

Reviewed for Code Compliance By: Andrew Nielsen Date: 6/28/2021 RESMSTR2021-00191

APPLICABLE CODES: APPLICABLE COUES:

BUILDING CODES = FLORIDA BUILDING CODE 7th EDITION (2020) RESIDENTIAL AND FLORIDA BUILDING CODE 7th EDITION (2020) BUILDING MECHANICAL CODE = FLORIDA BUILDING CODE 7th EDITION (2020) MECHANICAL PLUMBING CODE = FLORIDA BUILDING CODE 7th EDITION (2020) PLUMBING ELECTRICAL CODE = NFPA 70: NATIONAL ELECTRICAL CODE (2017) FIRE CODE = (2020) 7th EDITION OF FLORIDA FIRE PREVENTION CODE WITH (2018) NFPA 1 UNIFORM FIRE CODE AND (2018) NFPA 101 LIFE SAFETY CODE ACCESSIBILITY CODE = FLORIDA BUILDING CODE 7th EDITION (2020) ACCESSIBILITY ENERGY CODE = FLORIDA BUILDING CODE 7th EDITION (2020) ENERGY CONSERVATION

METHOD OF DESIGN: DESIGNED PURSUANT TO RESIDENTIAL FLORIDA BUILDING CODES 2020 CHAPTER 10, CHAPTER 3, ALTERATION LEVEL 2 AND CORRESPONDING CHAPTERS 5,6,7 AND SECTION 1609 OF THE 2020 FBC. BASIC WIND SPEED:

170 MPH (ULTIMATE DESIGN/3-SECOND GUST) = 132 MPH (NOMINAL DESIGN/FASTEST MILE) X 160 MPH (ULTIMATE DESIGN/3-SECOND GUST) = 124 MPH (NOMINAL DESIGN/FASTEST MILE) 150 MPH (ULTIMATE DESIGN/3-SECOND GUST) = 116 MPH (NOMINAL DESIGN/FASTEST MILE) RISK CATEGORY:

BUILDING OCCUPANCY CLASSIFICATION: GROUP A - ASSEMBLY GROUP B - BUSINESS

GROUP I - INSTITUTIONAL GROUP M - MERCANTILE GROUP D - DAY CARE CENTER GROUP E - EDUCATIONAL GROUP R - RESIDENTIAL GROUP F - FACTORY INDUSTRIAL GROUP S - STORAGE NOTE:

COMBINATION OF IMPACT RESISTANT GLAZING & COVERING INTERNAL PRESSURE COEFFICIENTS: 0.00 (OPEN) GROUP H - HAZARDOUS LEVEL 1 LEVEL 2

X +0.18, -0.18 (ENCLOSED) +0.55, -0.55, (PARTIALLY ENCLOSED) CLASSIFICATION OF WORK: LEVEL 3 X NEW CONSTRUCTION ☐ CHANGE OF OCCUPANCY ADDITION / REMODEL ☐ HISTORIC BUILDING

BUILDING CONSTRUCTION TYPE:

EXPOSURE CATEGORY: □ A□ CX B□ D

WINDBORNE DEBRIS REGION:

TYPE I-A TYPE II-B TYPE IV TYPE I-B TYPE III-A TYPE V-A TYPE II-A TYPE III-B X TYPE V-B

X IMPACT RESISTANT GLAZING

☐ IMPACT RESISTANT COVERING

Design Loads:

The structural system for this building has been designed in accordance with the Florida Building Codes (FBC), 7th Edition (2020), The following superimposed loading has been utilized.

Wind: Method of Design, per Section 1609 Florida Building Code 7th Edition (2020) **Enclosed Design**

Basic Wind Speed 160 MPH RISK CATEGORY: II

Wind Exposure Category "B" Internal Pressure Coefficient +/- 0.18 (for Enclosed)

<u>Components and Cladding Design Pressures:</u> Section 1609.6.3, Section 1609.6.4, & Figure 1609.6.2 Florida Building Code 7th Edition (2020)

Zone 1, 2e: 20.6 /-51.0 p.s.f. **Zone 2n,2r,3e:** 20.6 /-74.5 p.s.f.

Zone 3r: 20.6/-88.5 p.s.f. **Zone 4:** 27.6/-30.0 p.s.f. **Zone 5:** 27.6 /-37.0 p.s.f.

Structural Forces: Section 1606 & 1607 Florida Building Code 7th Edition (2020) Gravity Roof Design: Live load 20 PSF Dead load 25 PSF

Wind Roof Design: Dead load 10 PSF

Gravity Floor Design: Live load 40 PSF

Dead load 10 PSF Wind Floor Design:

Top Chord Dead load 10 PSF Bottom Chord Dead load 5 PSF

NOTE: Structural Calculations using Gravity and Wind Loads have been performed in the design of this structure.

SETBACK NOTE:
THESE PLANS ARE DESIGNED SO THAT THE HOUSE
SETBACK TO THE OVERHANGS SHALL NOT BE LESS

	Sheet List
Sheet Number	Sheet Name
A0	Cover Sheet
A1	Floor Plan
A2	Building Elevations
E1	Electrical Plan
S1	Foundation Plan
S2	Structural Notes & Details
S3	Structural Details & Roof Plan

PLANS MASTERING NOTE:

THESE PLANS HAVE BEEN REVIEWED TO BE MASTERED FOR THE 160 MPH WIND

THE CONTRACTOR MAY USE THESE PLANS MULTIPLE TIMES WITH A LETTER FROM THE ENGINEER FOR EACH OCCURRENCE.

NO PERMIT SHALL BE ISSUED WITHOUT THE LETTER FROM THE ENGINEER OF RECORD SHOWING THE ADDRESS FOR THE LOCATION & CONTRACTOR PULLING

TRUSS LIABILITY EXCLUSION NOTE

the truss plans in accordance with the building design.

QAI will not be liable for any errors in the truss design.

Quattrone and Associates, Inc. (QAI) did not prepare or design the truss plans attached to this file. The engineer of record on the truss plan is responsible for the truss engineering, reactions and uplifts. QAI is only referencing the truss plans for the purpose of designing the building structure.
The contractor / owner is responsible for reviewing the truss plan to determine the design, details, dimensions, and the accuracy of

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🏥 his itemā has b 🏟 ក្រី digitally signed by Alfred J. Quattrone, P.E. on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on

ំទី any electronic copies. 2021-06-22 14:57:27

ssociates,

Revision

1714 Master Plan June 18, 2021 ajq

Reviewed for Code Compliance By: Andrew Nielsen Date: 6/28/2021 RESMSTR2021-00191

SQUARE FOOTAGE Living Area 430 SF Garage Area Covered Lanai Area 141 SF 132 SF Entry Area TOTAL AREA 2417 SF

1 Floor Plan 1/4" = 1'-0"



LEGEND:

Dx = DOOR DESIGNATION

SLx = SKYLITE DESIGNATION

Wx = WINDOW DESIGNATION

W = WIDTH

H = HEIGHT

L	INTEL BEAM		
STANDARD 8F8-0B/1	T UNLESS NOTED OTHERWI	SE	
AT WINDOWS, DOOF	RS & OPENINGS 8F32-1B/1T (T	YP. U.N.O.)	
CAST-CRETE o BOTTOM OF PR	ELS SHALL BE MANUFACTUR r EQUAL RECAST TO BE AT MASONRY WINDOW / DOOR SCHEDULE	OPENING	
		•	
PROVIDE 2"x4" BLOCKING AT 4'-0" O.C. AT THE BOTTOM CHORD OF ALL TRUSSES IN LANAI AND ENTRY (AREAS EXPOSED TO WIND). CEILING SHEATHING IN THESE AREAS TO BE 5/8" EXTERIOR GRADE DRYWALL OR 1/2" EXTERIOR GRADE PLYWOOD. OPENING FROM A PRIVATE GARAGE DIRECTLY INTO A ROOM USED FOR SLEEPING PURPOSES SHALL NOT BE PERMITTED. OTHER OPENINGS BETWEEN THE GARAGE AND RESIDENCE SHALL BE EQUIPPED WITH SOLID WOOD DOORS NOT LESS THAN 1 3/8 INCHES (35 MM) IN THICKNESS, SOLID OR HONEYCOMB CORE STEEL DOORS NOT LESS THAN 1 3/8 INCHES (35MM) THICK, OR 20 MINUTE FIRE RATED DOORS, PER FBC 2020 7th EDITION SEC. R302.5.1 DUCTS IN THE GARAGE AND DUCTS PENETRATING THE WALLS OR CEILINGS SEPARATING THE DWELLING FROM THE GARAGE SHALL BE CONSTRUCTED OF A MINIMUM NO. 26 GAGE (0.48 MM) SHEET STEEL OR OTHER APPROVED MATERIAL AND SHALL HAVE NO OPENINGS INTO THE GARAGE.	O THE GARAGE SHALL BE SEPARATED FROM THE RESIDENCE AND ITS ATTIC AREA BY NOT LESS THAN 1/2 INCH (12.7 MM) GYPSUM BOARD APPLIED TO THE GARAGE SIDE. GARAGE BENEATH HABITABLE ROOMS SHALL BE SEPARATED FROM ALL HABITABLE ROOMS SHALL BE SEPARATED FROM ALL HABITABLE ROOMS SHALL BE SEPARATION IS A FLOOR CEILING ASSEMBLY, THE STRUCTURE SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED BY NOT LESS THAN 1/2 INCH (12.7MM) GYPSUM BOARD OR EQUIVALENT. PER FBC 2020 7th EDITION SECTION R309.2 O PER SEC. R302.1 OF THE 2020 7th EDITION FLORIDA RESIDENTIAL BUILDING CODE, EXTERIOR WALLS SEPARATED BY LESS THAN 6 FT., SHALL HAVE NOT LESS THAN 1 HR. FIRE RESISTIVE RATING WITH EXPOSURE ON BOTH SIDES. O PER TABLE R702.3.5 (FOOTNOTE D) ON CEILING APPLICATIONS TO RECEIVE A WATER BASED TEXTURE MATERIAL, EITHER HAND OR SPRAY APPLIED, THE GYPSUM BOARD SHALL BE APPLIED PERPENDICULAR TO FRAMING. THE MINIMUM GYPSUM BOARD THICKNESS SHALL BE 5/8 INCH FOR 24 INCH ON CENTER FRAMING OR 1/2 INCH SAG RESISTANT GYPSUM CEILING BOARD.	O THE DESIGN OF THIS BASED ON ALL FLOOF THE FEMA FLOOD ELE INCLUDING GARAGE. PROJECT IS IN A FEM ELEVATION ALL LIVIN ARE 1 FOOT ABOVE B ELEVATION. ALSO ALL ELECTRICA FIXTURES, MECHANIC & PADS SHALL BE 1 FI BASE FLOOD ELEVATI	RS ARE ABOVE EVATION, IF THIS A FLOOD G AREA FLOORS ASE FLOOD L, PLUMBING EAL EQUIPMENT OOT ABOVE

ABBREVIATIONS

3068 3'0" x 6'8" DOOR SIZE

AFF ABOVE FINISH FLOOR

AWN AWNING

CL CLEAR GLASS CWS COATED WIRE SHELF

FD FRENCH DOOR
FO FRAME OPENING
GL GLIDER

HDR HEADER
OBS OBSCURE GLASS

WBF WOOD BI-FOLD DOOR SGD SLIDING GLASS DOOR

INSTALLATION NOTES:

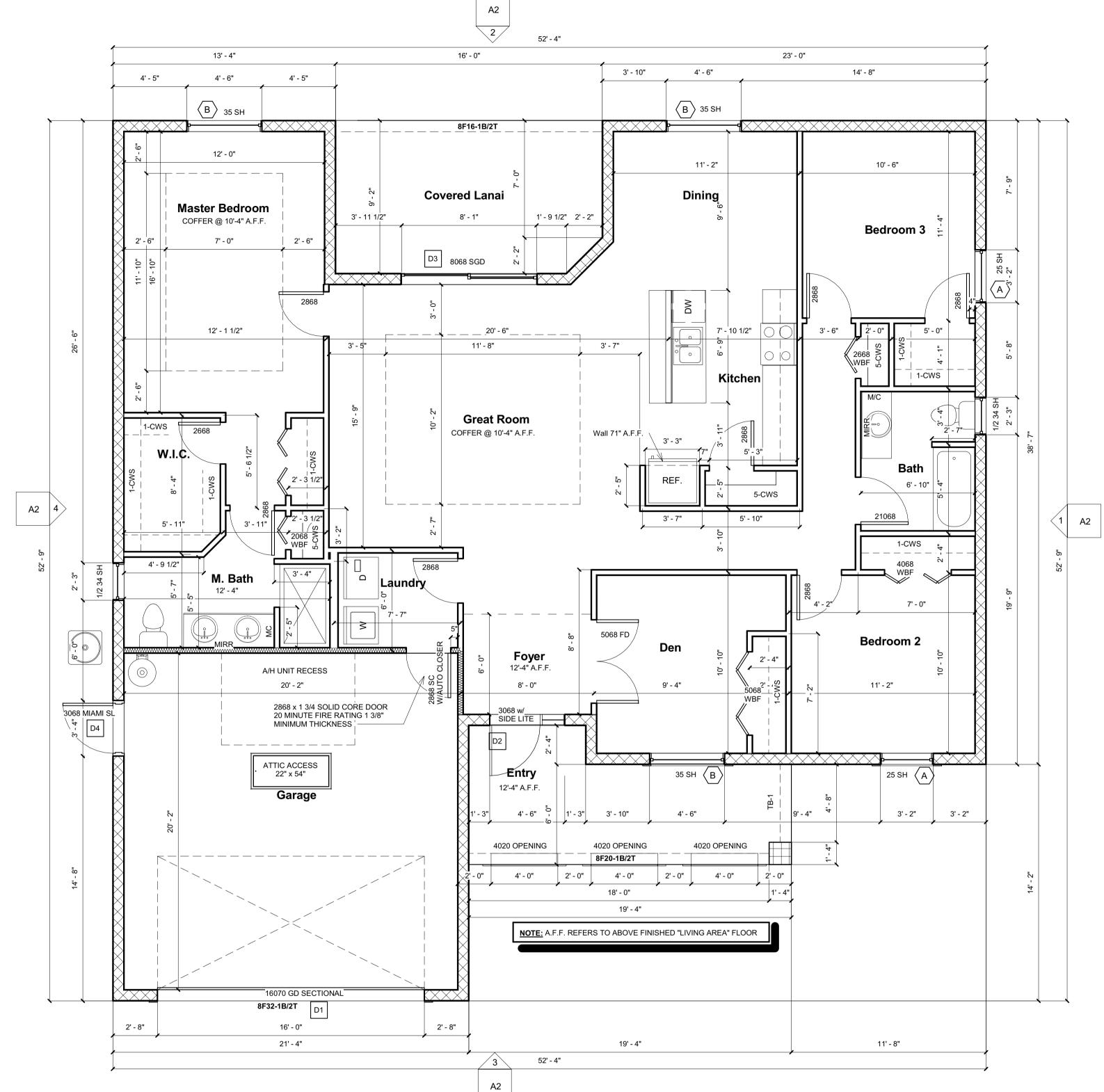
4. EGRESS PER FBC 2020 7th EDITION

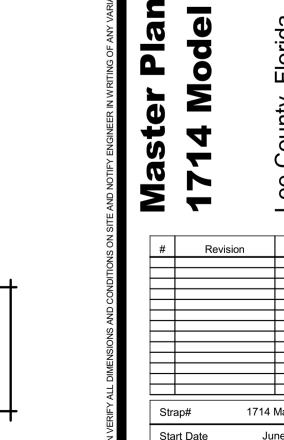
. MEANS OF EGRESS

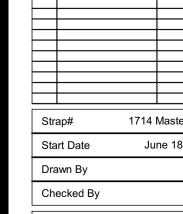
. TEMPERED WINDOW

3. O.H. GARAGE DOOR

M/C RECESSED MEDICINE CABINET U.N.O. UNLESS NOTED OTHERWISE







202

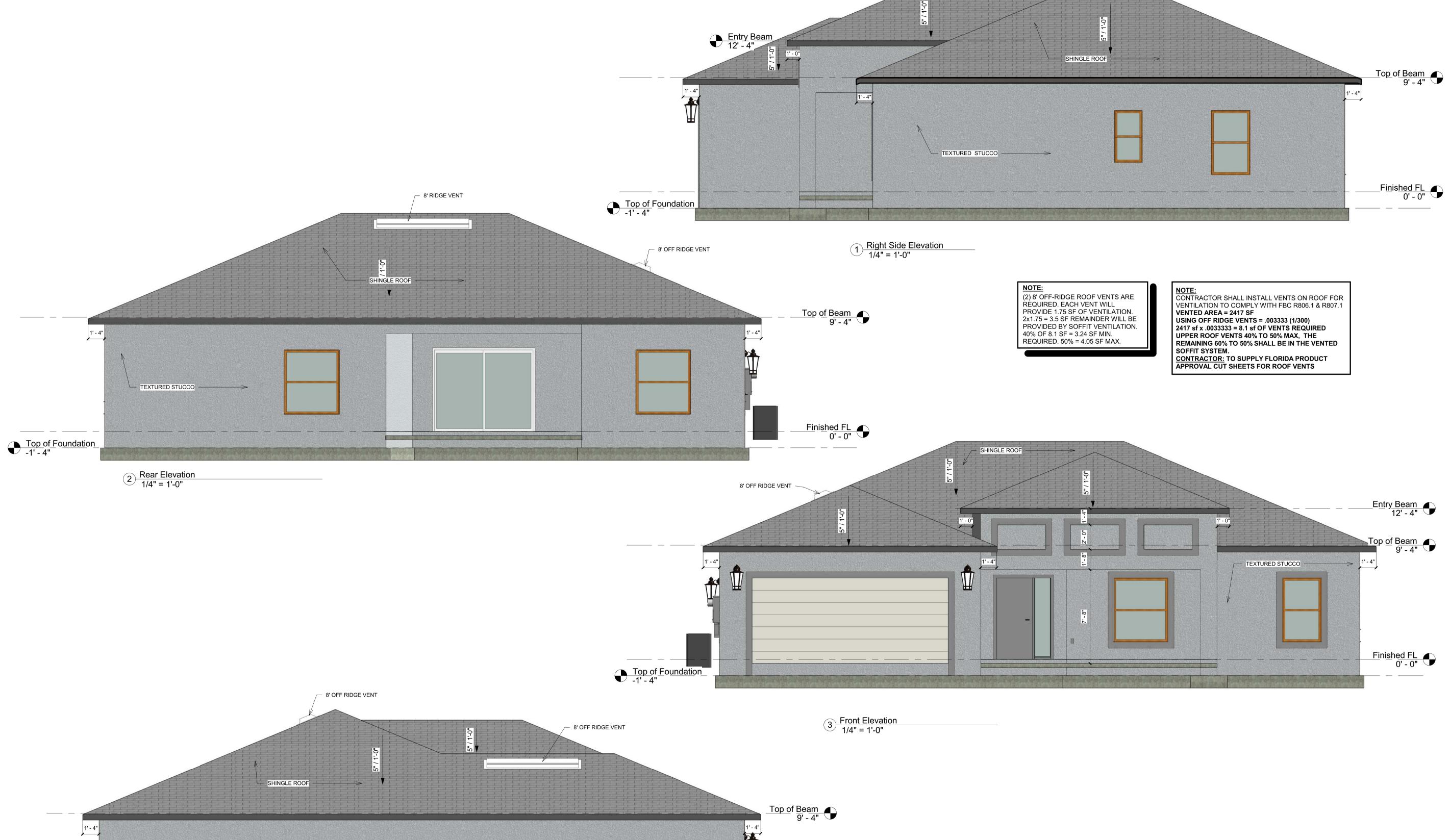
Model

1714 Master Plan June 18, 2021 jrr ajq

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ALLOWABLE SOIL LOAD: THIS PROJECT IS DESIGNED ASSUMING AN ALLOWABLE SOIL LOAD OF 2,000 PSF. THE CONTRACTOR SHALL VERIFY AND OBTAIN A SOILS REPORT. IF CONDITIONS ARE FOUND TO BE DIFFERENT, CONTRACTOR SHALL

NOTIFY ENGINEER PRIOR TO ANY CONSTRUCTION.



Revision Strap# 1714 Master Plan Start Date June 18, 2021 Drawn By jrr Checked By ajq

> **Building Elevations** Print Date: 6/17/2021 4:05:13 PM

Master Plan 1714 Model -

2021

J.R.

Associates, Inc.

Quattrone & 1
Engineers, Planners, & 4301 Veronica Shoemaker Blvd. - Fo

— 8' OFF RIDGE VENT

ALL ROOMS WITH BRANCH CIRCUITS THAT SUPPLY 125-VOLT, SINGLE PHASE, 15 AND 20 AMPERE RECEPTACLE OUTLETS SHALL BE PROTECTED BY AND ARC-FAULT CIRCUIT INTERRUPTER.

A LIGHTING FIXTURE WITH RECEPTACLE OUTLET, CONTROLLED BY A SWITCH LOCATED AT THE PASSAGEWAY OPENING, SHALL BE PROVIDED SO AS TO LIGHT THE PASSAGEWAY AND SERVICE AREA AND INSTALLED IN ACCORDANCE WITH CHAPTER 33 OF THE 2020 7th EDITION FLORIDA MECHANICAL CODE.

AIR HANDLING UNITS SHALL BE ALLOWED IN ATTICS IF THE FOLLOWING CONDITIONS ARE MET:

- 1. THE SERVICE PANEL OF THE EQUIPMENT IS LOCATED WITHIN 6 FEET (1829 MM) OF AN ATTIC ACCESS.
- 2. A DEVICE IS INSTALLED TO ALERT THE OWNER OR SHUT THE UNIT DOWN WHEN THE CONDENSATION DRAIN IS NOT WORKING PROPERLY.
- 3. THE ATTIC ACCESS OPENING IS OF SUFFICIENT SIZE TO REPLACE THE AIR HANDLER.

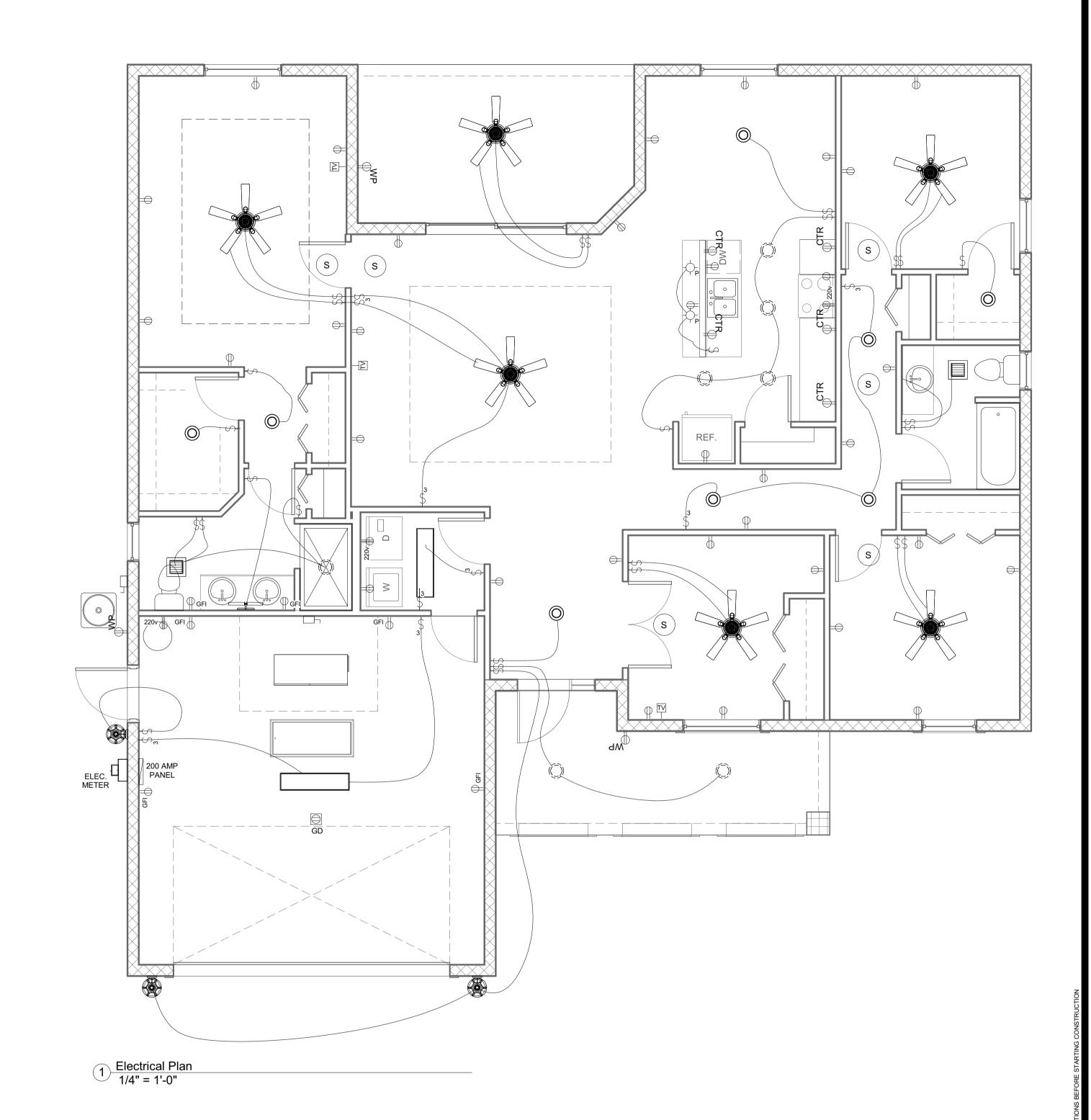
4. A NOTICE IS POSTED ON THE ELECTRIC SERVICE PANEL INDICATING TO THE HOMEOWNER THAT THE AIR HANDLER IS LOCATED IN THE ATTIC. SAID NOTICE SHALL BE IN ALL CAPITALS, IN 16 POINT TYPE, WITH THE TITLE AND FIRST PARAGRAPH IN

"NOTICE TO HOMEOWNER

A PART OF YOUR AIR CONDITIONING SYSTEM, THE AIR HANDLER, IS LOCATED IN THE ATTIC. FOR PROPER, EFFICIENT AND ECONOMIC OPERATION OF THE AIR CONDITIONING SYSTEM, YOU MUST ENSURE THAT REGULAR MAINTENANCE IS PERFORMED. YOUR AIR CONDITIONING SYSTEM IS EQUIPPED WITH ONE OR BOTH OF THE FOLLOWING:

1) A DEVICE THAT WILL ALERT YOU WHEN THE

CONDENSATION DRAIN IS NOT WORKING PROPERLY OR 2) A DEVICE THAT WILL SHUT THE SYSTEM DOWN WHEN THE CONDENSATION DRAIN IS NOT WORKING. TO LIMIT POTENTIAL DAMAGE TO YOUR HOME AND TO AVOID DISRUPTION OF SERVICE, IT IS RECOMMENDED THAT YOU ENSURE PROPER WORKING ORDER OF THESE DEVICES BEFORE EACH SEASON OF PEAK OPERATION."



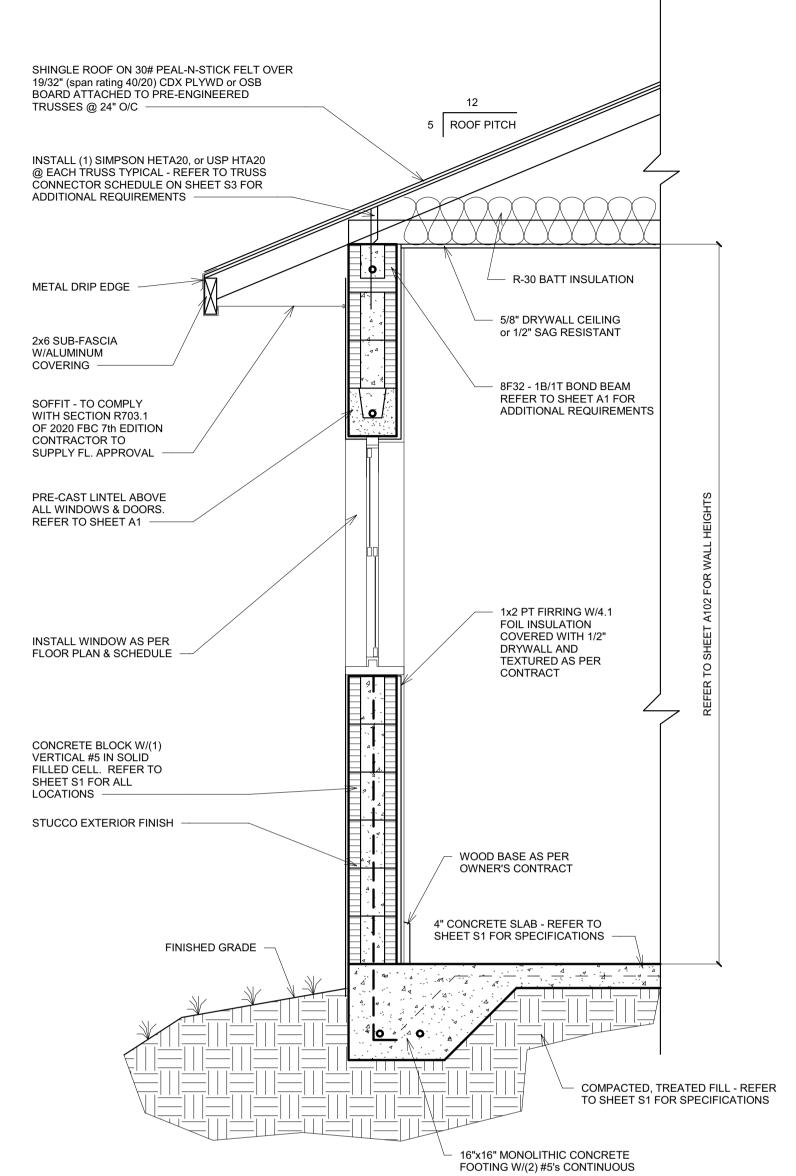
THE FOLLOWING DISCIPLINES ARE TO BE FIELD REVIEWED AND/OR FIELD INSPECTED FOR CODE COMPLIANCE ELECTRICAL MECHANICAL PLUMBING



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> Electrical Plan Print Date: 6/17/2021 4:05:14 PM

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2 Typ. Monolithic Wall Section 8F32-1B/1T 3/4" = 1'-0"

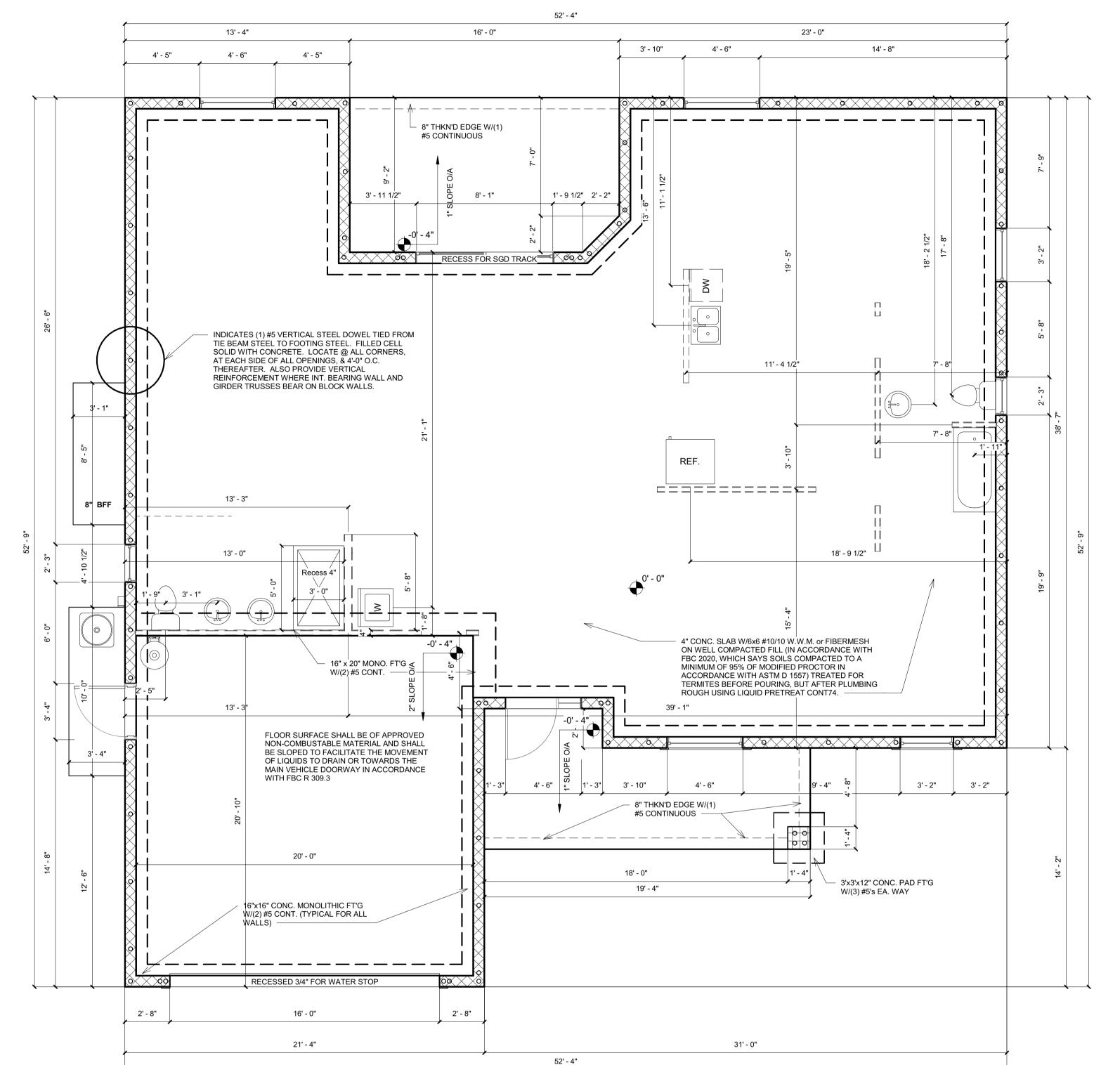
TERMITE PROTECTION Termite protection shall be provided by registered termiticides or other approved methods of termite protection labeled for use as a preventative treatment to new construction. If soil treatment is used for subterranean termite prevention, in initial chemical soil treatment inside the foundation perimeter shall be done after all excavation, backfilling and compaction is complete. Soil area disturbed after initial chemical soil treatment shall be retreated with chemical soil treatment, including space boxes or formed. Space in concrete floors boxed out or formed for subsequent installation of plumbing traps, drains or any other purpose shall be created by using plastic or metal permanently placed forms of sufficient depth to eliminate any planned soil disturbance after initial chemical soil treatment. Chemically treated soil shall be protected with a minimum 6 mil vapor retarded to protect against rainfall dilution. If rainfall occurs before vapor retarder placement, retreatment is required. Any work, including placement of reinforcing steel, done after chemical treatment until the concrete floor is poured, shall be done in such a manner as to avoid penetrating or disturbing treated soil. Concrete over pour or mortar accumulated along the exterior foundation perimeter, shall be removed prior to exterior chemical soil treatment, to enhance vertical penetration of the chemical. Chemical soil treatments shall also be applied under all exterior concrete or grade within one foot (305mm) of the primary structure sidewalls. Also a vertical chemical barrier shall be applied promptly after construction is completed. Including initial landscape and irrigation / sprinkler installation. Any soil disturbed after the chemical vertical barrier is applied shall be promptly retreated. All buildings shall have pre construction treatment protection against subterranean termites. The rules and laws as established by the Florida Department of Agriculture and consumer services shall be deemed as approved with respect to pre construction soil treatment for protection against subterranean termites. A certificate of compliance shall be issued to the building department by a licensed pest control company that

contains the following statement: The building has received a complete treatment for the prevention of subterranean termites.

application of a termiticide in annular space between sleeve and pipe.

Department of Agriculture and consumer services. Protective sleeves around metallic piping penetrating concrete slab-on-grade floors shall not be of cellulose containing materials and shall receive

Treatment is in accordance with rules and laws established by the Florida



1 Foundation Plan 1/4" = 1'-0"

THE FOLLOWING DISCIPLINES ARE TO BE FIELD REVIEWED AND/OR FIELD INSPECTED FOR CODE COMPLIANCE ELECTRICAL MECHANICAL PLUMBING

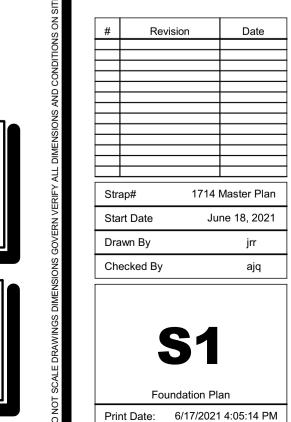
NOTE TO MASON CONTRACTOR:

THE MASON CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL MASONRY OPENINGS WITH PROPER MANUFACTURE'S SPEC. PRIOR TO CONSTRUCTION.

> THIS PROJECT IS DESIGNED ASSUMING AN ALLOWABLE SOIL LOAD OF 2,000 PSF. THE CONTRACTOR SHALL VERIFY AND OBTAIN A SOILS REPORT. IF CONDITIONS ARE FOUND TO BE DIFFERENT, CONTRACTOR SHALL NOTIFY ENGINEER PRIOR TO ANY CONSTRUCTION.



ALLOWABLE SOIL LOAD:



Plan

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2. The contractor shall supply, locate and build into the work all inserts, anchors, angles. plates, openings, sleeves, hangers, slab depressions and pitches as may be required to attach and accommodate other work.

3. All details and sections shown on the drawings are intended to be typical and shall be constructed to apply to any similar situation elsewhere in the work except where a different 4. Subsurface soil condition information is not available. Foundations are designed for a soil

bearing capacity of 2,000 PSF. The contractor shall report any differing conditions to the

designer prior to commencing work 5 Structural drawings shall be used in conjunction with Job Specification and Architectura Mechanical Electrical Plumbing and Site Drawings. Consult these Drawings for sleeves,

depressions, and other details not shown on Structural Drawings 6. All bolts, nuts, washers, straps, and fasteners including nails, shall be Hot-dip Galvanized or Stainless Steel. Continuous anchorage shall be provided between all trusses, wall sections, and 48 (30" for # 5 bar) bar diameters in masonry or 12" whichever is greater. beams, posts, and footings with the use of straps and connectors as specified herein, except

7. All specified fasteners may be substituted with equivalent fasteners. The

installation of the fasteners shall be in accordance with the manufacturer's specifications. 8. Wind bracing (1st and 2nd floors) with 18 gauge X 1-1/4" galvanized straps 10'-0" long placed diagonally, all bearing walls at corners or with 4'-0" X 10'-0" or 8'-0" X 5/8" exterior grade plywood siding.

9. The structure is designed to be self-supporting and stable after the building is complete. It is the contractor's sole responsibility to determine erection procedures and sequence to ensure safety of the building and its components during erection. This includes the addition of necessary shoring, sheeting, temporary bracing, guys or tie downs.

1. Concrete shall have (3/4" Maximum Diameter Aggregate) a minimum compressive strength of 3,000 PSI at 28 days, unless noted otherwise. Placement shall be in accordance with ACI

2. All concrete slabs on grade shall be the thickness indicated on the drawings, over minimum 6 Mil Polyethylene Vapor Barrier. Such slabs shall be reinforced with 6 X 6-W1.4 X W1.4 Welded Wire Fabric lapped 8" at edges and ends.

3. Fill under concrete slabs shall be clean sand or rock and free of debris and other deleterious material. Fill shall be compacted to a density of at least 95% of Standard Proctor Maximum Dry Density (ASTM D1557)

4. Footings shall bear upon undisturbed treated soil or upon soil compacted to at least 95% of Standard Proctor Maximum Dry Density (ASTM D1557) for a depth of at least three (3) feet below the bottom of the footing

5. Reinforcing steel shall be ASTM A-615 Grade 60 Deformed Steel. All continuous vertical and horizontal reinforcing steel in footings, beams, and columns shall be lap spliced a minimum of 30 Bar Diameter or 12", whichever is greater.

6. All 90 degree hooks with side cover (perpendicular to plane of hook) not less than 2.5 inches, and cover on bar extension beyond hook (also called end cover) not less than 2 inches: shall be embedded in support member a minimum of 15.5 Bar Diameters: and the hook shall be extended at least 12 Bar Diameters at the free end of the bar within the support member.

*All 90 degree hooks with side cover (perpendicular to plane of hook) less than 2.5 inches, and cover on bar extension beyond hook (also called end cover) less than 2 inches shall be embedded in support member a minimum of 22 Bar Diameters: and the hook shall be extended at least 12 Bar Diameters at the free end of the bar within the support

7. Concrete cover and spacing of reinforcing steel shall be as follows A. Footings: 3" Bottoms and Sides, 2" Top B. Beams: 1-1/2" Bottom, Sides and Top

D. Slabs on Grade: 2" Bottom, 1" Top E. Rebar C. to C.: 1 Bar Diameter or 1", whichever is greater (Center to Center spacing of rebar within concrete.)

8. Welded wire fabric shall conform to ASTM A-185, free from oil, scale, and rust and placed in accordance with the typical place details of ACI Standards and Specifications Minimum Ian shall be one space plus two (2) inches

Concrete Mono Footer Notes: 1. These notes and specifications apply to all concrete mono footers unless noted otherwise

2. See Concrete Notes for the concrete, and steel strengths and specifications. 3. All reinforcement shall have a minimum of 3" cover on all sides

4. All horizontal reinforcement shall be continuous.

5. All horizontal reinforcement bars shall be evenly spaced horizontally within the footer minus the 3" cover on each side. 6. All horizontal reinforcement bars shall have a minimum horizontal spacing within the footer

of (1) horizontal bar diameter or 1" whichever is greater. 7. All horizontal reinforcement bars shall be located at the depth specified in plan

8. All vertical reinforcement that terminates within the footer shall have (1) 90-degree hook of equal bar size as the vertical from above for each vertical reinforcing bar

9. All 90-degree hooks shall either be bent into the end of the vertical steel or lap spliced

10. All lap splices shall be a minimum of 30 (18.75" for # 5 bar) bar diameters in concrete and 48 (30" for # 5 bar) bar diameters in masonry or 12" whichever is greater

11. All 90 degree hooks shall have a minimum of a 12 bar diameters (8" for # 5 bar)

12. All hook extensions shall be located in the horizontal plane at 3" plus 1/2" bar diameter

from the center of the extension to the bottom edge of the concrete footer to the centerline

13. All hook extensions shall be run parallel to the horizontal steel in the footer

14. When specified by footing detail, slabs under vertical reinforced masonry walls shall have a #4 bar with a 90 degree hook, extending 30" into slab @ a depth of 2" below slab surface & extending 6" vertically down the mono footer @ 3" clear distance from exterior edge of mono footer or @ the center line of an interior footing. Provide these hooks @ every reinforced vertical cell extending up to the tie beam (except where noted otherwise) and within wall opening @ 48" o.c. from vertical at each end of wall opening 15. When specified by footing detail slabs under bearing frame walls shall have a #4 bar

with a 90 degree hook extending 30" into slab @ a depth of 2" below slab surface, extending 6" vertically down the mono footer @ 3" clear distance from exterior edge of mono footer or @ the center line of an interior footing. Provide these hooks @ 48" o.c under frame, @ every post location, and within wall opening @ 48" o.c. from post hook at each end of wall opening. Concrete Beam Notes:

1. These notes and specifications apply to all concrete beams unless noted otherwise

2. The typical Tie-Beam is an 8" x 16" concrete beam, with (2) #5 bars top and bottom, with no stirrups.

3. See the Concrete Notes for the concrete and steel strengths and specifications

4. Stirrups maybe used to secure top steel during concrete pouring in beam with no stirrups 5. Maintain 1-1/2" cover on all steel within the beam. 6. Horizontal steel shall be placed so that it is as close to the exterior surfaces of the

7. Horizontal steel over masonry walls shall be spliced in a vertical plane 8. Horizontal steel over openings shall be spliced in a horizontal plane within 1/3 of the

beam without violating the cover requirements.

beams span from the supports.

9. All horizontal steel shall be continuous and shall be lap spliced a minimum of 30 bar

diameters (18.75" for # 5 bar) or 12" whichever is greater

10. See the Plan View of Tie-Beam Reinforcement for Corners and Intersections detail for the steel placement and splicing at these locations.

1. These notes and specifications apply to all concrete pads unless noted otherwise:

2. See the Concrete Notes for the concrete and steel strengths and specifications 3. All reinforcement shall have a minimum of 3" cover on all sides.

4. All horizontal reinforcement shall have a length of the pad length, in the same direction

as the reinforcement, minus 2 times the 3" minimum cover. 5. All horizontal reinforcement bars shall be evenly spaced within the pad minus the 3" cover

6. All horizontal reinforcement bars shall be located at the depth specified in plan. 7. All vertical bars that terminate within the pad perimeter shall have (1) 90-degree hook of equal bar size as the vertical from above for each vertical reinforcing bar

8. All 90-degree hooks shall either be bent into the end of the vertical steel or lap spliced to the vertical steel.

9. All lap splices shall be a minimum of 30 (18.75" for # 5 bar) bar diameters in concrete 10. All 90 degree hooks shall have a minimum of a 12 bar diameters (8" for # 5 bar)

11. All hook extensions shall be located in the horizontal plane at 3" plus 1/2" bar diameter from the center of the extension to the bottom edge of the concrete pad to the centerline

12. All hook extensions shall be run in the horizontal plane away from the center of the pad when the center of the vertical of the hook extension is within 5" from the center of the

13. All hook extensions shall be run in the horizontal plane toward the center of the pad when the center of the vertical of the hook extension is not within 5" from the center of

Concrete Stem Wall Footer And Masonry Stem Wall Notes:

1. These notes and specifications apply to all stem walls unless noted otherwise: 2. See the Concrete and Masonry Notes for the masonry, grout, concrete, and steel strengths

3. All reinforcement shall have a minimum of 3" cover on all sides

All horizontal reinforcement shall be continuous.

5. All horizontal reinforcement bars shall be evenly spaced horizontally within the footer minus the 3" cover on each side

6. All horizontal reinforcement bars shall have a minimum horizontal spacing within the footer of (1) horizontal bar diameter or 1" whichever is greater.

7. All horizontal reinforcement bars shall be located at the depth specified in plan.

8. All vertical reinforcement that terminates within the footer shall have (1) 90-degree hook

of equal bar size as the vertical from above for each vertical reinforcing bar. 9. All 90-degree hooks shall either be bent into the end of the vertical steel or lap spliced

10. All lap splices shall be a minimum of 30 (18.75" for # 5 bar) bar diameters in concrete and 48 (30" for # 5 bar) bar diameters in masonry or 12" whichever is greater.

11. All 90 degree hooks shall have a minimum of a 12 bar diameters (8" for # 5 bar)

12. All hook extensions shall be located in the horizontal plane at 3" plus 1/2" bar diameter from the center of the extension to the bottom edge of the concrete footer to the centerline

13. All hook extensions shall be run parallel to the horizontal steel in the footer.

14. All stem walls shall have a #5 bar vertical in a grout filled cell @ 48" o.c.. @ both sides of every opening, @ every corner, and @ every reinforced vertical cell extending up to

15. All stem walls shall have horizontal masonry joint reinforcement provided above the footer

at 16" o.c., starting and ending 8" above the footer and below the finished floor. Use Truss-type with 6" lap at all splices. 16. When specified by footing detail, slabs under vertical reinforced masonry walls shall have

a #4 bar with a 90 degree hook, extending 30" into slab @ a depth of 2" below slab surface & extending 6" vertically down the stem wall @ 3" clear distance from exterior edge of stemwall or @ the center line of an interior stem wall. Provide these hooks @ every stending up to the tie beam (except where noted within wall opening @ 48" o.c. from vertical at each end of wall opening.

17. When specified by footing detail slabs under bearing frame walls shall have a #4 bar with a 90 degree book extending 30" into slab @ a depth of 2" below slab surface extending 6" vertically down the stem wall @ 3" clear distance from exterior edge of stem wall or @ the center line of an interior stem wall. Provide these hooks @ 48" o.c under frame, @ every post location, and within wall opening @ 48" o.c. from post hook at each end of wall opening.

Grout Filled Masonry Column Notes:

1. These notes and specifications apply to all Grout Filled Masonry Columns unless noted

2. See the Masonry Notes for the masonry, grout, and steel strengths and specifications. 3. All reinforcement shall have a minimum cover of 1/2" between the interior surface of the C.M.U. cell and all sides of the reinforcement.

4. All vertical steel shall be placed so that it is as close to the interior surfaces of the C.M.U. without violating the cover requirements

5. All vertical steel shall have a clear distance between bars not less than 1-1/2 times the vertical bar diameter, or less than 1-1/2" whichever is greater. 6. All lateral ties shall have a vertical spacing not exceeding 16 longitudinal (vertical) bar

diameters. 48 lateral tie bar or wire diameters, or lest cross-sectional dimension of the

7. All lateral ties shall be located vertically not more than ¢ a lateral tie spacing above the top of the footing or slab in any story, and shall be located vertically not more than & a lateral tie spacing below the lowest horizontal reinforcement in the beam, girder, slab, or

8. All vertical bars shall terminate at both ends with (1) 90-degree hook of equal bar size

9. All 90-degree hooks shall either be bent into the end of the vertical steel or lap spliced

10. All lap splices shall be a minimum of 48 (30" for # 5 bar) bar diameters in masonry or 12" whichever is greater.

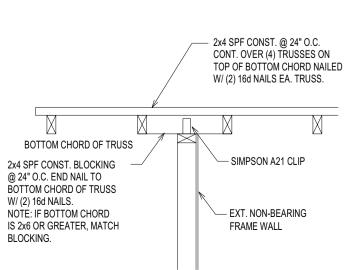
11. All 90 degree hooks shall have a minimum of a 12 bar diameters (8" for # 5 bar)

12. All vertical steel shall be terminated 2" plus 1/2" bar diameter from the center of the extension to the top of the tie-beam and provide a 90-degree hook with an extension that is parallel with the horizontal steel with in the tie-beam. 13. All hooks in the tie-beams shall have a minimum of 2.5" side cover and 2" end cover.

15. All hooks in the footers shall have a minimum of 2.5" side cover and 3" end cover.

14. All vertical steel shall be terminated 3" plus 1/2" bar diameter from the center of the

extension to the bottom of the footer and provide a 90-degree hook with an extension that



1. All wood framing shall be fabricated and installed per ATIC and TPI and National Design

2. All structural wood members shall be Southern Yellow Pine, unless noted otherwise.

3. Unless noted otherwise, the following minimum grades shall be used: A. Structural Light Framing 2" x 4" thick x 2" x 4" wide: #2 non-dense or better. B. Studs Size 2" to 4" Thick x 2" to 6" wide: #2 dense or better.

C. Structural Joist and Planks size 2" to 4" thick x 5" to 8" wide; #2 or better: 9 to 12" wide: #1 or better. D. Light Framing Size 2" to 4" thick x 3" to 4" wide; #2 non-dense or better. E. Alternate partition wall framing construction; SPF or Hem-Fir Stud, Non-bearing.

4. Structural Wood Panels (Sheathing) shall be specified by thickness, grade, APA Span

Rating, and Exposure Durability Classification as shown on the drawings. Nail as specified

5. All wood members exposed to weather or in contact with masonry, concrete or soil shall be pressure treated. Clearance between wood siding and earth on the exterior of a building

shall be not less than 6 inches except where siding, sheathing and wall framing are of approved pressure treated wood. 6. Contractor shall provide all fastening devices as shown on the drawings and as necessary

and suited for each application. Fastening subject to moisture shall be Hot-Dip Galvanized to ASTM A-153-80, or Stainless Steel.

7. All metal connections and fabrications shall comply with AISC Specifications. 8. Solid block all joists and rafters at interior points of support

9. Prefabricated structural trusses shall comply with NFPA, National Specifications for Wood Construction, TPI Design Specifications for Metal-Plated Wood Trusses and AITC 100. 10. All trusses shall be designed and certified by the Truss Manufacturer's State of Florida

Registered Engineer. 11. Contractor shall correlate with Truss Manufacturer to ensure that adequate bearing is provided at end reactions of all Girder Trusses.

12. Truss Manufacturer shall submit shop drawings to the Contractor and Designer for Review and approval prior to fabrication. Contractor shall be responsible for field Verification of dimensions, materials and conditions.

13. At volume ceiling conditions, align trusses to provide a smooth and unbroken interior wall

14. Brace trusses during erection and after permanent installation to comply with TPI BWT-76 15. Micro-Lams (or equal Paralams, LVL's, etc.) shall be used where specified on Engineered plans and installed in accordance with Manufacturer's recommendations. Any edges or ends exposed to the weather shall be protected by the installation of 26 Ga. Minimum, Galvanized

16. Splices in multi-board continuous beams shall be allowed for one board only per span and only at the quarter point of the span, unless shown otherwise.

17. Double top plates shall be overlapped at corners and secured with 4-10d nails. Mid-wall splices shall be 48" minimum w/ (2) rows of 10d nails at 4" O.C., staggered

1. Concrete masonry units shall be ASTM C90-75, Hollow Load Bearing concrete masonry units, Type I, Grade N-1, normal weight, with a minimum net area compressive strength =

2. Concrete masonry units shall be laid in running bond pattern with minimum of face shell

3. Mortar shall conform to ASTM C270 and be Type M or S. 4. Unless noted otherwise, the following minimum horizontal masonry joint reinforcement shall be provided (Use Truss-type with 6" lap at all splices):

A. No horizontal masonry joint reinforcement shall be provided above the finished floor, if the masonry wall has a bearing height less than 10' A. F. F. B. Horizontal masonry joint reinforcement shall be provided above the finished floor at 24" o.c., starting and ending 8" above the finished floor and below the

tie-beam, if the masonry wall has a bearing height equal to or greater than 10' and less than 12' A. F. F. C. Horizontal masonry joint reinforcement shall be provided above the finished floor at 16" o.c., starting and ending 8" above the finished floor and below the tie-beam, if the masonry wall has a bearing height equal to or greater than 12'

D. Horizontal masonry joint reinforcement shall be provided above the finished floor at 8" o.c., starting and ending 8" above the finished floor and below the tie-beam, if the masonry wall has a bearing height equal to or greater than 16

5. Masonry: Minimum compressive strength F'm=1.500 psi 6. Grout shall be Pea Rock (3/8" Maximum Diameter Aggregate) Concrete mix with F'c=3000

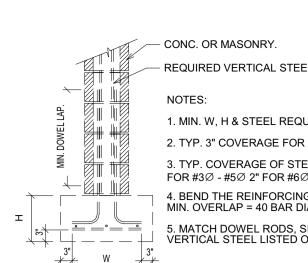
psi and having a slump between 8" and 11" at the time of placement.

7. Reinforcing steel shall be ASTM A-615 Grade 60 Deformed Steel. 8. Minimum lap splice shall be 48 Bar Diameter or 12" whichever is greater.

Exposed Ceiling Notes: All exposed ceilings in Entry's, Porches and Lanais shall be of one of the following types: Substitution ceiling type is allowed

1. 1/2" plywood or OSB sheathing fastened directly to trusses or framing.

2. 1/2" drywall fastened to min. 1x3 firing strips at 16" o.c. running perpendicular to 3. 1/2" drywall fastened to min. 2x4 bridge blocking at 48" o.c. running perpendicular to trusses or framing & supporting all drywall edges.



- 1X- PT WOOD BUCK TEMPORARY NAILED W/

1. INITIAL ATTACHMENT OF PT WOOD BUCKS TO MASONRY OPENINGS IS AT THE INSTALLERS DISCRETION AND

2. REFER TO MFG. CUT SHEETS FOR ADDITIONAL REQUIREMENTS FOR THE SPECIFIC WINDOW OR DOOR. THE

MAY BE BUT NOT LIMITED TO ADHESIVES OR CASE HARDENED NAILS MANUALLY OR PNEUMATICALLY

DRIVEN AS LONG AS THE BUCK IS NOT SPLIT. PERMANENT ATTACHMENT OF THE WINDOW/DOOR FRAME.

MIN. (2) PNEUMATIC NAILS. SET IN (2)-1/4"

BANDS OF POLYURETHANE TYPE CAULK.

3/16" OR 1/4" X 3 1/4" TAPCON @

MEG. SIZE AND SPACING OF ATTACHMENTS SUPERCEDE DETAILS ABOVE

TYPICAL WINDOW / DOOR ATTACHMENT DETAILS

WINDOW FRAME.

─ 8" CMU GROUTED SOLID

1X_ SQUARE WINDOW BUCK DETAIL

AND PT BUCK IS AS SHOWN ABOVE.

1. MIN. W, H & STEEL REQUIREMENTS PER FOUNDATION PLAN. 2. TYP. 3" COVERAGE FOR ALL FOUNDATION STEEL 3. TYP. COVERAGE OF STEEL FOR CONCRETE COLUMNS 1-1/2" FOR #3Ø - #5Ø 2" FOR #6Ø - #8Ø 4. BEND THE REINFORCING BARS OR USE STD. CORNER BARS. MIN. OVERLAP = 40 BAR DIAMETERS 5. MATCH DOWEL RODS, SIZE & NUMBER OF, TO REQUIRED VERTICAL STEEL LISTED ON FLOOR AND FOUNDATION PLANS

1X- PT WOOD BUCK TEMPORARY NAILED W

MIN. (2) PNEUMATIC NAILS. SET IN (2)-1/4"

BANDS OF POLYURETHANE TYPE CAULK.

- 3/16" OR 1/4" X 3 1/4" TAPCON @ MAX. 12"

N.T.S.

- 8" CMU GROUTED SOLID.

1X SQUARE ENTRY DOOR BUCK DETAIL

CONCRETE OR MASONRY COLUMN TO FOOTING

1. MIN. W, H & STEEL REQUIREMENTS PER FLOOR OR STRUCTURAL PLAN. 2. TYP. 1-1/2" COVERAGE FOR ALL BEAM STEEL - BOTTOM BEAM STEEL. 3. TYP. COVERAGE OF STEEL FOR CONCRETE COLUMNS 1-1/2" FOR #3Ø - #5Ø 2" FOR #6Ø-#8Ø BARS. MIN. OVERLAP = 40 BAR DIAMETERS. CONCRETE OR VERTICAL STEEL LISTED ON FLOOR AND FOUNDATION PLANS. MASONRY COL. 6. COLUMN TIES, IF REQUIRED BY NOTE ON PLANS, SPACE PER PLAN REQUIREMENTS. **CONCRETE OR MASONRY COLUMN TO BEAMS**

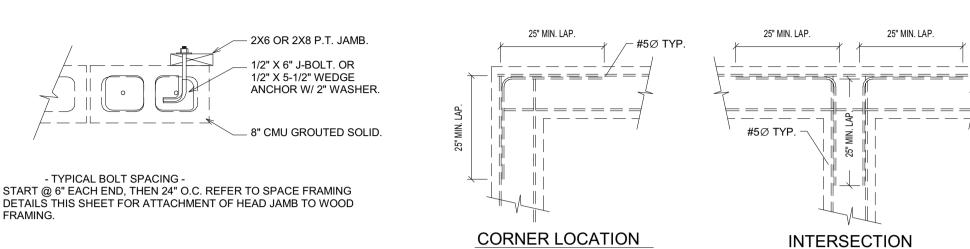
 BOTTOM BEAM STEEL POURED CELLS W/REINFORCEMENT CONTINUOUS FROM BEAM TO FOOTING 1. SEE FLOOR PLAN FOR FILLED CELL LOCATIONS AND PLACE VERTICAL REINFORCEMENT AS SHOWN 2. BEND THE REINFORCING BARS OR USE STD. CORNER BARS.

TOP BEAM STEEL

N.T.S.

N.T.S.

CONCRETE BEAM TO FILLED CEL



TRUSS OR ROOF FRAMING ATTACHED

TO HEADER BEAM W/ MTS20 UNLESS

NOTED OTHERWISE.

TYPICAL GARAGE DOOR JAMB DETAIL

TOP BEAM STEEL

1. PROVIDE FILLED CELL AND VERTICAL REINFORCEMENT AT CORNERS. 2. BEND THE REINFORCING BARS OR USE STD. CORNER BARS.

MTS12 EA. STUD

RUSS OR ROOF FRAMING.

CONTINUOUS DBL. 2X- TOP PLATE, NAIL

LOWER PLATE TO STUDS W/2-16d FOR

LARGER. NAIL UPPER PLATE TO LOWER

PLATE W/ SAME SPEC. MIN. 32" PLATE

FOR WALLS TO 8'-2" USE 2X- BLOCKING

@ MID-HEIGHT FOR WALLS OVER 8'-2"

BLOCKING W/ (2) 16d NAILS EA. END.

- 2X- P.T. SOLE PLATE NAIL TO STUDS

W/2-16d FOR 2X4 FRAMING & 3-16d

- 5/8" J-BOLT OR REDHEADS (TYP.)

@ 18" W/ 3" X 3" X 1/8" WASHER

MIN. END & SPLICE DISTANCE = 6"

CONNECTION "A" (BEAM TO COLUMN)

SPANS UP TO 7'-6" (2)LSTA 18 EA. SIDE

SPANS UP TO 10'-6" (2)LSTA 24 EA. SIDE

SPANS UP TO 12'-6" (2)LSTA 30 EA. SIDE

CONNECTION " B" (BASE OF COLUMN)

SPANS UP TO 10'-6" HTT16 EA. SIDE

SPANS UP TO 12'-6" HTT22 EA. SIDE

SPANS UP TO 7'-6" LTT20B EA. SIDE

FOR 2X6 & LARGER FRAMING.

BLOCKING IS @ 4'-0" O.C. MAX. ATTACH

2X4 FRAMING & 3-16d FOR 2X6 &

TYP. 2X- STUDS @ 16" O.C.

JACKS ON STUD LAYOUT

CONCRETE BEAM CORNERS

- DBL. 2X4 SPF #2 TOP PLATE. ON 2X4 SPF #2 STUDS @ 16" O.C.. USE SIMPSON H-5 @ TOP & SIMPSON H-2 @ BOTT. 2X4 SYP #2 BLOCKING @ 16" O.C BETWEEN TOP-CHORDS ATTACH W/2 16d NAILS EA END. 2X4 SPF #2 BOTT, PLATE ATTACH TO BLOCKING W/2 16d NAILS @ EA LOCATION.

TYPICAL FRAME KNEE WALL ON MFG. TRUSS DETAIL

HEADER BEAM @ TOP OF WALL BELOW DBL. TOP PLATE. CONNECTION "A" KING STUDS. HEADER SCHEDULE - HEADER STUDS. -NAIL TO KING 0 TO 12'-0" WALL HEIGHT STUDS W/16d @ KING STUDS HEADER STUDS 16" O.C. (1) EA. SIDE (1) EA. SIDE (2)-2X6 S.P.F. W/ 1/2" PLYWOOD 3'-4" TO 8'-0" (1) EA. SIDE (1) EA. SIDE (2)-2X8 S.P.F. W/ 1/2" PLYWOOD 2X- SUB SILL 8'-0" TO 10'-0" (1) EA. SIDE (2) EA. SIDE (2)-2X10 S.Y.P. NO. 2 W/ 1/2" PLYWOOD (WHERE APPLICABLE) 10'-0" TO 12'-0" (1) EA. SIDE (2) EA. SIDE (2)-2X12 S.Y.P. NO. 2 W/ 1/2" PLYWOOD . HEADER BEAMS ARE DESIGNED FOR A GRAVITY LOAD OF 1500#/TRUSS AND AN UPLIFT LOAD OF 1000"/TRUSS. REFER TO FLOOR/STRUCTURAL PLAN FOR ADDITIONAL HEADER BEAM SPECS 12d NAILS @ 16" O.C. ALONG EACH EDGE. SPLICES, IF NECESSARY, SHALL BE LOCATED @ 1/4 THE LENGTH OF THE BEAM BETWEEN SUPPORTS. 3. SPACE FRAMING FROM HEADER TO TOP OF WINDOW/DOOR OPENING IF NECESSARY. CONNECTION "B" —

R703.7.5 Curing Note The finish coat for two-coat cement plaster shall not be applied sooner than seven days after application of the first coat. For three-coat cement plaster, the second coat shall not be applied sooner than 48 hours after application of the first coat. The finish coat for threecoat cement plaster shall not be applied sooner than seven days after application of the second coat MTS16 TRUSS TO TOP PLATE - H6 @ EA STUD 1/2" CDX PLYWOOD w/Water-resistive barriers shall be installed as required in Section R703.2 and, where applied over wood-based sheathing shall include a water-resistive vapor-permeable barrier with a performance at least equivalent to two layers of Grade D paper. The individual layers shall be installed independently such that each layer provides a separate continuous plane and any flashing (installed in accordance with Section R703.4) intended to drain to the water-resistive barrier is directed between the layers. COVERED WITH STUCCO or SIDING. MSTM16 @ EA. STUD. (4) - TYP. WALL TO TRUSS CONN. 1/4"x 2-1/4" TAPCONS OR EQUAL. - CONC. TIE BEAM

---- A=DISTANCE OR MIN. 4'-0" ----PROVIDE 8d RING SHANK NAILS @ 4"O.C. @ EDGES & 4"O.C. IN INTERMEDIATE

FIELD AREA. SHEAR CAPACITY 240 PLF. PROVIDE 8d RING SHANK NAILS @ 4"O.C. @ EDGES & INTERMEDIATE FIELD

4"O.C. @ EDGES & INTERMEDIATE FIELD SHEAR CAPACITY 240 PLF. PROVIDE 2"x 4" EDGES OF SHEATHING. ATTACH BLOCKING TO TRUSSES W/ MIN. (3) 12d TOENAILED @ EACH END.

BLOCKING IN ROOF FRAMING @ ALL UNSUPPORTED

1. BEARING WALL DESIGNED FOR A GRAVITY LOAD OF 1500#/TRUSS 2. REFER TO FLOOR PLAN FOR FRAMING MATERIAL, SIZE & SPACING. 3. FASTEN 2X- BEAMS & STUDS W/ 16d AT 16" O.C. 4. SPACE FRAMING FROM HEADER TO TOP OF WINDOW/DOOR OPENING

> 5. EQUIVALENT FASTENERS OF OTHER MANUFACTURES MAY BE SUBSTITUTED FOR THE LISTED FASTENERS. TYPICAL BRG. FRAME WALL DETAIL (ALL WALL HEIGHTS) INTERIOR, EXTERIOR & EXTERIOR NON BRG.

R803.2.3.1Sheathing fastenings. Wood structural panel sheathing shall be fastened to roof framing in accordance with Table R803.2.3.1. Where the sheathing thickness is 15/32 inches and less, sheathing shall be fastened with ASTM F1667 RSRS-01 (23/g" × 0.113") nails. Where the sheathing thickness is greater than 15/32 inches, sheathing shall be fastened with ASTM F1667 RSRS-03 (21/g" × 0.131") nails or ASTM F1667 RSRS-04 (3 × 0.120") nails. RSRS-01, RSRS-03 and RSRS-04 are ring shank nails meeting the specifications in ASTM F1667

			RO	OF SHEAT	BLE R803 THING A		MENT ^{a, b}									
								WIND	SPEED							
Rafter/Truss Spacing24 in. o.c.	11	5 mph	12	20 mph	130	mph_	140	mph_	<u>150</u>	mph_	160	mph_	170	mph	180	mph
	<u>E</u>	<u>F</u>	<u>E</u>	<u>F</u>	<u>E</u>	E	E	E	E	<u>F</u>	E	E	E	<u>F</u>	E	E
			Al-		Exposure	B					Ver				33/4	
Rafter/Truss SG = 0.42	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	6	<u>6</u>	<u>6</u>	4	4	4	4	4	4
Rafter/Truss SG = 0.49	<u>6</u>	12	<u>6</u>	12	<u>6</u>	6	<u>6</u>	6	<u>6</u>	<u>6</u>	6	6	<u>6</u>	<u>6</u>	<u>6</u>	6
			A10		Exposure	<u>C</u>					Oir		104		518	
Rafter/Truss SG = 0.42	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	4	4	4	4	4	4	3	3	3	3
Rafter/Truss SG = 0.49	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	6	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	6	4	4	4	4
	\$1000 miles		A.		Exposure	D					Uer .		104		558	
Rafter/Truss SG = 0.42	<u>6</u>	<u>6</u>	<u>6</u>	<u>6</u>	4	4	4	4	4	4	3	3	3	3	3	3
Rafter/Truss SG = 0.49	<u>6</u>	<u>6</u>	<u>6</u>	6	6	<u>6</u>	6	<u>6</u>	4	4	4	4	4	4	4	4
= Nail spacing along panel edges (inches)		ēļe.	70				- Table 1	1.0		i.e.	U _O	-	200		550	

F = Nail spacing along intermediate supports in the panel field (inches)

a.For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.

The minimum thickness and span rating for wood structural panel roof sheathing shall not exceed the values set forth in Table R803.2.2.

panel roof sheathing shall not cantilever more than 9 inches beyond the gable end wall unless supported by gable overhang framing.

Reviewed for Code Compliance By: Andrew Nielsen Date: 6/28/2021 RESMSTR2021-00191

MINIMUM ROOF SHEATHING THICKNESS Rafter/Truss Spacing24 in. o.c. mum Sheathing Thickness, inches(Panel Span Rating) Exposure B num Sheathing Thickness, inches(Panel Span Rating) Exposure Minimum Sheathing Thickness, inches(Panel Span Rating) Exposure D Wood structural panel used as roof sheathing shall be installed with joints staggered in accordance with Section R803.2.3.1 for wood roof framing or with Section R804.1 for cold-formed steel roof framing. Wood structural

FRAME WALL PARALLEL TO TRUSS

ROOF SHEATHING NAILING SPECIFICATIONS

ALL FRAMING IS MIN. SPF NO. 2

TYPICAL KNEEWALL W/ TRUSS

N

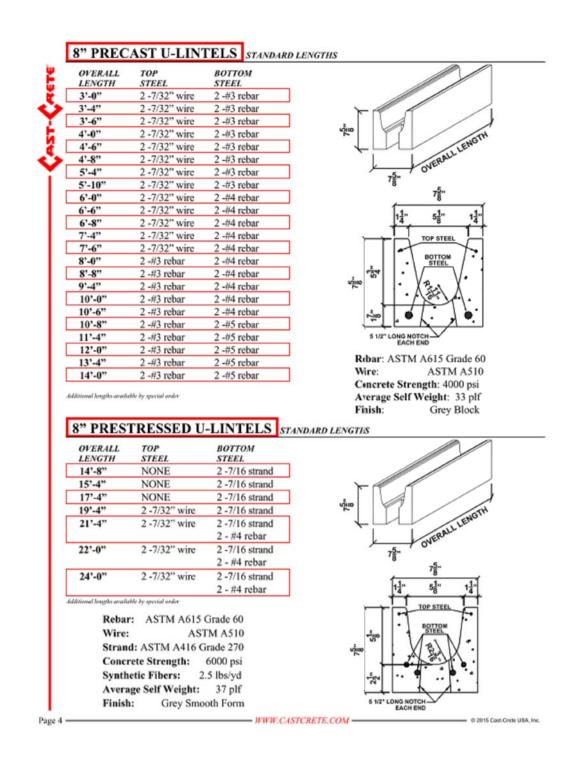
Revision 1714 Master Plan June 18, 2021 Start Date Drawn Bv jrr Checked By ajq

Structural Notes & Details Print Date: 6/17/2021 4:05:15 PM

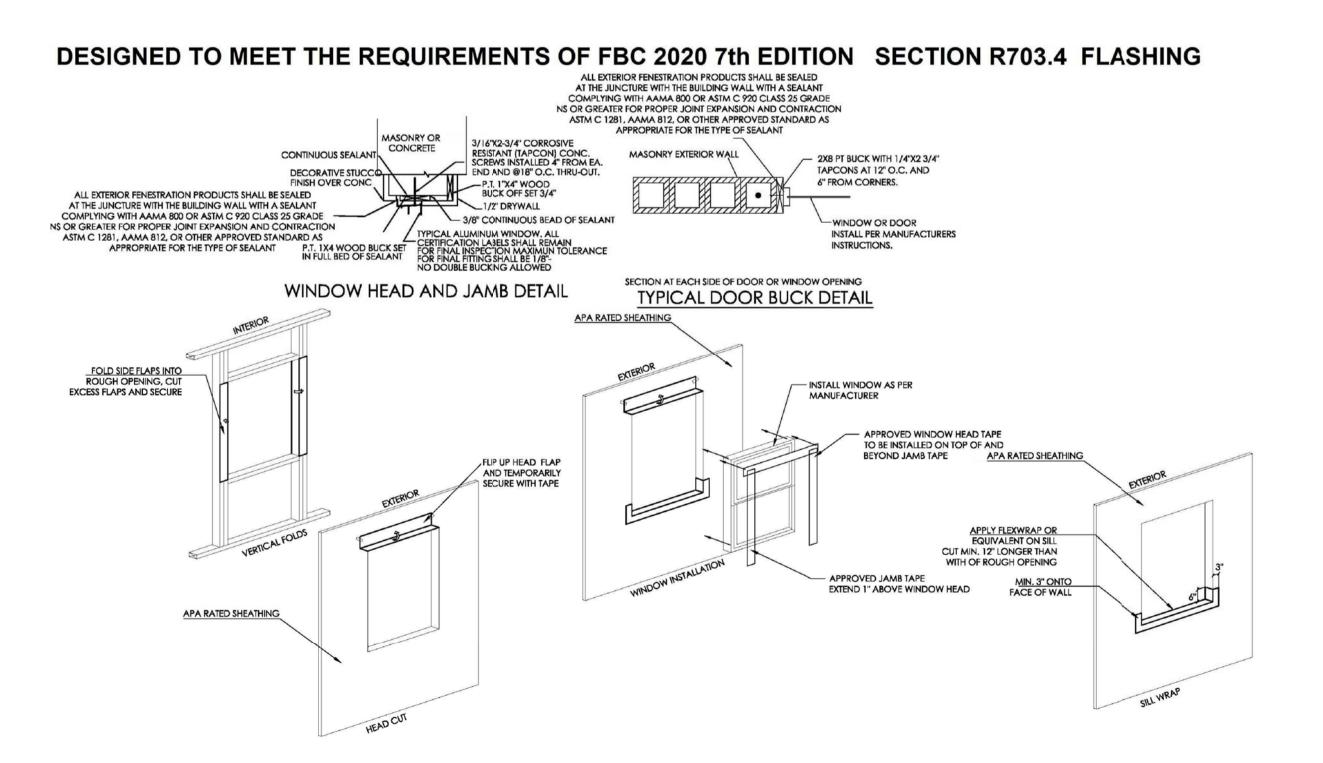
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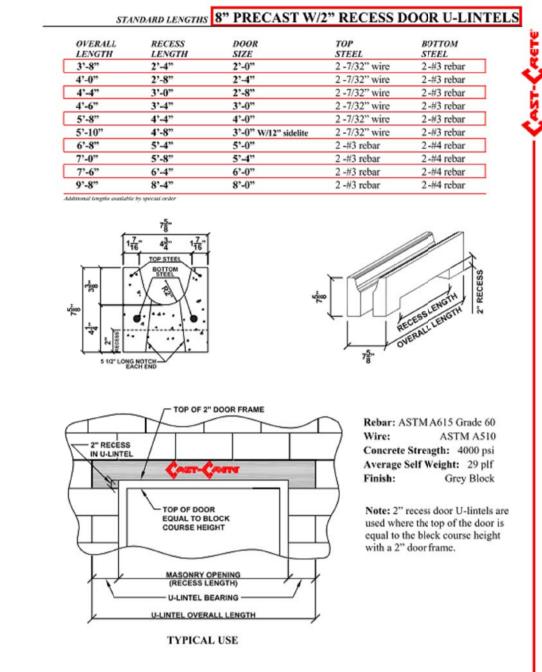
SHEAR CAPACITY 240 PLF.

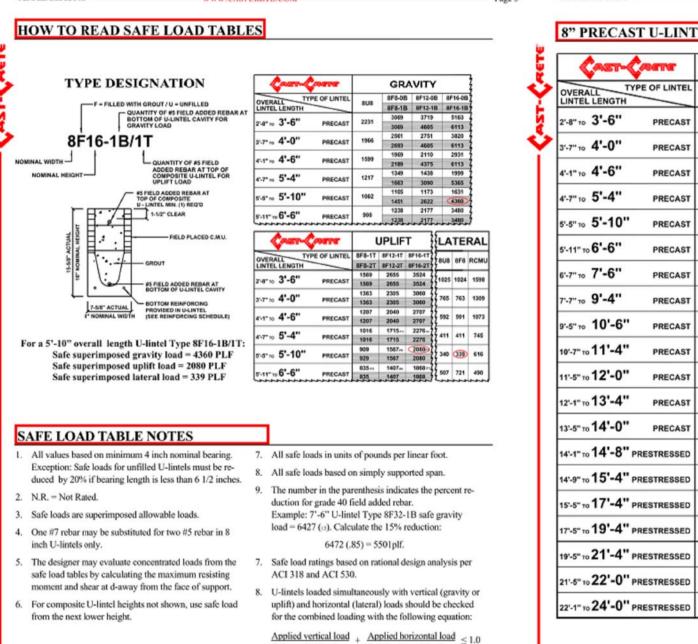
PROVIDE 8d RING SHANK NAILS @



MARK	BEARING	UPLIFT	NO.	MANUFA	CTURER	LOCATION	NOTES
	TYPE			SIMPSON	USP	1	
YPICAL TRUSS	CONCRETE	UP T0 1615	(1)	HETA20	HTA20	ALL UNMARKED ATTACHMENTS	1. EQUIVALENT FASTENERS OF OTHER MANUFACTURES
1-FT & BK	CONCRETE	1100	(1)	HETA20	HTA20	HG2	MAY BE SUBSTITUTED PER APPROVAL OF THE
2-LT & RT	CONCRETE	1000	(1)	HETA20	HTA20	HG3	PERMITTING BUILDING DEPARTMENT. 2. TRUSS ERECTOR SHALL USE CAUTION GIVEN TO
3-REAR	CONCRETE	1100	(1)	HETA20	HTA20	TG1	NAILING SO AS NOT TO SPLIT THE LUMBER.
3-FRONT	WOOD	1100	(1)	MTS16	MTW16	TG1	3. ALL TRUSS TO TRUSS CONNECTIONS BY TRUSS
							MANUFACTURER.
							 4. TRUSSES PREPARED BY: <u>Southwest Structural Systems</u>, <u>In</u> Dated: 06/16/2021 Job# 18712
							5. THESE PLANS HAVE BEEN COORDINATED WITH THE
							TRUSS LAYOUT LISTED IN #4 ABOVE. FOUNDATION
							HAS BEEN CHECKED FOR REACTIONS AND STRAPPING SPECIFIED HERE FOR ALL UPLIFTS.
							SPECIFIED HERE FOR ALL OPLIFTS.
TYPICAL TRUSS	WOOD	UP TO 1195	(1)	MTS16	MTW16	ALL UNMARKED ATTACHMENTS	7

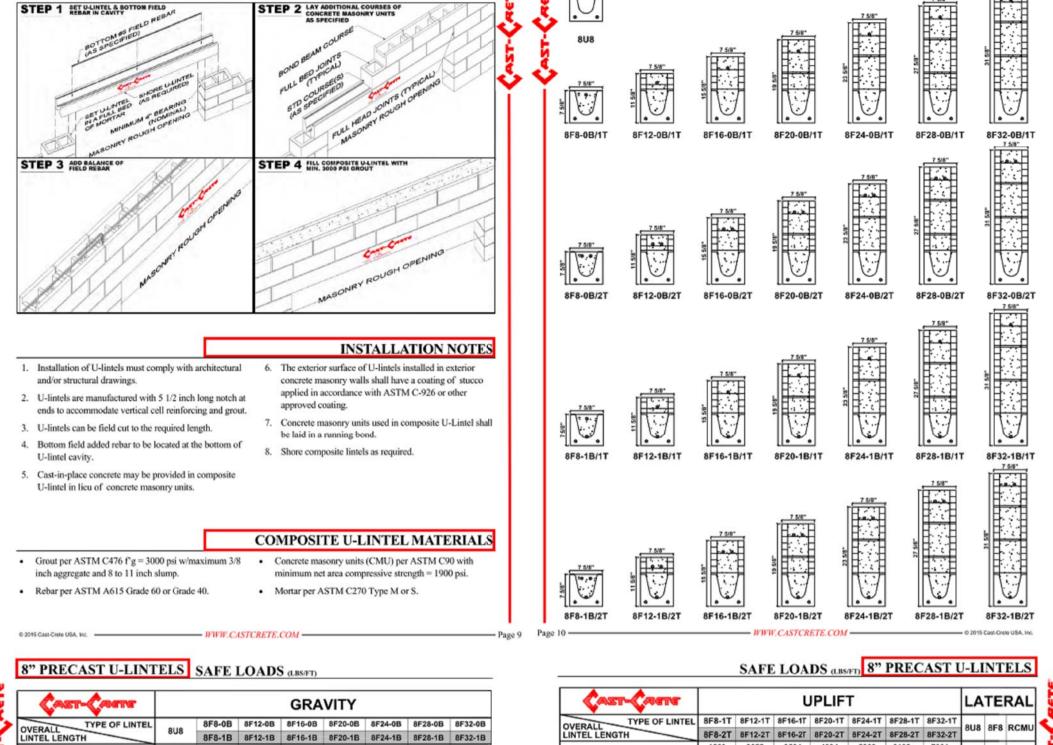






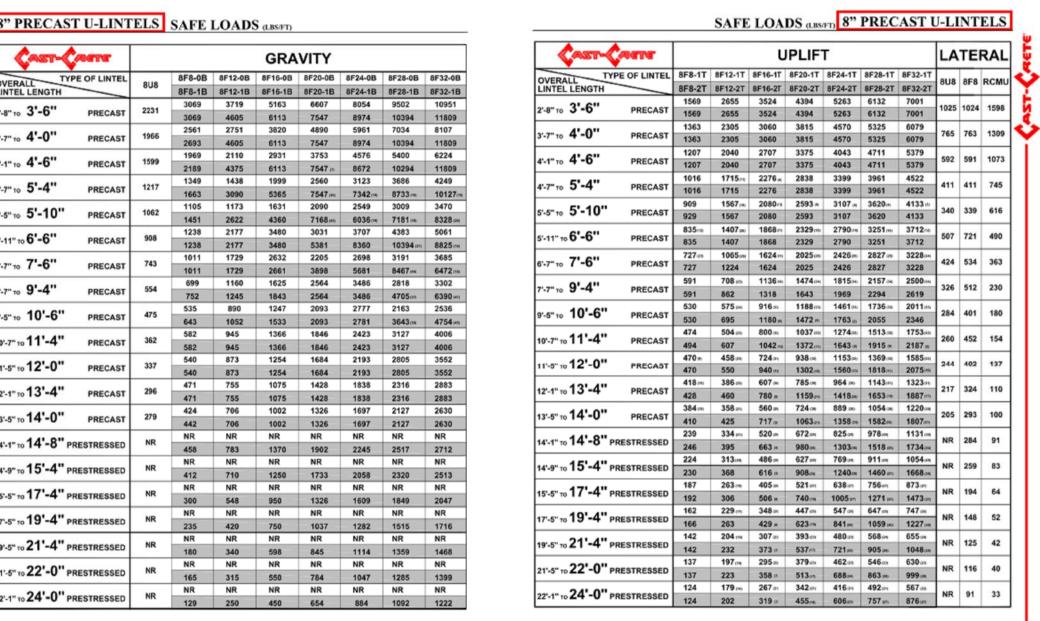
740	METE.				G	RAVI'	ΓY			
OVERALL	E OF LINTEL	8RU6	8RF6-0E	8RF10	0B 8RF1	4-0B 8RF	18-0B 8	RF22-08	8RF26-0	B BRF30
LINTEL LENGTH	•	9606	8RF6-1E	8RF10	1B 8RF1	4-1B 8RF	18-1B 8	RF22-18	8RF26-1	B 8RF30
з'-8" то 4'-4"	PRECAST	1635	1749	3355	32	80 4	349	5421	6493	7567
3-8-10 4 -4	PRECAST	1000	1891	3699	520	06 6	639	8060	9479	1089
4'-5" to 4'-6"	PRECAST	1494	1596	3063	29	92 3	968	4946	5924	6904
4-5 10 4 -0	PRECASI	30376.35	1756	3699	521	06 6	639	8060	9479	1089
4'-7" to 5'-8"	PRECAST	866	920	1770	17	16 2	277	2839	3402	3966
4-7 10 3 -0	PRECASI		1167	2481	45	67 6	389	8060 INI	7917 a	9311
5'-9" to 5'-10"	PRECAST	810	859	1653	160	00 2	124	2649	3174	3700
5-9 10 J - 10	PRECASI	5575.572	1113	2342	42	42 6	639 ∞	8060 m	7402 🕾	8706
5'-11" TO 6'-8"	PRECAST	797	901	1825	31:	20 5	048	7747	9448	7360
5-11 100 -0	PRECASI	2000	901	1825	31:	20 5	048	7915	9479	1089
6'-9" to 7'-6"	PRECAST	669	755	1490	24	59 3	776	5743	7239	5623
0.9 10 1 -0	PRECASI		755	1490	24	59 3	776	5743	8998 =	1089
7'-7" 10 9'-8"	PRECAST	411	466	999	150	68 2	253	3129	4091	3146
7-7 10 0 0	PRECASI	20,000	526	999	15	68 2	253	3129	4150	5891
CAST-C	SETE				JPLIF				-	TERA
OVERALL	E OF LINTEL	8RF6-1T	8RF10-1T	8RF14-1T	8RF18-1T	8RF22-1T			T 8RU6	TERA
FVD	700000000000000000000000000000000000000	8RF6-2T	8RF10-2T	8RF14-1T 8RF14-2T	8RF18-1T 8RF18-2T	8RF22-1T 8RF22-2T	8RF26-2	T 8RF30-2	T 8RU6	
OVERALL LINTEL LENGTH	E OF LINTEL	8RF6-2T 905	8RF10-2T 1668	8RF14-1T 8RF14-2T 2362	8RF18-1T 8RF18-2T 3056	8RF22-1T 8RF22-2T 3751	8RF26-2 4445	T 8RF30-2 5140	T 8RU6	
OVERALL	700000000000000000000000000000000000000	905 905	8RF10-2T 1668 1668	8RF14-1T 8RF14-2T 2362 2362	8RF18-1T 8RF18-2T 3056 3056	8RF22-1T 8RF22-2T 3751 3751	8RF26-2 4445 4445	T 8RF30-2 5140 5140	T 8RU6	8RF6 RC
OVERALL LINTEL LENGTH 3'-8" TO 4'-4"	PRECAST	8RF6-2T 905 905 867	8RF10-2T 1668 1668 1604	8RF14-1T 8RF14-2T 2362 2362 2272	8RF18-1T 8RF18-2T 3056 3056 2939	8RF22-1T 8RF22-2T 3751 3751 3607	8RF26-2 4445 4445 4275	T 8RF30-2 5140 5140 4943	T 8RU6	8RF6 RC
OVERALL LINTEL LENGTH	E OF LINTEL	8RF6-2T 905 905 867 867	8RF10-2T 1668 1668 1604 1604	8RF14-1T 8RF14-2T 2362 2362 2272 2272	8RF18-1T 8RF18-2T 3056 3056 2939 2939	8RF22-1T 8RF22-2T 3751 3751 3607 3607	8RF26-2 4445 4445 4275 4275	T 8RF30-2 5140 5140 4943 4943	7 8RU6 7 758	8RF6 RC
OVERALL LINTEL LENGTH 3'-8" TO 4'-4" 4'-5" TO 4'-6"	PRECAST	905 905 905 867 867 675	8RF10-2T 1668 1668 1604 1604 1269 per	8RF14-1T 8RF14-2T 2362 2362 2272 2272 1797	8RF18-1T 8RF18-2T 3056 3056 2939 2939 2939 2326,%	8RF22-1T 8RF22-2T 3751 3751 3607 3607 2854 in	8RF26-2 4445 4445 4275 4275 3382	T 8RF30-2 5140 5140 4943 4943 3911	7 8RU6 7 758	8RF6 RC
OVERALL LINTEL LENGTH 3'-8" TO 4'-4"	PRECAST	8RF6-2T 905 905 867 867 675 675	8RF10-2T 1668 1668 1604 1604 1269 (H)	8RF14-1T 8RF14-2T 2362 2362 2272 2272 1797 1797	8RF18-1T 8RF18-2T 3056 3056 2939 2939 2326 or 2326	8RF22-1T 8RF22-2T 3751 3751 3607 3607 2854 so	8RF26-2 4445 4445 4275 4275 3382 3382	5140 5140 5140 4943 4943 3911 3911	758 694	8RF6 RC 757 11 693 10
OVERALL LINTEL LENGTH 3'-8" TO 4'-4" 4'-5" TO 4'-6" 4'-7" TO 5'-8"	PRECAST PRECAST	8RF6-2T 905 905 867 867 675 675 655	8RF10-2T 1668 1668 1604 1604 1269 (10) 1269	8RF14-1T 8RF14-2T 2362 2362 2272 2272 1797 1797 1746 (m)	8RF18-1T 8RF18-2T 3056 3056 2939 2939 2326 (q 2326	8RF22-1T 8RF22-2T 3751 3751 3607 3607 2854 10 2854 2733 (s)	8RF26-2 4445 4445 4275 4275 4275 3382 3382 3286 n	5140 5140 5140 4943 4943 3911 3911	758 694	8RF6 RC 757 11 693 10
OVERALL LINTEL LENGTH 3'-8" TO 4'-4" 4'-5" TO 4'-6"	PRECAST	8RF6-2T 905 905 867 867 675 675 655	8RF10-2T 1668 1668 1604 1604 1269 (H) 1269 1207 (N)	8RF14-1T 8RF14-2T 2362 2362 2272 2272 1797 1797 1746 (m)	8RF18-1T 8RF18-2T 3056 3056 2939 2939 2326 (a 2326 2259 (a)	8RF22-1T 8RF22-2T 3751 3751 3607 3607 2854 so 2854 2733 ss 2733	8RF26-2 4445 4445 4275 4275 3382 3382 3286 ⋈	5140 5140 5140 4943 4943 3911 3911 3789	7 8RU6 758 694 408	8RF6 RC 757 11 693 10 407 69
OVERALL LINTEL LENGTH 3'-8" TO 4'-4" 4'-5" TO 4'-6" 4'-7" TO 5'-8" 5'-9" TO 5'-10"	PRECAST PRECAST PRECAST PRECAST	8RF6-2T 905 905 867 867 675 675 655 655	8RF10-2T 1668 1668 1604 1604 1269 1207(Hz) 1233 929(16)	8RF14-1T 8RF14-2T 2362 2362 2272 2272 1797 1797 1746 ⁽¹¹⁾ 1746 1530 ₍₁₀₎	8RF18-1T 8RF18-2T 3056 3056 2939 2939 2326 22259 1980	8RF22-1T 8RF22-2T 3751 3751 3607 3607 2854 iii 2733 ch 2733 ch 2733	8RF26-2 4445 4445 4275 4275 3382 3382 3286 2879	T 8RF30-2 5140 5140 4943 4943 3911 3911 3789 3789	7 8RU6 758 694 408	8RF6 RC 757 11 693 10 407 69
OVERALL LINTEL LENGTH 3'-8" TO 4'-4" 4'-5" TO 4'-6" 4'-7" TO 5'-8" 5'-9" TO 5'-10"	PRECAST PRECAST	8RF6-2T 905 905 867 867 675 675 655 655 570	8RF10-2T 1668 1668 1604 1604 1269 MI 1269 1207 (MI 1233 929 MI	8RF14-1T 8RF14-2T 2362 2362 2272 2272 1797 1797 1746-11 1530-11 1530	8RF18-1T 8RF18-2T 3056 3056 2939 2939 2326 2326 2259 1980 1980	8RF22-1T 8RF22-2T 3751 3751 3607 3607 2854 2733 ds 2733 2429 ms	8RF26-2 4445 4445 4275 4275 3382 3382 3286 a 3286 2879	T 8RF30-2 5140 5140 4943 4943 3911 3911 3789 3789 3329	7 8RU6 758 694 408 382 595	8RF6 RC 757 11 693 10 407 69
OVERALL LINTEL LENGTH 3'-8" to 4'-4" 4'-5" to 4'-6" 4'-7" to 5'-8" 5'-9" to 5'-10" 5'-11" to 6'-8"	PRECAST PRECAST PRECAST PRECAST PRECAST	8RF6-2T 905 905 867 867 675 675 655 570 570	8RF10-2T 1668 1668 1604 1604 1269 pth 1269 1207 pth 1233 929 pth 1080 742 cm	8RF14-1T 8RF14-2T 2362 2362 2272 2272 1797 1797 1746-11, 1530-11, 1530 1364-12	8RF18-1T 8RF18-2T 3056 3056 2939 2939 2326 2326 2259 1980 1765sin	8RF22-1T 8RF22-2T 3751 3751 3607 3607 2854 2733 d 2733 2429 2429 2166 d	8RF26-2 4445 4445 4275 4275 3382 3382 3286 2879 155 2879 2567 (mg)	T 8RF30-2 5140 5140 4943 4943 3911 3911 3799 3789 3329 2968	7 8RU6 758 694 408 382 595	8RF6 RC 757 11 693 10 407 69
OVERALL LINTEL LENGTH 3'-8" TO 4'-4" 4'-5" TO 4'-6" 4'-7" TO 5'-8" 5'-9" TO 5'-10"	PRECAST PRECAST PRECAST PRECAST	8RF6-2T 905 905 867 867 675 675 655 655 570 570 506	8RF10-2T 1668 1668 1604 1604 1269 pt 1269 1207 pt 1233 929 pt 1080 742 cs	8RF14-1T 8RF14-2T 2362 2362 2272 2772 1797 1746-11 1746 1530-11 1530 1364-01	8RF18-1T 8RF18-2T 3056 3056 3056 2939 2326 2259 1980 1980 1765	8RF22-1T 8RF22-2T 3751 3751 3607 2854 2733 dt 2733 2429 nt 2429 2166 on 2166	8RF26-2 4445 4445 4275 4275 3382 3382 3286 2879 2879 2567	T 8RF30-2 5140 5140 4943 4943 3911 3911 3799 3759 3329 2968 2968	7 8RU6 758 694 408 382 595	8RF6 RC 757 11 693 10 407 69 381 69 788 46
OVERALL LINTEL LENGTH 3'-8" to 4'-4" 4'-5" to 4'-6" 4'-7" to 5'-8" 5'-9" to 5'-10" 5'-11" to 6'-8"	PRECAST PRECAST PRECAST PRECAST PRECAST	8RF6-2T 905 905 867 867 675 675 655 570 570	8RF10-2T 1668 1668 1604 1604 1269 pth 1269 1207 pth 1233 929 pth 1080 742 cm	8RF14-1T 8RF14-2T 2362 2362 2272 2272 1797 1797 1746-11, 1530-11, 1530 1364-12	8RF18-1T 8RF18-2T 3056 3056 2939 2939 2326 2326 2259 1980 1765sin	8RF22-1T 8RF22-2T 3751 3751 3607 3607 2854 2733 d 2733 2429 2429 2166 d	8RF26-2 4445 4445 4275 4275 3382 3382 3286 2879 115 2879 2567 (mg)	T 8RF30-2 5140 5140 4943 4943 3911 3911 3799 3759 3329 2968 2968	7 8RU6 758 694 408 382 595	8RF6 RC 757 11 693 10 407 69 381 69 788 46

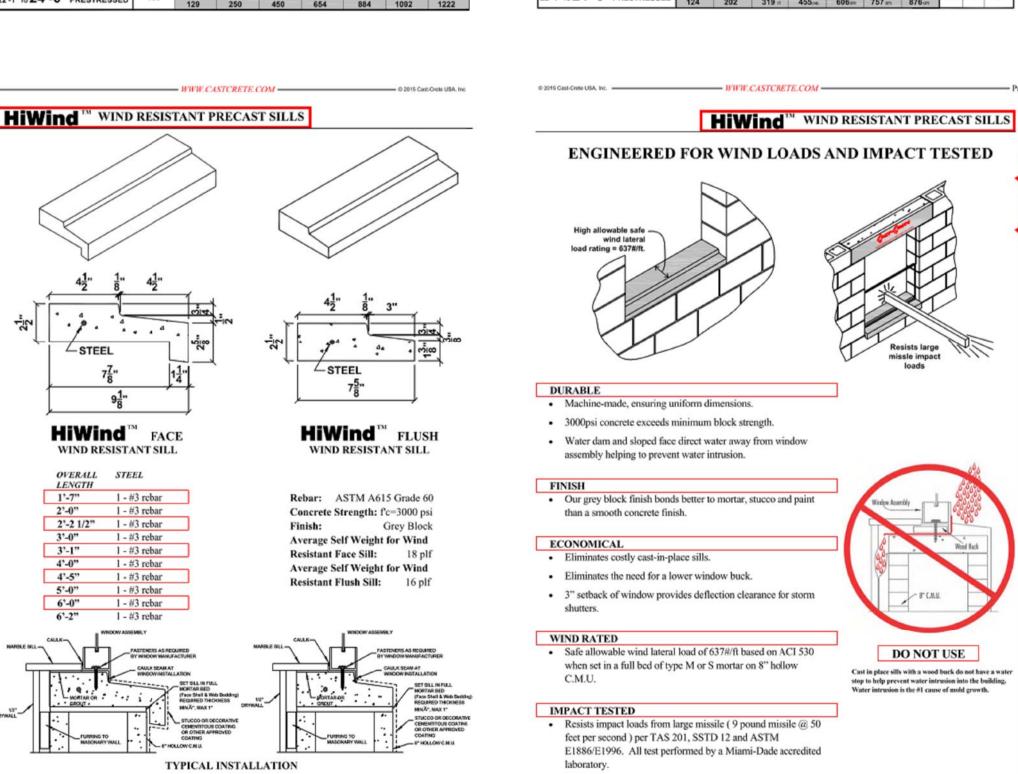
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INSTALLATION OF COMPOSITE U-LINTEL

8" COMPOSITE U-LINTEL TYPES





Reviewed for Code Compliance By: Andrew Nielsen Date: 6/28/2021

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Revision 1714 Master Plan June 18, 2021 Start Date Drawn Bv jrr Checked By ajq

Structural Details & Roof Plan

Print Date: 6/17/2021 4:05:22 PM

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