0.00	34.0	L	LK	1.09	143.9	

Table 11-23 (cont'd)
2032 No Action and 2032 With Action Conditions
Level of Service Analysis

				٧	Veekd	ay AM							Week	day PN	1						Satu	rday	Level of Sei	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
Ammusash			2032 No A				2032 With	Action			2032 No			,	2032 Witl	h Action			2032 No A	Action		.	2032 With	Action	
Approach		Lane		Delay		Lane		Delay		Lane		Delay		Lane				Lane		Delay		Lane		Delay	
		Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group				Group	v/c Ratio	Delay (sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS
				_		_				25: N	epperhan A		ton A	ve (Sig						1					
		L	0.66 1.01	77.3	E	L	0.79	89.0	F	L	0.71	85.2	F	L	0.83	96.0	F								4
	EB	T	63.8	E		1.02	64.6	E	T	1.06	73.3	E	T	1.06	74.7	E								4	
Ashburton Ave		R	0.07	0.8	Α	R	0.07	0.7	A	R	0.05	0.2	Α	R	0.05	0.2	Α								4
	WB	L	0.56	59.2	E	L	0.56	59.2	E	L	0.79	77.6	Е	L	0.79	77.6	E								4
		TR	1.04	86.4	F	TR	1.07	94.4	F	TR	1.26	164.3	F	TR	1.31	183.3	F								4
	NB	L	0.60	61.4	E	L	0.60	61.4	E	L	0.53	56.8	E	L	0.53	56.8	E								
NI		TR	0.67	32.6	пО	TR	0.71	34.0	C	TR	0.48	31.6	С	TR	0.52	32.4	C								4
Nepperhan Ave	OD.	L	1.73	410.1		T	1.73	410.1		T	0.93	101.8	F	T	0.93	101.8									4
	SB	l D	0.36 0.16	30.5 4.2	C	R	0.37 0.18	30.6	C	R	0.36 0.16	30.4	С	R	0.39 0.21	30.8 4.2	C								
		R		4.2 69.4	A E		ersection	4.0 70.9	A E		ersection	4.0 81.2	A F		ersection	85.1	A F								4
		inte	rsection	09.4	E	int	ersection	70.9	E		Nepperhan					85.1	Г								
	EB	т	0.62	27.4	С	т	0.66	28.4	С		0.40	23.6	C	(Signa	0.44	24.0	С	1							
Nepperhan Ave	WB	T	0.02	23.4	C	T	0.00	23.5	C	T	0.40	23.6	С	T	0.44	24.0	C								
Copcutt Ln	SB	LR	0.34	27.7	C	LR	0.39	27.7	C	LR	0.42	23.6	С	LR	0.44	23.6	С								
Copcuit Lii	35		rsection	25.9	С		ersection	26.6	C		ersection	23.6	С		ersection	24.0	C								4
		IIILO	1 Section	20.9	U	1110	.ersection	20.0			7: Nepperha					24.0	U								
		1	0.75	50.2	D	- 1	1.33	200.1	F		0.87	63.5	E	Ignanz	1.37	215.8	F		0.64	40.7	D	- 1	1.11	119.7	F
Elm St	EB	TR	0.30	29.0	С	TR	0.32	29.4	С	TR	0.27	27.7	С	TR	0.28	28.2	С	TR	0.15	21.7	C	TR	0.17	23.2	С
Liiii Ot	WB	LR	1.41	227.2	F	LR	1.48	255.5	F	LR	0.67	31.0	C	LR	0.77	37.9	D	LR	0.49	21.1	C		0.57	25.4	C
	NB	TR	0.83	35.2	D	TR	0.91	40.1	D	TR	1.02	59.2	E	TR	1.09	83.9	F	TR	0.70	31.0	C		0.75	32.5	C
Nepperhan Ave		L	0.42	18.6	В	L	0.42	19.1	В	L	0.43	19.5	В	L	0.43	19.6	В	L	0.20	12.2	В	L	0.22	12.3	В
	SB	T		15.3	В	T	0.56	15.8	В	T	0.60	16.2	В	T	0.70	18.5	В	T	0.45	14.3	В	T	0.55	15.7	В
		Inte		55.3	Е	Int	ersection	73.2	E	Int	ersection	38.8	D	Inte	ersection	62.8	Е	Inte	ersection	23.4	C	Int	ersection	33.1	C
					ı			_			8: Walnut St			Signaliz	zed)					_			<u> </u>		
		L	0.40	13.3	В	L	0.44	14.9	В	L	0.81	42.4	D	L	0.94	76.1	Е								
V A	EB	TR	0.50	18.9	В	TR	0.65	21.8	С	TR	0.83	29.3	С	TR	0.96	41.5	D								
Yonkers Ave	MD	L	0.28	18.0	В	L	0.37	18.0	В	L	0.68	34.4	С	L	0.68	30.8	С								
	WB	TR	0.65	38.5	D	TR	0.70	38.5	D	TR	0.70	40.5	D	TR	0.85	43.4	D								
Walnut St	NB	LTR	1.33	196.7	F	LTR	1.33	196.7	F	LTR	0.61	37.4	D	LTR	0.61	37.4	D								
wallut St	SB	LTR	1.84	421.6	F	LTR	1.84	421.6	F	LTR	0.75	51.5	D	LTR	0.75	51.5	D								
		Inte	rsection	108.7	F	Int	ersection	101.3	F	Int	ersection	36.4	D	Int	ersection	43.9	D								
										29	e: Prescott S	t & Yonkers	Ave (Signali	zed)										
	EB	L	0.04	13.0	В	L	0.05	10.6	В	L	0.28	9.2	Α	Ĺ	0.45	13.4	В								
Yonkers Ave	⊏D	TR	T 0.53 1 Intersection 5 L 0.40 1 TR 0.50 1 L 0.28 1 TR 0.65 3 LTR 1.33 19 LTR 1.84 42 Intersection 10 L 0.04 1 TR 0.077 2		С	TR	0.95	34.2	С	TR	0.89	20.2	С	TR	1.04	44.0	D								
I OHKEIS AVE	WB	L	0.31	22.7	С	L	0.33	30.6	С	L	0.44	39.3	D	L	0.44	35.3	D								
		TR	0.55	6.4	Α	TR	0.59	6.3	Α	TR	0.60	5.8	Α	TR	0.72	6.2	Α								
Prescott St	NB	LTR	0.86	60.8	Е	LTR	0.86	60.8	Е	LTR	0.26	36.1	D	LTR	0.26	36.1	D								
		Inte	rsection	22.7	С	Int	ersection	25.8	С	Int	ersection	15.0	В	Inte	ersection	25.7	С								

Table 11-23 (cont'd)
2032 No Action and 2032 With Action Conditions
Level of Service Analysis

				V	Veek	day AM							Weekday F	PM						Satu		Level of Sei		J ===
			2032 No A			1	2032 With	Action			2032 No		Troonady I		th Action			2032 No A	Action	Juli	raay	2032 With	Action	
Approach	1	Lane		Delay		Lane		Delay		Lane		Delay	Lane				Lane		Delay		Lane		Delay	\top
		Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio		LOS Grou	p v/c Ratio	Delay (sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS
									30	: Drive	way/Ashburt	on Ave & Y	onkers Ave	(Signalized)										
	EB	TR	0.82	10.1	В	TR	0.97	19.1	В	TR	0.76	8.0	A TR	0.89	12.9	В								
Yonkers Ave	WB	Т	0.70	27.9	С	Т	0.81	31.1	С	Т	0.79	30.3	C T	1.01	52.3	D								
		R	0.41	0.6	Α	R	0.41	0.6	Α	R	0.47	1.0	A R	0.47	1.0	Α								
Driveway	NB	LR	0.20	56.6		LR	0.20	56.6	Е	LR	0.17	55.7	E LR	0.17	55.7	Е								
Ashburton Ave	SB	L	1.12	116.8	F	L	1.12	116.8	F	L	1.00	82.6	F L	1.00	82.6	F								
7 ISTIBUTION 7 WC		LR	0.97	64.1		LR	0.97	64.1	Е	LR	0.99	81.4	F LR	0.99	81.4	F								
		Inte	rsection	32.2	С	Int	ersection	35.1	D		ersection	28.8		ntersection	37.2	D								
	1				1					31: You	nkers Ave &			<u> </u>			•		1					
	EB	L	0.91	43.4	D	L	1.20	126.6	F	L	0.73	34.3	C L	0.97	55.5	Е								
Yonkers Ave		Т	0.57	5.6	Α	Т	0.59	5.0	Α	Т	0.56	5.9	A T	0.58	5.2	Α								
TOTIKOTO 7 (VO	WB	Т	0.82	42.2	D	Т	0.84	42.7	D	Т	0.88	45.8	D T	0.92	58.6	Е								
		R	0.15	0.2	Α	R	0.15	0.2	Α	R	0.11	0.1	A R	0.11	0.1	Α								
Saw Mill NB	SB	L	0.30	46.2	D	L	0.30	46.2	D	L	0.26	45.3	D L	0.26	45.3	D								
Ramps		R	0.56	26.5	С	R	0.60	28.0	С	R	0.63	29.4	C R	0.74	34.4	С								
		Inte	rsection	24.6	С	Int	ersection	36.3	D		ersection	25.9		ntersection	33.4	С								
	1	r 1												Ln (Signalized		-			1					_
Yonkers Ave	EB	LT	0.50	3.5	Α	LT	0.52	3.5	Α	LT	0.48	1.7	A LT	0.50	1.6	Α								
	WB	TR	0.47	7.4	Α	TR	0.48	7.5	Α	TR	0.48	7.9	A TR	0.50	8.3	Α								
Fox Terrace	SB	LR	0.00	0.0	Α	LR	0.00	0.0	Α	LR	0.00	0.0	A LR	0.00	0.0	Α								
Wasylenko Ln	SWB		0.02	42.0	D	LR	0.02	42.0	D	LR	0.22	52.9	D LR	0.22	52.9	D								
		Inte	rsection	5.4	Α	Int	ersection	5.5	Α		ersection	5.1		ntersection	5.3	Α								
	1									33: Yo	nkers Ave 8		1 1 1		1				1	_		- I		_
Midland Ave	WB	L L	0.43	15.9	В	L L	0.43	15.8	В	L	0.37	14.3	B L	0.38	13.9	В								
	1	R	0.42	6.1	Α	R	0.43	6.2	Α	R	0.40	6.1	A R	0.42	6.1	Α								
	NB		0.78	27.3	С	T	0.79	27.6	С	T	0.80	28.4	C T	0.83	29.6	С				-				
Yonkers Ave		R	0.18	6.0	Α	R	0.18	6.1	Α	R	0.24	6.4	A R	0.24	6.6	Α								
	SB	L	1.15	134.0	F	L	1.28	181.7	F	L	1.05	103.8	F L	1.17	141.2	F								4
			0.62	18.0	В	T	0.63	19.1	В	T	0.62	16.0	B T	0.63	16.2	В				-				4
		Inte	rsection	30.9	С	Int	ersection	37.2	D		ersection	26.5		ntersection	30.9	С								
	_	· - ·	Т		1	T -			kers A	ve and	Saw Mill Riv	er Parkway	Southbou	nd Ramps (Ur										_
Vanks - A	EB		-	-	-		-	-	-	I	-	-	- [-	-	-								
Yonkers Ave		R	-	-	-	R	-	-	-	R	-	-	- R	-	-	-								4
Cow Mill CD	WB	Т	-	-	-	T	-	-	-	Т	-	-	- T	-	-	-								
Saw Mill SB Off-Ramp	SB	R	0.75	44.2	Е	R	0.94	76.4	F	R	1.25	176.1	F R	1.93	464.7	F								
	_	1	0.0-	70.0	1 -					and Cro					st) (Signalized)									
	EB	L	0.95	72.9	E		0.96	75.6	E	L	0.67	42.8	D L	0.70	44.6	ם נ								
Midland Ave		TR	0.19	9.3	A	TR	0.19	9.3	A	TR	0.16	11.5	B TR	0.16	11.5	В								
	WB	LTR	0.12	15.4	В	LTR	0.12	15.4		LTR	0.14	12.2	B LTR		12.2	В								
	NB	<u>L</u>	0.25	12.2	В	L	0.25	12.2	В	L	0.28	12.7	B L	0.28	12.7	В								
Yonkers Ave	<u> </u>	TR	0.45	13.8	В	TR	0.46	13.9	В	TR	0.49	14.0	B TR	0.50	14.2	В								
	SB	L	0.08	17.8	В	L	0.08	16.6	В	L	0.23	19.1	B L	0.24	18.0	В								
	1 -	TR	1.09	78.0	E	TR	1.12	88.5	F	TR	1.05	62.1	E TR		70.6	E								
		Inte	rsection	51.4	D	Int	ersection	57.0	E	Inte	ersection	38.8	D I	ntersection	43.1	D								

Table 11-23 (cont'd) 2032 No Action and 2032 With Action Conditions **Level of Service Analysis**

				1	Weekd	ay AM							Week	day PN	Л						Satu	rday			
Annroach			2032 No A	ction			2032 With	Action			2032 No	Action			2032 Witl	n Action			2032 No A	ction		_	2032 With	Action	
Approach	l	Lane		Delay		Lane		Delay		Lane		Delay		Lane				Lane		Delay		Lane		Delay	
		Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	Delay (sec)	LOS	Group	v/c Ratio	(sec)	LOS	Group	v/c Ratio	(sec)	LOS
										36: H	lawthorne Av	/e & Prosp	ect St	(Unsig	nalized)										
	EB	LTR	0.41	12.1	В	LTR	0.47	12.8	В	LTR	0.66	16.8	С	LTR	0.82	24.2	С	LTR	0.25	10.5	В	LTR	0.30	11.0	В
Prospect St	WB	L	0.42	23.6	С	L	0.53	33.1	D	L	1.14	159.1	F	L	1.50	313.3	F	L	0.24	15.7	С	L	0.28	18.2	С
	VVD	TR	0.59	14.2	В	TR	0.63	15.0	С	TR	0.77	19.9	С	TR	0.92	34.6	D	TR	0.51	12.8	В	TR	0.65	15.8	С
										37: Lo	cust Hill Ave	& Ashburt	on Av		gnalized)			-	_			-	_		
Ashburton Ave	EB	TR	-	-	-	TR	-	-	-	TR	-	-	-	TR	-	-	-								
	WB	LT	0.06	9.3	Α	LT	0.07	9.4	Α	LT	0.09	9.6	Α	LT	0.11	9.8	Α								
Locust Hill Ave	NB	LR	0.29	20.7	С	LR	0.45	27.4	D	LR	0.28	29.1	D	LR	0.53	45.7	Е								
									,	38: Pali	sade Ave & l	_afayette P	I/Wals	h Rd (S	Signalized)										
Lafayette Pl	EB	LTR	0.10	7.4	Α	LTR	0.10	7.4	Α	LTR	0.06	6.9	Α	LTR	0.06	6.9	Α								
Walsh Rd	WB	LTR	0.10	5.4	Α	LTR	0.10	5.4	Α	LTR	0.15	6.5	Α	LTR	0.15	6.5	Α								
Palisade Ave	NB	TR	0.47	13.0	В	TR	0.51	13.6	В	TR	0.44	12.4	В	TR	0.47	12.9	В								
ralisaue Ave	SB	LT	0.47	13.6	В	LT	0.50	14.0	В	LT	0.58	15.4	В	LT	0.64	16.8	В								
		Inte	rsection	12.2	В	Inte	ersection	12.6	В	Int	ersection	12.9	В	Int	ersection	13.9	В								
									50	: Locus	t Hill Avenue	& Overloo	k Ter	race (U	nsignalized)										
Overlook Terr	EB	LR	-	0.0	Α	LR	0.15	10.1	В	LR	-	0.0	Α	LR	0.17	10.3	В	LR	-	0.0	Α	LR	0.13	9.9	Α
Locust Hill Ave	NB	LT	-	0.0	Α	LT	0.04	7.6	Α	LT	-	0.0	Α	LT	0.07	7.6	Α	LT	-	0.0	Α	LT	0.07	7.6	Α
Locust Till Ave	SB	TR	-	-	-	TR	-	-	-	TR	-	-	-	TR	-	-	-	TR	-	-	-	TR	-	-	-
									Buen	a Vista	Avenue & Te	utonia Hal	I Sout	h Drive	way (Unsigna					•					
South Driveway						LR	0.26	15.8	С					LR	0.29	20.8	С					LR	0.19	13.4	В
Buena Vista	NB					LT	0.03	8.6	Α					LT	0.12	9.6	Α					LT	0.09	8.4	Α
Avenue	SB					TR	-	-	-					TR	-	-	-					TR	-	-	-
										130: Ce	entre Street &	& Palisade	Avenu	ıe (Uns	ignalized)					•					
Palisade	EB					TR								TR			-					TR			
Avenue							-		ļ <u>.</u>						_	-							_	-	<u> </u>
Centre Street	NB					R	0.01	10.5	В					R	0.01	12.2	В					R	0.01	10.7	В

Notes:
EB = Eastbound, WB = Westbound, NB = Northbound, SB = Southbound, NEB = Northeastbound, SWB = Southwestbound v/c = volume to capacity, LOS = Level of Service
L = Left Turn, T = Through, R = Right Turn, dr = defacto right- turn lane
= LOS E, = LOS F

Red bold text = Impact

Table 11-29 2032 No Action, 2032 With Action, and 2032 Mitigation Conditions Level of Service Analysis

																																	<u> Jevel</u>	of Serv	vice .	Analysi
							Veekda												Weekda												Saturd					
		20)32 No					h Actio			32 Miti					Action			032 With					igation				Action			2 With					gation
_		Lane		Delay		Lane		Delay		Lane		Delay		Lane		Delay		Lane		Delay		Lane		Delay				Delay			v/c [Lane v		
Approac	ch	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS																		Ratio	(sec)	LOS Gr	oup	Ratio (sec)	LOS	roup∣R:	atio ((sec) LOS
	1		I I			1 -				3:				Prosp				and Wi	th Actio			<u>ed – M</u>				<u>d)</u>										
Prospect St	WB	L	0.27	13.4		L	0.28	13.9		L	0.40		С	<u>L</u>	0.38			L	0.40	16.9	С	L		37.1	D											
	ļ <u>.</u>	- 17	0.54	16.7		R	0.62	19.1	С	R	0.60	9.1	Α	R	0.61	19.5		R	0.89	35.9	E	R	0.70		В											
	NB	TR	0.65	19.6	С	TR	0.71	21.6	С	T		27.7	С	TR	0.86	31.4	D	TR	1.02	51.0	F	T			С											
Buena Vista								1	-	R	0.42	7.5	Α						1		-	R		14.0	В		1	1 1		-						
Ave	SB	LI	1.03	70.6	F	LT	1.25	155.0	F	<u> </u>		23.1	С	LI	1.28	168.8	<u> </u>	LT	1.54	282.8	F	<u> </u>	0.88		C								\longrightarrow			
										T	0.22	5.0	Α									T	0.16	4.8	<u>A</u>											
										Interse	ection	13.1										Inters		19.8	В											
	1		1			T	1	1	1	T	т т	8: 1	Neppe	erhan S	t & Wa	rburto	n Ave	& Do	ck St/Ma	nor Ho	ouse S	Sq (Sig	nalized	l)			1									
Nepperhan St	NEB	LR	0.24	9.9	Α	LR	0.26	11.2	В	LR	0.42	16.7	В	LR	0.41	16.9		LR	0.43	17.6	В	LR	0.83	53.3	D											
\^/	NB	L	0.40	21.5		L	0.41	21.9		L	0.29	13.6	В	L		34.8	С	L	0.71	39.1	D	L	0.48	18.1	В											
Warburton Ave		TR	0.81			TR	0.83	30.2	С	TR	0.69		В	TR	0.59			TR	0.61	18.8	В	TR	0.49		В											
Ave	SB	LTR	0.86	39.6		LTR	0.93	69.4	Е	LTR	0.77					56.5		LTR	1.01	86.7	F				D											
		Inters	ection	32.8	С	Interse	ection	49.0	D	Interse	ection	22.4	С			41.0			section	58.7	Е		ection	30.3	С											
																		rton A	ve & Ma	in St (S	Signal	ized)														
	EB													LTR	0.88	52.2		LTR	0.92	58.4	Е	LTR		51.2	D											
Main St	WB													L	0.79	50.5		L	0.79	50.1	D	L			D											
	VVD													TR	0.64	29.1		TR	0.65	29.2	С	TR			С											
Riverdale	NB													L	0.11	12.5		L	0.12	12.8	В	L	0.12		В											
Ave	IND													Т	0.15	10.7	В	Т	0.15	10.8	В	Т	0.15	11.5	В											
Warburton Ave	SB													TR	0.74	33.4	С	TR	0.78	34.9	С	TR	0.76	33.5	С											
	•						•							Inters	ection	34.0	С	Inters	section	35.5	D	Inters	ection	33.2	С		,			-						
								•	•	•	•	•	-	10:	Riverd	ale Av	enue	& Hud	son Str	et (Sic	naliz	ed)		•		-					•	•			•	
Hudson St	EB													LTR	0.85	54.0		LTR	0.88	57.2	E	LTR	0.86	53.8	D											
	NB													TR	0.45	15.6	В	TR	0.49	18.1	В	TR	0.50	18.5	В											
Riverdale														L		24.8		1	0.63	27.6	C	T T	0.63	28.3	C											
Ave	SB													T	0.38	11.4	В	T	0.40	12.3	В	Ť	0.40	12.9	В											
														Inters		22.1	C	Inters	section	24.8	C	Inters	ection	24.6	C									1		
								•											pect St		_			,												
	EB	LTR	0.58	34.0	С	LTR	0.70	37.8	D	LTR	0.73	40.3	D		0.88	50.4		LTR	0.97	64.0		LTR	0.89	48.8	D											
		L	1.20	125.9	F	L	1.51	260.5	F	L		104.8	F	L	1.34	189.6	F	L	1.63	313.8	F	L	1.20	128.3												
Prospect St	WB	T	0.66		В	T	0.70	11.3	В	T	0.62	8.5	A	T	0.93	28.4	С	T	1.08	71.3	Ē	T			C											
			0.24			R		1.3		Ŕ	0.22		Α	R		0.9		R	0.21					1.1	A											
	1	L		46.4		L	0.57			L				L		55.4		L	0.67	57.0		L		64.0												
	NB	TR	1.27dr	104.3	F	TR		109.0		Т		47.6		TR		85.7		TR	1.09dr			Т		41.4												
Riverdale										R	0.77											R		48.0								I .				
Ave		L	0.82	146.0	F	L	0.84	173.9	F	L		131.7		L	1.00	157.6	F	L	1.05	169.2	F	L		168.3												
	SB	Т		33.9		Т		33.9		Т		37.1		Т		34.0		Т	0.51	34.0		Т		38.4	D											
			0.08			R	0.08			R		0.4	Α	R	0.09			R	0.09	0.3	Α	R	0.10		Α											
	•		ection	65.1	Е	Inters	ection	87.7	F		ection	48.1		Inters		71.9	Е	Inters	section	104.5	F	Inters					,									

Table 11-29 (cont'd)
2032 No Action, 2032 With Action, and 2032 Mitigation Conditions
Level of Service Analysis

																																	Leve	i of Serv	ice Analy
							Veekda							-		-			Weekda												Satur				
		2		Action)		32 With				32 Mit		1)32 No				32 Witl					tigatio				Action				n Actio	n		Mitigation
_		Lane		Delay		Lane		Delay		Lane		Delay		Lane		Delay		Lane	v/c	Delay		Lane		Delay		Lane		Delay		Lane		Delay			c Delay
Approa	ch	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)								`				(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group Ra	tio (sec) L
		I	I				I	ı		1	1		16:						ay & Hu		t (Uns	signali:		1000		1	1	1			I				
Hudson St	EB													LR	1.08	85.5	F	LR	1.21	135.8	F	R	0.77	29.9 15.6	С										
South Broadway	NB													Т	0.91	38.2	Е	Т	1.00	48.6	Е	Т	0.90	41.6	Е										
									-				17	: Sout	n Broa	dwav 8	k Pros	pect S	t/Neppe	rhan A	ve (Si	ignaliz	ed)	· I	1										
Prospect		L	0.20	23.0	С	L	0.20	25.3	С	L	0.28	28.5	С	L	0.21			L	0.21	26.3		L		28.2	С	L	0.22	20.6	С	L	0.24	20.8	С	L 0.	23 20.8
St	EB	TR	0.93	82.3	F	TR	1.00	89.6	F	TR	0.98	76.1	Е	TR	0.97	89.9	F	TR	1.05	87.8	F	TR	1.06			TR	0.76			TR	0.83	65.5	Е		79 38.2
		L	0.82	30.8	С	L	1.08	84.8	F	L	0.99	52.9	D	L	1.04	80.1	F	L	1.21	138.6	F	L	1.07	79.2	Е	L	0.19	6.9	Α	L	0.35	10.8	В		36 8.8
Nepperhan	WB	Т	0.68	11.8	В	Т	0.74	12.6	В	Т	0.66	3.5	Α	Т	0.78	15.5	В	Т	0.89	24.8	С	Т	0.81	9.1	Α	Т	0.49	6.9	Α	Т	0.60	8.5	Α	T 0.	60 4.5
Ave		R	0.16	1.6	Α	R	0.16	1.3	Α	R	0.14	0.3	Α	R	0.35	1.7	Α	R	0.36	1.6	Α	R	0.33	8.0	Α	R	0.11	0.5	Α	R	0.11	0.9	Α	R 0.	11 0.3
		L	0.34	32.3	С	L	0.35			L	0.37			L	0.34			L	0.38	33.1	С	L	0.41			L	0.13		С	L	0.16	29.0	С		16 29.3
South	NB	TR	0.76	45.8	D	TR	0.83	51.7	D	Т	0.37	33.6	С	TR	0.94	66.8	Е	TR	1.11	110.5	F	Т	0.51			TR	0.48	34.6	С	TR	0.64	39.5	D	T 0.	
Broadway										R	0.51	37.7	D									R	0.72											R 0.	
Dioauway	SB	L	0.35	35.6		L	0.69	57.0	Е	L	0.39			L	1.60	335.2		L	3.37	1117.7	F	L	0.84			L		33.3	С	L			D		43 35.9
	OD	TR	0.24	24.7	С	TR	0.24				0.26	26.9	С	TR	0.29			TR	0.29	20.2	С	TR		21.7		TR	0.18	24.3	С	TR	0.18	24.1	С		18 24.3
		Inters	ection	41.4	D	Inters	ection	51.4	D	Interse	ection	38.5	D		ection	66.3	Е	Inters	ection	121.5	F		ection	47.4	D	Inters	ection	24.5	С	Interse	ection	36.0	D	Intersecti	on 23.2
					1				1	1	1								Park H								1						1		
Vark St	EB													LTR	0.75	45.8	D	LTR	0.75	46.2	D	LTR	0.74	45.1	D										
Park Hill Ave	WB													LTR	0.75	48.5	D	LTR	0.75	49.0	D	LTR	0.74	47.8	D										
Cauth	ND													LTR	0.95	53.2	D	LTR	1.08	87.5	F	LT	0.85	38.0	D										
South Broadway	NB		,	•	•				•						•						*	R	0.13												
Бгоацway	SB													LTR	0.81				0.91	47.3	D	LTR	0.93	50.2											
															ection	46.0			ection				ection	43.0	D										
														- 2	20: Nev	v Main	St &	Nepper	han Av	e (Sign	alized	d)													
	EB																									L	0.06	4.5	Α	L	0.18	6.3	Α	L 0.	
Nepperhan																										TR	0.52	8.4	Α	TR	0.59	11.6	В		62 22.6
Ave																											0.84	59.1		L	0.98	80.6	F	L 0.	
,	WB																									Т	0.42	8.6	Α	Т	0.52	11.7	В		55 31.6
																										R	0.45		В	R	0.56		В		59 34.6
New Main	NB																									LTR	0.85	51.9	D	LTR	0.90	57.9	Е		29 30.7
St				1				1	1							1	1			ı	I							T	I -				_	R 0.	39 17.4
																											ection	21.3	С	Interse	ection	25.6	С	Intersecti	on 30.5
						-				1	1				n Ave	(No A	ction		th Actic	n Unsi	gnaliz						1								
Nepperhan	EB	TR	-	-	-	TR	-	-	-	TR	0.46	5.5	Α	TR	-	-	-	TR	-	-	-	TR	0.58			TR	-	-	-	TR	-	-	-		36 4.0
Ave	WB	L_	-	0.0	Α	L	-	0.0	Α	L L	0.00	0.0	Α	<u>L</u>	0.01	27.1		L	0.01	30.9	D	L L	0.01	12.0		<u> </u>	0.00	14.9	В	<u> </u>	0.00	16.7	С		00 7.0
		T	-	-	-	T	-	-	-	T	0.54	5.8	A	T	-	-	-	T	-	-		T	0.59			T	-	-	-	Т	-	-			45 5.9
Waverly St	NB	LR	1.21	185.4	F	LR	1.71	403.4	F	LR	0.83	51.0		LR	1.78	450.2	F	LR	3.47	1235.8	F	LR	0.86			LR	0.60	34.6	D	LR	1.09	143.9	F	LR 0.	
										Interse	ection	8.5	Α									Inters	ection	11.1	l B									Intersecti	on 8.2

Table 11-29 (cont'd)
2032 No Action, 2032 With Action, and 2032 Mitigation Conditions
Level of Service Analysis

		I					Maakda	A B.A											Maakda	, DM											Cotu		Leve	1 01 Se	rvice	Апа	13818
		20	032 No	A otion	. 1		<u>Veekda</u> 32 With		<u> </u>	2	032 Mit	igation		20	122 No.	Action	1		<u>Neekda</u> 32 With			20	22 Mit	igation		20	122 No	Action	. 1	202	Satu	ruay h Actio	<u> </u>	20	32 Mit	iaatia	
		Lane		Delay		Lane		Delay		Lane		Delay	ı	Lane		Delay		Lane		Delay		Lane		Delay		Lane		Delay		Lane		Delay		Lane		Delay	
Appro	ach				109				109				109				109					Group			1 09				108								
Дррго	ucii	Огоир	Itatio	(300)	LUU	Стоир	Itatio	(300)	LOC	Cioup	ritatio	(300)							irton A				itatio	(300)		Group	itatio	(300)	LOO	Огоар	itatio	(300)	LOO	Cioup	Itatio	(300)	
			0.66	77.3	Е	- 1	0.79	89 N	F	1	0.58	35.2	D		0.71	85.2	F	L	0.83	96.0	F	ı	0.61	41.9	D												
	EB	Ť	1.01	63.8	$\overline{}$	Ŧ	1.02	64.6	Ē	Ť	1.00	58.4	E	Ť	1.06	73.3	Ė	Ť	1.06	74.7	Ė	Ť	1.04	67.0	E												
Ashburto	n	R	0.07	0.8	A	R	0.07	0.7	A	R	0.07	0.7		R	0.05	0.2	A	R	0.05	0.2	A	R	0.05	0.2	A												
Ave		_	0.56	59.2	E	I	0.56	59.2	E	1	0.38	21.5		ı	0.79	77.6	E	1	0.79	77.6	E	I I	0.58	28.7	C												
	WB	TR	1.04	86.4	F	TR	1.07	94.4	F	TR	1.05	87.1	F	TR	1.26	164.3	F	TR	1.31	183.3	F	TR	1.28	170.5	F												
		1		61.4	F		0.60	61.4	Е	1	0.60	61.4	E				E	L	0.53		E	L		56.8	E												
	NB	TR		32.6		TR	0.71	34.0	С	TR	0.71			TR		31.6	С	TR	0.52		С	TR			С												
Nepperha	ın	L	1.73	410.1	F	L	1.73	410.1	F	L	1.73	410.1	F	L	0.93	101.8	F	L	0.93	101.8	F	L	0.93	101.8	F												
Ave	SB	Т	0.36	30.5	С	Т	0.37	30.6	С	Т	0.37	30.6	С	Т	0.36	30.4	С	Т	0.39	30.8	С	Т	0.39	30.8	С												
		R	0.16	4.2	Α	R	0.18	4.0	Α	R	0.18	4.0	Α	R	0.16	4.0	Α	R	0.21	4.2	Α	R	0.22	4.2	Α												
	·•	Inters	ection	69.4	Е	Inters		70.9	Е	Inters	ection	65.0	Е	Inters		81.2	F	Inters		85.1	F	Interse		75.6	Е						L						
																lepperl	nan A		lm St (S		ed)																\neg
		L	0.75	50.2	D	L	1.33	200.1	F	L	0.69	32.7	С	L		63.5		L	1.37	215.8	É	L	0.95	68.8	Е	L	0.64	40.7	D	L	1.11	119.7	F	L	0.75	42.3	D
Elm St	EB	TR	0.30	29.0	С	TR	0.32	29.4	С	TR	0.24	20.3	С	TR	0.27	27.7	С	TR	0.28	28.2	С	TR	0.28	28.3	С	TR	0.15	21.7	С	TR	0.17	23.2	С	TR	0.16	22.0	С
EIIII St	WB	LTR	1.41	227.2	F	LTR	1.48	255.5	F	L	1.00	73.4	Е	LTR	0.67	31.0	С	LTR	0.77	37.9	D	L	0.77	50.0	D	LTR	0.49	21.1	С	LTR	0.57	25.4	С	L	0.55	34.5	С
	VVD									R	0.17	9.7	Α									R	0.09	10.4	В									R	0.05	4.8	Α
Nonnorha	NB	TR	0.83	35.2		TR	0.91	40.1	D	TR	0.94	38.7	D	TR	1.02		Е	TR	1.09	83.9	F	TR	0.96	47.7	D	TR		31.0		TR		32.5	С	TR	0.80	16.9	В
Nepperha Ave	SB	L		18.6		L	0.42	19.1	В	L	0.61			L	0.43	19.5	В	L	0.43	19.6	В	L	0.62	28.1	С	L		12.2	В	L	0.22		В	L		14.0	
AVC	36	Т	0.53	15.3		T	0.56	15.8	В	Т	0.70	20.4		Т	0.60	16.2	В	Т	0.70	18.5	В	Т	0.70	14.7	В	Т	0.45	14.3	В	T	0.55		В	T		17.5	
		Inters	ection	55.3	Е	Inters	ection	73.2	Е	Inters	ection	31.9	С	Inters		38.8	D	Inters		62.8	Е	Interse	ection	35.0	D	Interse	ection	23.4	С	Interse	ection	33.1	С	Interse	ection	20.2	С
	-		1				1	1	1	1	1							onker:	Ave (1			1	1		1			
	EB													L	0.81	42.4	D	L	0.94	76.1	Е	L		48.2	D												
Yonkers														TR	0.83	29.3	С	TR	0.96	41.5	D	TR	0.98	31.7	С												
Ave	WB													L		34.4	С	<u> </u>	0.68	30.8	С	L_	0.63	25.5	С												
														TR		40.5	D	TR	0.85	43.4	D	TR	0.88	39.0	D												
Walnut S	t NB													LTR	0.61	37.4		LTR	0.61	37.4	D	LTR	0.61	37.4	D												
	SB													LTR		51.5		LTR	0.75	51.5	D		0.75		D												
															ection	36.4		Inters		43.9	D	Interse	ection	36.7	D												
	1	1 1	0.01	43.4	Ь	-	4 20	126.6	-	1	0.05	51.6	<u> </u>	313	0.73	34.3		W WIIII I	NB Ram 0.97	ps (5)	gnaliz	ea) '	0.06	47.4	П		1	1									
Yonkers	EB	L	0.91	5.6	D A	T	0.59	5.0	A		0.85	6.2	D A	T	0.73	5.9	C A	T	0.58	5.2	A		0.86	7.8	D A												
Ave		<u>'</u>	0.82	42.2		<u> </u>	0.84	42.7	D	+	0.99	54.5		<u> </u>		45.8	D	+	0.92	58.6	E	T	1.00	50.0	D												1
Ave	WB	R	0.02	0.2	A	R	0.15	0.2	A	R	0.35	0.2	A	R	0.00	0.1	A	R	0.11	0.1	A	R	0.11	0.1	A												
Saw Mill		1 1	0.13	46.2	D	1	0.13	46.2	D	I	0.13	53.4		IN I	0.11	45.3	D		0.11	45.3	D	1	0.11	45.5	D												
NB Ramp		R				R				R				R				R				R															
14B Ramp	,01	Inters	ection	24.6	C	Inters	ection	36.3	D	Inters	ection	29.9	C	Inters	ection	25.9	C	Inters	ection	33.4	C	Interse	ection	29.8	С												
		IIICIS	COLIOIT	۲.0	U	111013	COLIOIT	50.0	U	Inters		20.0	U						Ave (V				JOHO11	20.0	J												-
Midland		lι	0.43	15.9	В	1	0.43	15.8	В		0.43	15.8	В	00. 1			10 0 1	maiame	1) 017.	1001) (0	Jigiiai	l Lou,															
Ave	WB	R	0.42	6.1	A	R	0.43		A	R		6.2																									
	+	Т	0.78			T	0.79	27.6		T		32.4																									
Yonkers	NB	R	0.18			R	0.18		A	R		7.6																									
		1		134.0		L	1.28	181.7	F	L		127.8																									
Ave	~ -															1					1	1					1	1				1	1				
Ave	SB		0.62 ection			Т	0.63	19.1	В	T		18.1																									

Table 11-29 (cont'd) 2032 No Action, 2032 With Action, and 2032 Mitigation Conditions Level of Service Analysis

																																TC V C	or Sci	VICC	Anaiysis
						Neekda	y AM										-	Weekda	ay PM		-														
	20	32 No	Action		20	32 With	Actio	n	203	32 Mitig	ation		20	32 No	Action	1	20	32 Witl	n Action	n	20	032 Mit	igation	1	20	032 No	Actio	n	203	2 With	Action	1	203	2 Miti	gation
	Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c D	elay		Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay	,	Lane	v/c	Delay		Lane	v/c	Delay
ch	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio (sec) L	os	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec) L	os	Group F	Ratio	(sec) LOS
																																		•	
	Т	-	-	_	Т	-	-	-				Α	Т	-	-	-	I T	_	-	-	Т			Α	Ĭ		1								
FB	R	-	-	-	R	-	-	-			0.1	Α	R	_	_	-	R	_	_	_	R			Α											
WB	Т	_	_	_	Т	_	_	_				Α	Т	_	_	_	Т	_	_	_	Т			F											
	·				·				•	0.7 1	0.0	<u> </u>	·									0.00	02.0	_											
SB	R	0.75	44 2	F	R	0.94	76.4	F	R	0.81	54 8	D	R	1 25	176 1	F	R	1.93	464.7	F	R	0.96	79.5	F											
0,5	.,	0.70		_		0.0				0.01	3 1.0	- I		1.20	17 0.1					1		0.00	70.0	_											
						ļ			Interse	ction	5.9	Α						<u> </u>			Inters	ection	32.5	С											
									micoroo				vo and	Cross	Count	ty Dai	rkway (n_Pam	n/Midla	nd A												1			
		0.05	72.0	Е		0.06	75.6	_					ve and	CIUSS	Count	ly Fai	I Kway C	Jii-Kaiii	Privilaia	IIIu A	ve (Las	i) (Sigi	lializec	,											
EB	TD			^	TD	0.90				0.00																									
MD								A				A																							
WB	LIK				LIK							В																							
NB -	L				L							В																							
	IR				IR							<u> </u>																							
SB	L				L		16.6	В																											
					TR	1.12	88.5	F																											
	Interse	ection	51.4	D	Inters	ection	57.0	E																											
									36	: Hawth	orne /	Ave 8												nalize	d)										
EB													LTR	0.66	16.8	С	LTR	0.82	24.2	С	LTR	0.28	2.7	Α											
\A/D													L	1.14	159.1	F	L	1.50	313.3	F	L	0.35	5.4	Α											
VVD													TR	0.77	19.9	С	TR	0.92	34.6	D	TR	0.47	4.3	Α											
															•	•		•	•		Inters	ection	3.7	Α		•							,		
	EB SB WB SB SB SB	EB L TR WB LTR WB LTR NB LTR SB L TR TR SB L TR TR SB L TR TR SB L TR TR TR SB L TR TR SB L TR TR TR SB L TR TR TR SB L TR TR SB L TR	EB	EB	Ch Group Ratio (sec) LOS EB T - - - R - - - - WB T - - - SB R 0.75 44.2 E EB L 0.95 72.9 E TR 0.19 9.3 A WB LTR 0.12 15.4 B NB L 0.25 12.2 B TR 0.45 13.8 B SB L 0.08 17.8 B TR 1.09 78.0 E Intersection 51.4 D	Ch	Ch	Lane V/c Group Ratio (sec) LOS Group Ratio (sec) Ratio (sec) CS Group Ratio (sec) CS CS CS CS CS CS CS C	Chapter Chap	Chamber Cham	2032 No Action 2032 With Action 2032 Mitig Lane V/c Delay Group Ratio (sec) LOS LOS	2032 No Action 2032 With Action 2032 Mitigation	2032 No Action 2032 With Action 2032 Mitigation Lane V/c Delay Group Ratio (sec) LOS Group Ratio (sec) LOS	Column	Ch	2032 No Action 2032 With Action 2032 Mitigation 2032 No Action 2032 No Action	2032 No Action	2032 No Action	2032 No Action	2032 No Action 2032 With Action 2032 With Action Lane V/c Group Ratio Ratio	2032 No Action 2032 With Action 2032 With Action 2032 Milty attle 2032 Milty at	Column C	Column C	Column C	March Marc	Part	Column C	March Color Colo	Mart Mart	2032 No Action 2032 With Act	Part	Note 1	Statistical Content of the content	Note 1 1 2 2 3 3 3 3 3 3 3 3	State Stat

Notes:
EB = Eastbound, WB = Westbound, NB = Northbound, SB = Southbound, NEB = Northeastbound, SWB = Southwestbound

v/c = volume to capacity, LOS = Level of Service
L = Left Turn, T = Through, R = Right Turn, dr = defacto right- turn lane
= LOS E, = LOS F

Red bold text = Significant Impact