

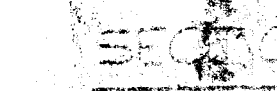


12. ALL FLOOR JOIST FRAMING TO BE 2"X12 L.D. @ 16" O.C. UNLESS NOTED OTHERWISE.
13. SEE PLAN FOR FLOOR JOIST SECTION.
14. ALL FLOOR JOISTS WITHIN WALL & DECK SECTIONS
TO BE 2"X12 L.D. @ 16" O.C. UNLESS NOTED OTHERWISE.
15. ALL FLOOR JOISTS WITHIN WALL & DECK SECTIONS
TO BE 2"X12 L.D. @ 16" O.C. UNLESS NOTED OTHERWISE.
16. ALL FLOOR JOISTS WITHIN WALL & DECK SECTIONS
TO BE 2"X12 L.D. @ 16" O.C. UNLESS NOTED OTHERWISE.

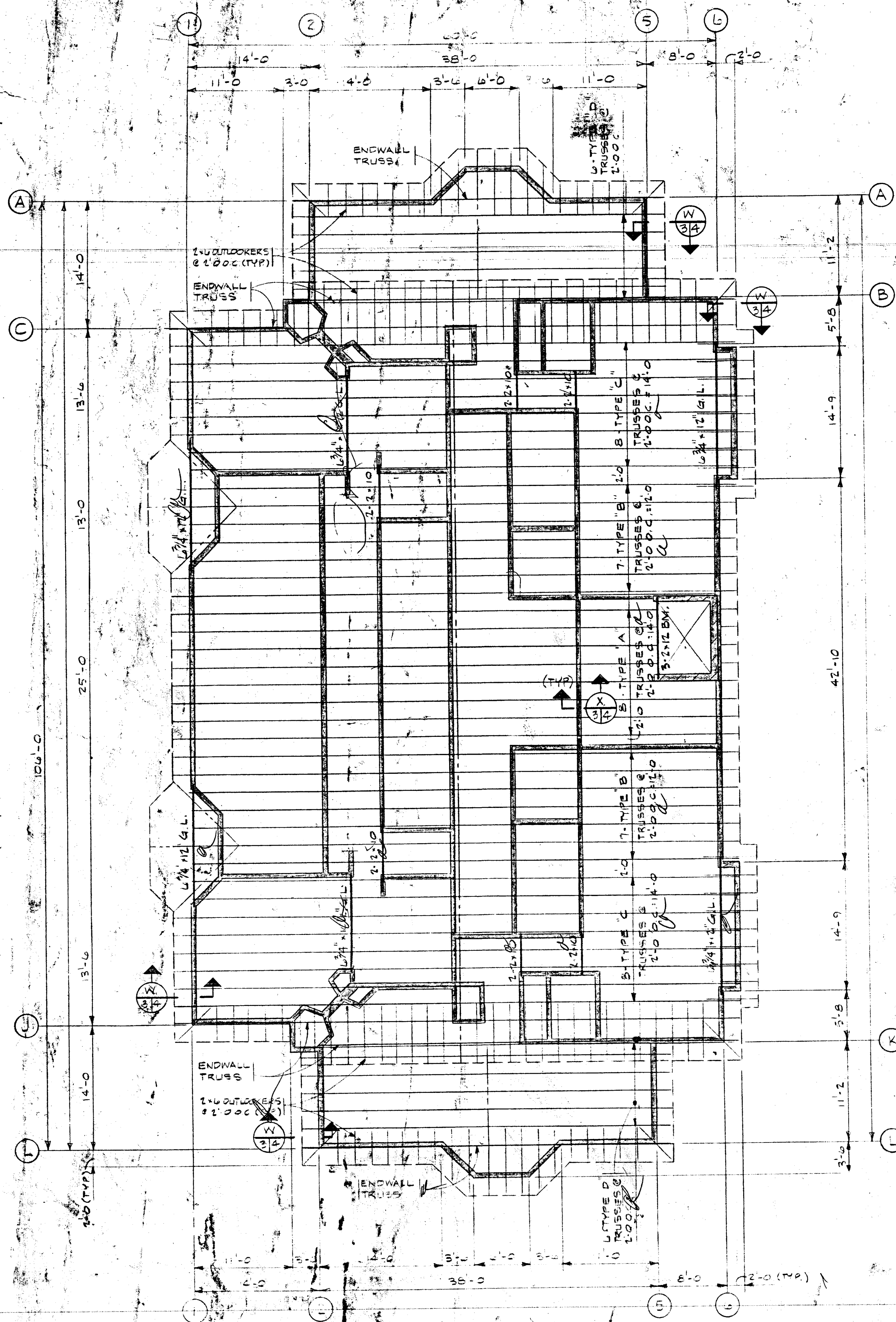


1. ALL FLOOR JOIST FRAMING TO BE 2" X 10" O.C. - SEE PLAN FOR FRAMING DIRECTION
2. CONN. ALL WALL TOP PLATES TO ALL FLOOR JOISTS WITH 10D NAILS
 PROVIDE SOLID BLOCKING BETWEEN JOISTS ABOVE WALLS WHEN WALLS RUN PERPENDICULAR TO JOISTS - SEE SECTIONS


$$3/4'' = 1 - 1/4''$$

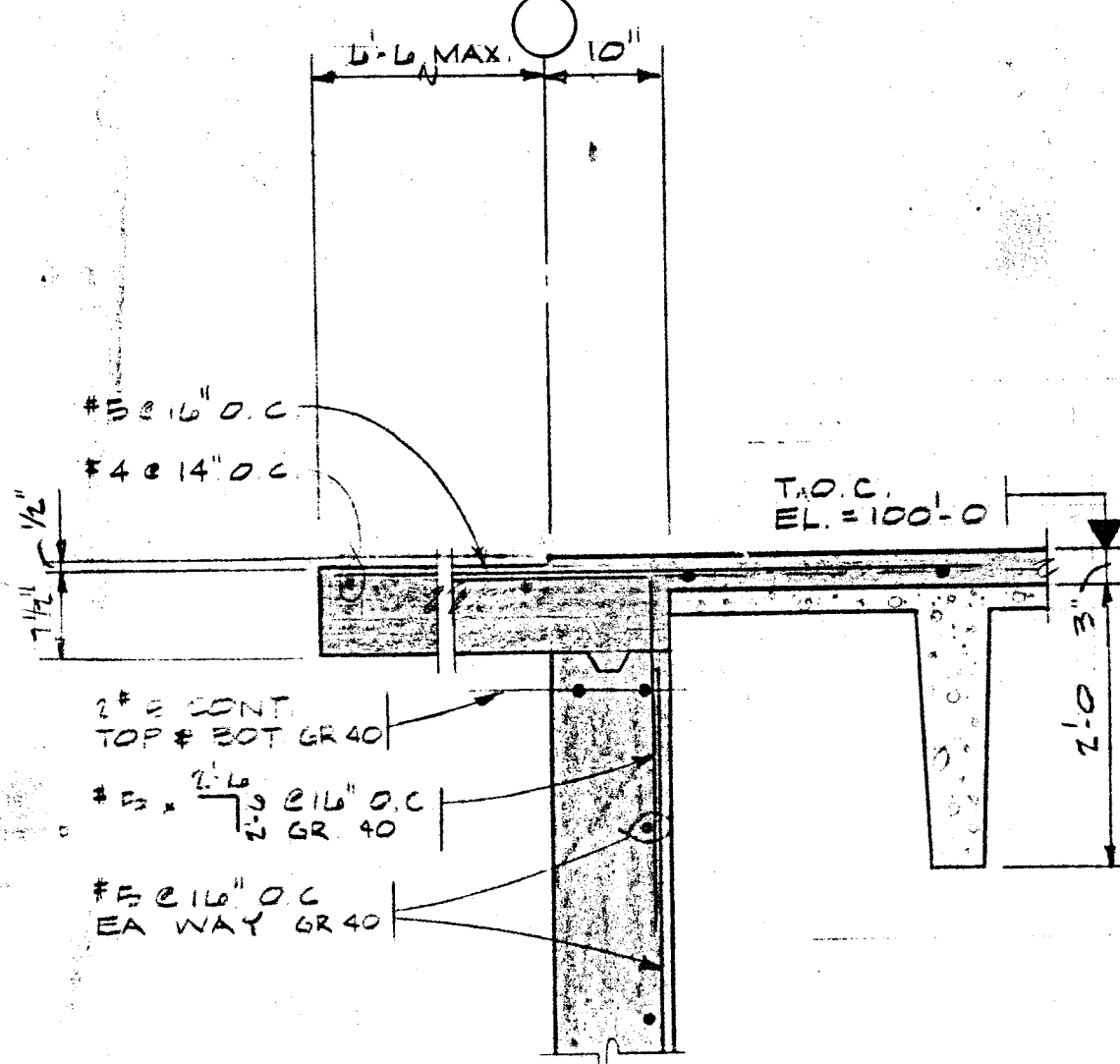
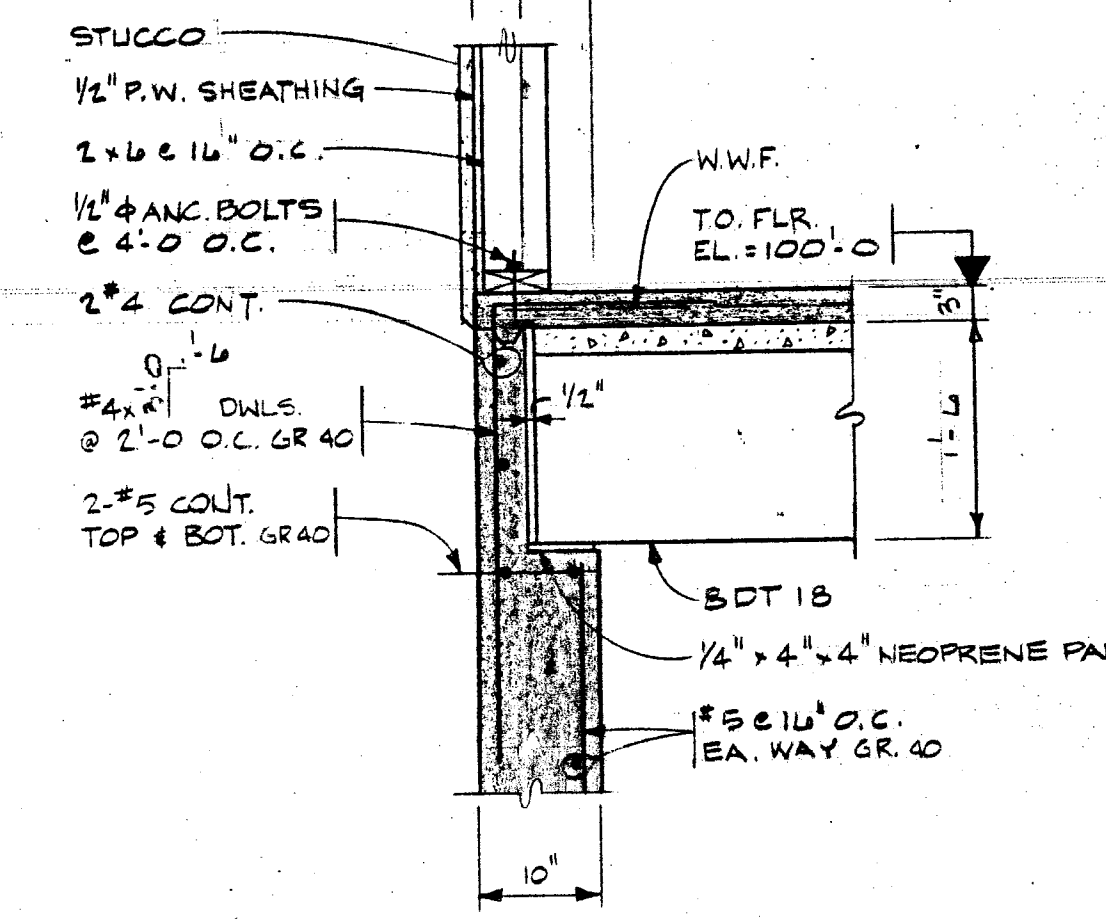
$$\frac{2}{4} = 1 -$$


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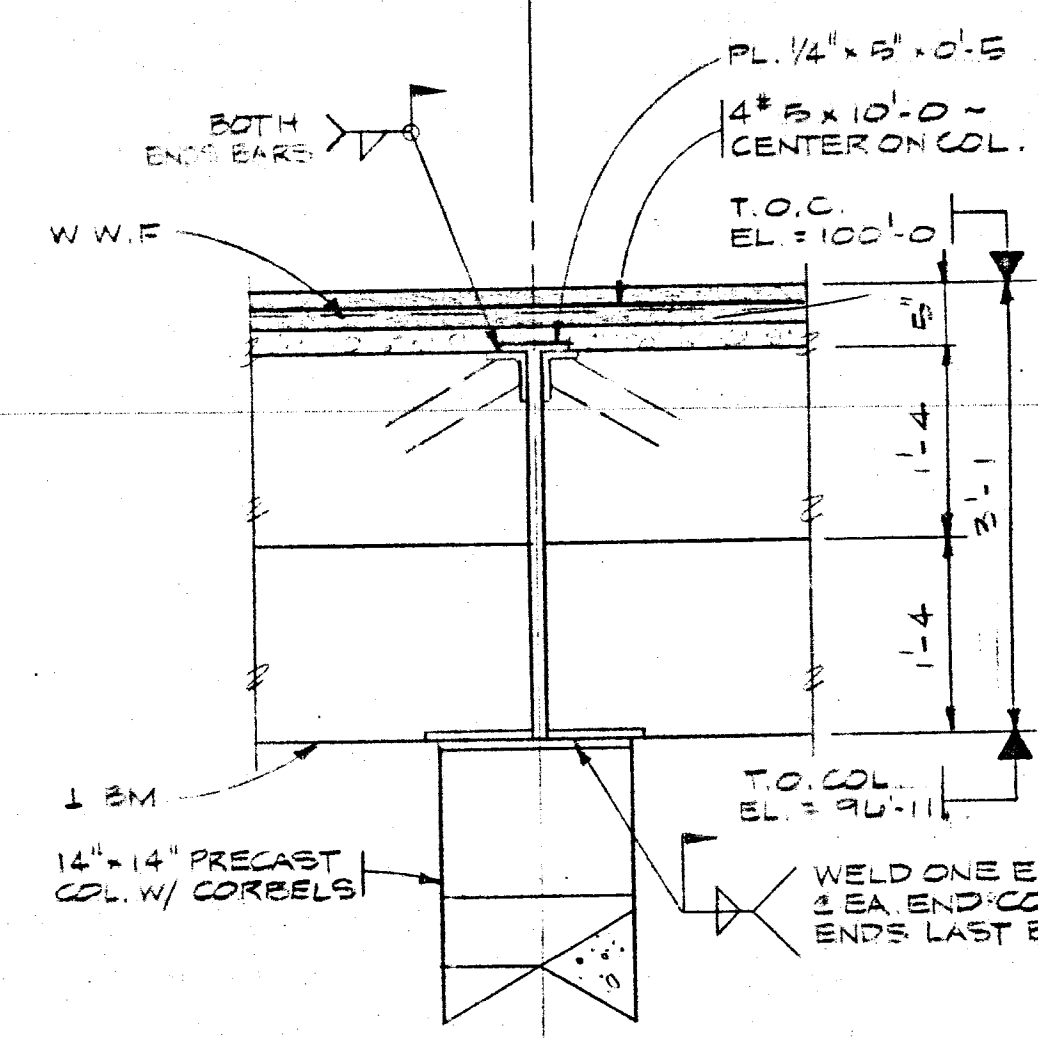


ROOF FRAMING PLAN 1/8" = 1'-0"

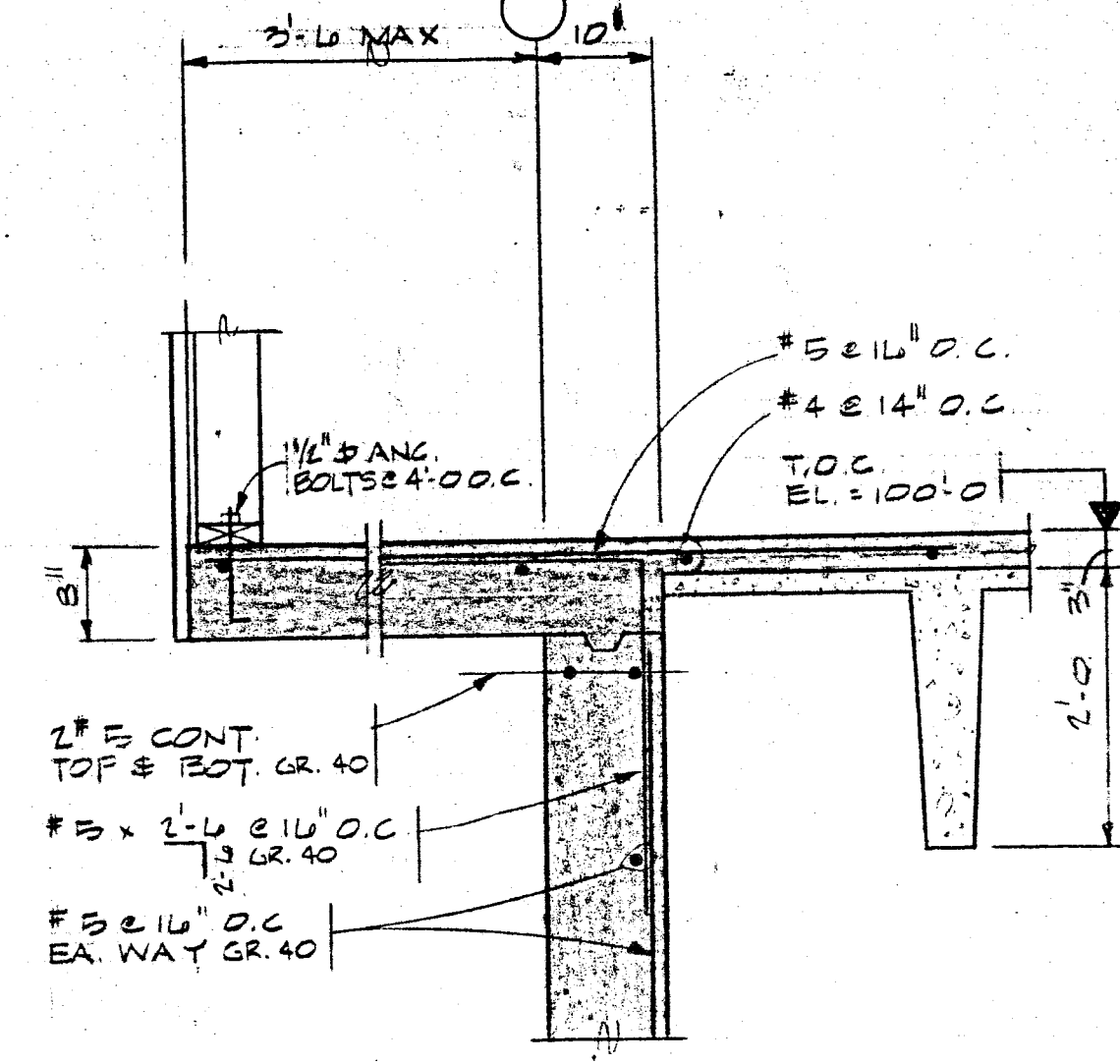
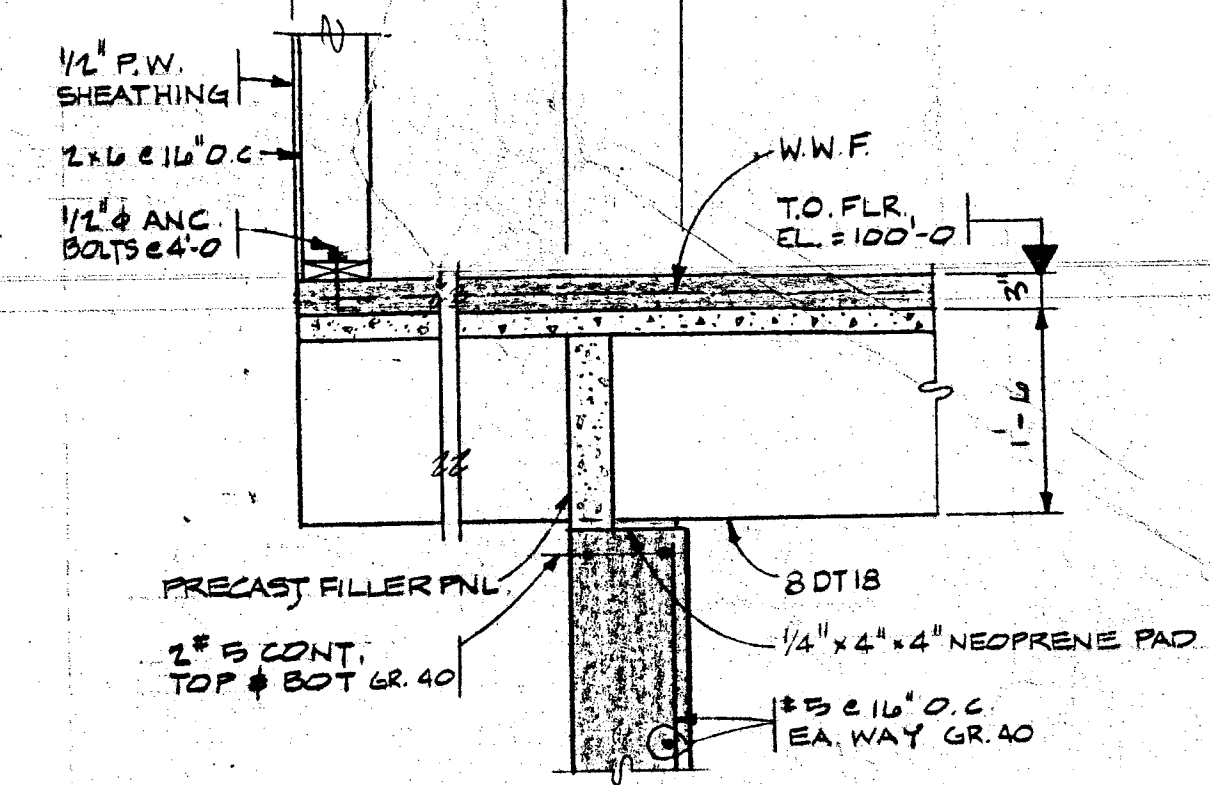
NOTES: ALL WALLS SHOWN F-HADED ARE BEARING WALLS AND BEAMS SUPPORTING ROOF TRUSSES.



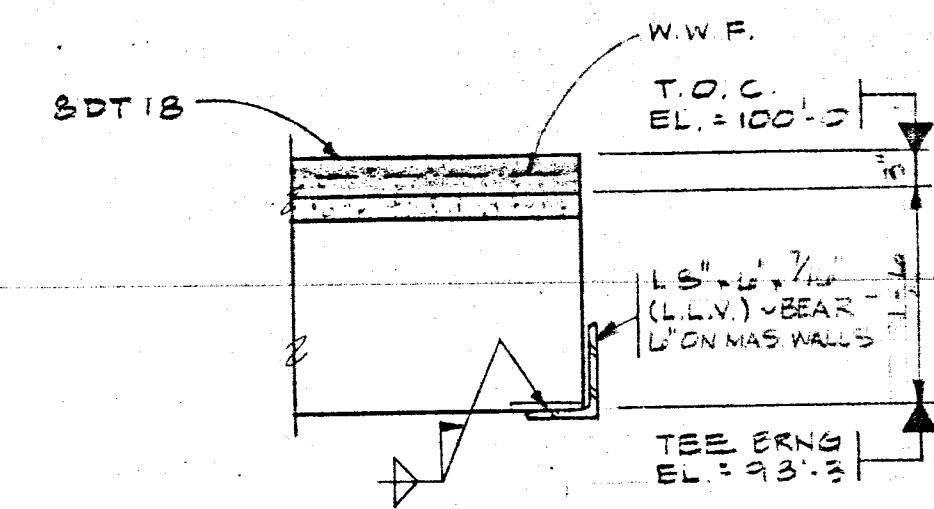
SECTION K/K13 3/4" = 1'-0"



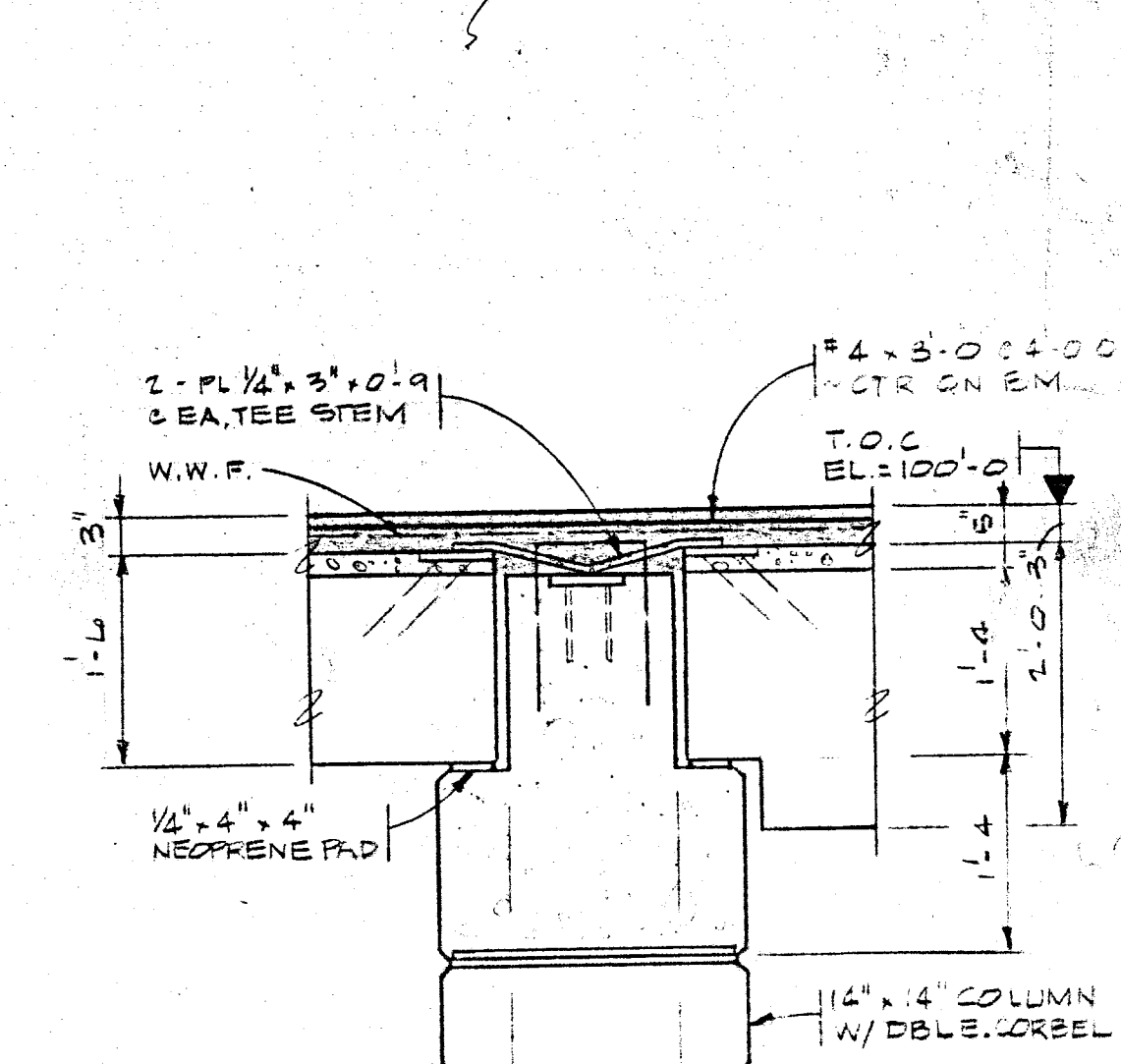
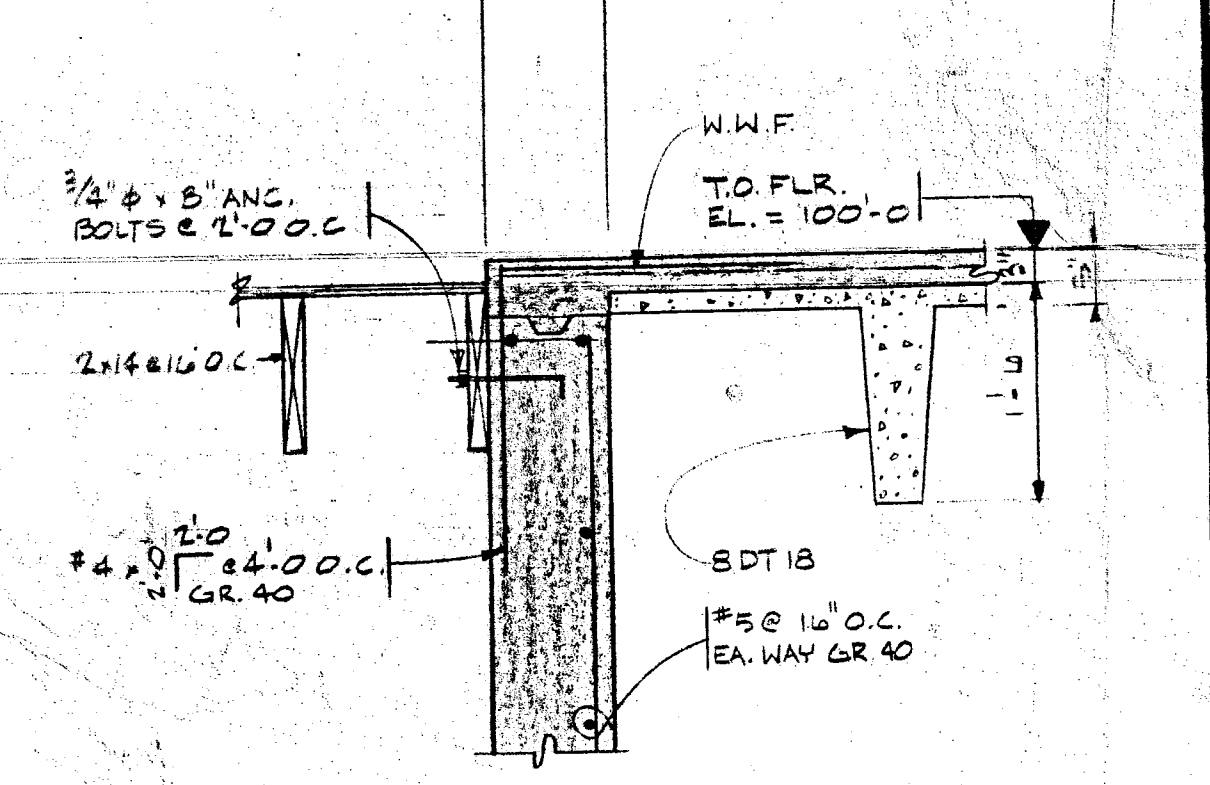
SECTION M/M13 3/4" = 1'-0"



SECTION P/P13 3/4" = 1'-0"



SECTION R/R13 3/4" = 1'-0"

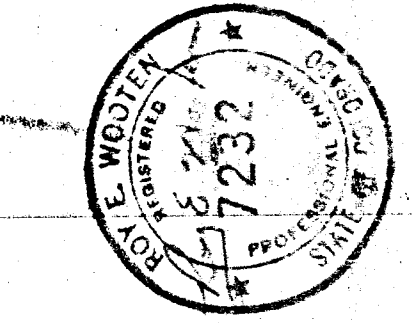


SECTION U/U13 3/4" = 1'-0"

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DENVER, COLORADO



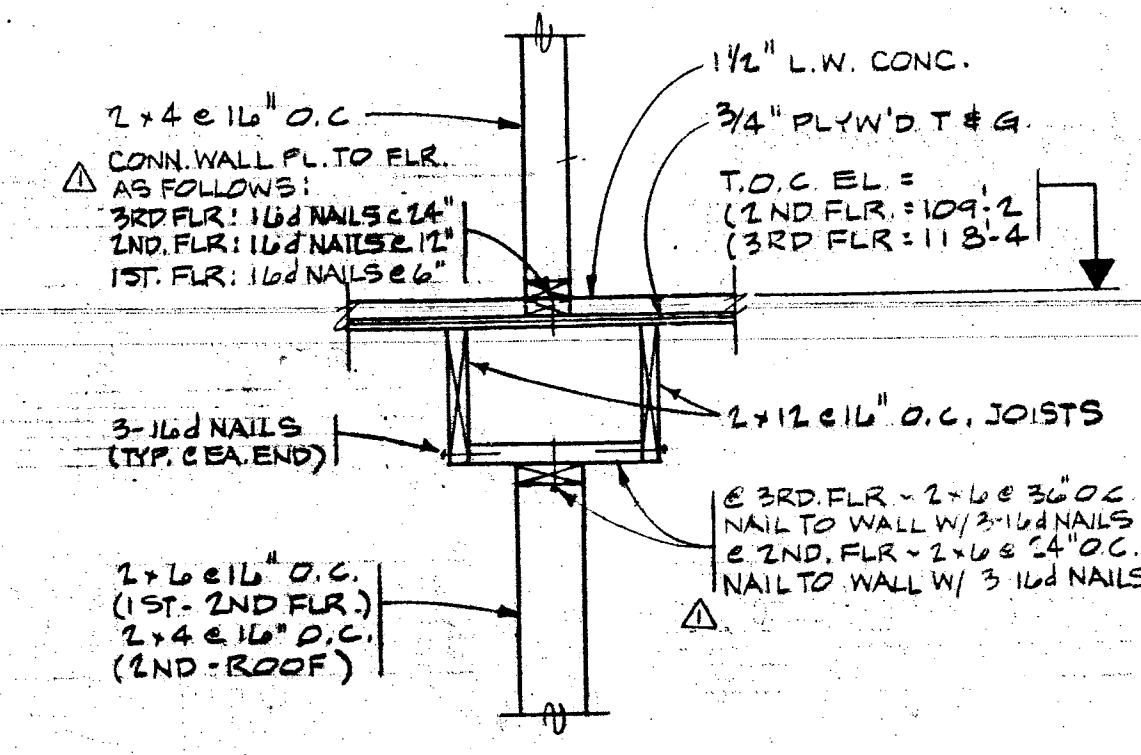
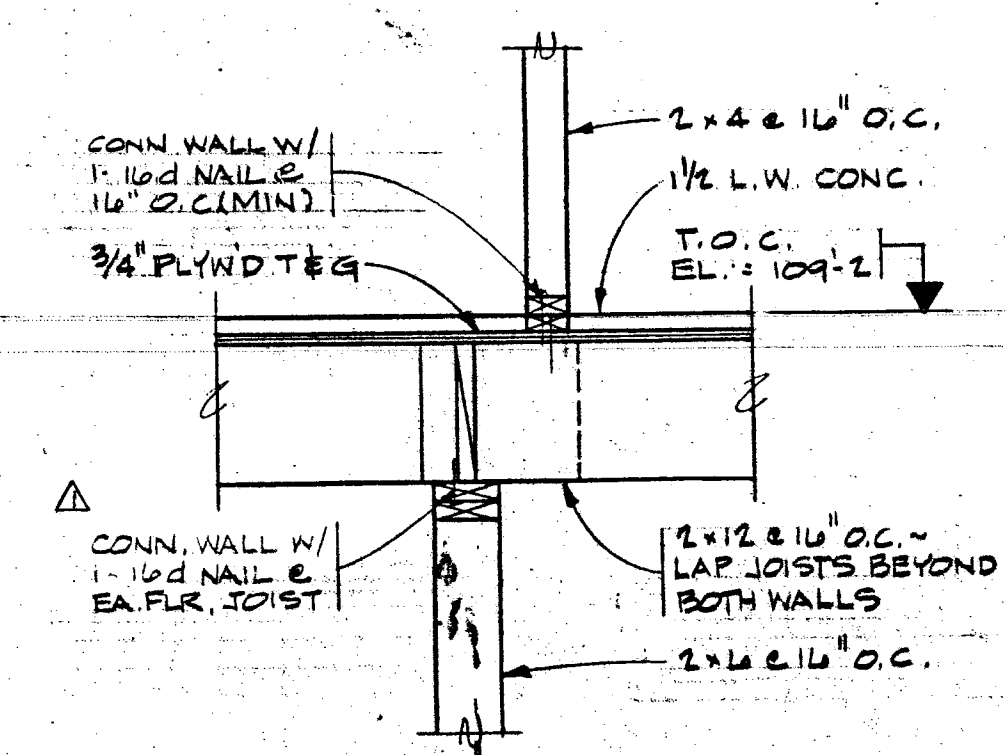
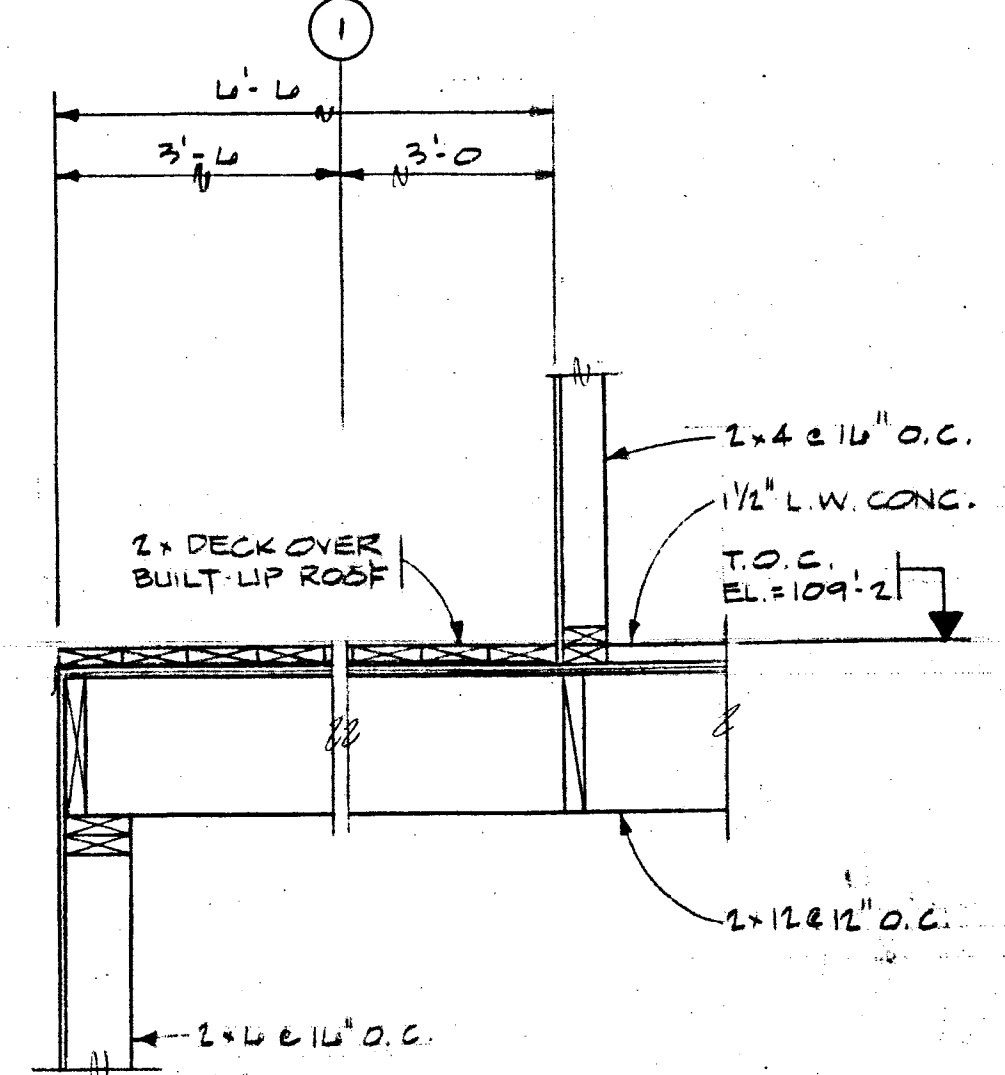
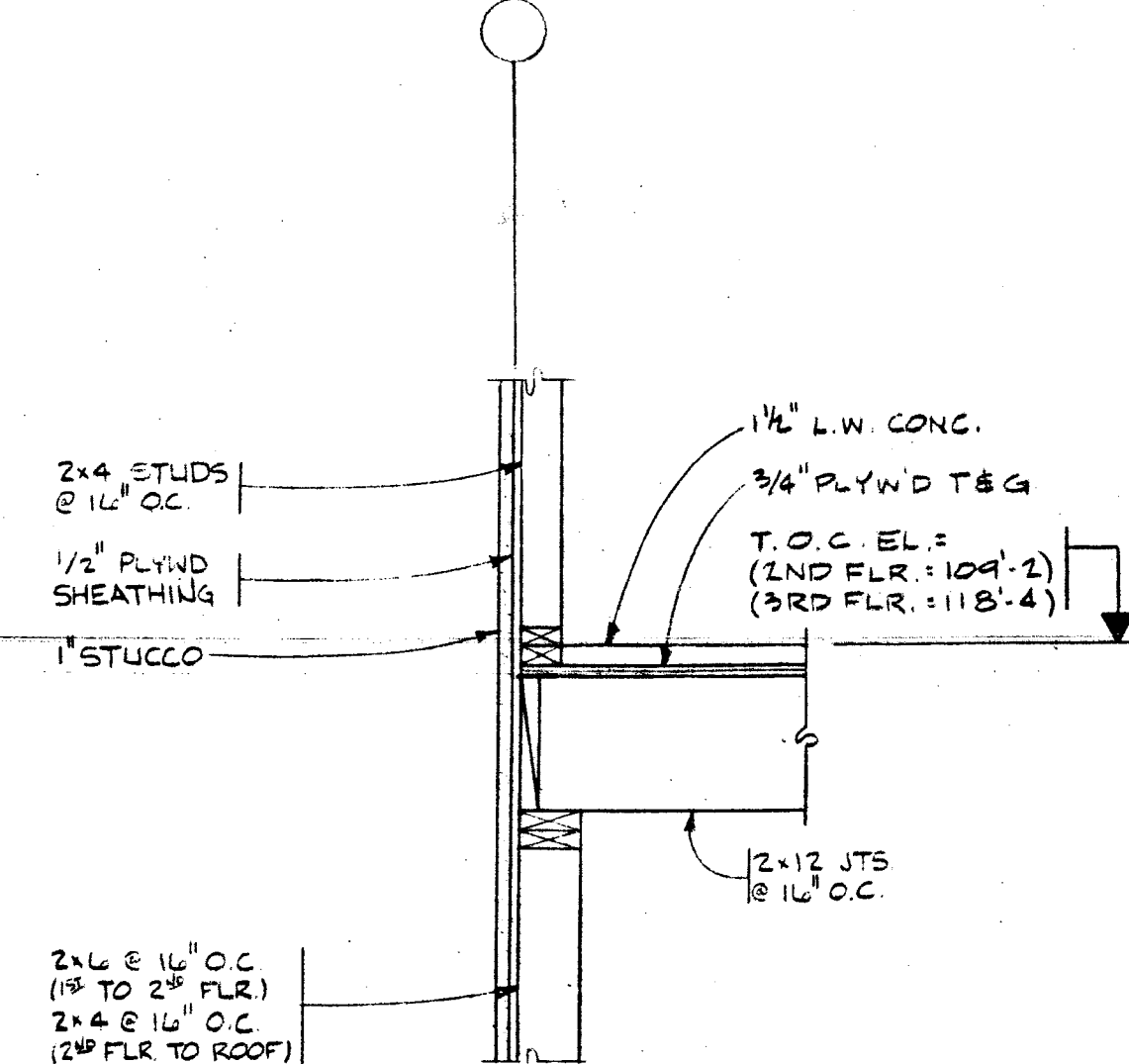
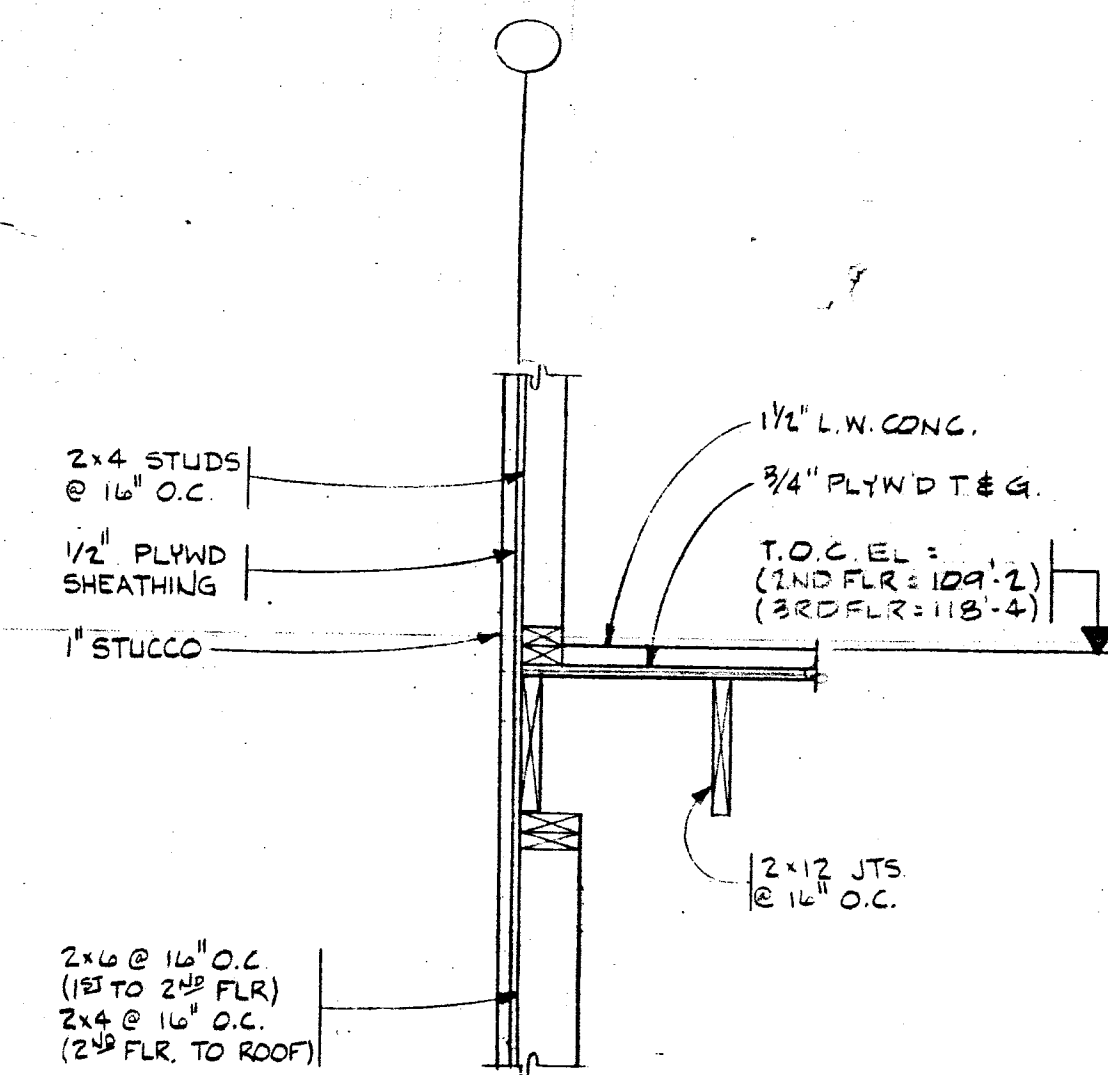
WOODS MAJOR PROJECT FOR
A CONDOMINIUM PROJECT FOR
WES CHILPALA
BRECKENRIDGE, COLORADO



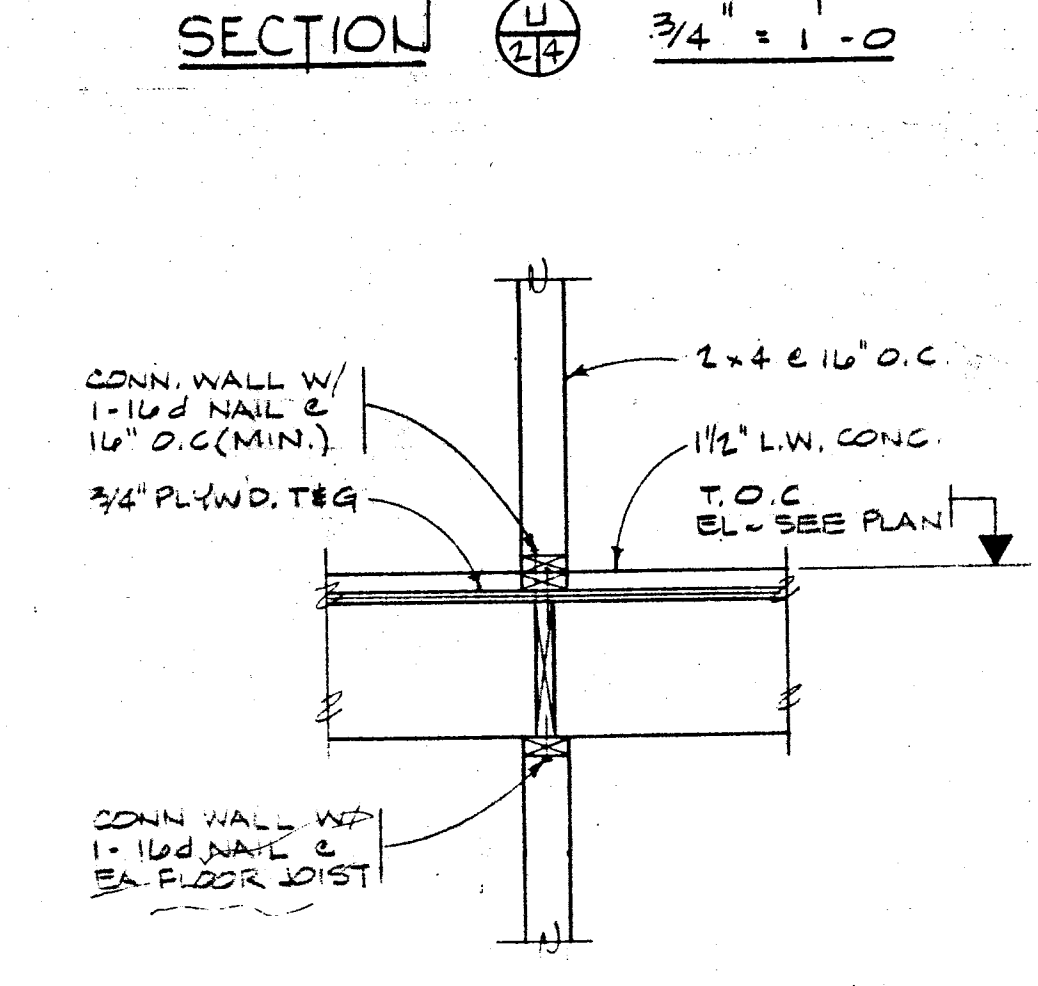
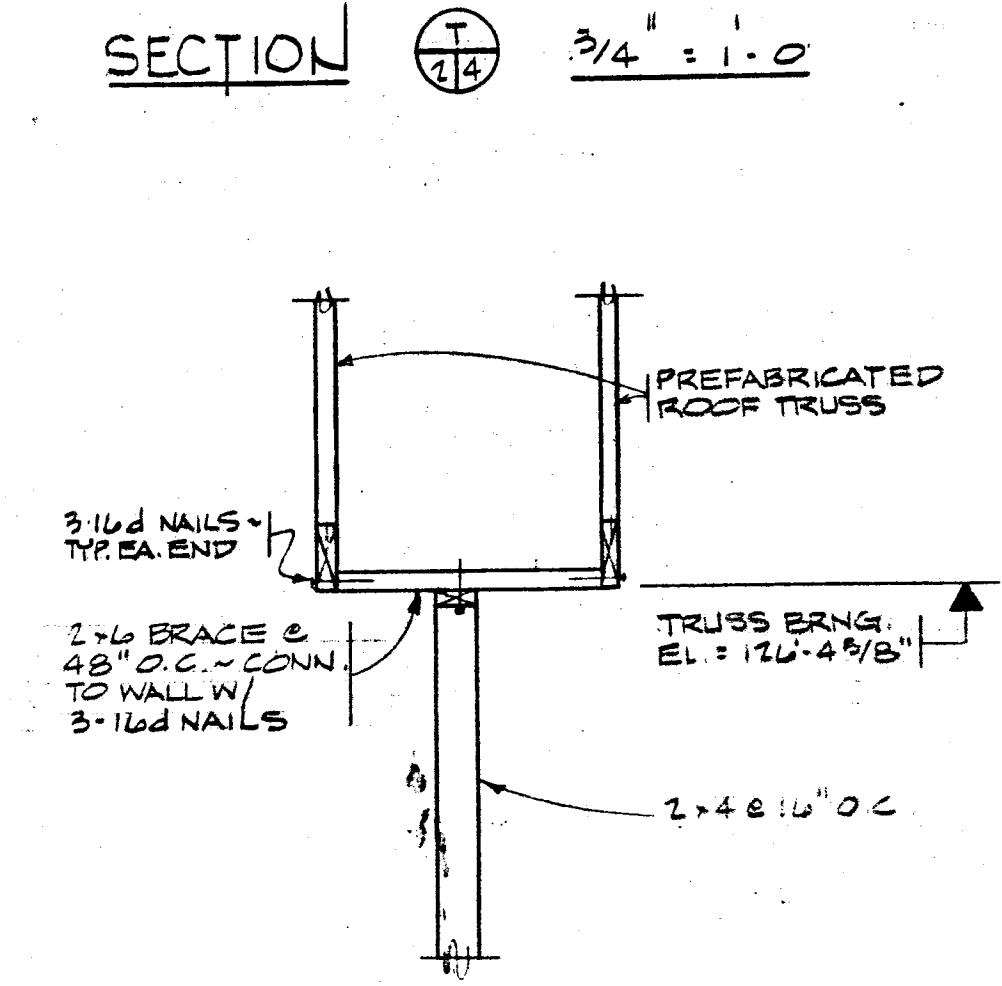
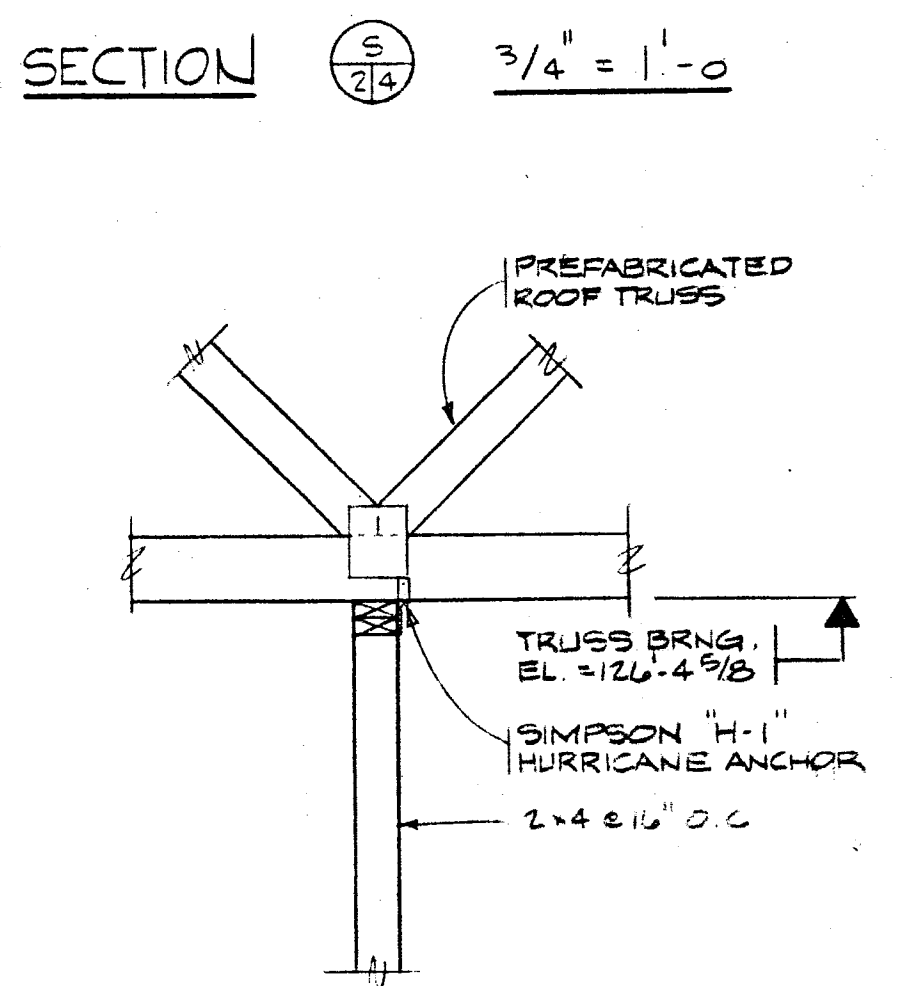
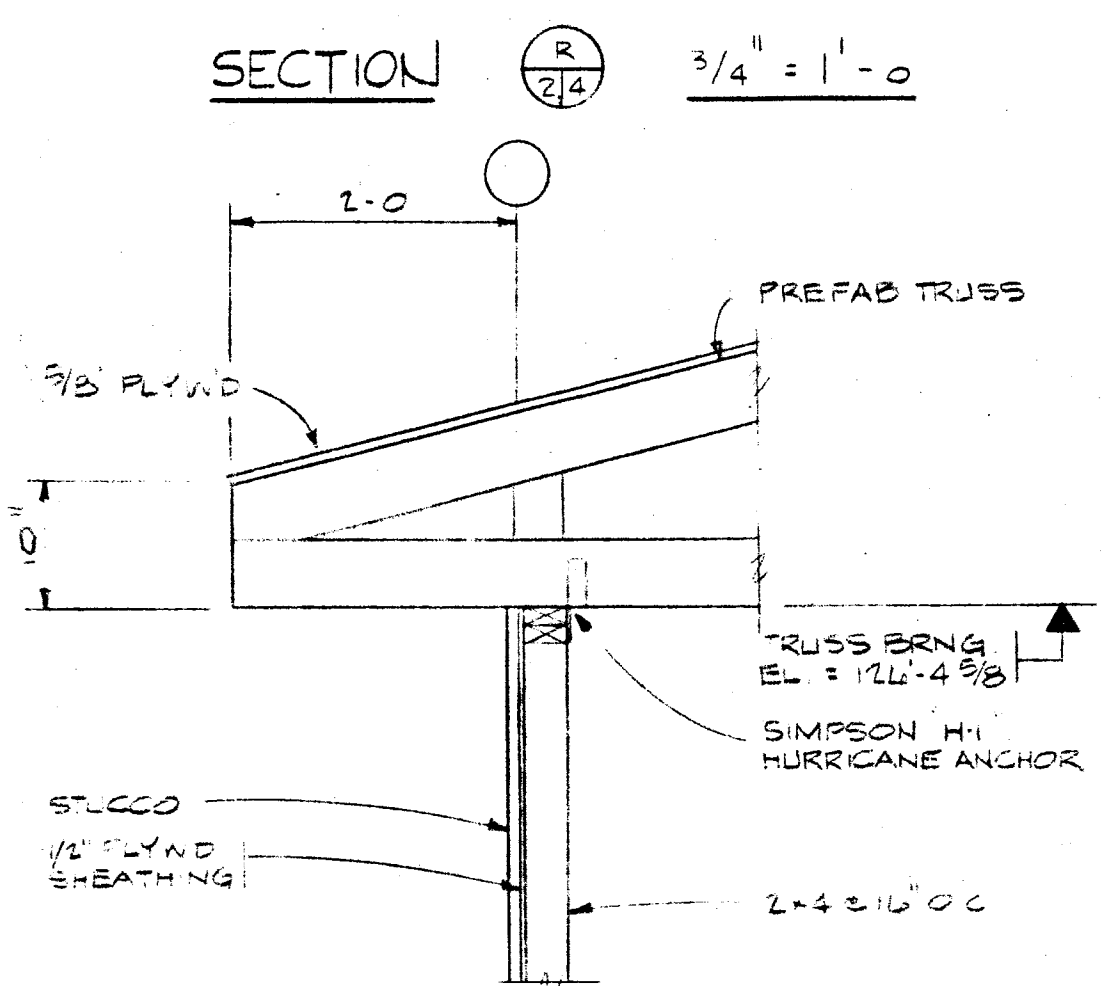
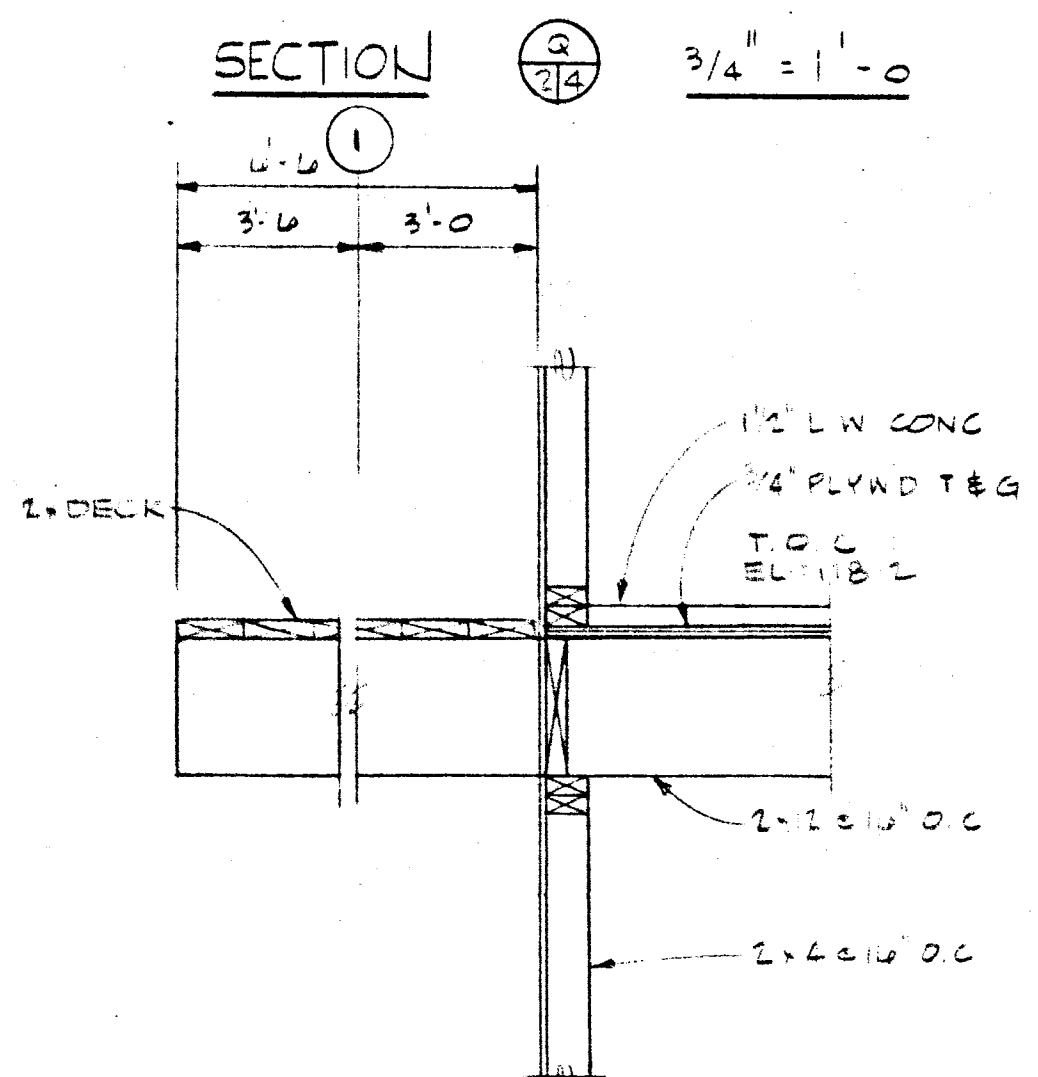
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drawn by: RB
checked by: JLV
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NOTE: TYPICAL TOP PLATE CONN. FOR ALL WALLS PARALLEL TO FLR JOISTS.



NOTE: WALL TO FLOOR CONNECTIONS TYP. FOR ALL WALLS PERPENDICULAR TO FLR JOISTS

SECTION Q-2 3/4" = 1'-0"

SECTION R-2 3/4" = 1'-0"

SECTION S-2 3/4" = 1'-0"

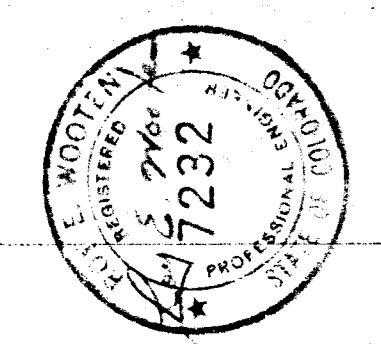
SECTION T-2 3/4" = 1'-0"

SECTION U-2 3/4" = 1'-0"

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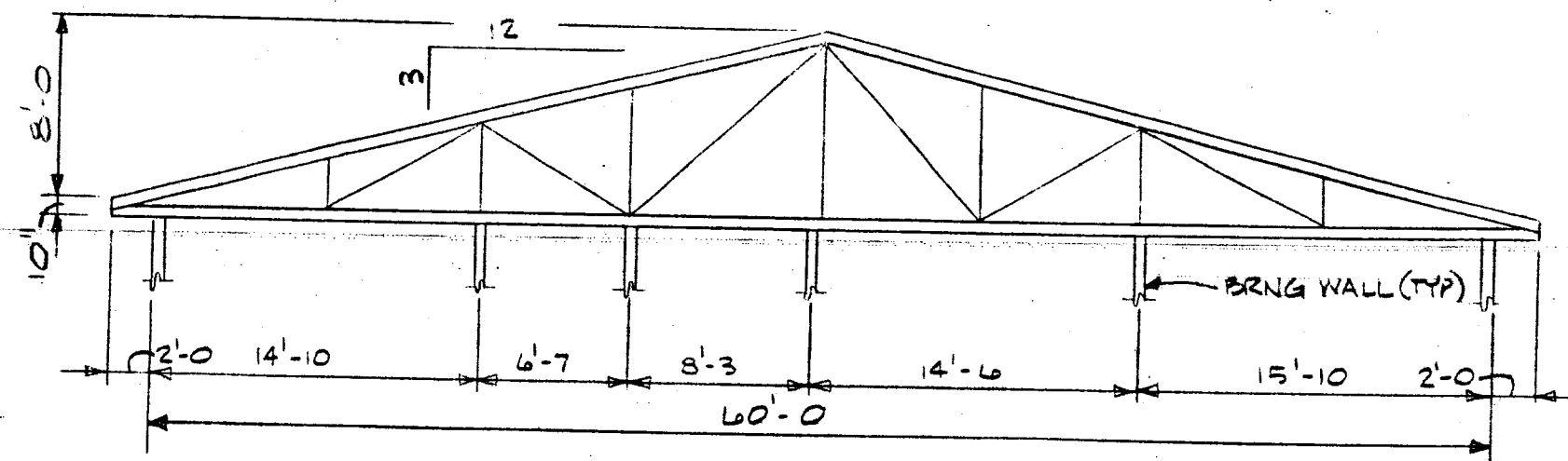
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A CONDOMINIUM PROJECT FOR
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BRECKENRIDGE, COLORADO



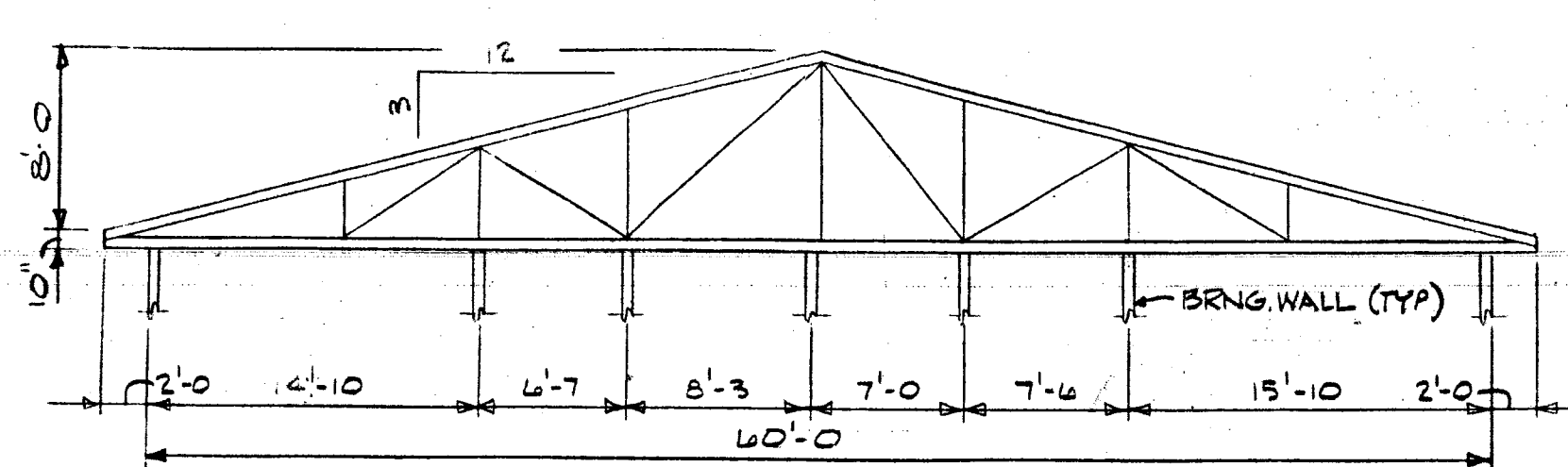
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checked by EW
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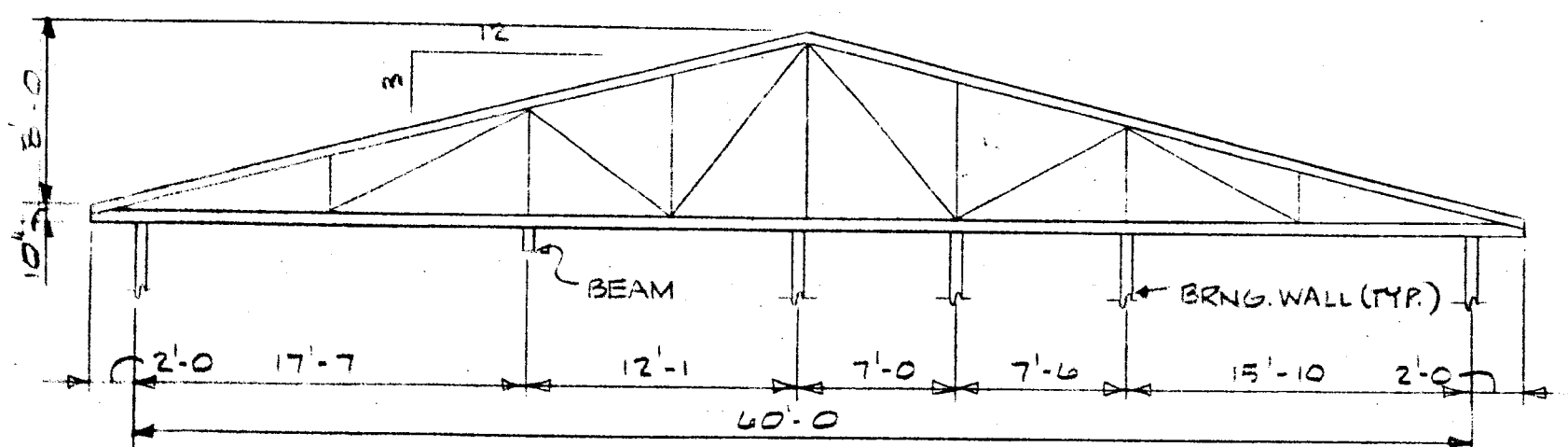
SHEET
S-4



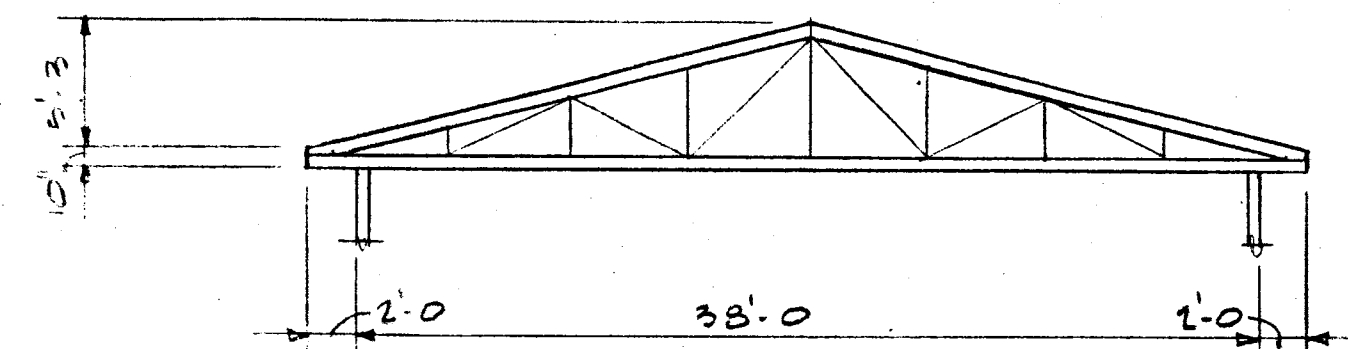
TRUSS A



TRUSS B



TRUSS C



TRUSS D

DETAIL OF ROOF TRUSSES

GENERAL NOTES

1. LIVE LOADS USED IN DESIGN

- A. ROOF 90 PSF
- B. RESIDENTIAL FLOORS 40 PSF
- C. CORRIDORS AND STAIRWAYS 100 PSF
- D. WIND U.B.C. ZONE 80
- E. SEISMIC U.B.C. ZONE 1

2. CONCRETE

- A. ALL CONCRETE SHALL BE MADE WITH STONE AGGREGATE AND TYPE I PORTLAND CEMENT. AIR ENTRAINMENT SHALL BE 4-1/2 PERCENT PLUS OR MINUS 1-1/2 PERCENT. CONCRETE SHALL DEVELOP THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH IN 28 DAYS.
 - 1. SLAB-ON-GRADE, WALLS 3,000 PSF
 - 2. DOUBLE TEE TOPPING 4,000 PSF
- B. PRESTRESSED CONCRETE SHALL BE MADE WITH "STONE" AGGREGATE AND SHALL DEVELOP 5,000 PSI, COMPRESSIVE STRENGTH IN 28 DAYS.
- C. SLABS AND BEAMS SHALL NOT HAVE JOINTS IN A HORIZONTAL PLANE. ANY STOP IN CONCRETE WORK MUST BE MADE AT CENTER OF SPAN WITH VERTICAL BULKHEADS AND HORIZONTAL KEYS, UNLESS OTHERWISE SHOWN. ALL CONSTRUCTION JOINTS SHALL BE AS DETAILED OR AS APPROVED BY THE ARCHITECT.
- D. ALL EXPOSED EDGES OF CONCRETE SLABS AND WALLS SHALL HAVE A 1/2" CHAMFER.
- E. ALL CONCRETE WORK AND REINFORCEMENT DETAILING SHALL BE IN ACCORDANCE WITH ACI BUILDING CODE 318-82.
- F. FLOOR SLABS-ON-GRADE SHALL BE POURED IN CHECKER PATTERN AVOIDING RE-ENTRANT CORNERS WITH CONSTRUCTION JOINTS LOCATED UNDER PARTITIONS WHERE PRACTICAL AND WITH NO DIMENSION EXCEEDING 30'-0" OR TOTAL AREA OF 900 SQUARE FEET. AT LEAST 48 HOURS SHALL ELAPSE BEFORE POURING ADJACENT SECTION.
- G. MINIMUM AREA OF REINFORCEMENT FOR PRECAST CONCRETE SHALL BE 0.002 bd.
- H. PRECAST CONCRETE CONNECTIONS SHALL BE OF READILY WELDABLE STEEL AND SHALL BE AS DETAILED. ALTERNATIVES MAY BE SUBSTITUTED ONLY UPON WRITTEN APPROVAL OF THE ENGINEER.
- I. PRECAST CONCRETE SHALL BE ERECTED INTO POSITION IN A MANNER WHICH WILL NOT IMPAIR THE STRENGTH OF THE MEMBERS AND ADEQUATE TEMPORARY BRACING SHALL BE PROVIDED UNTIL ALL RELATED FIELD CONNECTIONS HAVE BEEN FULLY COMPLETED. PROVIDE LIFTING DEVICES THAT WILL SUPPORT TWICE THE WEIGHT OF THE PRECAST MEMBER.

3. REINFORCEMENT

- A. ALL REINFORCING SHALL BE HIGH STRENGTH DEFORMED BARS (ASTM DESIGNATION A615, GRADE 60) WITH 60,000 PSI MINIMUM YIELD POINT (EXCEPT #3 COLUMN TIES AND BEAM STIRRUPS WHICH MAY BE GRADE 40 DEFORMED BARS).
- B. WIRE STRAND USED IN PRE-TENSIONING SHALL HAVE 270 KSI OR 240 KSI ULTIMATE STRENGTH AND SHALL CONFORM TO "SPECIFICATIONS FOR UNCOATED SEVEN-WIRE STRESS RELIEVED STRAND FOR PRESTRESSING CONCRETE" (ASTM A416-68).

C. REINFORCEMENT PROTECTION:

- 1. CONCRETE POURED AGAINST EARTH 3"
- 2. CONCRETE POURED IN FORMS BUT EXPOSED TO WEATHER OR EARTH:
 - a. IF BARS ARE LARGER THAN #5 2"
 - b. IF BARS ARE #5 OR SMALLER 1-1/2"
- 3. SLABS AND WALLS 3/4"
- D. ALL BAR LENGTHS ARE DRAWN TO SCALE UNLESS NOTED. NO SLICES OR REINFORCEMENT SHALL BE MADE EXCEPT AS DETAILED OR AUTHORIZED BY THE STRUCTURAL ENGINEER. LAP SPLICES, WHERE PERMITTED, SHALL BE A MINIMUM OF 24 BAR DIAMETERS FOR VERTICALS AND 36 BAR DIAMETERS FOR ALL OTHERS. MAKE ALL BARS CONTINUOUS AROUND CORNERS.
- E. DETAIL BARS IN ACCORDANCE WITH ACI DETAILING MANUAL AND ACI BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, LATEST EDITIONS.
- F. PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCING AT POSITIONS SHOWN ON THE PLANS.
- G. PLACE TWO #5 (1 EACH FACE) WITH 2'-0" PROJECTION AROUND ALL OPENINGS IN CONCRETE. WHERE OPENINGS ARE RECTANGULAR, ALSO PROVIDE ONE #4 x 4'-0" DIAGONALLY AT EACH CORNER.
- H. ALL STIRRUPS SHALL HAVE TWO #3 SPACERS FOR LENGTH OF STIRRUP SPACING.
- I. WIRE MESH REINFORCEMENT MUST LAP TWO FULL MESHES AT SIDE AND END LAPS, BUT NOT LESS THAN 12", AND SHALL BE WIRED TOGETHER.

4. STRUCTURAL STEEL

- A. ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM SPECIFICATION A36, LATEST EDITION.
- B. PROVIDE GOVERNMENT ANCHORS FOR BEAMS BEARING ON MASONRY WHERE ANCHOR BOLTS ARE NOT SPECIFIED.
- C. STRUCTURAL STEEL FABRICATOR SHALL FURNISH STEEL LINTELS FOR MASONRY OPENINGS.

5. MASONRY

- A. ALL WALLS SHALL BE REINFORCED HORIZONTALLY WITH STANDARD GRADE DUR-O-WALL (OR EQUAL) MASONRY WALL REINFORCEMENT AT 16" O.C. STRUCTURAL WALLS SHALL HAVE MINIMUM #4 AT 48" VERTICAL REINFORCEMENT.
- B. ALL REINFORCING IN MASONRY WALLS SHALL BE FULLY GROUTED IN PLACE.

6. WOOD

- A. ALL FRAMING LUMBER SHALL BE KILN DRIED DOUGLAS FIR COAST REGION, CONSTRUCTION GRADE (F = 1200 PSI MINIMUM).
- B. PROVIDE 1" x 4" CROSS BRIDGING NOT OVER 8 FEET O.C. FOR ALL WOOD JOISTS AND 2x SOLID BLOCKING BETWEEN JOISTS AT ALL SUPPORTS.
- C. LAMINATED BEAMS:
 - 1. ALL LAMINATED MEMBERS SHALL BE FABRICATED WITH LUMBER OF ONE OF THE FOLLOWING SPECIES: WEST COAST DOUGLAS FIR, WEST COAST HEMLOCK, WESTERN LARCH OR SOUTHERN PINE.
 - 2. LAMINATED MEMBERS SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR THE DESIGN AND FABRICATION OF STRUCTURAL GLUED LAMINATED LUMBER, PUBLISHED BY THE A.I.T.C. AND THE APPROPRIATE LUMBER PRODUCERS' ASSOCIATION.
 - 3. ALLOWABLE UNIT STRESSES REQUIRED FOR DRY CONDITIONS OF USE FOR LAMINATED MEMBERS ARE AS FOLLOWS:
 - a. BENDING 2,270 PSI
 - b. TENSION PARALLEL TO GRAIN 1,660 PSI
 - c. COMPRESSION PARALLEL TO GRAIN 1,500 PSI
 - d. HORIZONTAL SHEAR 165 PSI
 - e. COMPRESSION PERPENDICULAR TO GRAIN 575 PSI
 - 4. LAMINATED MEMBERS SHALL BE BUILT UP USING 2" NOMINAL MATERIAL.
- D. LAMINATED MEMBERS SIZES SHOWN ARE NET; OTHER MEMBER SIZES ARE NOMINAL.

7. FOUNDATIONS

- A. MAXIMUM SOIL PRESSURE USED IN DESIGN (BASED ON D.L. + 1/2 L.L.) 1,000 PSF
- B. ALL FOOTINGS ARE TO BE PLACED ON FIRM, UNDISTURBED NATURAL SOIL OR PROPERLY RECOMPACTED BACKFILL.
- C. DO NOT PLACE BACKFILL AGAINST PARKING GARAGE WALLS UNTIL SLAB-ON-GRADE AND FIRST FLOORS ARE IN PLACE.
- D. SEE SOIL REPORT DATED MARCH 2, 1984 BY PHILLIP J. BELLING BEFORE BEGINNING CONSTRUCTION.

8. GENERAL

- A. CONTRACTOR SHALL CHECK ALL DIMENSIONS ON STRUCTURAL DRAWINGS AGAINST ARCHITECTURAL.
- B. ENGINEER'S APPROVAL MUST BE SECURED FOR ALL STRUCTURAL SUBSTITUTIONS.
- C. VERIFY ALL OPENINGS THROUGH ROOF, FLOOR AND WALLS WITH MECHANICAL AND ELECTRICAL CONTRACTORS.
- D. WATERPROOFING, VAPOR BARRIERS, WATERSTOP AND DRAINAGE DRAINS SHALL BE AS INDICATED IN THE STRUCTURAL AND MECHANICAL DRAWINGS.
- E. PRIOR TO INSTALLATION OF MECHANICAL EQUIPMENT, MECHANICAL ITEMS, ENGINEER'S APPROVAL OF CONNECTIONS AND DETAILS SHALL BE OBTAINED.
- F. SUBMIT SHOP AND ERECTION DRAWINGS FOR ALL PRECAST CONCRETE UNITS, WOOD ROOF TRUSSES, MISCELLANEOUS STEEL, ETC., TO ENGINEER FOR REVIEW AND THE MANUFACTURE OR FABRICATION OF ANY ITEM WITHOUT WRITTEN APPROVAL OF SHOP DRAWINGS WILL BE AT THE RISK OF THE CONTRACTOR.

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RWA

WOODS MANOR
A CONDOMINIUM PROJECT FOR
WES CHILIPALA
BRECKENRIDGE, COLORADO

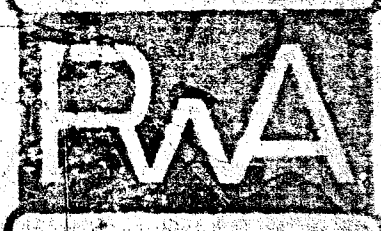


date APR. 17, 1984
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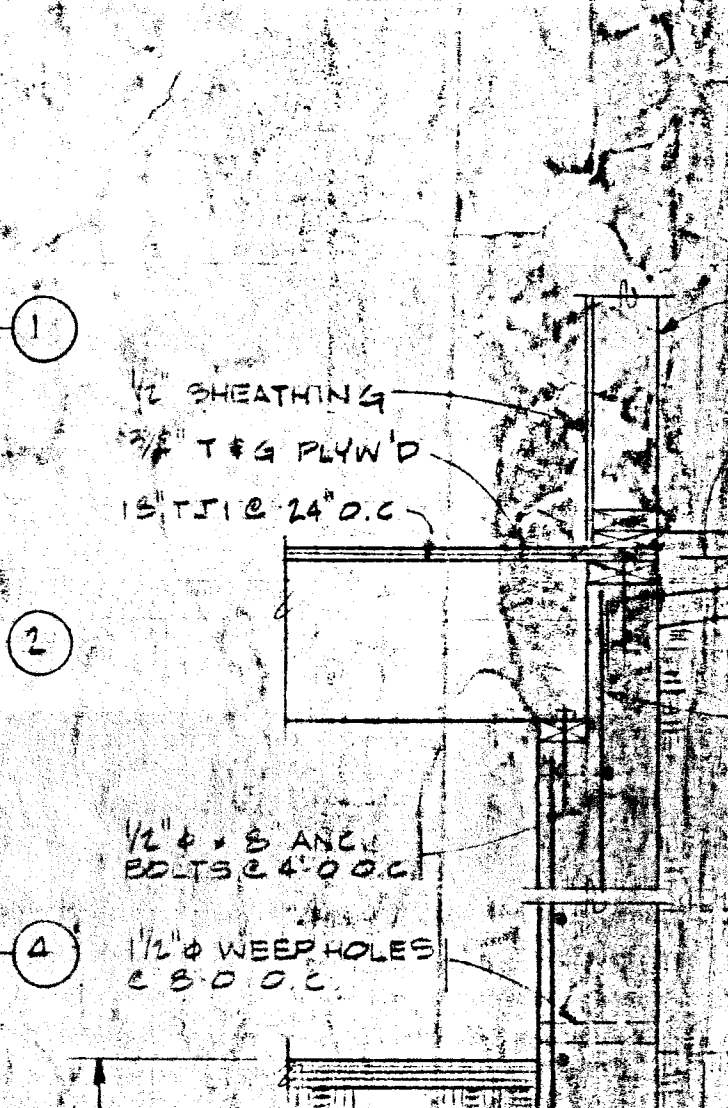
SECOND FLOOR FRAMING PLAN 1/8" = 1'-0"



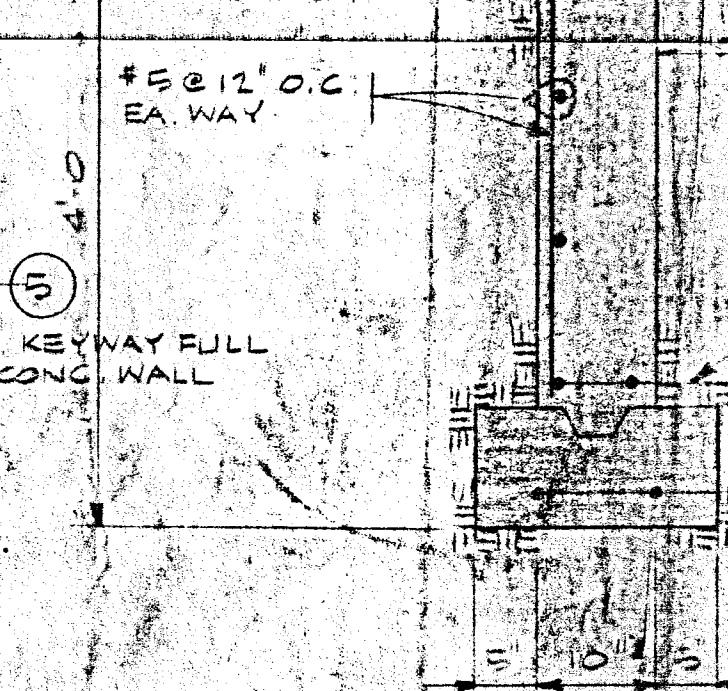
FOUNDATION AND FIRST FLOOR FRAMING PLAN 1/8" = 1'-0"



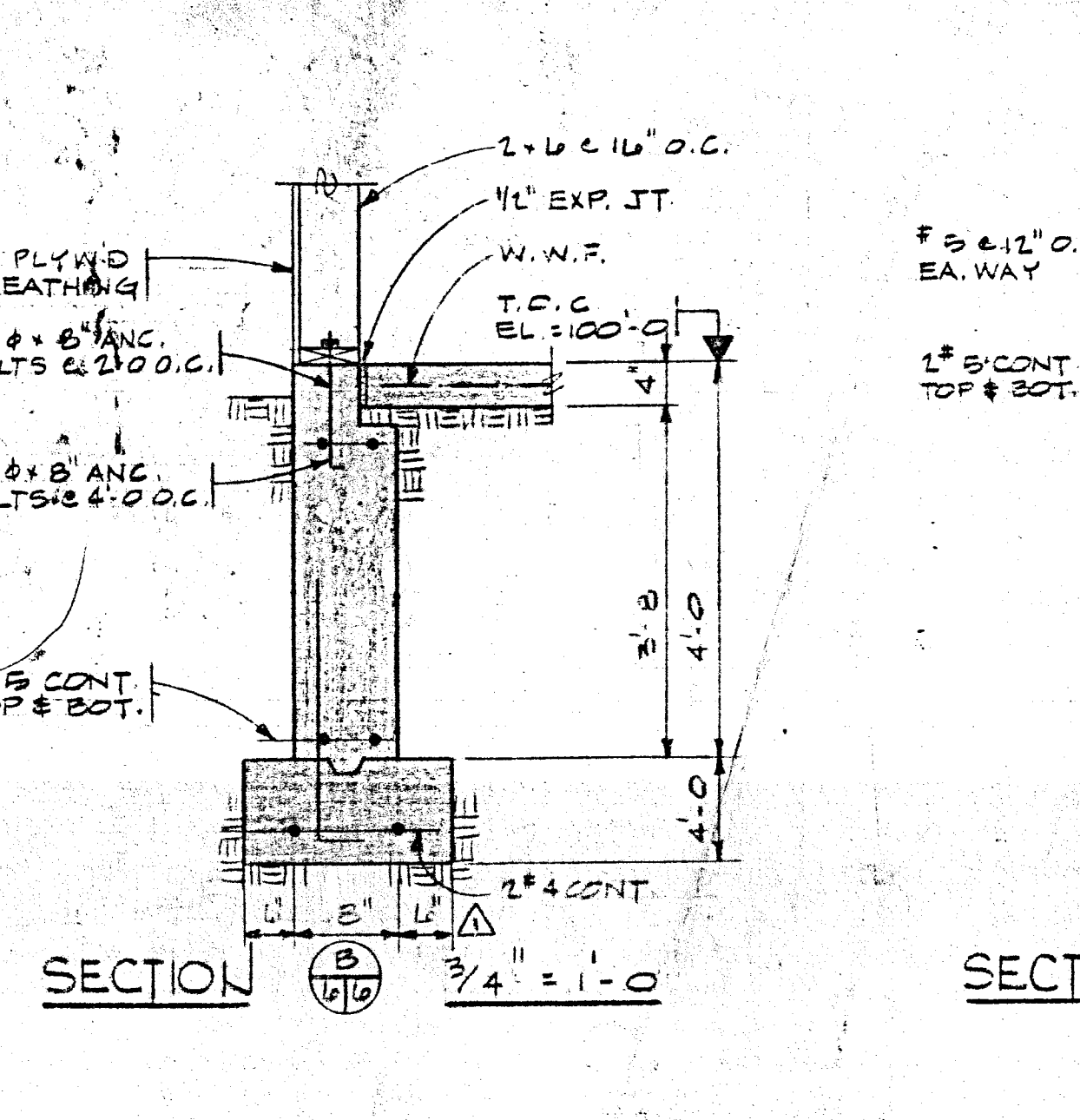
DETAIL 1
PLAN - COLUMN CAP



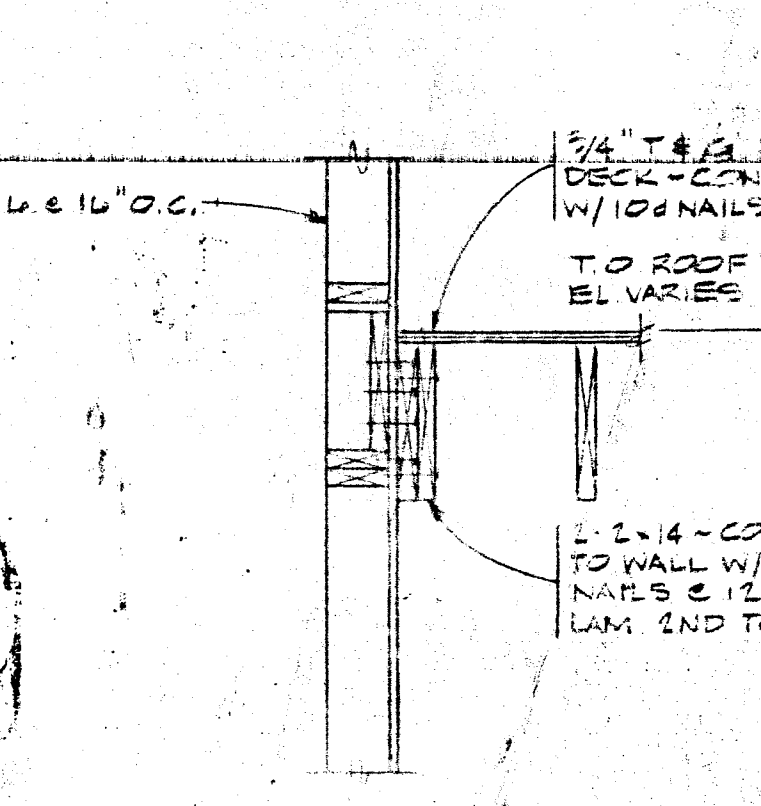
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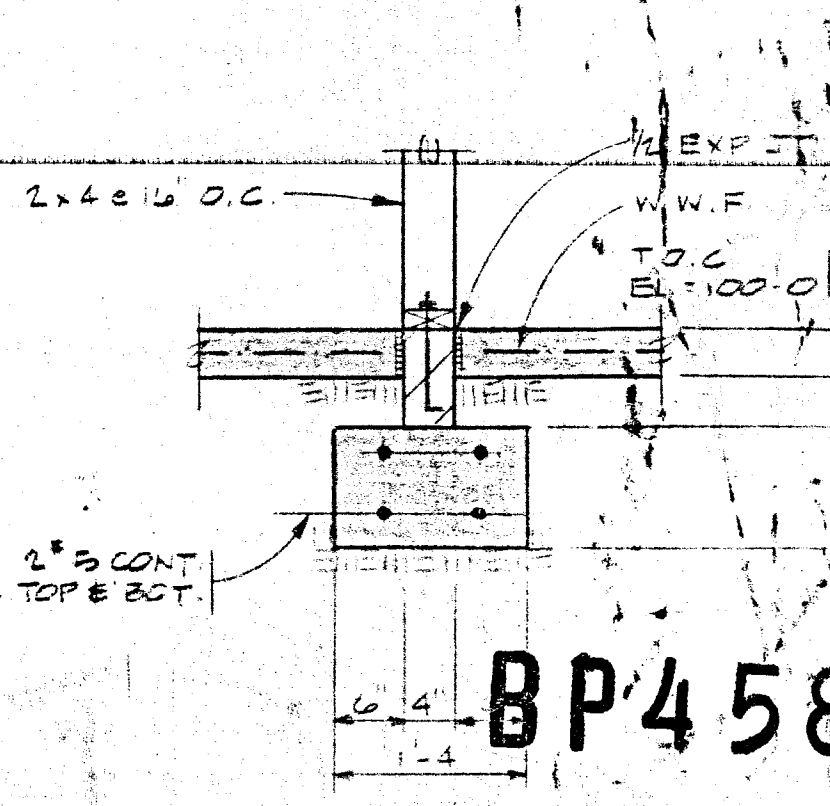
ROOF FRAMING PLAN 1/8" = 1'-0"



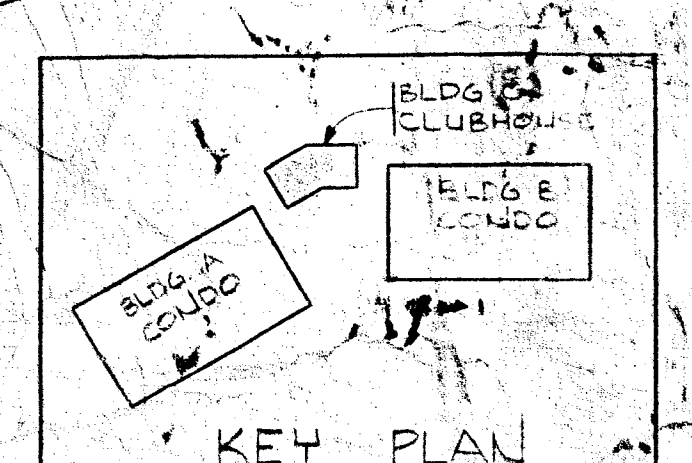
SECTION B-B 3/4" = 1'-0"



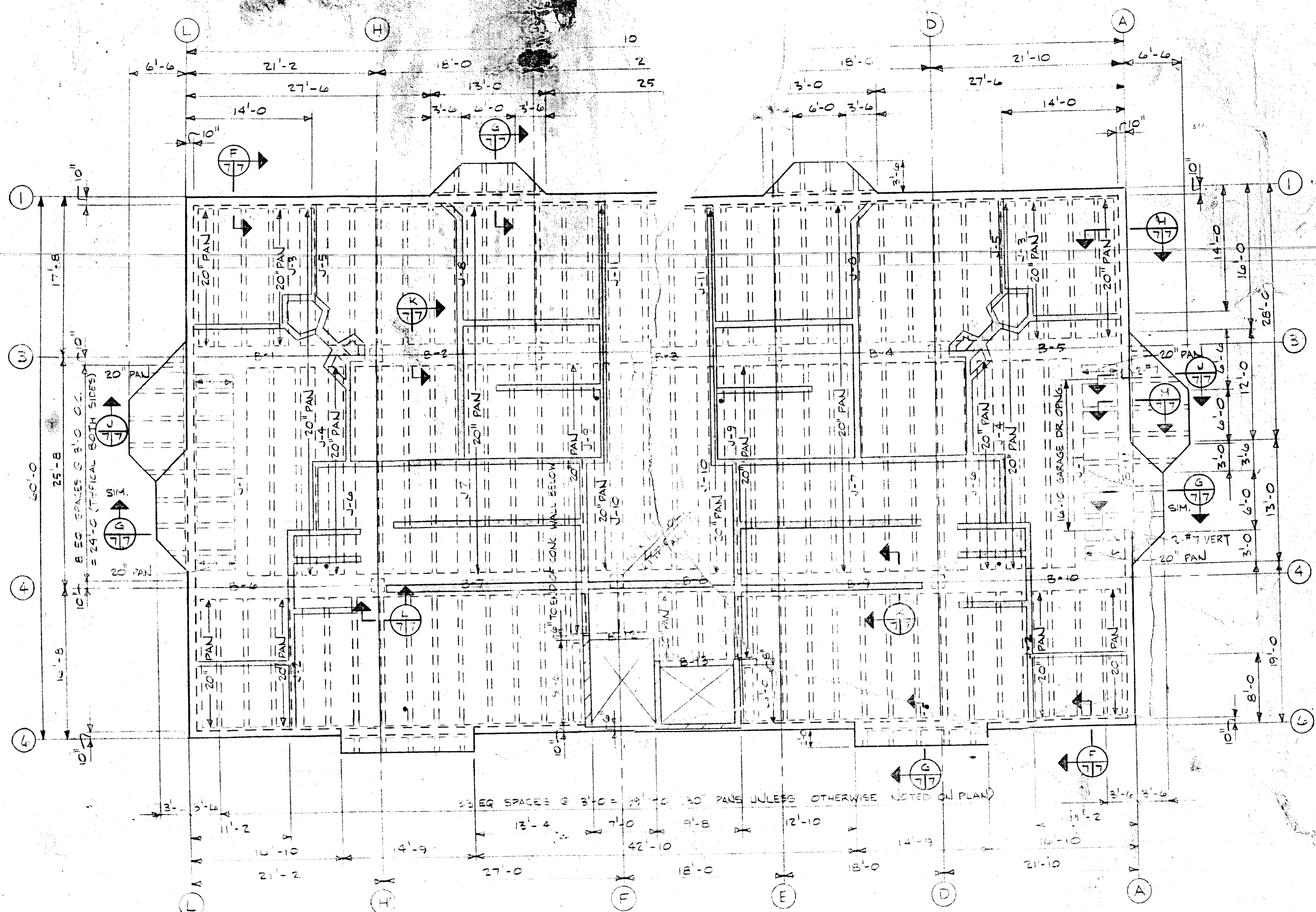
SECTION C-C 3/4" = 1'-0"



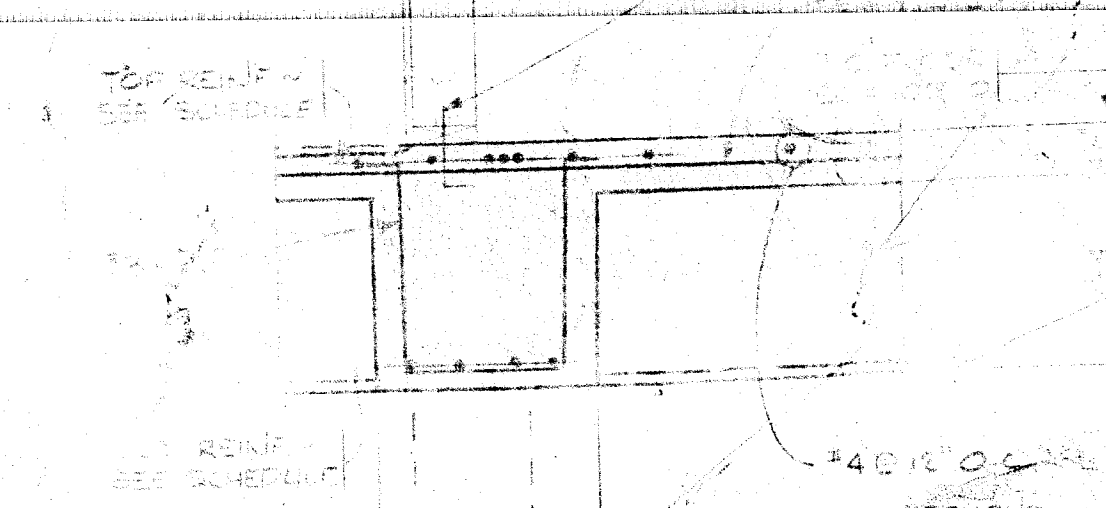
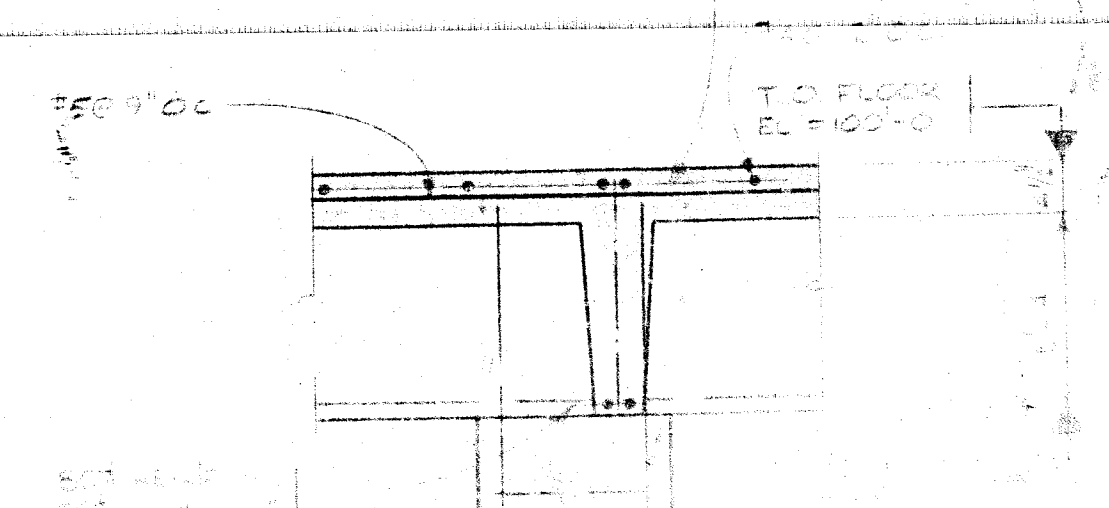
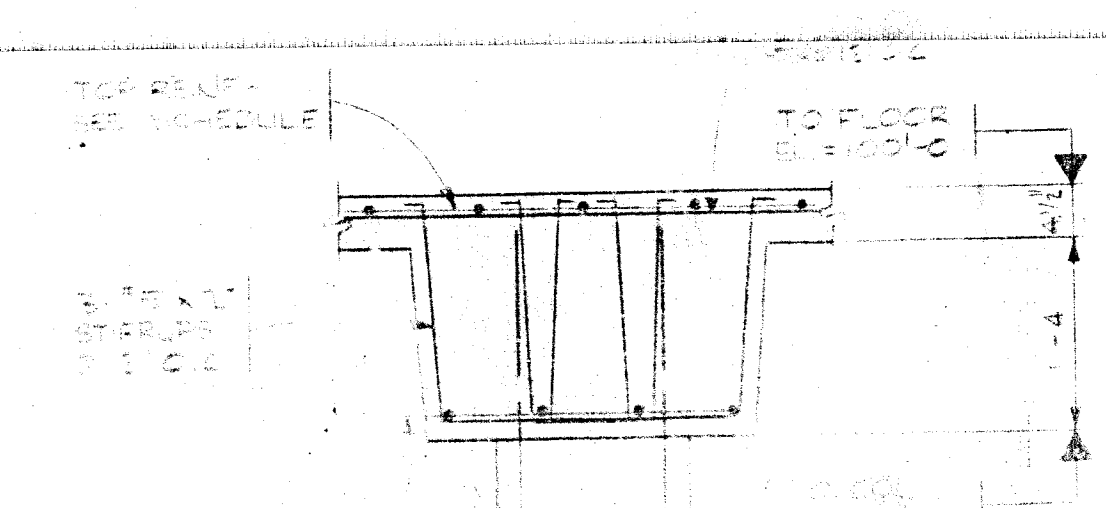
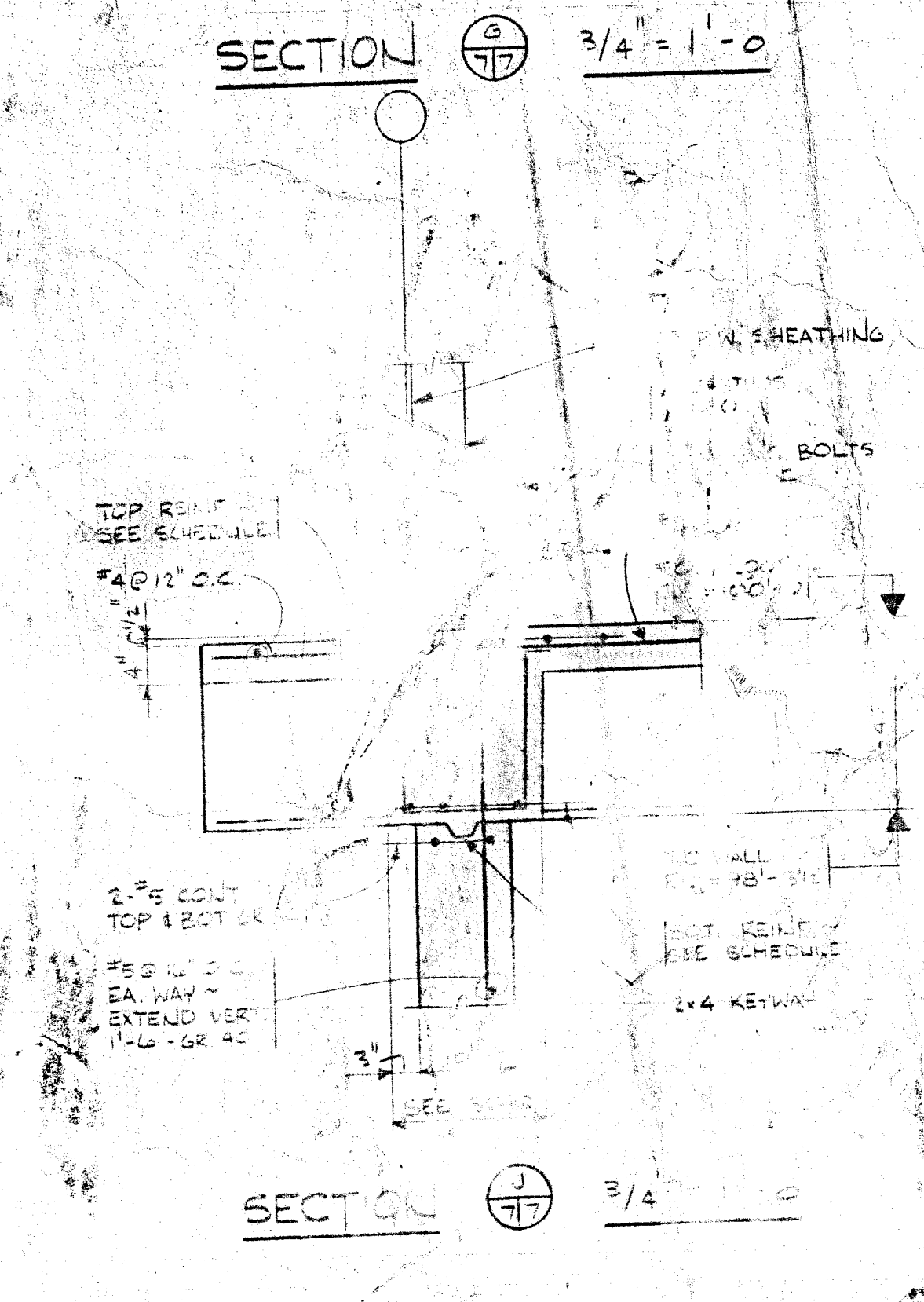
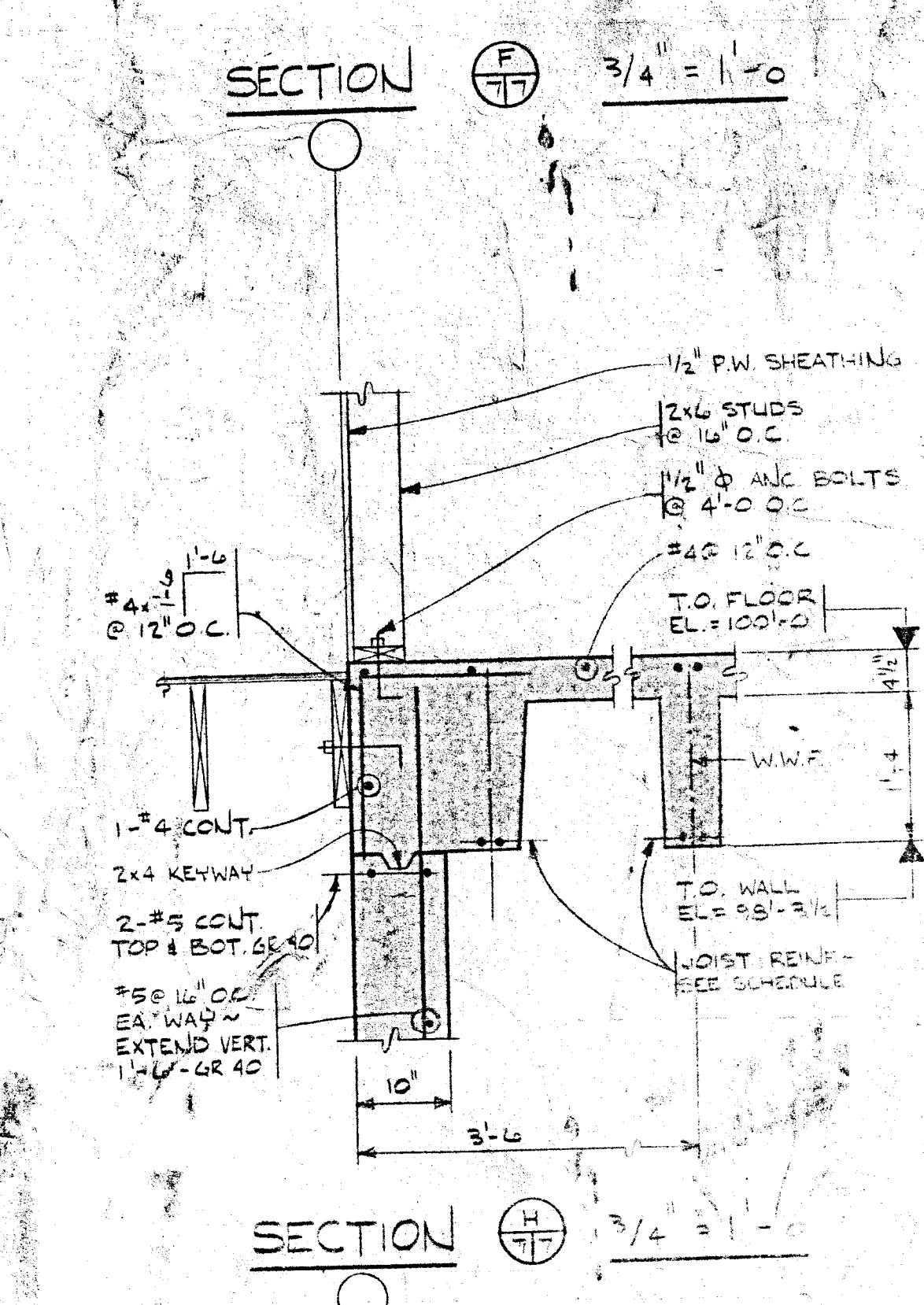
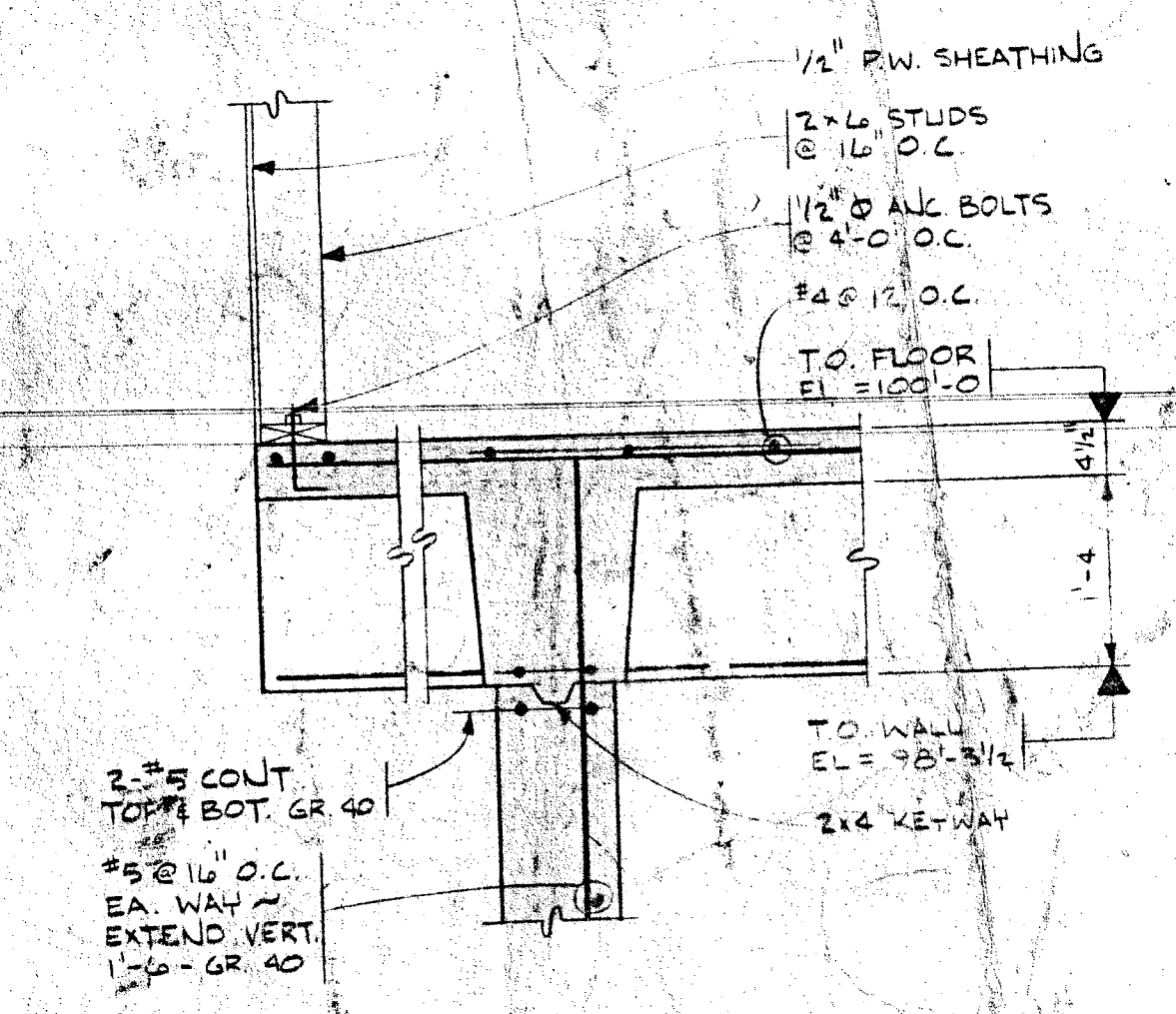
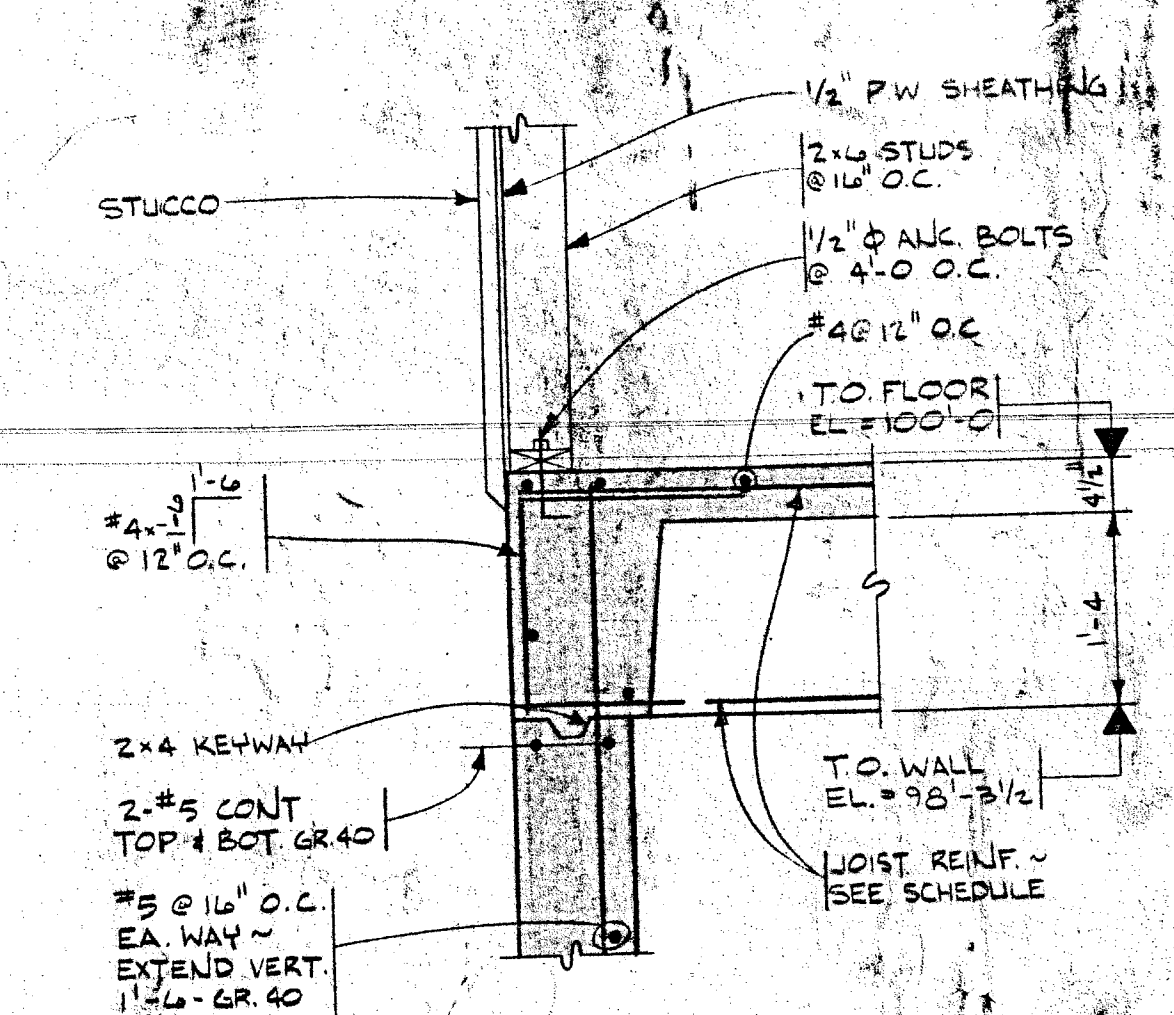
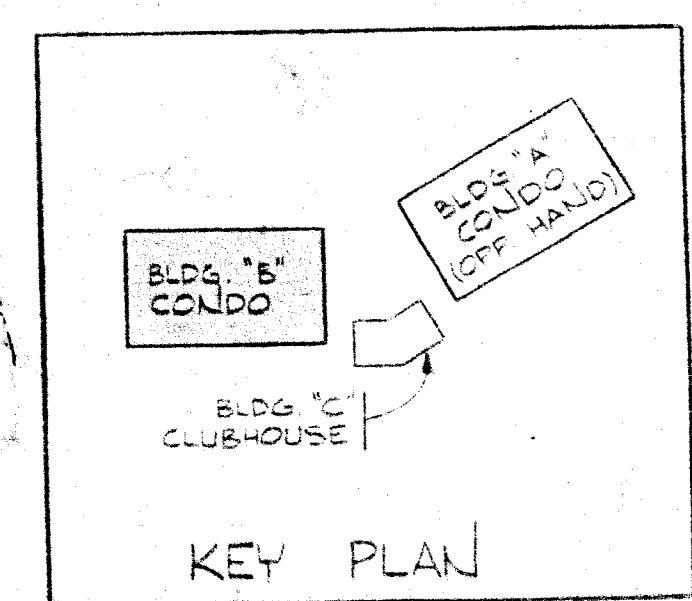
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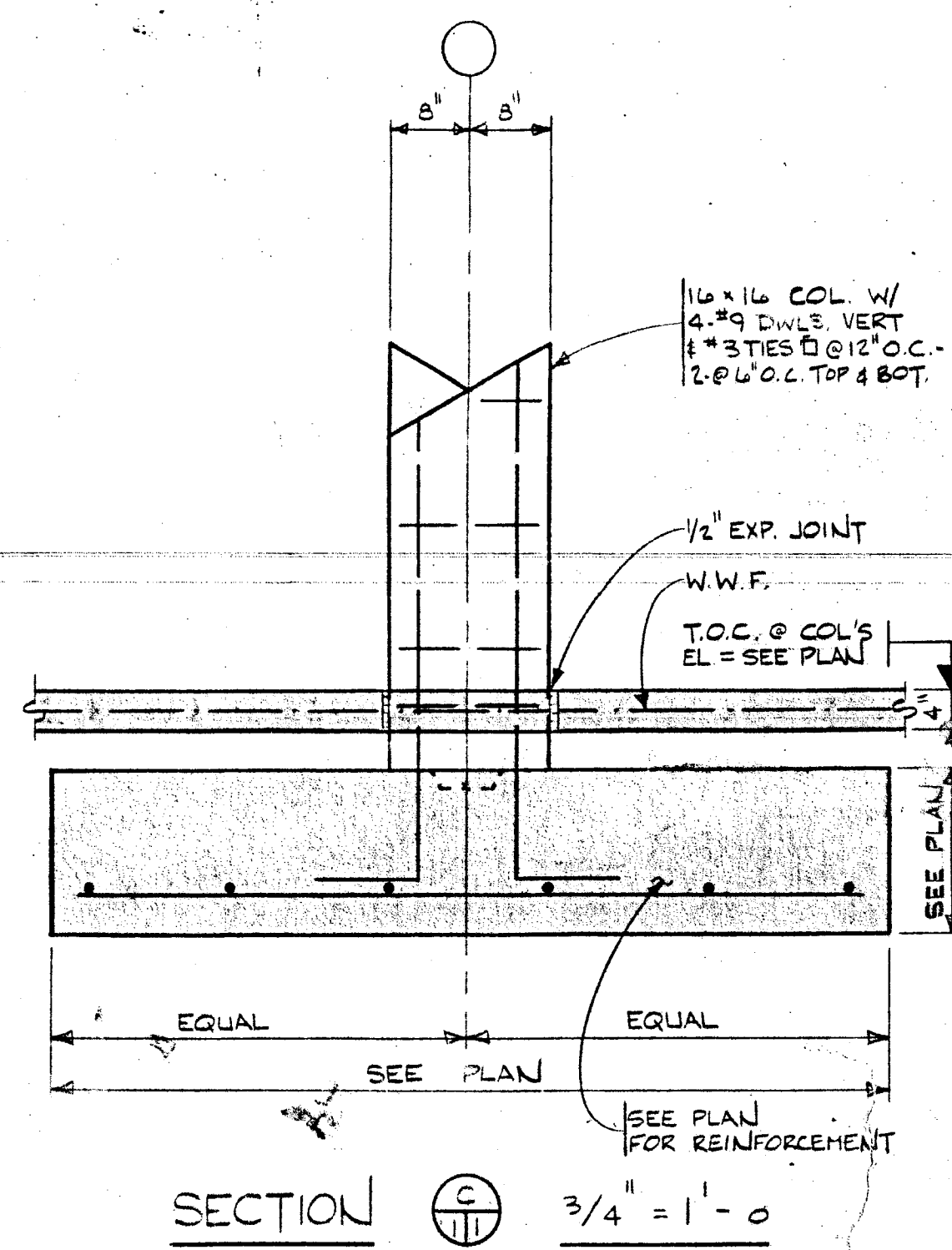
FIRST FLOOR FRAMING PLAN 1/8" = 1'-0"



BP4585

JOIST / BEAM SCHEDULE					
JOIST OR BEAM NO.	SIZE W x D	REINFORCEMENT			STIRRUPS LEFT TO RIGHT
		TOP LEFT	BOTTOM	TOP RIGHT	
J 16-B	6 x 20 1/2	2-#5	2-#6	4-#5	6x6-4/4 W.W.F.
J 25-B	6 x 20 1/2		2-#6	4-#5	6x6-4/4 W.W.F.
J 17-B	6 x 20 1/2		2-#6	2-#5	6x6-4/4 W.W.F.
J-1	16 x 20 1/2	6-#5	4-#6	6-#5	#3 @ 12" O.C.
J-2	16 x 20 1/2	2-#5	4-#5	6-#5	#3 @ 12" O.C.
J-3	16 x 20 1/2	6-#5	4-#5	2-#5	#3 @ 12" O.C.
J-4	16 x 20 1/2	6-#5	4-#6	6-#5	#3 @ 12" O.C.
J-5	16 x 20 1/2		4-#5	2-#5	#3 @ 12" O.C.
J-6	16 x 20 1/2	6-#5	4-#6	6-#5	#3 @ 12" O.C.
J-7	16 x 20 1/2	6-#5	4-#6	6-#5	#3 @ 12" O.C.
J-8	16 x 20 1/2		4-#5	2-#5	#3 @ 12" O.C.
J-9	16 x 20 1/2	6-#5	4-#6 EXT. 2 TOL.	6-#5	#3 @ 12" O.C.
J-10	16 x 20 1/2	6-#5	4-#6 EXT. 2 TOL.	6-#5	#3 @ 12" O.C.
J-11	16 x 20 1/2		4-#5	2-#5	#3 @ 12" O.C.
B-1	30 x 20 1/2	4-#9 CONT.	4-#8 + 3-#9	4-#9 + 2-#8 + 1-#9	4-#8 @ 12" 7-#8 @ 12"
B-2	30 x 20 1/2		4-#8	4-#9 + 5-#9 + 1-#10	4-#8 @ 12" 7-#8 @ 12"
B-3	30 x 20 1/2		4-#8 + 3-#11	4-#9 + 5-#9 + 1-#10	8-#8 @ 12" 8-#8 @ 12"
B-4	30 x 20 1/2		4-#8	4-#9 + 2-#8 + 1-#9	7-#8 @ 12" 6-#8 @ 12"
B-5	30 x 20 1/2		4-#8 + 3-#9	4-#9	8-#8 @ 12" 4-#8 @ 12"
B-6	30 x 20 1/2	4-#9	4-#8 + 4-#9	4-#9 + 2-#9 + 1-#10	4-#8 @ 12" 7-#8 @ 12"
B-7	30 x 20 1/2		4-#8	4-#9	7-#8 @ 12" 4-#8 @ 12"
B-8	30 x 20 1/2		4-#8	4-#9 + 3-#9 + 1-#10	5-#8 @ 12" 8-#8 @ 12"
B-9	30 x 20 1/2		4-#8 + 4-#9	4-#9 + 3-#9 + 1-#10	7-#8 @ 12" 7-#8 @ 12"
B-10	30 x 20 1/2		4-#8 + 2-#7	4-#9	7-#8 @ 12" 6-#8 @ 12"
B-11	16 x 20 1/2	3-#8 + 10-#5	2-#8	3-#8 + 10-#5	#3 @ 12"
B-12	8 x 20 1/2		2-#6 T & B		6x6-4/4 W.W.F.
B-13	8 x 20 1/2		2-#6 T & B		6x6-4/4 W.W.F.

5- SET, REFER SECTION ③



SECTION ③ 3/4" = 1' - 0"

GENERAL NOTES	
CONCRETE	
A.	ALL CONCRETE SHALL BE MADE WITH STONE AGGREGATE AND TYPE I PORTLAND CEMENT AND SHALL DEVELOP 4,000 PSI MINIMUM COMPRESSIVE STRENGTH IN 28 DAYS. AIR ENTRAINMENT SHALL BE 4-1/2 PERCENT PLUS OR MINUS 1-1/2 PERCENT.
B.	SLABS, BEAMS, AND JOISTS SHALL NOT HAVE JOINTS IN A HORIZONTAL PLANE. ANY STOP IN CONCRETE WORK MUST BE MADE AT CENTER OF SPAN WITH VERTICAL BULKHEADS AND HORIZONTAL KEYS UNLESS OTHERWISE SHOWN. ALL CONSTRUCTION JOINTS SHALL BE AS DETAILED OR AS APPROVED BY THE ARCHITECT.
C.	ALL EXPOSED EDGES OF CONCRETE SLABS AND WALLS SHALL HAVE A 1/2" CHAMFER.
D.	ALL CONCRETE WORK AND REINFORCEMENT DETAILING SHALL BE IN ACCORDANCE WITH ACI BUILDING CODE 318-82.
REINFORCEMENT	
A.	ALL REINFORCING SHALL BE HIGH STRENGTH DEFORMED BARS (ASTM DESIGNATION A615, GRADE 60) WITH 60,000 PSI MINIMUM YIELD POINT (EXCEPT #3 COLUMN TIES AND BEAM STIRRUPS WHICH MAY BE GRADE 40 DEFORMED BARS).
B.	WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185-70 AND SHALL BE LAPPED ONE FULL MESH AT SIDE AND END SPLICES AND SHALL BE WIRE TOGETHER.
C.	REINFORCEMENT PROTECTION:
1.	CONCRETE POURED AGAINST EARTH. 3"
2.	CONCRETE POURED IN FORMS BUT EXPOSED TO WEATHER OR EARTH:
a.	IF BARS ARE LARGER THAN #5 2"
b.	IF BARS ARE #5 OR SMALLER 1-1/2"
3.	BEAMS AND COLUMNS
a.	PRIMARY REINFORCEMENT. 1-1/2"
b.	TIES, STIRRUPS 1"
4.	SLABS, WALLS, JOISTS 1"
D.	ALL BAR LENGTHS ARE DRAWN TO SCALE UNLESS NOTED. NO SPLICES OF REINFORCEMENT SHALL BE MADE EXCEPT AS DETAILED OR AUTHORIZED BY THE STRUCTURAL ENGINEER. LAP SPLICES, WHERE PERMITTED, SHALL BE A MINIMUM OF 24 BAR DIAMETERS FOR VERTICALS, AND 36 BAR DIAMETERS FOR ALL OTHERS. MAKE ALL BARS CONTINUOUS AROUND CORNERS.
E.	DETAIL BARS IN ACCORDANCE WITH ACI DETAILING MANUAL AND ACI BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, LATEST EDITIONS.
F.	PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCING AT POSITIONS SHOWN ON THE PLANS.
G.	PLACE TWO #5 (1 EACH FACE) WITH 2'-0" PROJECTION AROUND ALL OPENINGS IN CONCRETE. WHERE OPENINGS ARE RECTANGULAR, ALSO, PROVIDE ONE #4 X 4'-0" DIAGONALLY AT EACH CORNER.
H.	CONTINUOUS TOP AND BOTTOM BARS IN JOISTS AND BEAMS SHALL BE SPLICED AS FOLLOWS:
1.	TOP BARS - AT MIDSPAN
2.	BOTTOM BARS - OVER SUPPORT
GENERAL	
A.	SEE SHEET S-5 FOR ALL OTHER NOTES

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WOODS FLAVOR

CONDOMINIUM PROJECT FOR

THE CITY OF CHILIPALA

BRECKENRIDGE, COLORADO