TUNNEL SUPPORTS CIRCULAR TUNNEL (IN CONTINUATION OF TUNNEL IN WEAK ROCK)

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BASIC CONCEPT

This presentation is based on paper by Hoek : Tunnels in weak rocks

Based on basic concepts:

- * The process of designing of support system.
- * How rock mass surrounding tunnel deforms
- * How support system acts to control this deformation.

Dimensionless Plots of Tunnel Deformation



This Figure is a plot between

 Percentage strain i.e. ratio of tunnel wall displacement to tunnel radius and

• the ratio of rock mass strength to in situ stress.

Dimensionless Plots of Tunnel Deformation This plot shows that: **Rock mass Strength Vs In situ Stress**



If Rock mass strength (σ_{cm}) reduced below < 20% of in situ stresses (p_o) .

Deformation increases substantially, if it is not controlled by any support system, the opening will collapse.

Tunnel closure Vs Tunnel Diameter



Based on observations and measurements it is being suggested:

If Tunnel Closure > 1% of Tunnel Diameter

There is onset of instabilities in the tunnels

Therefore from the **above two conditions** the conclusion is the following

- ➢ If Rock mass strength (σ_{cm}) reduced below < 20% of in situ stresses (p₀).</p>
- Tunnel Closure > 1% of Tunnel Diameter

Deformation increases substantially, if it is not controlled by any support system, the opening will collapse

Tunnel Displacement for Different Support Pressures



- rp = Plastic zone radius
- u_i = Tunnel sidewall deformation
- r_o = Original tunnel radius in metres
- pi = Internal support pressure
- $p_o =$ In situ stress = depth below surface × unit weight of rock mass
- σ_{cm} = Rock mass strength = $2c' \cos \phi' / (1 \sin \phi')$

Internal Support Pressure pi

pi which called here internal pressure means support pressure like rock bolts, shotcrete, steel ribs etc.

pi = 0 means unsupported tunnel

Here we talk internal pressures with respect to in situ

Rockmass Strength σ_{cm} Stress i.e. pi/p0

Strength of the rock mass surrounding the tunnel plays an important roll in determining the support pressures. This strength we usually determined from

Roclab software. The parameters required for determining its www.geotechnic value are USC, GSI, Ei, depth from surface, D blasting factor. 7

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 σ_{cm} = Rock mass strength = $2c' \cos \phi' / (1 - \sin \phi')$

Y – **axis** : Percentage strain ε (tunnel closure/tunnel diameter) As per slide number 5, the convergence shall be less than 1%.

X-axis: Rockmass strength/in situ stress

Statistical Curve : for different support pressures/in situ stress

The series of curves shown in figure are defined by equation below:

 p_o

$$\varepsilon\% = \frac{ui}{ro} \times 100 = \left(0.2 - 0.25 \frac{p_i}{p_o}\right) \frac{\sigma_{cm}}{p_o}$$

Practical Example

A drainage tunnel of 4m is to excavate in rock mass behind the slope of open pit mine. So following are inputs of this example:

Inputs

Tunnel Span = 4m Tunnel depth = 150m Unit weight of rock = 0.027MN/m3

Granodiorite

UCS of intact rock = 100Mpa (from lab) GSI = 55

Fault Zone

UCS of rock in fault zone = 10MPa (from lab) GSI = 15



Fault Zone Rock mass

All above parameters will be used to determine the rock mass properties

Rock mass properties of Granodiorite and Fault zone RocLab (Rocscience software)

Practical Example

Rock mass properties of Granodiorite and Fault zone

RocLab (Rocscience software)

Material	σ_{ci} - MPa	GSI	mi	σ_{cm}	σ_{cm}/po
Granodiotite	100	55	30	33	8.25
Fault	10	15	8	0.6	0.15

Granodiotite

Po = $150 \ge 0.027 = 4.05$ MPa σ cm/po = $33/4.05 = 8.15 \cong 8.25$ As per graph

- The size of the plastic zone and the induced deformations will be negligible
- No support system is reauired.
- Spot bolting and shotcrete provided.



Practical Example

Rock mass properties of Granodiorite and Fault zone

RocLab (Rocscience software)

Material	σ_{ci} - MPa	GSI	mi	σ_{cm}	σ_{cm}/po
Granodiotite	100	55	30	33	8.25
Fault	10	15	8	0.6	0.15

Fault

 σ cm/po = 0.6/4.05 = 0.15 For unsupported tunnel with 2m radius Pi = 0 (internal support pressure inside tunnel)

$$\frac{rp}{ro} = \left(1.25 - 0.625 \frac{p_i}{p_o}\right) \frac{\sigma_{cm}}{p_o} \left(\frac{p_i}{p_o} - 0.57\right)$$

rp = 7.5m (Radius of plastic zone)In Graph for $p0/pi = 0 \& \sigma cm/po = 0.15$ Stain in tunnel = 9%, therefore Deformation = 0.18m



Practical Example Determination of Support Measures Support System for Tunnel in Fault Zone Deformation in tunnel = 180mm (Deformation is very High needs Support system)

Strain Tunnel = 9%

Therefore substantial support is required in order convergence to an acceptable level.

Assume the acceptable limit is 2%

and $\sigma_{cm}/p_0 = 0.15$ From graph the determined internal support pressure is approximately $p_i/p_0 = 0.25$ $p_0 = 4$ MPa (provided in slide 10) \therefore **pi = 0.25x 4 = 1MPa** (required internal press to support tunnel at 2% convergence.



This is the figure from where we will select supports to generate support pressures inside tunnel greater that 1MP (determined in last slide.

Support Capacity > 1MPa

Supports that can be provided in the tunnel are:

- 1. Steel Sets
- 2. Lattice Girder
- 3. Shotcrete
- 4. Concrete lining
- 5. Rock bolts or cables
- 6. Combination of above supports





Maximum support



Figure 8: Approximate maximum capacities for different support systems installed in circular tunnels. Note that



Figure 8: Approximate maximum capacities for different support systems installed in circular tunnels. Note that steel sets and rockbolts are all spaced at 1 m.

From Figure 8, select: **X** – **Axis** – 4m tunnel diameter **Y** – **Axis** – 1Mpa Support Pressure

In this **figure number** are given, which are corresponding to support measures provided in figure in last slide.

Therefore, now numbers can be select for support pressure greater than > 1MPa can be selected. And support can be selected to support tunnel.

There are many constraints of this method which are not covered in this presentation. To know more, read Tunnels in Weak Rock paper by Hoek.

The required **internal support pressure is 1MPa**. Therefore the provided shall be more than 1Mpa. In graph in **slide 14, there are curve numbers 1 to 26** which represents type of support like steel ribs, lattice girder, shotcrete, concrete lining, rock bolts which are presented in figure provide **slide no 13**.

From figure 8 in slide 13, you can decide the type of support you need to provide to support the tunnel.

Support system depends upon the rock class tunnel is going through like in Good Rock – Spot Bolting & Shotcrete Fair Rock – Pattern Bolting & Shotcrete Poor Rock – Pattern Bolting & Shotcrete Very Poor to Extremely Poor – Steel Ribs and Shotcrete

Here Tunnel is passing through fault zone so the it is very poor to extremely poor rock. Therefore the support system should be steel ribs .

From **Slide 14**, we will **select no. 3** curve number as it is providing internal support pressure more than required support pressure of 1Mpa .

Curve No. 3

Maximum Support pimax = $7.0 \text{ D}^{-1.4}/\text{s} = 7.0 \text{ x } 4^{-1.4}/1$

= 4.9 MPa is the provided **pressure by Steel Ribs** spaced at 1m distance.

Ground Reaction Curve Vs Support Reaction

Ground Reaction Curve is shown as curve and Support Reaction Curve is shown

as linear.

- **Red line** shows failure of support if support being applied as the deformation of tunnel just initiated. In this case as tunnel deforms all the pressure come on support and it fails.
- Blue line shows the right time of installing supports, allowing a certain amount of convergence in tunnel (like 2% in example we adopted).



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