WALL OPENING SCHEDULE HEADER OPENING OPENING DOOR | WIDTH | HEIGHT TYPE GIRT **JAMBS** C4X2.5 1 - 2 10'-0" 10'-0' ROLL UP DOOR DOUBLE XI4 CHN4X 3 - 5 6'-0" PERSONNEL DOOR DOUBLE 6'-8" 3XI8 CHN4X 6 3'-0" PERSONNEL DOOR DOUBLE 6'-8" 3XI8 CHN4X 7 - 10 6'-0" 4'-0" MINDOM NOTE #4 3XI8

MINDOM

NOTES:

11-16

6'-0"

4'-0"

- I) JAMB MEMBERS SHOWN AS "CHN" ARE CHANNEL MEMBERS (WITHOUT STIFFENER LIPS) AND THOSE SHOWN AS "C" ARE CEE MEMBERS. FIRST NUMBER IS WEB DEPTH IN INCHES, SECOND NUMBER IS FLANGE WIDTH IN INCHES, AND THIRD NUMBER IS MATERIAL THICKNESS (GAUGE).
- 2) SEE DETAILS J/9 AND /O FOR OPENING FRAMING INFORMATION.
- 3) SIZE OF HEADER GIRT MEMBER TO BE SAME AS SIDEMALL OR ENDWALL GIRT, AS APPROPRIATE, PER ELEVATIONS. AT WINDOWS, INSTALL HEADER GIRT SPECIFIED ABOVE AND BELOW WINDOWS, U.N.O.
- 4) AT OPENINGS NOTED, INSTEAD OF ATTACHING DOOR JAMBS TO HEADER GIRT PER DETAIL LI/IO ATTACH DOOR JAMBS TO UNDERSIDE OF ENDWALL RAFTER PER DETAIL L2/10.
- 5) ALL OPENINGS AND ACCESSORIES SHALL BE CAPABLE OF SUPPORTING ALL WIND PRESSURES PERPENDICULAR TO THE SURFACE (GENERATED BY WINDS AT THE SPEED AND EXPOSURE INDICATED ABOVE) BY SPANNING BETWEEN THE JAMBS.

PROJECT DESIGN CRITERIA

ROOF DEAD LOAD: 3 psf

ROOF COLLATERAL LOAD: O psf

GROUND SNOW LOAD: O psf Ct = 1.2

ROOF SNOW LOAD: O psf ROOF LIVE LOAD: 20 psf

WIND SPEED: 110 mph

WIND EXPOSURE: C

CHN4X

3XI8

SINGLE

Ss: 1.984 Sds: 1.587 SI: 0.774 Sdl: 0.877 SEISMIC DESIGN CATEGORY: E ('short' period) E ('l-sec' period) R transverse: 3.0 R Ionaitudinal: 3.0

RISK CATEGORY: I

SOIL BEARING PRESSURE: 1500 psf

WIND DESIGN OF LATERAL FORCE-RESISTING SYSTEMS IS BASED ON THE DIRECTIONAL DESIGN PROCEDURE OF ASCE 7-16, CHAPTER

SEISMIC DESIGN OF LATERAL FORCE-RESISTING SYSTEMS ARE AS FOLLOWS:

-- TRANSVERSE: ORDINARY STEEL MOMENT FRAME (SEISMIC DESIGN IS BASED ON ASCE 07-16, SECTIONS 12.1 - 12.13) -- LONGITUDINAL: ORDINARY STEEL BRACED FRAME. (SEISMIC DESIGN IS PERFORMED USING THE SIMPLIFIED DESIGN PROCEDURE (ASCE 07-16, SECTION 12.14).

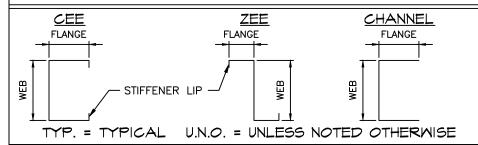
DESIGN BASE SHEAR: IS SHOWN ON CALCULATION SHEET M2.

IMPORTANT: IN ADDITION TO THESE ENGINEERING PLANS (WHICH ALWAYS TAKE PRECEDENCE), YOU SHOULD HAVE THE FOLLOWING FROM ACT BUILDING SYSTEMS:

- CONSTRUCTION PACKAGE
- INSTALLATION MANUALS
- CONSTRUCTION VIDEOS

PLEASE CONTACT YOUR SALES REP IF YOU HAVE NOT RECEIVED THESE PRIOR TO STARTING CONSTRUCTION.

COMPONENT DIAGRAM



FOUNDATION DETAIL KEYS

ENDWALL COLUMN (SEE DETAIL C/8 FOR TOP (A)CONNECTION AND G/9 FOR BASE CONNECTION) Engineering by:

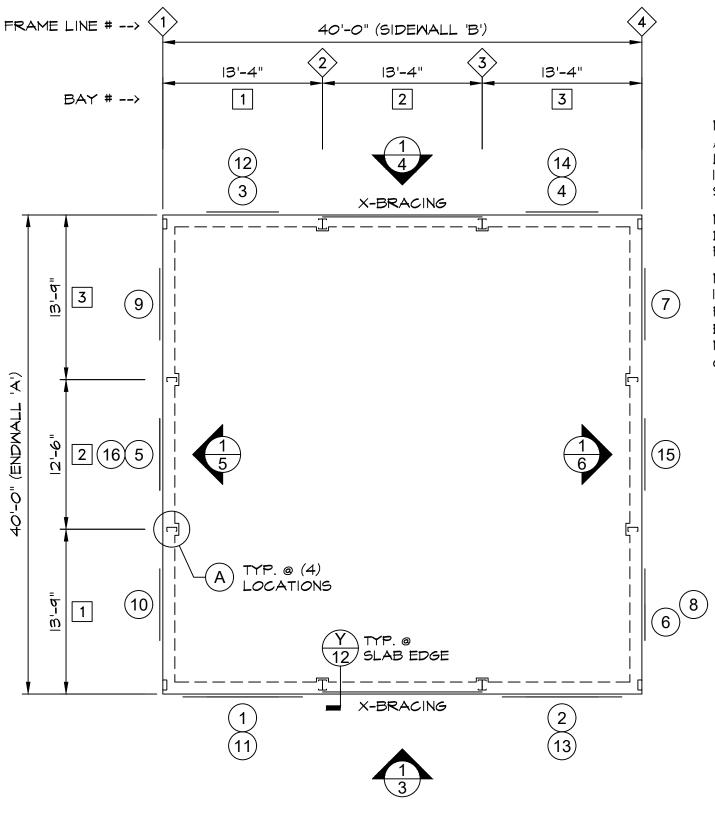
, SC 29073
uildingsve** Building E Lexington, gsupport@actbui Vetal

COTTRACK

STEEL BUT DING

Buildings, Steel I Ohana JOB NAME:

9/9/2022



NOTE: USE %" X 5" DEWALT 'SCREM-BOLT+' ANCHOR IN 51/2" DEEP HOLES AT ANCHOR LOCATIONS PER BASE DETAIL F/9, INSTALLED PER ICC REPORT ESR-3889, SECTION 4.3.

NOTE: SEE "TYP. FRAME CROSS-SECTION" DETAIL ON SHEET II FOR SPECIFIC FRAME DETAIL INFORMATION.

NOTE: EXCEPT AT DOOR OPENINGS, INSTALL 18G FORMED BASE TO FOUNDATION (FOR ATTACHMENT OF BOTTOM OF WALL SIDING) WITH 1/4in X Iin MUSHROOM HEAD SPIKE ANCHORS AT 48" O.C. (6" MAX. FROM ANY END).

Structural Engineering by:

Metal Building Engineering,

Lexington, SC 29073

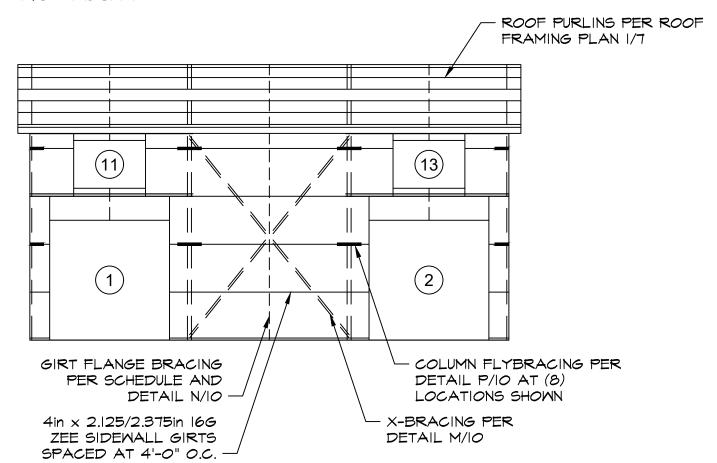
engsupport@actbuildingsystems.com DISTRIBUTOR: Ohana Steel Buildings, LLC JOB NAME: 9/9/2022

1 FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

	BAY #I	BAY #2	BAY #3
Sidewall 'A'	M/S	M/S	M/S

M/S = MIDSPAN



SIDEWALL 'A' EXTERIOR ELEVATION

SCALE: 1/8" = 1'-0"

Structural Engineering by:

Metal Building Engineering, L
Lexington, SC 29073
engsupport@actbuildingsystems.com

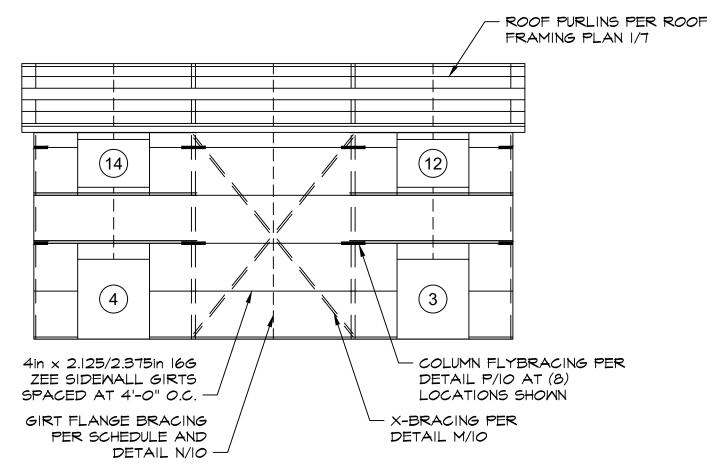
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9/9/2022

3

	BAY #I	BAY #2	BAY #3
Sidewall 'B'	M/S	M/S	M/S

M/S = MIDSPAN



SIDEMALL 'B' EXTERIOR ELEVATION

SCALE: 1/8" = 1'-0"

Structural Engineering by:

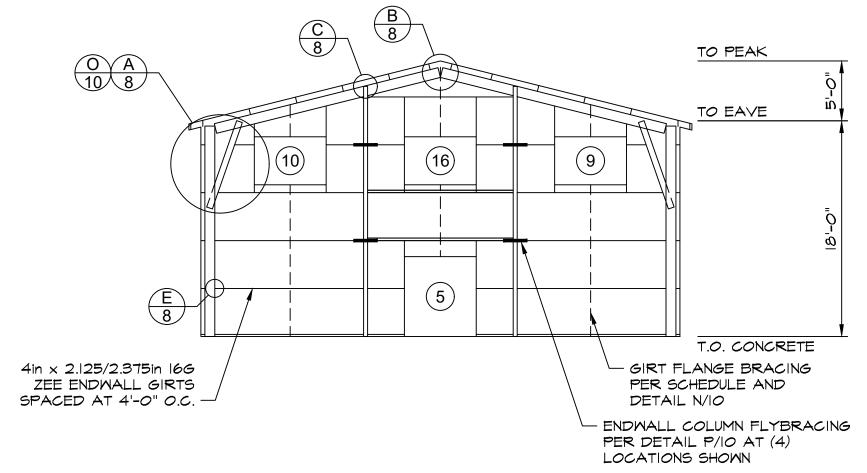
Metal Building Engineering, L
Lexington, SC 20073
engsupport@actbuildingsystems.com

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9/9/2022

	BAY #I	BAY #2	BAY #3
Endwall 'A'	M/S	M/S	M/S

M/S = MIDSPAN



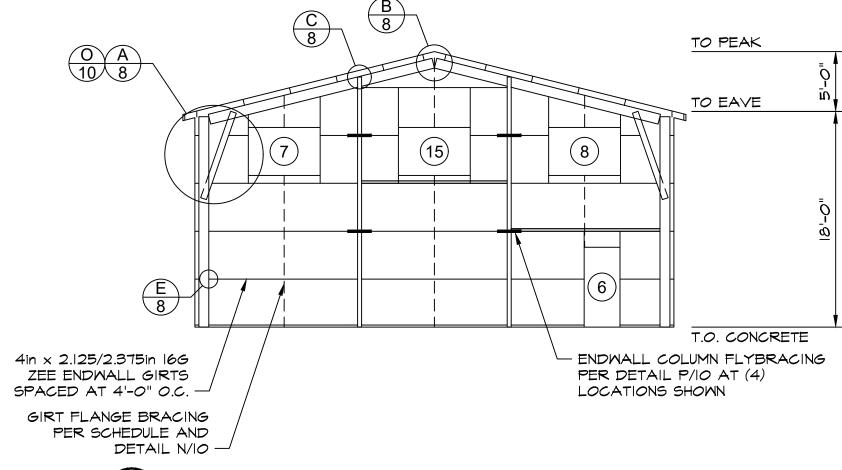


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	BAY #I	BAY #2	BAY #3
Endwall 'B'	M/S	M/9	Y/9

M/S = MIDSPAN



1 ENDWALL 'B' INTERIOR ELEVATION
6 SCALE: 1/8" = 1'-0" FRAME #4

Structural Engineering by:

Metal Building Engineering, LLC
Lexington, SC 29073
engsupport@actbuildingsystems.com

WWW.outhacksteelbuildings.com

OUTBACK
STEEL BUILDINGS
TRROUGHOUT THE

STEEL BUILDINGS
US.AND.CANDA

DISTRIBUTOR: Ohana Steel Buildings, LLC JOB NAME:

JOB NAME: JOB ADDRES

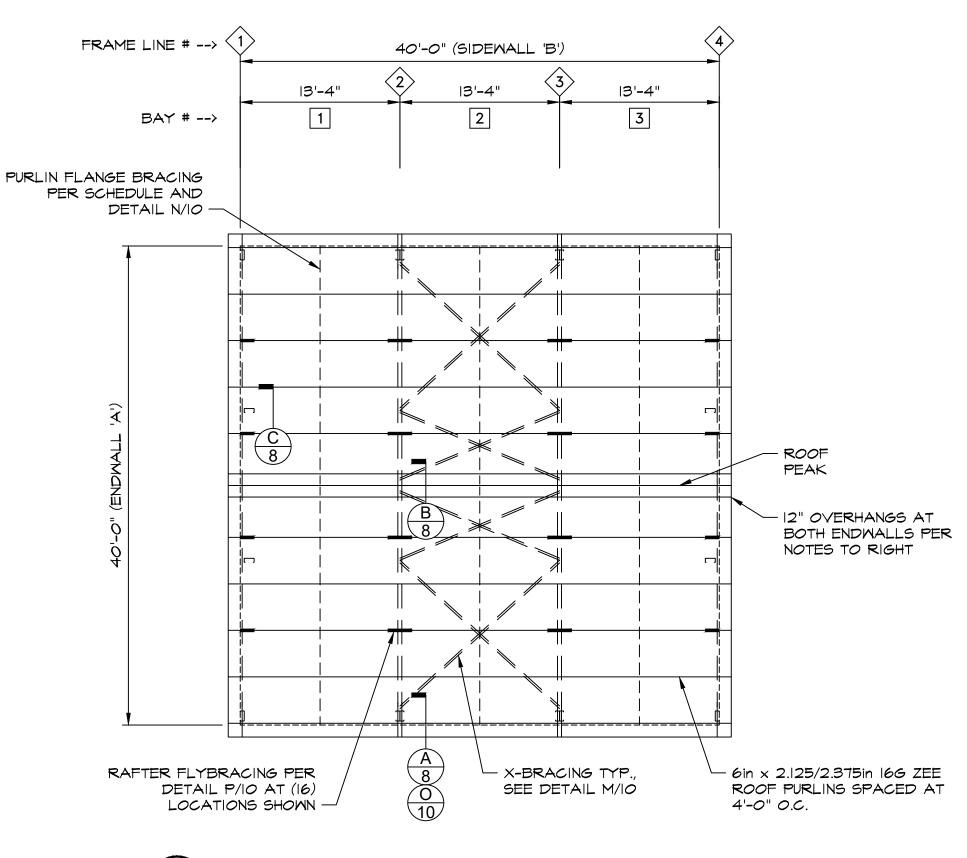
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9/9/2022

JOB NO.

6 0F 13



PURLIN FLANGE BRACING SCHEDULE

	BAY #I	BAY #2	BAY #3
Main Roof	M/S	M/S	M/S

M/S = MIDSPAN

TYPICAL ENDWALL OVERHANG INFORMATION: 1) CONTINUE ROOF PURLINS 12" MAX. BEYOND OUTSIDE FACE OF ENDWALL GIRTS AND ENCLOSE PURLIN ENDS WITH 6in x 3in x 166 CHANNEL (INSTALL #14 SCREW AT EACH CHANNEL FLANGE TO PURLIN).

- 2) INFILL BETWEEN CHANNEL AT PURLIN ENDS AND 'OUTRIGGER' AT ENDWALL FRAME (SEE DETAIL O/IO) WITH TYP. STEEL ROOF PURLINS.
- 3) INFILL WITH PURLIN MATERIAL BETWEEN PURLINS ABOVE ENDWALL RAFTER TO SEAL OFF BUILDING OR ENCLOSE BOTTOM OF OVERHANG WITH MATERIAL OF CUSTOMER'S CHOICE.

LLC

Structural Engineering by:

Metal Building Engineering, L
Lexington, SC 29073
engsupport@actbuildingsystems.com

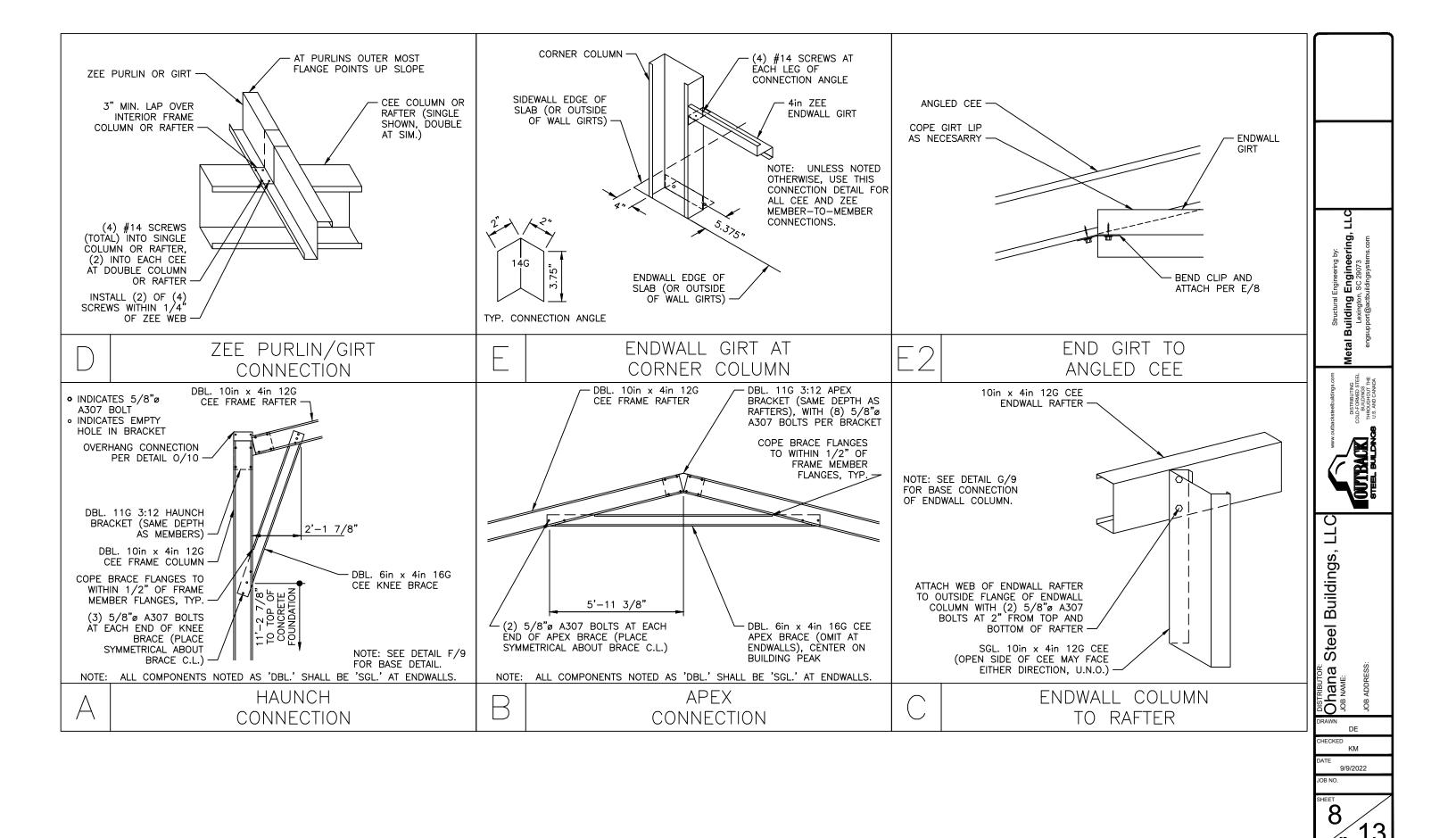


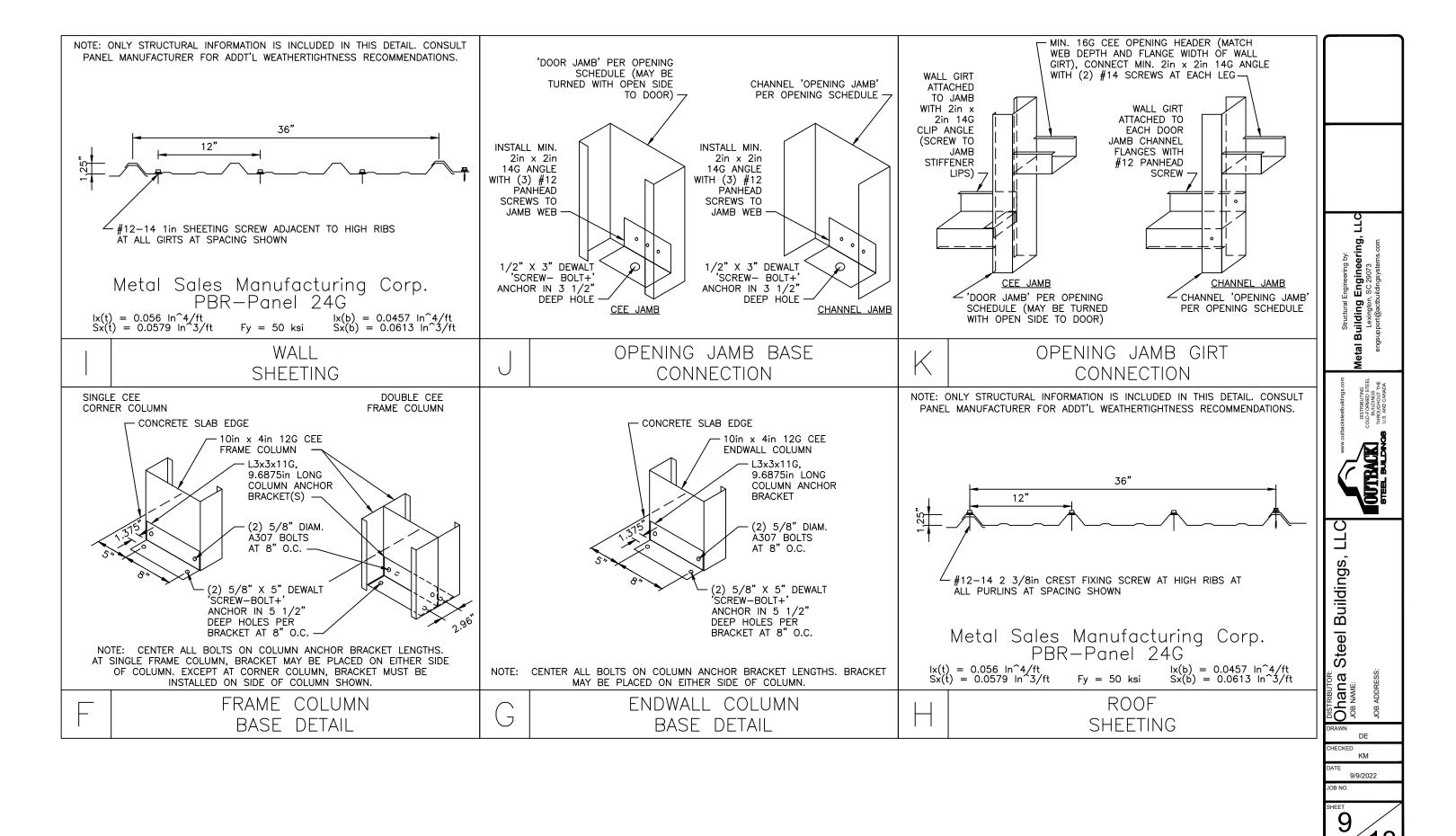
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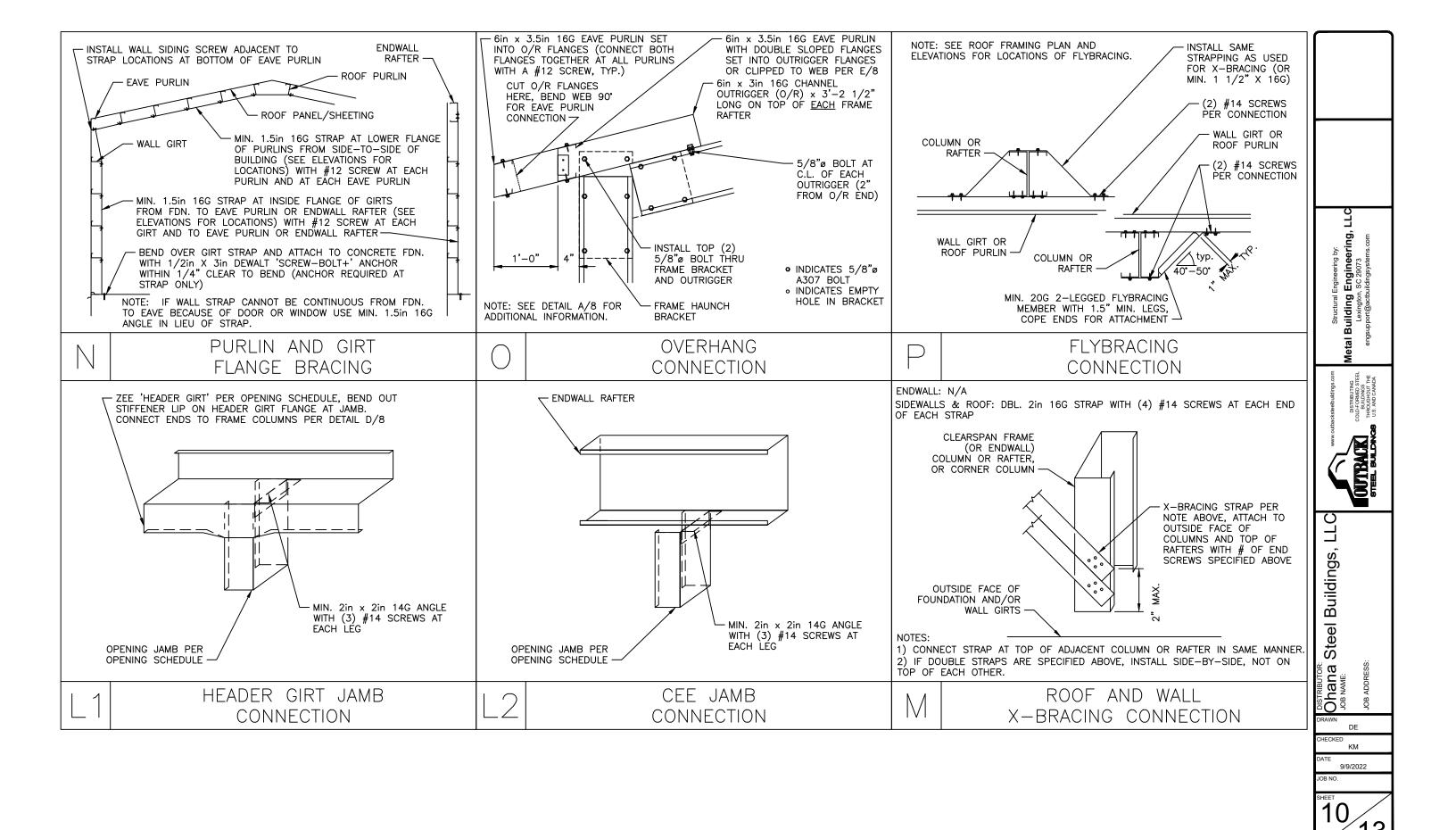
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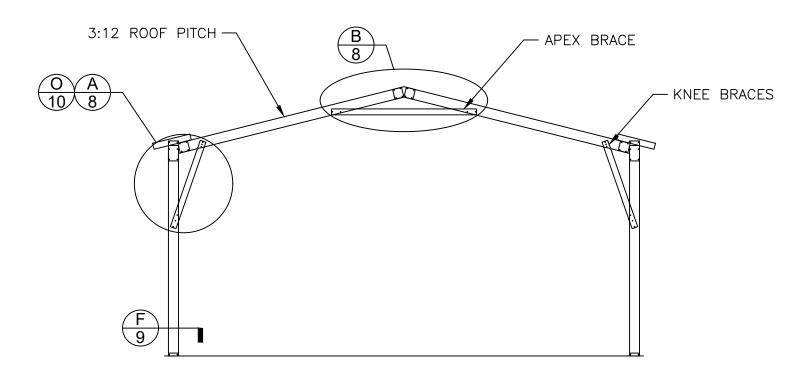
ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

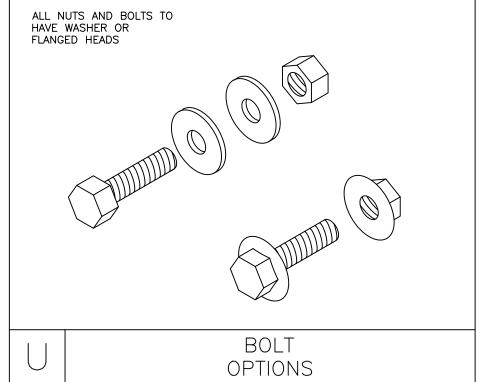


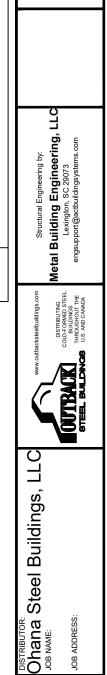








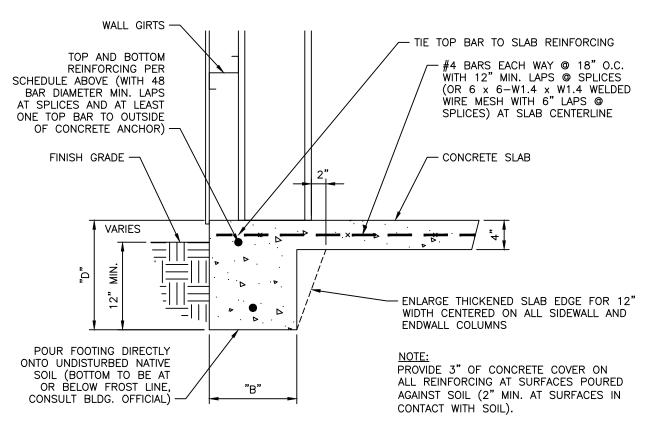




9/9/2022

LOCATIONS	"B"	"D"	TOP REINF.	BOTTOM REINF.
SIDEWALLS	12"	14"	(1)-#4	(1)-#4
ENDWALLS	12"	12"	(1)-#4	(1)-#4

* NOTE: INSTALL TOP REINFORCING BAR AT A DISTANCE FROM EDGE OF SLAB EQUAL TO DEPTH OF WALL GIRTS.



NOTES

- 1) SLAB REINFORCING SHOWN ABOVE IS SPECIFIED AS A METHOD OF CONTROLLING SHRINKAGE CRACKING AT THE SLAB SURFACE. CUSTOMER MAY CHOOSE ANOTHER METHOD OR REMOVE SLAB REINFORCING ALL TOGETHER BUT CUSTOMER WILL ASSUME ALL RESPONSIBILITY FOR THE PERFORMANCE OF THE SLAB. IF ANY OTHER METHOD IS CHOSEN BESIDES WHAT IS SHOWN ON THIS DETAIL, CUSTOMER SHALL INSTALL, AT EACH FRAME, (1) #4 CONT. BAR SIDEWALL—TO—SIDEWALL WITH A 6" 90" HOOK AT EACH END AROUND FRAME COLUMN CONCRETE ANCHOR GROUP.
- 2) SLAB DESIGN AND REINFORCING SPECIFIED IS BASED SOLELY ON THE 'SOIL DESIGN PRESSURE' INDICATED ON DRAWING SHEET 1 ASSUMING A MAX. POINT (WHEEL) LOAD OF 3000#. SLAB DESIGN FOR ANY OTHER SLAB THICKNESS OR POINT LOADS SHALL BE PROVIDED BY OTHERS.
- 3) SLAB AND FOUNDATION DESIGN SHOWN IS FOR STRUCTURAL PURPOSES ONLY. SLAB DESIGN SHOWN DOES NOT ADDRESS SLAB CONCRETE STRENGTH FOR WEAR RESISTANCE, VAPOR OR MOISTURE BARRIERS, SLAB SUBBASE MATERIAL SPECIFICATIONS OR INSTALLATION METHODS, OR THERMAL (INSULATION) REQUIREMENTS. A QUALIFIED PROFESSIONAL SHOULD BE RETAINED TO PROVIDE GUIDANCE FOR THOSE CONCERNS NOT ADDRESSED ON THIS DETAIL.



EDGE OF SLAB DETAIL

SCALE: NO SCALE



STRUCTURAL GENERAL NOTES

1. GOVERNING CODE: 2018 INTERNATIONAL BUILDING CODE

2. DRAWING OWNERSHIP

THESE DRAWINGS ARE JOINTLY OWNED BY OUTBACK STEEL BUILDINGS (OSB) AND METAL BUILDING ENGINEERING, LLC. DRAWINGS ARE PROVIDED FOR THE SOLE PURPOSE OF OBTAINING BUILDING PERMITS. ENGINEERING SEAL IS VALID FOR THE CONSTRUCTION OF A SINGLE BUILDING AT THE JOB ADDRESS SHOWN IN DRAWING TITLEBLOCK. ANY OTHER USE OF THESE DRAWINGS WITHOUT WRITTEN AUTHORIZATION FROM OSB AND METAL BUILDING ENGINEERING, LLC IS PROHIBITED.

THESE DRAWINGS ARE NOT VALID UNLESS 1) THE SEAL (STAMP) ON A PAPER COPY IS WET SIGNED IN INK BY THE ENGINEER, OR 2) THE PAPER COPIES ARE OF A DRAWING DISTALLY SIGNED BY THE ENGINEER, OR 3) THE ELECTRONIC FILE OF THE DRAWING IS DISTALLY SIGNED BY THE ENGINEER. IF A COPY OF THESE DRAWINGS IS DISTRIBUTED WITHOUT EITHER A PROPER WET SIGNATURE OR A DIGITAL SIGNATURE, THE DRAWING IS CONSIDERED INVALID. IF A COPY OF THESE DRAWINGS IS DISTRIBUTED WITHOUT EITHER A PROPER WET SIGNATURE OR A DIGITAL SIGNATURE, THE DRAWING IS CONSIDERED INVALID. THE ENGINEER ACCEPTS NO LIABILITY OR RESPONSIBILITY FOR DRAWINGS CONSIDERED INVALID AS NOTED ABOVE.

4. CONTRACTOR RESPONSIBILITIES:

CONTRACTOR SHALL VERIFY AND CONFIRM ALL EXISTING CONDITIONS AND DIMENSIONS. METAL BUILDING ENGINEERING, LLC (ENGINEER) SHALL BE NOTIFIED OF ANY DISCREPANCIES BETWEEN DRAWINGS AND EXISTING CONDITIONS PRIOR TO START OF WORK.

DISCREPANCIES BETWEEN DRAWINGS AND EXISTING CONDITIONS PRIOR TO START OF WORK.

CONTRACTOR MUST SUBMIT IN WRITING ANY REQUEST FOR MODIFICATION TO THE PLANS AND/OR SPECIFICATIONS AND NO STRUCTURAL CHANGES FROM THE APPROVED PLANS SHALL BE MADE IN THE FIELD UNLESS, PRIOR TO MAKING CHANGES, WRITTEN APPROVAL IS OBTAINED FROM THE ENGINEER. SHOP DRAWINGS SUBMITTED TO THE ENGINEER FOR REVIEW DO NOT CONSTITUTE "IN WRITING" UNLESS IT IS NOTED THAT SPECIFIC CHANGES ARE BEING REQUESTED. IF CHANGES ARE MADE WITHOUT WRITTEN APPROVAL, SUCH CHANGES SHALL BE THE LEGAL AND FINANCIAL RESPONSIBILITY OF THE CONTRACTOR OR SUB—CONTRACTORS INVOLVED AND IT SHALL BE THEIR FULL RESPONSIBILITY TO REPLACE OR REPART HE CONDITION AS DIRECTED BY THE ENGINEER.

CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING, SHORING, GUYING, OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING ERECTION. THESE TEMPORARY PROVISIONS SHALL REMAIN IN PLACE UNTIL SUFFICIENT PERMANENT MEMBERS ARE ERECTED TO INSURE THE SAFETY OF PARTIALLY ERECTED STRUCTURES. CONTRACTOR IS RESPONSIBLE FOR MEETING ALL LAWS REGULATING THE ERECTION OF STEEL BUILDINGS.

THESE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. BUILDING IS NOT CONSIDERED COMPLETE UNTIL THE INSTALLATION OF ALL COMPONENTS AND DETAILS SHOWN HEREIN ARE INSTALLED ACCORDING TO THE DRAWINGS.

THE SUPPLYING OF STAMPED ENGINEERING CALCULATIONS AND DRAWINGS FOR THIS METAL BUILDING DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT METAL BUILDING ENGINEERING, LLC IS ACTING AS THE ENGINEER OR ARCHITECT OF RECORD OR THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE FOR THE WHOLE OF THE PROJECT.

THIS BUILDING HAS BEEN REVIEWED BY METAL BUILDING ENGINEERING, LLC FOR CONFORMITY ONLY TO THE STRUCTURAL DESIGN PORTIONS OF THE GOVERNING CODE. THE BUILDING OWNER IS RESPONSIBLE TO SEEK PROFESSIONAL ADVICE IN ADDRESSING ANY OTHER CODE REQUIREMENTS (INCLUDING, BUT NOT LIMITED TO, FIRE AND LIFE SAFETY, ENVIRONMENTAL, ACCESSIBILITY, OR ELECTRICAL) THAT MAY APPLY TO THIS PROJECT.

DRAWINGS SCALES INDICATED ON DRAWINGS ARE APPROXIMATE AND INTENDED TO BE USED FOR REFERENCE ONLY. DO NOT SCALE DRAWINGS FOR CONSTRUCTION

THESE DOCUMENTS ARE STAMPED ONLY AS TO THE COMPONENTS FURNISHED BY OSB. IT IS THE RESPONSIBILITY OF THE PURCHASER TO COORDINATE DRAWINGS PROVIDED BY METAL BUILDING ENGINEERING, LLC WITH OTHER PLANS AND/OR OTHER COMPONENTS THAT ARE PART OF THE OVERALL PROJECT. IN CASES OF DISCREPANCIES, DRAWINGS PROVIDED BY METAL BUILDING ENGINEERING, LLCSHALL GOVERN. THE UNDERSIGNED ENGINEER WILL NOT SUPERVISE THE FABRICATION OR ERECTION OF THIS STRUCTURE. ANY OBSERVATION VISITS TO THE PROJECT SITE BY THE UNDERSIGNED ENGINEER ARE NOT TO BE CONSTRUED AS BEING INSPECTIONS FOR THE CONSTRUCTION OF ANY

6. INSPECTIONS:
NO SPECIAL INSPECTIONS ARE REQUIRED BY THE GOVERNING CODE ON THIS JOB.
ALL SPECIAL INSPECTIONS AND ANY OTHER ADDITIONAL INSPECTIONS REQUESTED BY BUILDING DEPARTMENT SHALL BE AT OWNER'S EXPENSE.

ALLOWABLE SOIL BEARING VALUE INDICATED ON DRAWING SHEET 1 OCCURS AT 12" BELOW FINISH GRADE, OR EXISTING NATURAL GRADE, OR AT FROST DEPTH SPECIFIED BY BUILDING DEPARTMENT, WHICHEVER IS THE LOWEST ELEVATION. FOUNDATION DESIGN SHOWN ASSUMES BOTTOM OF FOOTING BEARS ON NATIVE SOILS. FOUNDATION DESIGN SHOWN DOES NOT ACCOUNT FOR EXPANSIVE SOIL CONDITIONS OR FOR CONCRETE THAT WILL BE EXPOSED TO SULFATE CONTAINING SOLUTIONS OR CHLORIDES. OWNER SHALL CONTACT ENGINEER PRIOR TO CONSTRUCTION IF ANY OF THESE CONDITIONS EXIST.

8. CONCRETE REQUIREMENTS:

ALL CONCRETE SHALL HAVE A MIN. 28-DAY STRENGTH OF 2500 psi. HIGHER STRENGTH CONCRETE MAY BE USED, AT OWNER'S DISCRETION, FOR FINISH AND DURABILITY PURPOSES. CEMENT SHALL COMPLY WITH ASTM C150, TYPE 2, AND SHALL CONTAIN NO FLYASH.

ALL CONCRETE PLACEMENT SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE". WHICH IS HEREBY MADE A PART OF THESE DOCUMENTS.

CONCRETE REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60 FOR #4 BARS AND LARGER, GRADE 40 FOR #3 BARS. WELDED WIRE MESH SHALL CONFORM TO ASTM

LAP SPLICES TO BE 48 BAR DIAMETERS MIN., U.N.O.

CONCRETE GRADE BEAMS, THICKENED SLAB EDGES, PIERS, AND SPREAD FOOTINGS SHALL BE POURED ONTO UNDISTURBED, NATIVE SOIL WHICH IS FREE FROM ANY MATERIAL THAT WILL ADVERSELY AFFECT THE MIN. ALLOWABLE SOIL BEARING PRESSURE SPECIFIED ON SHEET 1.

CONCRETE ANCHOR INSTALLATION SHALL BE DONE IN ACCORDANCE WITH ICC REPORT ESR—3889, SECTION 4.3.

ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 (Fy MIN. OF 36000 psi), U.N.O. ALL BOLTS SHALL CONFORM TO ASTM A307, U.N.O. BOLT HOLE DIAMETERS SHALL BE 1/16" LARGER THAN NOMINAL BOLT DIAMETER. ALL INSTALLATION SHALL BE IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE" NO WELDING IS REQUIRED ON THIS JOB.

10 LIGHT GAUGE STRUCTURAL STEEL REQUIREMENTS:

ALL LIGHT CAUGE STEEL FRAMING MATERIAL AND ERECTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE (AISI)

"NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD—FORMED STEEL STRUCTURAL MEMBERS".

ALL LIGHT GAUGE STEEL MATERIAL SHALL CONFORM TO ASTM A653 HAVING A MINIMUM YIELD STRENGTH OF 55000 psi. THE GRADE AND ASTM SPECIFICATION NUMBER SHALL BE INDICATED BY PAINTING, DECAL, TAGGING, OR OTHER SUITABLE MEANS, ON EACH LIFT OR BUNDLE OF FABRICATED ELEMENTS.

UNLESS NOTED OTHERWISE, CEE, ZEE, AND CHANNEL MEMBERS' WEB AND FLANGE DIMENSIONS (IN INCHES) SHALL BE AS NOTED IN DETAILS IN THE FOLLOWING FORMAT:

UNLESS NOTED OTHERWISE, CEE, ZEE, AND CHANNEL MEMBERS' WEB AND FLANGE DIMENSIONS (IN INCHES) SHALL BE AS NOTED IN DETAILS IN THE FOLLOWING FORMAT:

[WEB DEPTH]In x [FLANGE WIDTH]In [GAUGE]G. FOR ZEES WITH UNEQUAL FLANGES, THE WIDTHS FOR BOTH FLANGES WILD BE LISTED, SEPARATED BY A " /". MIN. FLANGE

STIFFENER LIPS SHALL BE 0.885" FOR 12G CEES, 0.800" FOR 14G CEES, 0.773" FOR 16G CEES, 0.900" FOR 12G ZEES, 0.900" FOR 14G ZEES, AND 0.900" FOR 16G ZEES.

ALL BEND RADIUSES SHALL BE .1875". FOR ANGLES, THE FIRST TWO NUMBERS ARE THE LEG DIMENSIONS.

DECIMAL THICKNESS OF THE DELIVERED LIGHT GAUGE STEEL MATERIAL, ACCORDING TO NOMINAL GAUGES, SHALL MEET OR EXCEED 95% THE FOLLOWING DESIGN VALUES

GAUGE NO. DECIMAL THICKNESS, IN.

GAUGE NO. DECIMAL THICKNESS, IN.

10 0.135 14 0.070 18 0.046

12 0.105 16 0.070 18 0.036

EXCEPTION ON DEVALUE OF COLUMN AND PARTER MEMBERS SHALL NOT DE DRIVET ON NOTIFIED WITHOUT DRIVE ADDROVAL OF THE ENCINEER DOOR LAMP.

EXCEPT AS SHOWN ON DRAWINGS, CEE COLUMN AND RAFTER MEMBERS SHALL NOT BE DRILLED OR NOTCHED WITHOUT PRIOR APPROVAL OF THE ENGINEER. DOOR JAMB, ROOF PURLIN, AND WALL GIRT ENDS MAY HAVE FLANGES COPED 3" MAX. IF CONNECTION IS MADE TO PERPENDICULAR MEMBER PER DETAIL E/8. ROUND HOLES MAY BE DRILLED THROUGH ANY GIRT OR PURLIN MEMBER WITHIN THE MIDDLE THIRD OF THE DEPTH OF THAT MEMBER AND NOT WITHIN 24" OF MEMBER END (FIELD-DRILLED BOLT

HOLES INDICATED AT ENDS OF KNEE OR APEX BRACE WEBS AND SHOP-PUNCHED HOLES IN BRACE FLANGES EXCEPTED).

ALL BOLTS USED TO CONNECT LIGHT GAUGE MATERIAL SHALL CONFORM TO ASTM A307. BOLTS TO BE SNUG TIGHT PER THE RCSC AND AISC SPECIFICATIONS, UNLESS SPECIFICALLY NOTED OTHERWISE. BOLTS SHALL BE SPACED NO LESS THAN 3 BOLT DIAMETERS BETWEEN CENTERS. DISTANCE FROM BOLT CENTER TO THE END OR EDGE OF ANY LIGHT GAUGE MEMBER SHALL BE AMIN. OF 1.5 BOLT DIAMETERS. ALL SCREWS USED TO CONNECT LIGHT GAUGE MATERIAL SHALL BE SELF-PORTILLING SCREWS AND SHALL HAVE A MIN. TENSILE BREAKING STRENGTH OF 100,000 psi. SCREWS SHALL BE SPACED NO LESS THAN 1" O.C. AND EDGE OR END DISTANCE SHALL NOT BE LESS THAN 1". UNLESS NOTED OTHERWISE, ALL REFERENCES TO 'SCREWS' CONNECTING MATERIAL THICKER THAN 20 gg. SHALL BE MIN. #14 SCREWS AND SHALL HAVE MIN. 14 THREADS PER INCH.

SCREW ROOT DIAMETERS SHALL NOT BE LESS THAN: #14 SCREW: .200" #12 SCREW: .177" #10 SCREW: .153"

11. STEEL ROOF AND WALL PANELS (CLADDING):
LIGHT GAUGE STEEL ROOF AND WALL PANELS SHALL CONFORM TO ASTM A653 AND THE STEEL DECK INSTITUTE SPECIFICATIONS AND HAVE A MIN. YIELD STRENGTH OF

LIGHT GAUGE STEEL NOOT AND WINEL STEEL STE GAUGE NO. DECIMAL THICKNESS, IN. 0.0134 0.0120 SEE DETAILS H/9 AND I/9 FOR ROOF AND WALL PANEL FASTENER TYPES AND SPACINGS.

ĭ Engineering, SC 29073 Building Ē ᆸ Buildings Steel hana 30° КM 9/9/2022

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