

GENERAL & STRUCTURAL NOTES

Miscellaneous Notes:

- The contractor shall verify all conditions and dimensions at the job site prior to commencing work. The contractor shall report all discrepancies between the drawings and existing conditions to the designer prior to commencing work.
- The contractor shall supply, locate and build into the wall all inserts, anchors, angles, plate openings, sleeves, hangers, slab depressions and pieces as may be required to attach and accommodate other work.
- All details and sections shown on the drawings are intended to be typical and shall be constructed to apply to any similar situation elsewhere in the work except where a different detail is shown.
- Subsurface soil condition information is not available. Foundations are designed for a soil bearing capacity of 2,000 PSF. The contractor shall report any differing conditions to the designer prior to commencing work.
- Structural drawings shall be used in conjunction with Job Specifications and Architectural, Mechanical, Electrical, Plumbing, and Site Drawings. Consult these Drawings for sleeves, depressions, and other details not shown on Structural Drawings.
- All bolts, nuts, washers, straps, and fasteners including nails, shall be Hot-Dip Galvanized or Stainless Steel. Continuous anchorage shall be provided between all trusses, wall sections, beams, posts, and footings with the use of straps and connectors as specified herein, except for interior framing.
- All specified fasteners may be substituted with equivalent fasteners. The installation of the fasteners shall be in accordance with the manufacturer's specifications.
- Wind bracing (1st and 2nd floors) with 18 Gauge 1-1/4" galvanized straps 10'-0" long placed diagonally, all bearing walls all corners with 4'-0" X 10'-0" or 8'-0" X 5'-0" exterior grade plywood siding.
- The structure is designed to be self-supporting and stable after the building is complete. It is the contractor's sole responsibility to determine erection procedures and sequence to ensure safety of the building and its components during erection. This includes the addition of necessary shoring, sheathing, temporary bracing, girts or bracing.

Concrete Notes:

- Concrete shall have (34" Maximum Diameter Aggregate) a minimum compressive strength of 3,000 PSI at 28 days, unless noted otherwise. Placement shall be in accordance with ACI 318.
- All concrete slabs on grade shall be the thickness indicated on the drawings, over minimum 5/8" Polyethylene Vapor Barrier. Such slabs shall be reinforced with 3 X 6 #4/11 X W 14 Welded Wire Fabric lapped 6" at edges and ends.
- Fill under concrete slabs shall be clean sand or rock and free of debris and other deleterious material. Fill shall be compacted to a density of at least 95% of Standard Proctor Maximum Dry Density (ASTM D1557).
- Footings shall bear upon undisturbed treated soil or upon soil compacted to at least 95% of Standard Proctor Maximum Dry Density (ASTM D1557) for a depth of at least three (3) feet below the bottom of the footing.
- Reinforcing steel shall be ASTM A-615 Grade 60 Deformed Steel. All continuous vertical and horizontal reinforcing steel in footings, beams, and columns shall be lap spliced a minimum of 30 Bar Diameters or 12", whichever is greater.
- All 90-degree hooks with side cover (perpendicular to plane of hook) not less than 2.5 inches, and cover on bar extension beyond hook (also called end cover) not less than 2 inches; shall be embedded in support member a minimum of 15.5 Bar Diameters; and the hook shall be extended at least 12 Bar Diameters at the free end of the bar within the support member.
- All 90-degree hooks with side cover (perpendicular to plane of hook) less than 2.5 inches, and cover on bar extension beyond hook (also called end cover) less than 2 inches shall be embedded in support member a minimum of 22 Bar Diameters; and the hook shall be extended at least 12 Bar Diameters at the free end of the bar within the support member.

- Concrete cover and spacing of reinforcing steel shall be as follows:
A. Footings: 3" Bottoms and Sides, 2" Top
B. Beams: 1-1/2" Bottom, Sides and Top
C. Columns: 1-1/2"
D. Slabs on Grade: 2" Bottom, 1" Top
E. Other: C to C: 1" Bar Diameter or 1", whichever is greater
(Center to Center spacing of rebar within concrete.)
F. Others: Per ACI
- Welded wire fabric shall conform to ASTM A185, free from oil, scale, rust and placed in accordance with the typical place details of ACI Standards and Specifications. Minimum lap shall be one space plus two (2) inches.

Concrete Mono Footer Notes:

- These notes and specifications apply to all concrete mono footers unless noted otherwise
- See Concrete Notes for the concrete, and steel strengths and specifications.
- All reinforcement shall have a minimum of 3" cover on all sides
- All horizontal reinforcement shall be continuous.
- All horizontal reinforcement bars shall be evenly spaced horizontally within the footer minus the 3" cover on each side.
- All horizontal reinforcement bars shall have a minimum horizontal spacing within the footer of (1) horizontal bar diameter or 1" whichever is greater.
- All horizontal reinforcement bars shall be located at the depth specified in plan
- All vertical reinforcement that terminates within the footer shall have (1) 90-degree hook of equal bar size as the vertical from above for each vertical reinforcing bar
- All 90-degree hooks shall be bent into the end of the vertical steel or lap spliced to the vertical steel.
- All lap splices shall be a minimum of 30 (18.75" for # 5 bar) bar diameters in concrete and 48 (30" for # 5 bar) bar diameters in masonry or 12" whichever is greater
- All 90-degree hooks shall have a minimum of 12 bar diameters (8" for # 5 bar) extension.

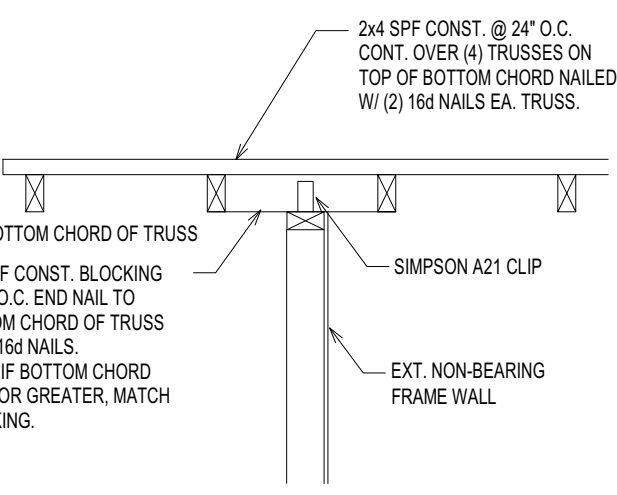
- When specified by footing detail, slabs under vertical reinforced masonry walls shall have a #4 bar with a 90 degree hook, extending 30" into slab @ a depth of 2" below slab surface & extending 9" vertically down the stem wall @ 3" clear distance from exterior edge of stem wall or @ the center line of an interior stem wall. Provide these hooks @ every reinforced vertical call extending up to the tie beam (except where noted otherwise) and within wall opening @ 48" o.c. from vertical at each end of wall opening.
- When specified by footing detail, slabs under bearing frame walls shall have a #4 bar with a 90 degree hook extending 30" into slab @ a depth of 2" below slab surface, extending 9" vertically down the stem wall @ 3" clear distance from exterior edge of stem wall or @ the center line of an interior stem wall. Provide these hooks @ 48" o.c. under frame, @ every post location, and within wall opening @ 48" o.c. from post hook at each end of wall opening.

Grout Filled Masonry Column Notes:

- These notes and specifications apply to all Grout Filled Masonry Columns unless noted otherwise:
- See the Masonry Notes for the masonry, grout, and steel strengths and specifications.
- All reinforcement shall have a minimum cover of 1/2" between the interior surface of the C.M.U. cell and all sides of the reinforcement.
- All vertical steel shall be placed so that it is as close to the interior surfaces of the C.M.U. without violating the cover requirements.
- All vertical steel shall have a clear distance between bars not less than 1-1/2 times the vertical bar diameter, or less than 1-1/2" whichever is greater.
- All lateral ties shall have a vertical spacing not exceeding 16 longitudinal (vertical) bar diameters, 48 lateral tie bar or wire diameters, or least cross-sectional dimension of the column.
- All lateral ties shall be located vertically not more than a lateral tie spacing above the top of the footing or slab in any story, and shall be located vertically not more than a lateral tie spacing below the lowest horizontal reinforcement in the beam, girder, slab, or drop panel.
- All vertical bars shall terminate at both ends with (1) 90-degree hook of equal bar size as the vertical.
- All 90-degree hooks shall either be bent into the end of the vertical steel or lap spliced to the vertical steel.
- All lap splices shall be a minimum of 48 (30" for # 5 bar) bar diameters in masonry or 12" whichever is greater.
- All 90-degree hooks shall have a minimum of 12 bar diameters (8" for # 5 bar) extension.
- All vertical steel shall be terminated 2" plus 1/2" bar diameter from the center of the extension to the top of the tie-beam and provide a 90-degree hook with an extension that is parallel to the horizontal steel with in the tie-beam.
- All hooks in the tie-beams shall have a minimum of 2.5" side cover and 2" end cover.
- All vertical steel shall be terminated 3" plus 1/2" bar diameter from the center of the extension to the bottom of the footer and provide a 90-degree hook with an extension that is parallel with the horizontal steel.
- All hooks in the footers shall have a minimum of 2.5" side cover and 3" end cover.

Concrete Beam Notes:

- These notes and specifications apply to all concrete beams unless noted otherwise
- The typical Tie-Beam is an 8" x 16" concrete beam, with (2) #5 bars top and bottom, with no stirrups.
- See the Concrete Notes for the concrete and steel strengths and specifications
- Stirrups may be used to secure top steel during concrete pouring in beam with no stirrups
- Maintain 1-1/2" cover on all steel within the beam.
- Horizontal steel shall be placed so that it is as close to the exterior surfaces of the beam without violating the cover requirements.
- Horizontal steel over masonry walls shall be spliced in a vertical plane
- Horizontal steel over openings shall be spliced in a horizontal plane within 1/3 of the beams span from the supports.
- All horizontal steel shall be continuous and shall be lap spliced a minimum of 30 bar diameters (18.75" for # 5 bar) or 12" whichever is greater
- See the Plan View of Tie-Beam Reinforcement for Corners and Intersections detail for the steel placement and splicing at these locations.



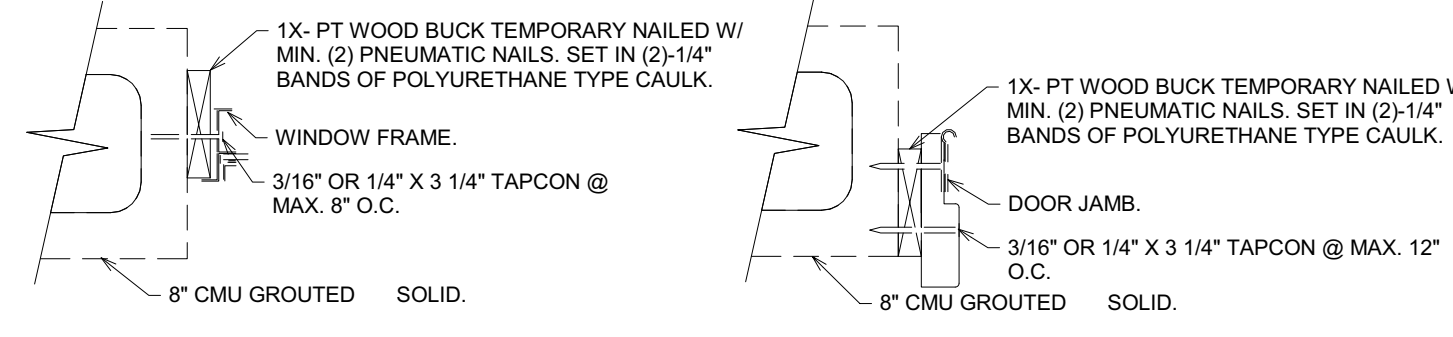
FRAME WALL PARALLEL TO TRUSS N.T.S.

Wood Framing Notes:

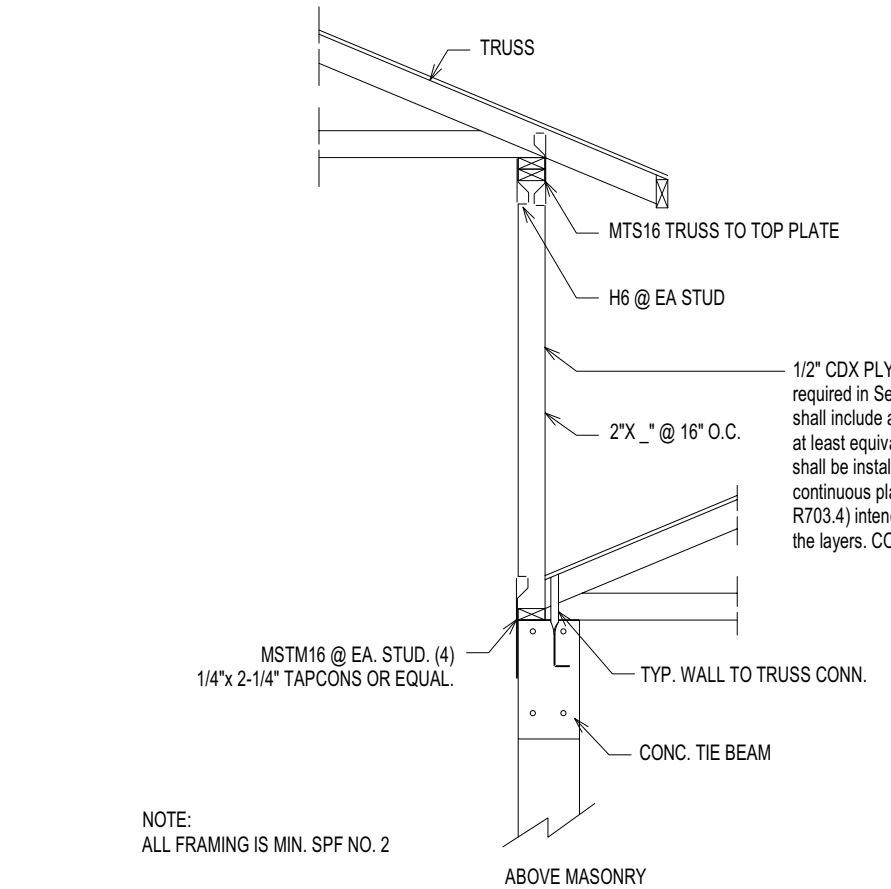
- All wood framing shall be fabricated and installed per ATIC and TPI and National Design Specifications for Wood Construction.
- All structural wood members shall be Southern Yellow Pine, unless noted otherwise.
- Unless noted otherwise, the following minimum grades shall be used:
A. Structural Lignum Framing 2" x 4" Thick x 2" x 4" wide; #2 non-dense or better.
B. Studs Size 2" x 4" Thick x 2" to 6" wide; #2 dense or better.
C. Structural Joist and Planks size 2" x 4" Thick x 4" to 6" wide; #2 or better; 6" to 12" wide; #1 or better.
D. Light Framing Size 2" to 4" Thick x 2" to 4" wide; #2 non-dense or better.
E. Alternate partition wall framing construction: SPF or Hem-Fir Stud, Non-bearing.
- Structural Wood Panels (Sheathing) shall be specified by thickness, grade, APA Span Rating, and Exposure Durability Classification as shown by the drawing. Nail as specified per detail or note.
- All wood members exposed to weather or in contact with masonry, concrete or soil shall be pressure treated. Clearance between wood siding and earth on the exterior of a building shall not be less than 6 inches except where siding, sheathing and wall framing are of approved pressure treated wood.
- Contractor shall provide all fastening devices as shown on the drawings and as necessary and suited for each application. Fastening subject to moisture shall be Hot-Dip Galvanized to ASTM A-153-80, or Stainless Steel.
- All metal connections and fabrications shall comply with AISC Specifications.
- Solid block joists and rafters at interior points of support.
- Pre-fabricated structural trusses shall comply with NFPA, National Specifications for Wood Construction, TPI Design Specifications for Metal-Plated Wood Trusses and ATIC 100.
- All trusses shall be designed and certified by the Truss Manufacturer's State of Florida Registered Engineer.
- Contractor shall consult with Truss Manufacturer to ensure that adequate bearing is provided at end reactions of all Girder Trusses.
- Truss Manufacturer shall submit shop drawings to the Contractor and Designer for Review and approval prior to fabrication. Contractor shall be responsible for field Verification of dimensions, materials and conditions.
- All volume ceiling conditions, ceiling trusses to provide a smooth and unbroken interior wall surface from floor to ceiling.
- Brace trusses during erection and after permanent installation to comply with TPI BWT-76
- Inco-Lams (or equal Parallams, LVL's, etc.) shall be used where specified on Engineered plans and installed in accordance with Manufacturer's recommendations. Any girders or ends exposed to the weather shall be protected by the installation of 2x6s. Minimum Gypsum based Steel Sheathing.
- Sllices in multi-board continuous beams shall be allowed for one board only per span and only at the quarter point of the span, unless shown otherwise.
- Double top plates shall be overlapped at corners and secured with 4-10d nails. Mid-wall splices shall be 48" minimum w/ (2) rows of 10d nails at 4" O.C., staggered.

- Initial attachment of PT wood bucks to masonry openings is at the installers discretion and may be but not limited to adhesives or case hardened nails manually or pneumatically driven as long as the buck is not split. Permanent attachment of the window/door frame and PT buck is as shown above.
- Refer to MFG. CUT SHEETS FOR ADDITIONAL REQUIREMENTS FOR THE SPECIFIC WINDOW OR DOOR. THE MFG. SIZE AND SPACING OF ATTACHMENTS SUPERCEDE DETAILS ABOVE.

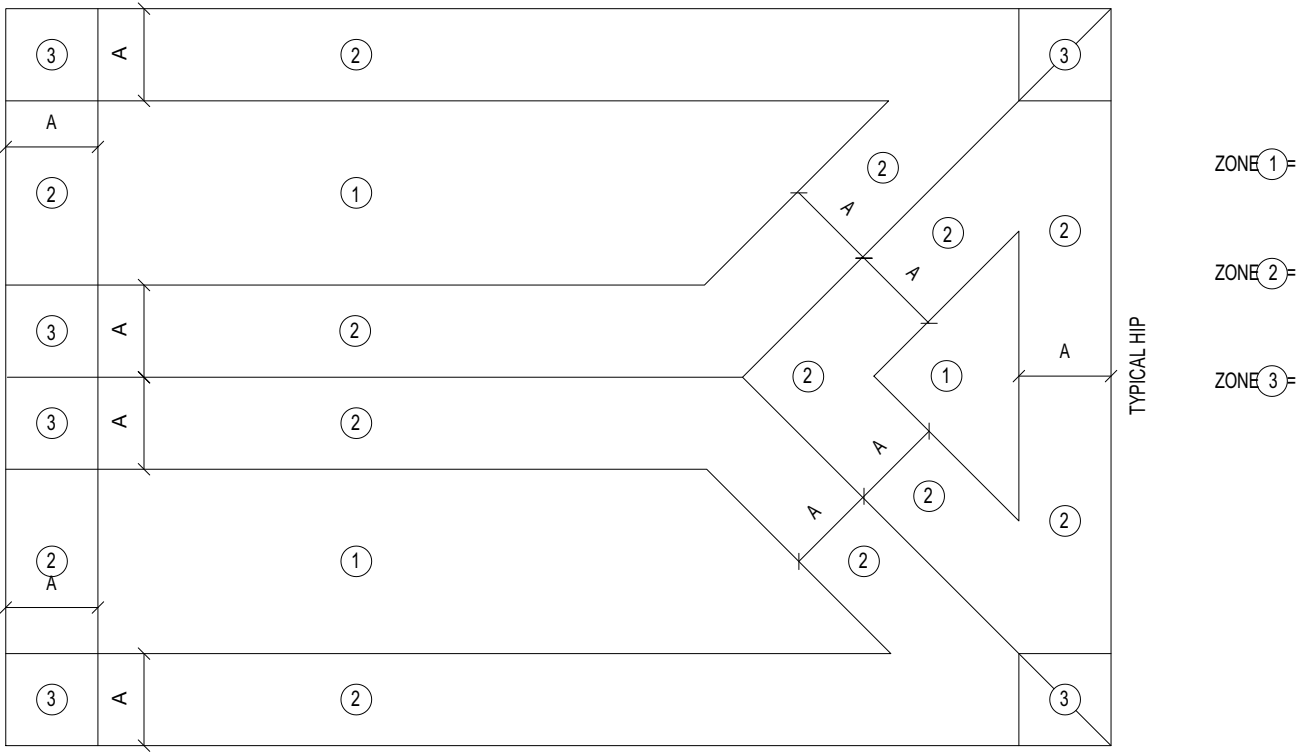
TYPICAL WINDOW / DOOR ATTACHMENT DETAILS N.T.S.



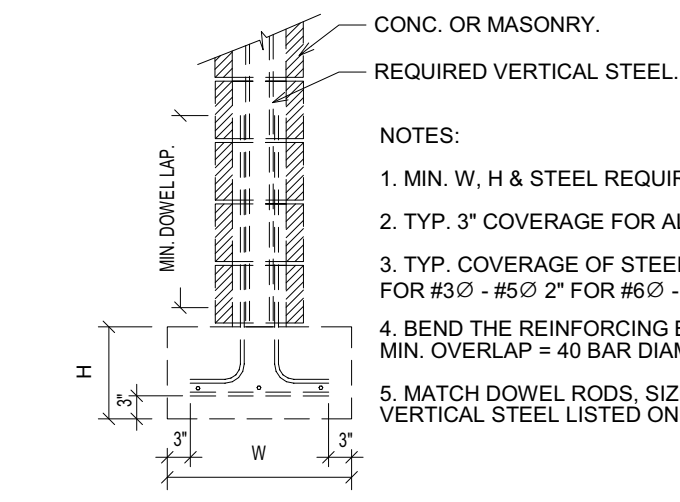
TYPICAL FRAME KNEE WALL ON MFG. TRUSS DETAIL N.T.S.



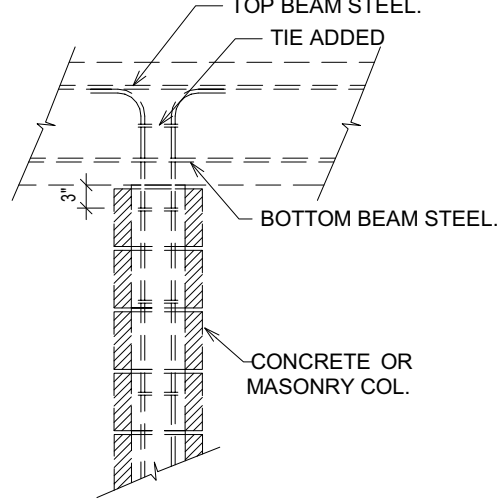
TYPICAL KNEEWALL W/ TRUSS N.T.S.



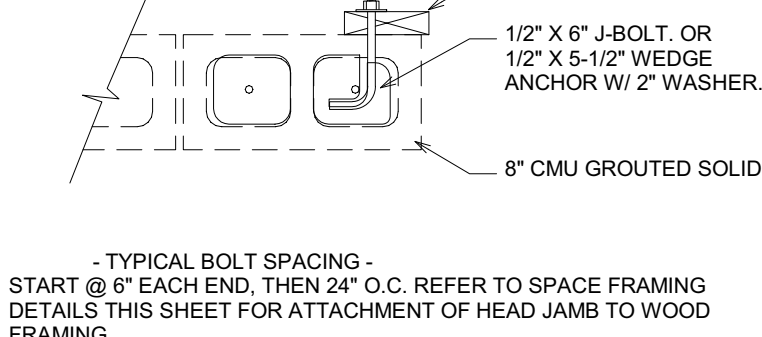
ROOF SHEATHING NAILING SPECIFICATIONS N.T.S.



CONCRETE OR MASONRY COLUMN TO FOOTING N.T.S.



CONCRETE OR MASONRY COLUMN TO BEAMS N.T.S.



TYPICAL GARAGE DOOR JAMB DETAIL N.T.S.

HEADER SCHEDULE						
OPENING	SIZE	KING STUDS	0 TO 12'-0\"			
			(1) EA. SIDE	(1) EA. SIDE		
0 TO 3'-4"	(2) 2x6 S.P.F. W/ 1/2" PLYWOOD	(1) EA. SIDE	(1) EA. SIDE	(1) EA. SIDE	(1) EA. SIDE	
3'-4" TO 3'-8"	(2) 2x6 S.P.F. W/ 1/2" PLYWOOD	(1) EA. SIDE	(1) EA. SIDE	(1) EA. SIDE	(1) EA. SIDE	
3'-8" TO 10'-0"	(2) 2x12 S.P.F. NO. 2 W/ 1/2" PLYWOOD	(1) EA. SIDE	(2) EA. SIDE	(1) EA. SIDE	(1) EA. SIDE	
10'-0" TO 12'-0"	(2) 2x12 S.P.F. NO. 2 W/ 1/2" PLYWOOD	(1) EA. SIDE	(2) EA. SIDE	(1) EA. SIDE	(2) EA. SIDE	

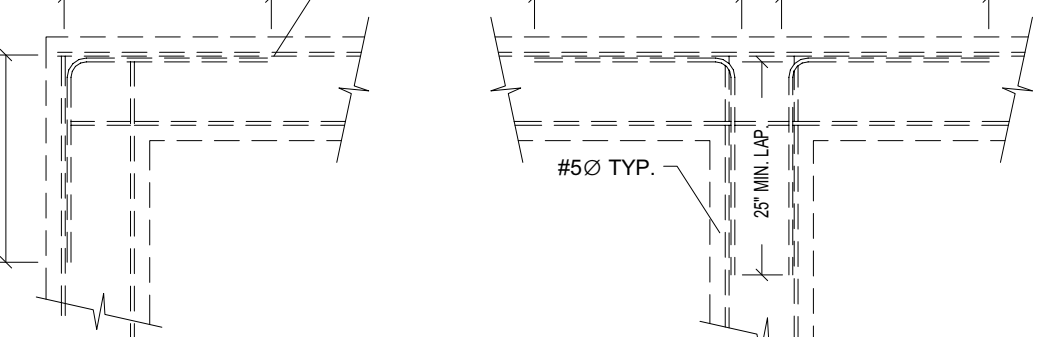
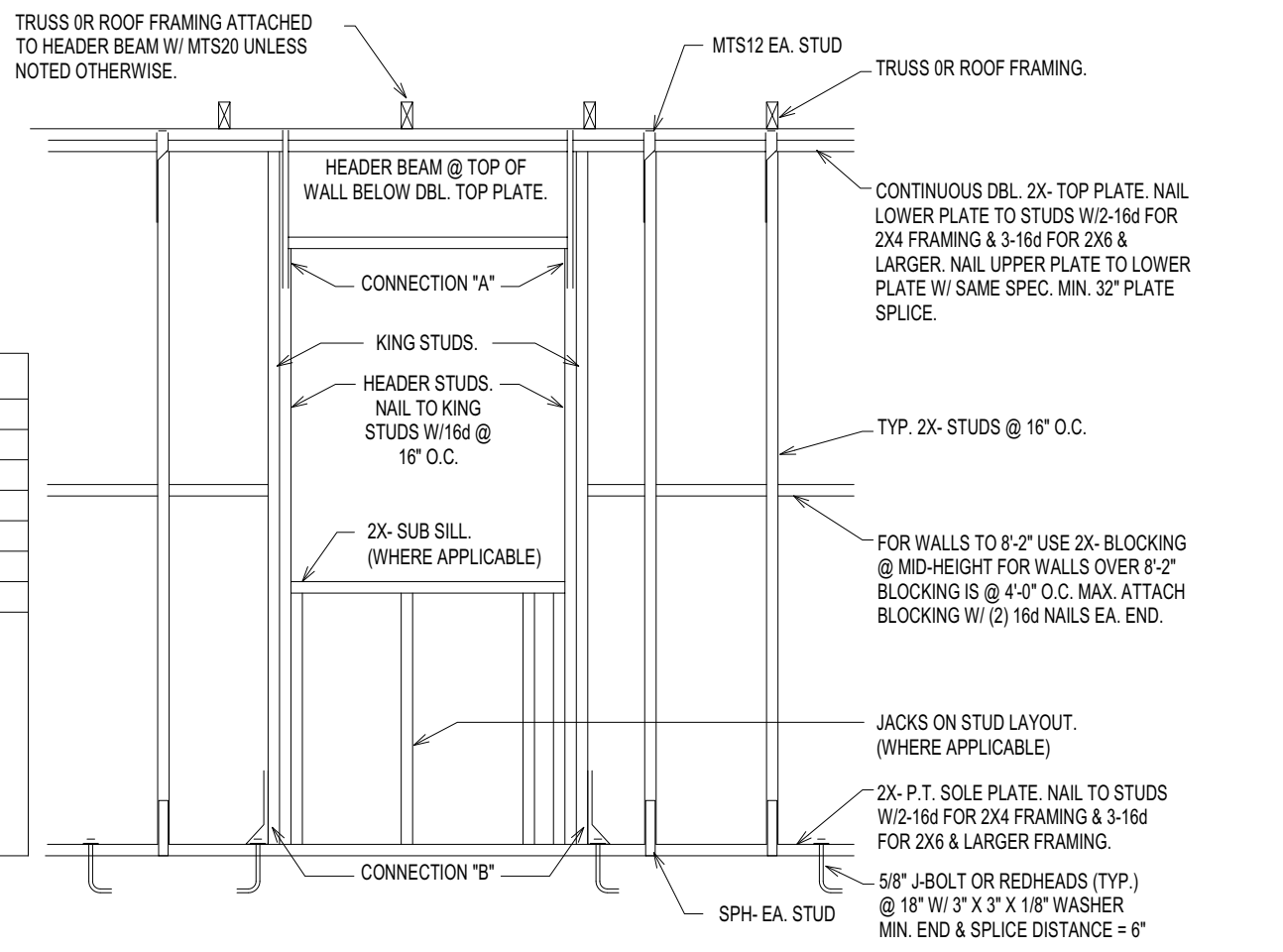
NOTE:

- HEADER BEAMS ARE DESIGNED FOR A GRAVITY LOAD OF 1500#/TRUSS AND AN UPLIFT LOAD OF 1000#/TRUSS. REFER TO STRUCTURAL PLAN FOR ADDITIONAL HEADER BEAM SPECS.
- ALL HEADER BEAMS SHALL BE BUILT UP WITH 1/2" PLYWOOD FILLER. NAILING SHALL BE 12d NAILS @ 16" O.C. ALONG EACH-EDGE SPLICES. IF NECESSARY, SHALL BE LOCATED @ 1/4 THE LENGTH OF THE BEAM BETWEEN SUPPORTS.
- SPACE FRAMING FROM HEADER TO TOP OF WINDOW/DOOR OPENING IF NECESSARY.

- NOTE:
- BEARING WALL DESIGNED FOR A GRAVITY LOAD OF 1500#/TRUSS AND AN UPLIFT LOAD OF 1000#/TRUSS. REFER TO STRUCTURAL PLAN FOR ADDITIONAL HEADER BEAM SPECS.
 - REFER TO FLOOR PLAN FOR FRAMING MATERIAL, SIZE & SPACING.
 - FASTEN 2X- BEAMS & STUDS W/ 16d AT 16" O.C.
 - SPACE FRAMING FROM HEADER TO TOP OF WINDOW/DOOR OPENING IF NECESSARY.
 - EQUIVALENT FASTENERS OF OTHER MANUFACTURES MAY BE SUBSTITUTED FOR THE LISTED FASTENERS.

TYPICAL BRG. FRAME WALL DETAIL (ALL WALL HEIGHTS) N.T.S.

INTERIOR, EXTERIOR & EXTERIOR NON BRG.



CORNER LOCATION N.T.S.

INTERSECTION N.T.S.

CONCRETE BEAM CORNERS N.T.S.

NOTE:

- PROVIDE FILLED CELL AND VERTICAL REINFORCEMENT AT CORNERS.
- BEND THE REINFORCING BARS OR USE STD. CORNER BARS. MIN. OVERLAP = 40 BAR DIAMETERS.

CADD Services Provided By:
J.R. ROST Associates, Inc.
561 Lee Street, Suite 201
Fort Myers, Florida 33916
Tel: 239.935.9500 Fax: 239.935.2125

Master by:
J.R. Rost Associates, Inc.
Engineers, Planners, & Development Consultants
4301 Veronica Shores Blvd. - Fort Myers, Florida 33916 - 239.935.5222
Professional Seal Number: 14670

Reviewed for Code Compliance
By: Andrew Nielsen Date: 6/28/2021
RESMSTR2021-00191

DO NOT SCALE DRAWINGS. DIMENSIONS GOVERN. VERIFY ALL DIMENSIONS AND CONDITIONS ON SITE AND NOTIFY ENGINEER IN WRITING OF ANY VARIATIONS BEFORE STARTING CONSTRUCTION.

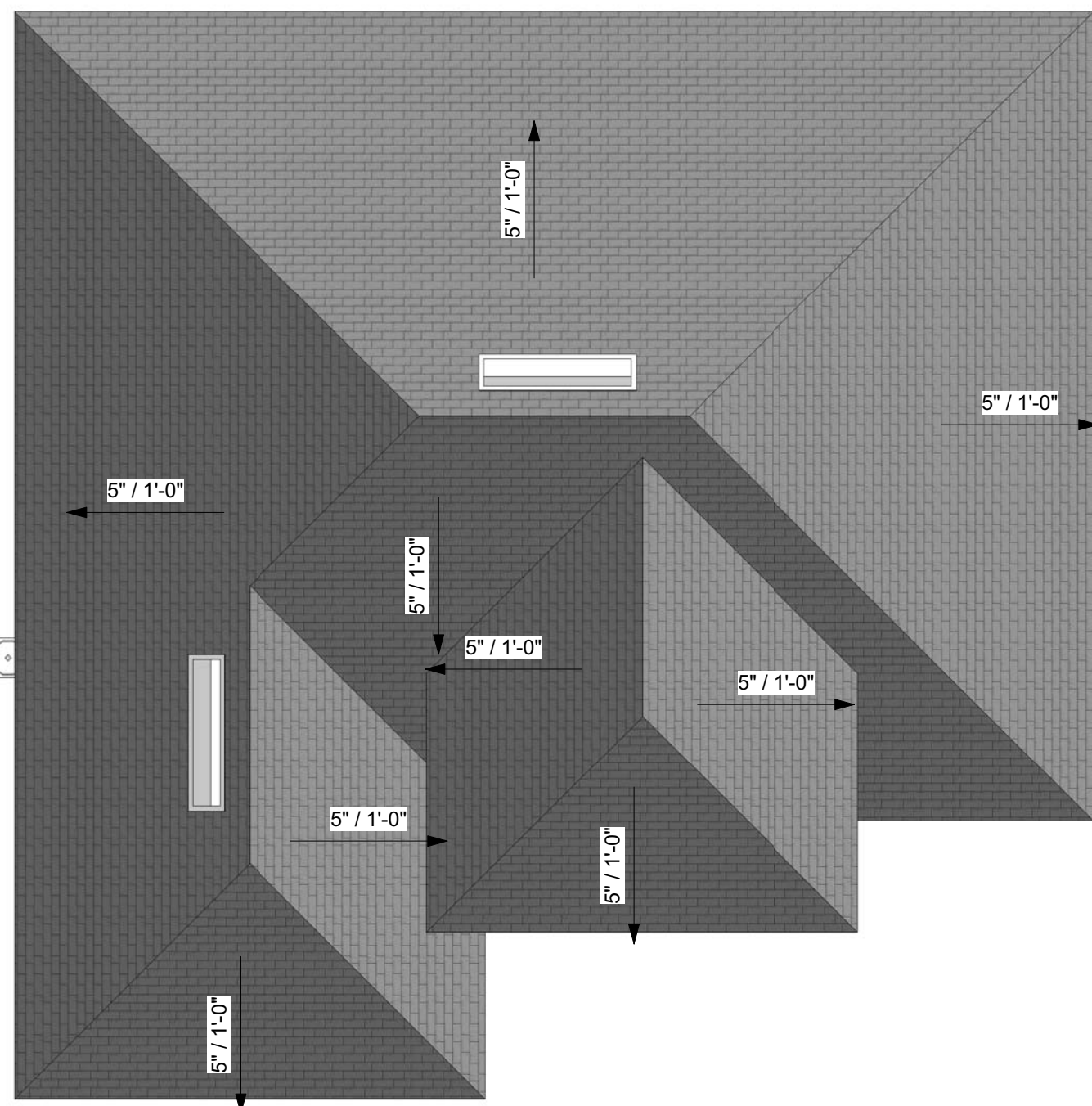
Master Plan
1714 Model - 2021

Lee County, Florida

#	Revision	Date

Strap# 1714 Master Plan
Start Date June 18, 2021
Drawn By jfr
Checked By ajq

S2



8" PRECAST U-INTELS STANDARD LENGTHS

OVERALL LENGTH	TOP STEEL	BOTTOM STEEL
3'-0"	2-7/32" wire	2-#3 rebar
3'-4"	2-7/32" wire	2-#3 rebar
4'-0"	2-7/32" wire	2-#3 rebar
4'-4"	2-7/32" wire	2-#3 rebar
4'-8"	2-7/32" wire	2-#3 rebar
5'-4"	2-7/32" wire	2-#3 rebar
5'-10"	2-7/32" wire	2-#3 rebar
6'-0"	2-7/32" wire	2-#4 rebar
6'-6"	2-7/32" wire	2-#4 rebar
6'-8"	2-7/32" wire	2-#4 rebar
7'-4"	2-7/32" wire	2-#4 rebar
7'-6"	2-7/32" wire	2-#4 rebar
8'-0"	2-#3 rebar	2-#4 rebar
8'-8"	2-#3 rebar	2-#4 rebar
9'-4"	2-#3 rebar	2-#4 rebar
10'-0"	2-#3 rebar	2-#4 rebar
10'-6"	2-#3 rebar	2-#4 rebar
10'-8"	2-#3 rebar	2-#5 rebar
11'-4"	2-#3 rebar	2-#5 rebar
12'-0"	2-#3 rebar	2-#5 rebar
13'-4"	2-#3 rebar	2-#5 rebar
14'-0"	2-#3 rebar	2-#5 rebar

Rebar: ASTM A615 Grade 60
Wire: ASTM A510
Concrete Strength: 4000 psi
Average Self Weight: 33 plf
Finish: Grey Block

8" PRESTRESSED U-INTELS STANDARD LENGTHS

OVERALL LENGTH	TOP STEEL	BOTTOM STEEL
14'-8"	NONE	2-7/16 strand
15'-4"	NONE	2-7/16 strand
17'-4"	NONE	2-7/16 strand
19'-4"	2-7/32" wire	2-7/16 strand
21'-4"	2-7/32" wire	2-7/16 strand
22'-0"	2-7/32" wire	2-7/16 strand
24'-0"	2-7/32" wire	2-7/16 strand

Rebar: ASTM A615 Grade 60
Wire: ASTM A510
Strand: ASTM A116 Grade 270
Concrete Strength: 6000 psi
Synthetic Fibers: 2.5 lbs/yd
Average Self Weight: 37 plf
Finish: Grey Smooth Form

8" PRECAST W/2" RECESS DOOR U-INTELS STANDARD LENGTHS

OVERALL LENGTH	RECESS LENGTH	DOOR SIZE	TOP STEEL	BOTTOM STEEL
3'-0"	3'-4"	2'-0"	2-7/32" wire	2-#3 rebar
4'-0"	3'-0"	2'-8"	2-7/32" wire	2-#3 rebar
4'-4"	3'-4"	3'-0"	2-7/32" wire	2-#3 rebar
5'-8"	4'-4"	4'-0"	2-7/32" wire	2-#3 rebar
5'-10"	4'-8"	3'-0" W/1/2" scribe	2-7/32" wire	2-#3 rebar
6'-8"	5'-4"	5'-0"	2-#3 rebar	2-#4 rebar
7'-4"	5'-8"	5'-4"	2-#3 rebar	2-#4 rebar
7'-6"	6'-4"	6'-0"	2-#3 rebar	2-#4 rebar
8'-4"	8'-0"	8'-0"	2-#3 rebar	2-#4 rebar

Rebar: ASTM A615 Grade 60
Wire: ASTM A510
Concrete Strength: 4000 psi
Average Self Weight: 29 plf
Finish: Grey Block

Note: 2" recess door U-intels are used where the top of the door is equal to the block course height with a 2" door frame.

INSTALLATION OF COMPOSITE U-INTEL

INSTALLATION NOTES

- Installation of U-intels must comply with architectural and/or structural drawings.
- U-intels are manufactured with 5/16 inch long rebar at ends to accommodate vertical cell reinforcing and grout.
- U-intels can be field cut to the required length.
- Bottom field added rebar to be located at the bottom of U-intel cavity.
- Cast-in-place concrete may be provided in composite U-intel in lieu of concrete masonry units.
- The exterior surface of U-intels installed in exterior concrete masonry walls shall have a coating of sealer applied in accordance with ASTM C-926 or other approved coating.
- Concrete masonry units used in composite U-Intel shall be laid in a running bond.
- Show composite finish as required.

COMPOSITE U-INTEL MATERIALS

- Grout per ASTM C476, F_c = 3000 psi w/maximum 3/8 inch aggregate and 8 to 11 inch slump.
- Rebar per ASTM A615 Grade 60 or Grade 40.
- Concrete masonry units (CMU) per ASTM C90 with minimum net area compressive strength = 1900 psi.
- Mortar per ASTM C270 Type II or S.

8" COMPOSITE U-INTEL TYPES

TYPE	DESCRIPTION
8F8-0B1T	BUS
8F12-0B1T	
8F16-0B1T	
8F20-0B1T	
8F24-0B1T	
8F28-0B1T	
8F32-0B1T	
8F8-0B2T	
8F12-0B2T	
8F16-0B2T	
8F20-0B2T	
8F24-0B2T	
8F28-0B2T	
8F32-0B2T	
8F8-1B1T	
8F12-1B1T	
8F16-1B1T	
8F20-1B1T	
8F24-1B1T	
8F28-1B1T	
8F32-1B1T	
8F8-1B2T	
8F12-1B2T	
8F16-1B2T	
8F20-1B2T	
8F24-1B2T	
8F28-1B2T	
8F32-1B2T	

1 Site
1/8" = 1'-0"

TRUSS FASTENER SCHEDULE

MARK	BEARING TYPE	UPLIFT	NO	MANUFACTURER		LOCATION	NOTES
				SIMPSON	USP		
TYPICAL TRUSS	CONCRETE	UP TO 1615	(1)	HETA20	HTA20	ALL UNMARKED ATTACHMENTS	1. EQUIVALENT FASTENERS OF OTHER MANUFACTURERS MAY BE SUBSTITUTED PER APPROVAL OF THE PERMITTING BUILDING DEPARTMENT. 2. TRUSS ERECTOR SHALL USE CAUTION GIVEN TO NAILING SO AS NOT TO SPLIT THE LUMBER. 3. ALL TRUSSES TO TRUSS CONNECTIONS BY TRUSS MANUFACTURER. 4. TRUSSES PREPARED BY: Southwest Structural Systems, Inc. Dated: 08/16/2021, Job# 18742 5. THESE PLANS HAVE BEEN COORDINATED WITH THE TRUSS LAYOUT LISTED IN #4 ABOVE. FOUNDATION HAS BEEN CHECKED FOR REACTIONS AND STRAPPING SPECIFIED HERE FOR ALL UPLIFTS.
1-Ft & BK	CONCRETE	1100	(1)	HETA20	HTA20	HG2	
2-LT & RT	CONCRETE	1000	(1)	HETA20	HTA20	HG3	
3-REAR	CONCRETE	1100	(1)	HETA20	HTA20	TG1	
3-FRONT	WOOD	1100	(1)	MTS16	MTW16	TG1	
TYPICAL TRUSS	WOOD	UP TO 1195	(1)	MTS16	MTW16	ALL UNMARKED ATTACHMENTS	

HOW TO READ SAFE LOAD TABLES

TYPE DESIGNATION
8F16-1B1T

GRAVITY

OVERALL LENGTH	TYPE OF U-INTEL	8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0B
3'-0"	PRECAST	3069	3719	5163	6607	8054	9502	10951
4'-0"	PRECAST	3069	4055	6113	7547	8974	10394	11809
5'-0"	PRECAST	2963	4600	6113	7547	8974	10394	11809
6'-0"	PRECAST	1989	2180	4375	6113	7547	8974	10394
7'-0"	PRECAST	1249	1438	1999	2569	3129	3689	4249
8'-0"	PRECAST	845	1022	1366	1746	2126	2506	2886
9'-0"	PRECAST	536	656	824	992	1160	1328	1496
10'-0"	PRECAST	327	387	483	583	683	783	883
11'-0"	PRECAST	218	258	318	378	438	498	558
12'-0"	PRECAST	169	199	239	279	319	359	399
13'-0"	PRECAST	120	140	170	190	220	250	280
14'-0"	PRECAST	71	81	101	121	141	161	181
15'-0"	PRECAST	22	32	52	72	92	112	132

UPLIFT

OVERALL LENGTH	TYPE OF U-INTEL	8F8-1B	8F12-1B	8F16-1B	8F20-1B	8F24-1B	8F28-1B	8F32-1B
3'-0"	PRECAST	1611	1729	2432	2956	3480	4004	4528
4'-0"	PRECAST	1611	1729	2432	2956	3480	4004	4528
5'-0"	PRECAST	1031	1129	1561	1885	2209	2533	2857
6'-0"	PRECAST	651	699	924	1084	1244	1404	1564
7'-0"	PRECAST	471	504	643	754	865	976	1087
8'-0"	PRECAST	291	308	387	456	525	594	663
9'-0"	PRECAST	211	222	271	311	351	391	431
10'-0"	PRECAST	131	139	171	191	211	231	251
11'-0"	PRECAST	51	54	68	78	88	98	108
12'-0"	PRECAST	11	12	15	17	19	21	23
13'-0"	PRECAST	1	1	2	2	3	3	4
14'-0"	PRECAST	0	0	0	0	0	0	0
15'-0"	PRECAST	0	0	0	0	0	0	0

LATERAL

OVERALL LENGTH	TYPE OF U-INTEL	8F8-RCM	8F12-RCM	8F16-RCM	8F20-RCM	8F24-RCM	8F28-RCM	8F32-RCM
3'-0"	PRECAST	1031	1129	1561	1885	2209	2533	2857
4'-0"	PRECAST	1031	1129	1561	1885	2209	2533	2857
5'-0"	PRECAST	651	699	924	1084	1244	1404	1564
6'-0"	PRECAST	471	504	643	754	865	976	1087
7'-0"	PRECAST	291	308	387	456	525	594	663
8'-0"	PRECAST	211	222	271	311	351	391	431
9'-0"	PRECAST	131	139	171	191	211	231	251
10'-0"	PRECAST	51	54	68	78	88	98	108
11'-0"	PRECAST	11	12	15	17	19	21	23
12'-0"	PRECAST	1	1	2	2	3	3	4
13'-0"	PRECAST	0	0	0	0	0	0	0
14'-0"	PRECAST	0	0	0	0	0	0	0
15'-0"	PRECAST	0	0	0	0	0	0	0

SAFE LOAD TABLE NOTES

- All values based on minimum 4 inch nominal bearing. Exception: Safe loads for unframed U-intels must be reduced by 20% if bearing length is less than 6 1/2 inches.
- N.R. = Not Rated.
- Safe loads are superimposed allowable loads.
- One #7 rebar may be substituted for two #3 rebar in 8 inch U-intels only.
- The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at a distance from the face of support. Example: 4" U-Intel Type 8F12-1B safe gravity load = 4427 (-). Calculate the 15% reduction: 6472 (85) = 5501 plf.
- Safe load ratings based on rational design analysis per ACI 318 and ACI 530.
- U-intels loaded simultaneously with vertical (gravity) or uplift and horizontal (lateral) loads should be checked for combined loading with the following equation: Applied vertical load + Applied horizontal load ≤ 1.0 Safe vertical load + Safe horizontal load
- All safe loads in units of pounds per linear foot.
- All safe loads based on simply supported span.
- The number in the parenthesis indicates the percent reduction for grade 40 field added rebar. Example: 4" U-Intel Type 8F12-1B safe gravity load = 4427 (-). Calculate the 15% reduction: 6472 (85) = 5501 plf.

8" PRECAST U-INTELS SAFE LOADS (LB/FT)

GRAVITY

OVERALL LENGTH	TYPE OF U-INTEL	8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0B
3'-0"	PRECAST	3069	3719	5163	6607	8054	9502	10951
4'-0"	PRECAST	3069	4055	6113	7547	8974	10394	11809
5'-0"	PRECAST	2963	4600	6113	7547	8974	10394	11809
6'-0"	PRECAST	1989	2180	4375	6113	7547	8974	10394
7'-0"	PRECAST	1249	1438	1999	2569	3129	3689	4249
8'-0"	PRECAST	845	1022	1366	1746	2126	2506	2886
9'-0"	PRECAST	536	656	824	992	1160	1328	1496
10'-0"	PRECAST	327	387	483	583	683	783	883
11'-0"	PRECAST	218	258	318	378	438	498	558
12'-0"	PRECAST	169	199	239	279	319	359	399
13'-0"	PRECAST	120	140	170	190	220	250	280
14'-0"	PRECAST	71	81	101	121	141	161	181
15'-0"	PRECAST	22	32	52	72	92	112	132

UPLIFT

OVERALL LENGTH	TYPE OF U-INTEL	8F8-1B	8F12-1B	8F16-1B	8F20-1B	8F24-1B	8F28-1B	8F32-1B
3'-0"	PRECAST	1611	1729	2432	2956	3480	4004	4528
4'-0"	PRECAST	1611	1729	2432	2956	3480	4004	4528
5'-0"	PRECAST	1031	1129	1561	1885	2209	2533	2857
6'-0"	PRECAST	651	699	924	1084	1244	1404	1564
7'-0"	PRECAST	471	504	643	754	865	976	1087
8'-0"	PRECAST	291	308	387	456	525	594	663
9'-0"	PRECAST	211	222	271	311	351	391	431
10'-0"	PRECAST	131	139	171	191	211	231	251
11'-0"	PRECAST	51	54	68	78	88	98	108
12'-0"	PRECAST	11	12	15	17	19	21	23
13'-0"	PRECAST	1	1	2	2	3	3	4
14'-0"	PRECAST	0	0	0	0	0	0	0
15'-0"	PRECAST	0	0	0	0	0	0	0

LATERAL

OVERALL LENGTH	TYPE OF U-INTEL	8F8-RCM	8F12-RCM	8F16-RCM	8F20-RCM	8F24-RCM	8F28-RCM	8F32-RCM
3'-0"	PRECAST	1031	1129	1561	1885	2209	2533	2857
4'-0"	PRECAST	1031	1129	1561	1885	2209	2533	2857
5'-0"	PRECAST	651	699	924	1084	1244	1404	1564
6'-0"	PRECAST	471	504	643	754	865	976	1087
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11'-0"	PRECAST	11	12	15	17	19	21	23
12'-0"	PRECAST	1	1	2	2	3	3	4
13'-0"	PRECAST	0	0	0	0	0	0	0
14'-0"	PRECAST	0	0	0	0	0	0	0
15'-0"	PRECAST	0	0	0	0	0	0	0

8" PRECAST U-INTELS SAFE LOADS (LB/FT)

UPLIFT

OVERALL LENGTH	TYPE OF U-INTEL	8F8-1B	8F12-1B	8F16-1B	8F20-1B	8F24-1B	8F28-1B	8F32-1B
3'-0"	PRECAST	1611	1729	2432	2956	3480	4004	4528
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8'-0"	PRECAST	291	308	387	456	525	594	663
9'-0"	PRECAST	211	222	271	311	351	391	431
10'-0"	PRECAST	131	139	171				

READ ALL NOTES. TRUSSES WILL NOT BE MANUFACTURED WITHOUT APPROVAL OF THIS DRAWING.

WARNING AND INSTRUCTIONS: THOSE INSTALLING AND USING THESE COMPONENTS MUST READ AND FOLLOW THE WARNINGS AND INSTRUCTIONS INCLUDED IN THE DELIVERY AND ENGINEERING PACKAGE. DO NOT INSTALL OR USE THESE COMPONENTS UNTIL THE FULL DELIVERY PACKAGE IS RECEIVED AND ALL THE TRUSSES ARE VERIFIED FOR ACCURACY. REFER TO BCSI SUMMARY SHEET (TRUSS PLATE INSTITUTE RECOMMENDATIONS) AND INDIVIDUAL TRUSS DESIGNS FOR IMPORTANT INFORMATION REGARDING BRACING AND INSTALLATION GUIDELINES.

THE ADVICE OF A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT MUST BE SOUGHT ON MATTERS RELATING TO BEARING DESIGN, ANCHORAGE, BRACING, INSTALLATION AND USE OF COMPONENTS. DADE TRUSS COMPANY IS NOT RESPONSIBLE FOR THESE MATTERS. A PERMIT FROM THE BUILDING DEPARTMENT IS REQUIRED TO INSTALL THESE COMPONENTS. DO NOT INSTALL THESE COMPONENTS UNTIL ONE IS OBTAINED. CONTRACTOR MUST ADHERE TO ALL BUILDING CODE REQUIREMENTS REGARDING THE INSTALLATION AND USE OF TRUSSES.

INSTALLATION AND USE: DO NOT CUT OR ALTER TRUSSES. DO NOT INSTALL OR USE DAMAGED TRUSSES AND REPORT ANY DAMAGED TRUSSES TO FABRICATOR. PROPER INSTALLATION AND USE OF THESE COMPONENTS IS THE SOLE RESPONSIBILITY OF THOSE PERSON INSTALLING AND USING THESE COMPONENTS. DADE TRUSS COMPANY, INC. IS NOT RESPONSIBLE FOR THE LIABILITIES THAT MAY RESULT FROM FIELD STORAGE, MISUSE, OR IMPROPER INSTALLATION OF THESE COMPONENTS WHICH MAY RESULT IN FAILURES, BODILY INJURY, LOSS OR PROPERTY, AND/OR LIFE. TRUSSES MUST BE INSTALLED BY CONTRACTORS WITH SUFFICIENT EXPERIENCE IN TRUSS INSTALLATION AND HANDLING.

ERECTION SUPERVISION IS REQUIRED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT FOR TRUSSES OVER 40 FEET IN LENGTH. NO ALTERATION OF THE TRUSSES IS ALLOWED WITHOUT PREVIOUS APPROVAL OF TRUSSES MANUFACTURER. ANY UNAUTHORIZED ALTERATION, REPAIR, OR MODIFICATION OF THE TRUSSES WILL CAUSE DADE TRUSS COMPANY TO RELINQUISH RESPONSIBILITY FOR THE STRUCTURAL SAFETY OF THOSE TRUSSES AND TO NOTIFY BUILDING OFFICIALS. THIS IS A TRUSS PLACEMENT DRAWING ONLY. TRUSSES SHOWN ON THIS PLAN ARE A COMPONENT PART OF A STRUCTURE. THIS PLAN IDENTIFIES TRUSS LOCATION. INSTALLER MUST REFER TO INDIVIDUAL ENGINEERING DRAWINGS FOR PROPER IDENTIFICATION OF TRUSSES.

BRACING: ERECTION AND PERMANENT BRACING WHICH IS ALWAYS REQUIRED ARE THE RESPONSIBILITY OF THE CONTRACTOR NOT THE TRUSS FABRICATOR. REFER TO INDIVIDUAL TRUSS DESIGNS AND ARCHITECTURAL OR ENGINEERING DRAWINGS FOR ADDITIONAL BRACING REQUIRED TO BE INSTALLED DURING ERECTION. REFER TO ARCHITECTURAL DRAWINGS FOR BRACING REQUIRED TO RESIST WIND AND OTHER SPECIFIC LOADING CONDITIONS. PERSONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVENT TOPPLING AND COLLAPSING DURING INSTALLATION. TRUSSES SHALL BE ERECTED AND FASTENED IN A STRAIGHT AND PLUMB POSITION.

BEARINGS: ALL BEARINGS, BEARING DESIGNS, BRACING, AND ANCHORAGE, ARE RESPONSIBILITY OF THE PROJECT DESIGNER. REFER TO INDIVIDUAL TRUSS DESIGNS FOR REACTIONS AND UPLIFTS. TRUSSES MAY NOT BEAR ON ANY INTERIOR WALL OR PARTITION UNLESS DESIGNED FOR THE SAME.

GIRDERS: GIRDER PLIES SHOULD BE FIELD CONNECTED BY BUILDER AS SHOWN ON THE INDIVIDUAL ENGINEERING DESIGNS.

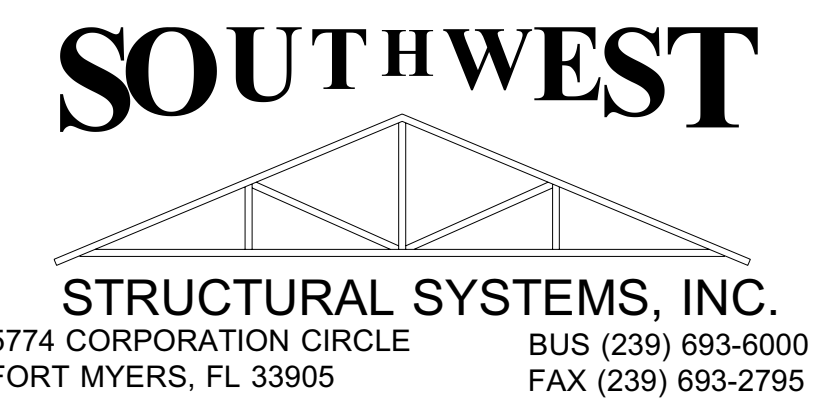
SPACING: TRUSS SPACING 24" OC UNLESS OTHERWISE NOTED ON LAYOUT.

HANGERS: TRUSS MANUFACTURER WILL ONLY SUPPLY STANDARD LIGHT GAUGE TRUSS TO TRUSS CONNECTORS AS SHOWN IN THE ENGINEERING PACKAGE AND ONLY FOR SPANS OVER 12'. ALL HANGERS REQUIRED FOR TRUSSES OF LESS THAN 12' IN SPAN AND WITH REACTIONS OF LESS THAN 600 LBS MUST BE SUPPLIED BY BUILDER. ALL CONNECTIONS REQUIRING SPECIALLY MANUFACTURED HANGERS ARE TO BE SUPPLIED BY BUILDER.

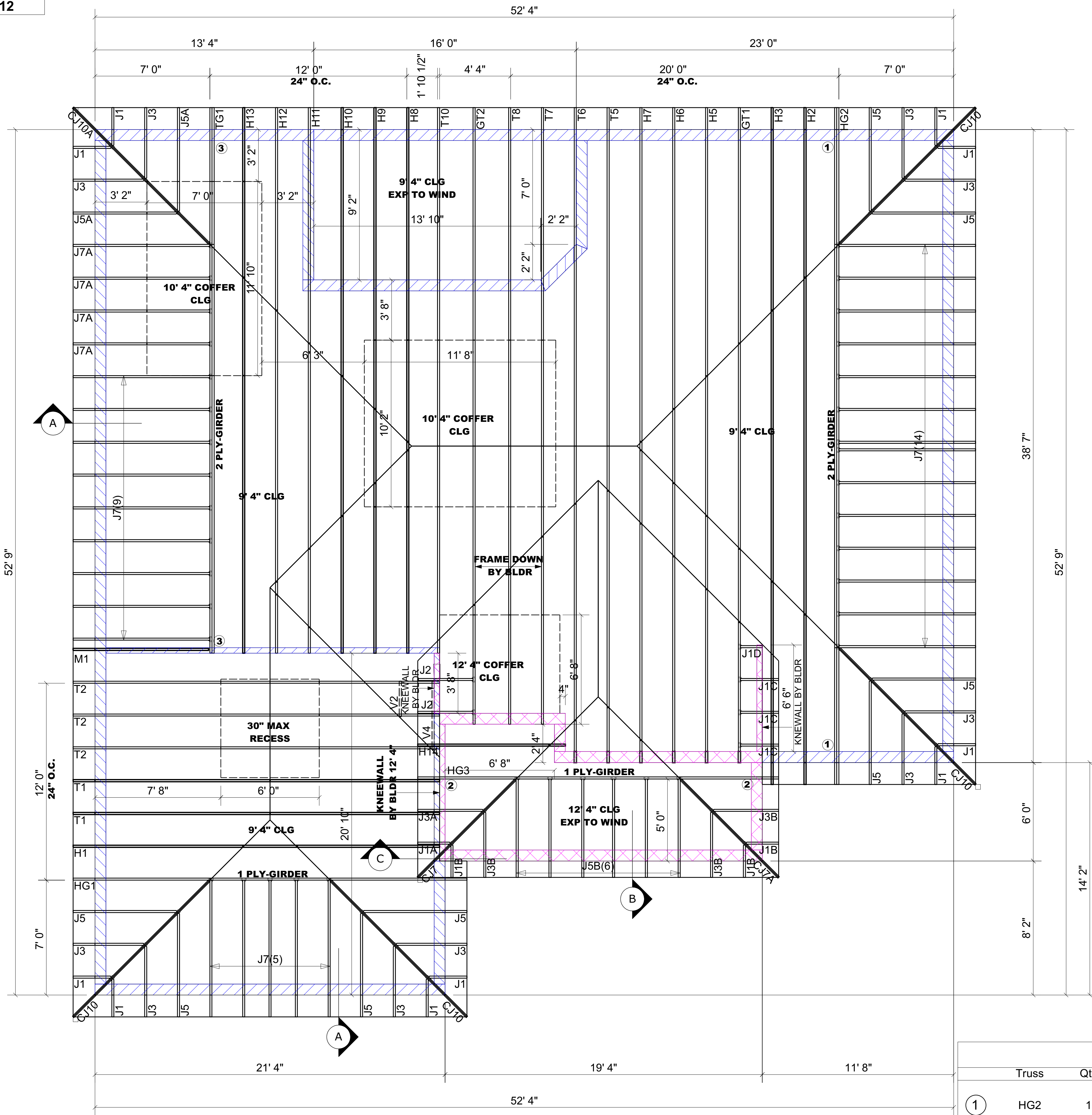
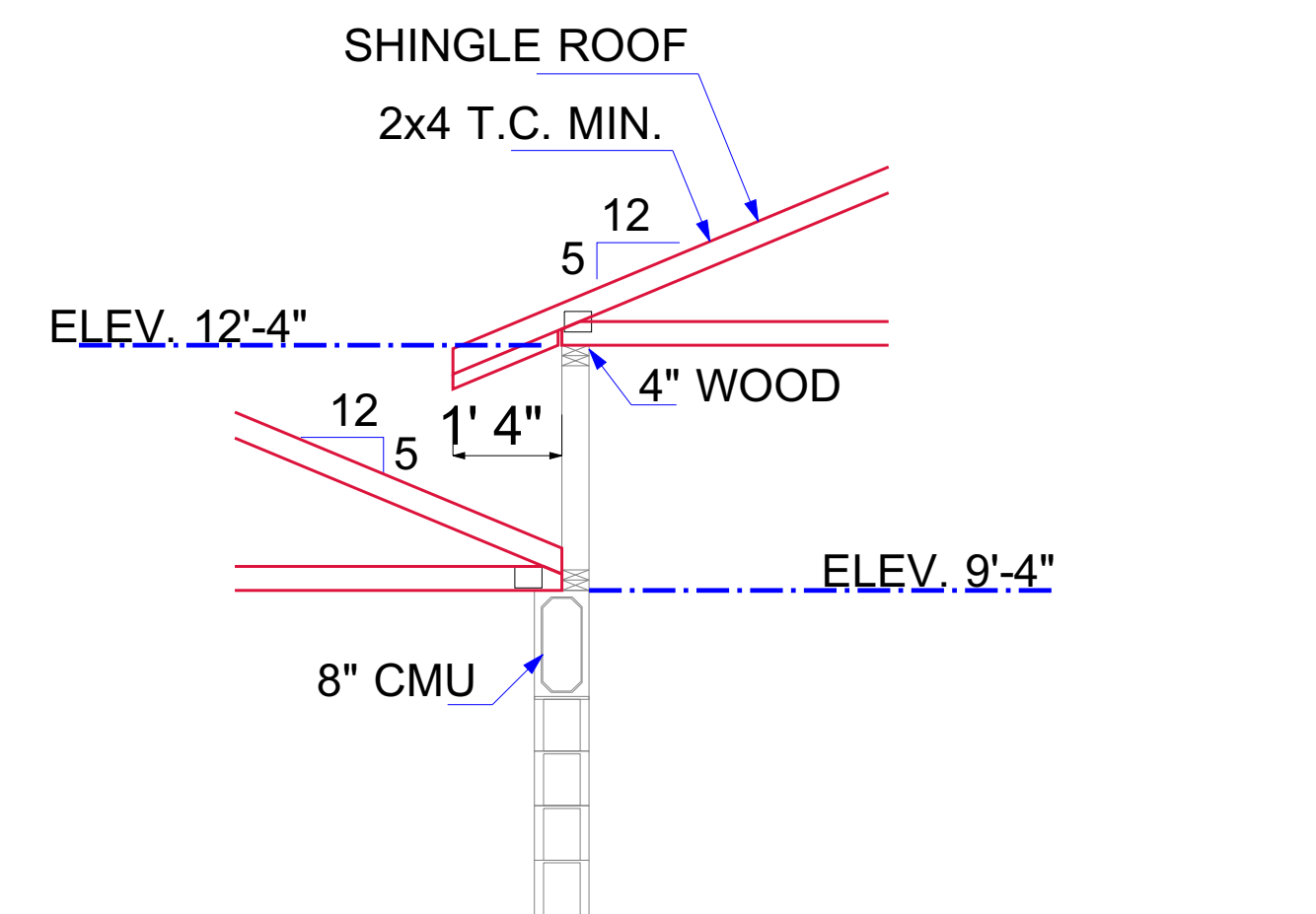
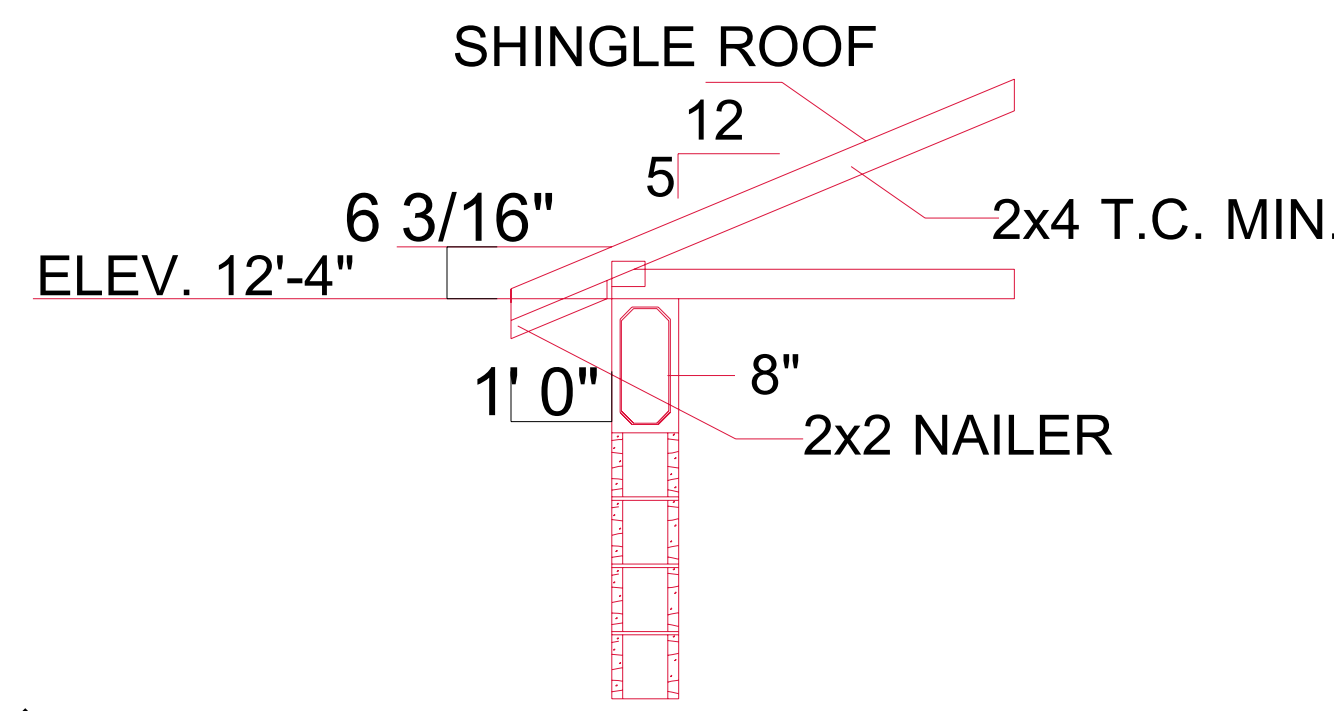
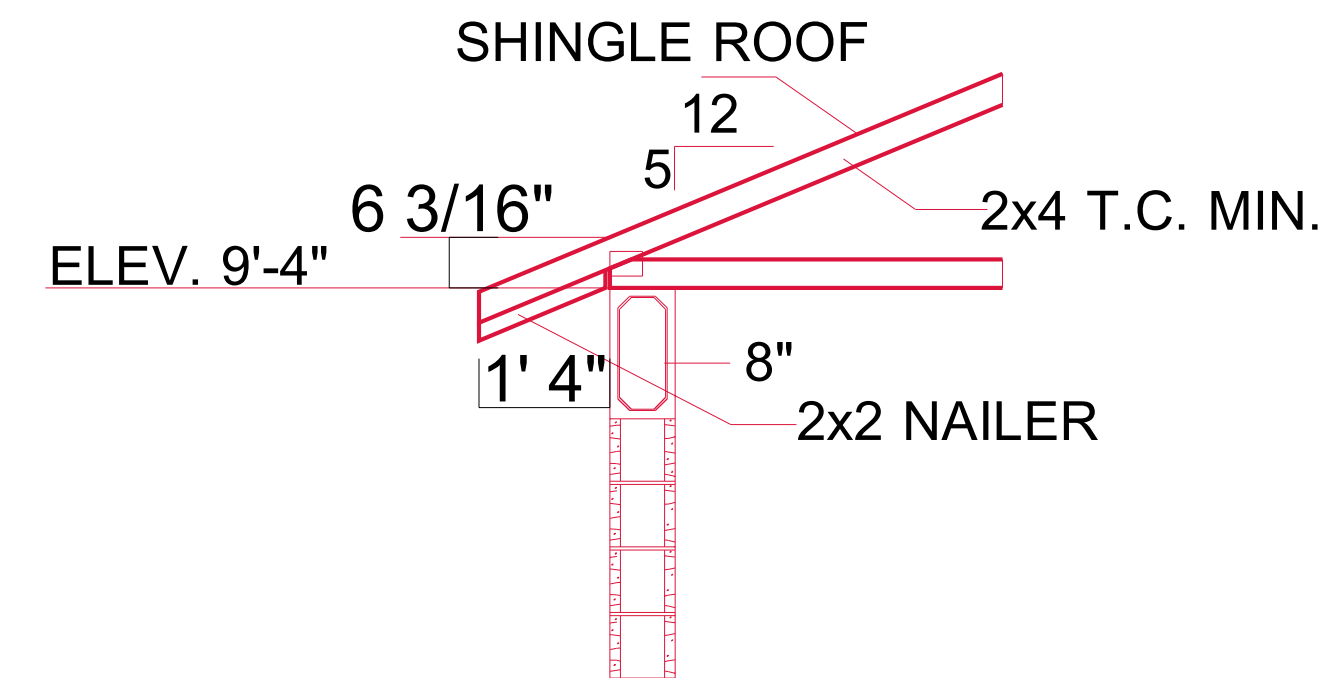
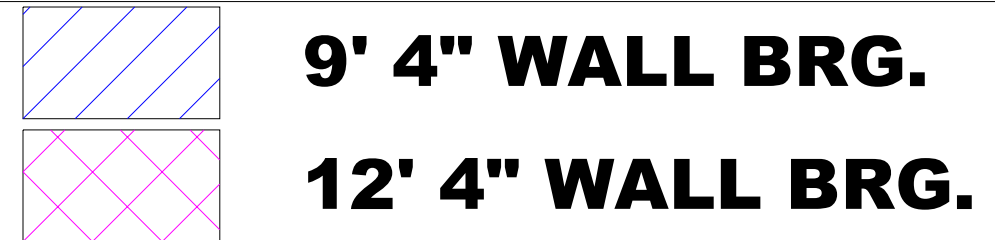
ACCEPTANCE AND APPROVAL: ALL DIMENSIONS, QUANTITIES, LOADING, AND DETAILS ON THIS PLAN AND ON INDIVIDUAL TRUSS DESIGNS MUST BE REVIEWED AND APPROVED BY THE PROJECT ARCHITECT, ENGINEER, AND/OR CONTRACTOR BEFORE FABRICATION. BY ACCEPTING, REVIEWING OR AUTHORIZING FOR FABRICATION THE TRUSSES DESCRIBED IN THIS DRAWING, THE BUYER OR BUYER'S REPRESENTATIVE ACCEPTS ALL CONDITIONS DESCRIBED HEREIN.

TRUSS PLACEMENT PLAN AND INDIVIDUAL TRUSS DESIGNS ACCEPTED AND APPROVED.

By: _____
Date: _____
Client: **J R Rost Associates Inc**
Job Name: _____
Model: _____ Date: 06-16-21
Lot #: _____ Block: _____
Job Addr: **Lee County, Florida**
Architect: _____
DES. H.P.



WALL BEARING LEGEND



Truss List					
Truss	Qty	Ply	Reactions		
① HG2	1	2	N/A	N/A	-1100 lb
② HG3	1	1	N/A	N/A	-1000 lb
③ TG1	1	2	N/A	N/A	-1100 lb

DESIGN DATA

LOADING CRITERIA: FBC2020 / TP2014
 ROOF LIVE: 20 PSF ROOF DEAD: 20 PSF
 FLOOR LIVE: PSF FLOOR DEAD: PSF
 WIND LOADING CRITERIA: ASCE 7-16
 WIND DESIGN VELOCITY: 160 MPH
 WIND DURATION FACTOR: 1.60
 EXPOSURE: B CATEGORY: II
 DESIGN ELEVATION: 15'-0"
 DEAD LOAD FOR UPLIFT: 10 PSF
 IMPORTANCE FACTOR: 1.00

COMPONENTS MANAGEMENT SYSTEMS, INC.
 CERTIFICATE OF AUTHORIZATION # 30680
 6401 NW 74 AVENUE MIAMI, FL 33166
 305-592-8245
 SALVADOR A. JURADO PE, FLORIDA REGISTRATION # 19939

Reviewed for Code Compliance
 By: Andrew Nielsen Date: 6/28/2021
 RESMSTR2021-00191

NOT TO SCALE

ROOF TRUSS LAYOUT

1234567