





#### City of Elyria WTP

Proposed Improvements for Increasing Plant Capacity Present<u>ed to OTCO</u>

December 4, 2018



# Agenda

- 1. Plant History
- 2. Plant Overview
- 3. Present Capacity
- 4. Why Increase Capacity?
- 5. Plant Capacity Analysis
- 6. High Rate Demonstration Study
- 7. Plant Capacity Improvements
- 8. Ancillary Improvements

City of Elyria WTP - Proposed Improvements for Increasing Plant Capacity

#### Plant History

# Plant History

- City of Elyria purchased property on the shore of Lake Erie in late 1800's (area now inside the city limits of Lorain)
- Original Water Works Plant on West 15<sup>th</sup> Street sold to Elyria Iron and Steel Company in 1904
- New water plant began pumping treated water from Lake Erie to Elyria in 20-inch cast iron main (still in use today)
- Believed to be the first inland City in the USA to pump treated water from the Great Lakes





# Plant History

- In 1922, original plant was at capacity and a new plant replaced the old, expanding to total capacity of 8 MGD
- Over the years, various improvements and expansions have increased to current capacity of 22 MGD
- Plant supplies water to the following:
  - City of Elyria
  - City of North Ridgeville
  - City of Amherst
  - Erie Huron County Rural Water Authority
  - Several adjacent townships



City of Elyria WTP - Proposed Improvements for Increasing Plant Capacity

#### Plant Overview



## Process Flow Diagram - Liquids



#### Process Flow Diagram - Solids



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#### Present Plant Capacity

# Present Plant Capacity – 22 MGD

- Based on Ohio EPA assessment of the entire process
- Limited by the lowest rated component in the process
  - Water Supply Source
  - Treatment Plant
  - "Essential" Chemicals
    - Coagulant Aluminum Chlorohydrate
    - Chlorine Gas
    - Phosphoric Acid
    - Sodium Fluorosilicate
- Not impacted by "Non-essential" chemicals
  - Powder Activated Carbon
  - Potassium Permanganate
  - Caustic
  - Cationic Polymer

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#### Plant Capacity Analysis

# Plant Capacity Analysis

- Plant specific data used to determine de-rating of process components
- Analyzed plant data from Jan 2016 to Dec 2017 to determine:
  - Average Day Production: 12.2 MGD
  - Maximum Day Production: 18.0 MGD
  - Peak-hour Treatment: 22.8 MGD
  - Peak-hour Production: 19.8 MGD
- Allows determining the following factors:
  - Max Day / Average Day = 1.5
  - Peak Hour (prod) / Max Day =
     1.5 (based on individual day data, not maximums for the year)
  - Peak Hour (treatment) / Max Day = 1.3



# Plant Capacity Analysis – Water Supply Source

Component	No. of Units	Existing System	Component Capacity (MGD)	Flow Basis of Component Capacity/Ratio	Equivalent Max- Day Capacity (MGD)	
Source-water Supply	1	Lake Erie	~	Avg-day 1.5	~	
Intake Structure	1	Four 66" flare openings One 24" opening Total Area = 108 sf	35	Max-day 1.0	35.0	
Intake Pipe	2	One 42" pipe One 24" pipe	57.6	Max-day 1.0	57.6	
Travelling Screens	2	(2) - 30 MGD	30	1.0	30	
Source-water Pumps	-water nps -Water 10 MG 7 LS Pump #3 - 10 MG LS Pump #4 - 7.0 M LS Pump #6 - 11.5 M LS Pump #8 - 12 M LS Pump #9 - 11.5 M		54	Max-day 1.0	54.0	

# Plant Capacity Analysis – Treatment Plant

Component	No. of Units	Existing System	Component Capacity (MGD)	Flow Basis of Component Capacity/Ratio	Equivalent Max- Day Capacity (MGD)
Rapid Mixers	2	10.33' x 10.33' x 17' SWD each; Total Volume = 27,140 gal G value = 300 fps/ft	73.6	Max-day 1.0	73.6
Flocculation Tanks	4	66' x 16' x 16' SWD Total Volume = 505,550 gal (2) – 30" BFV/tank	24.3	Max-day 1.0	24.3
Settling Tanks	4	(2) - 198' x 37' x 16 SWD (2) - 254' x 41.5' x 16 SWD	15.1 10.5	Max-day 1.0	25.7
		Total Volume = 4.55 MG Basins 1-2 (2) - 516' of weir Basins 3-4 (2) - 640' of weir	10.3 12.8	Max-day 1.0	23.1

# Plant Capacity Analysis – Treatment Plant

Component	No. of Units	Existing System	Component Capacity (MGD)	Flow Basis of Component Capacity/Ratio	Equivalent Max- Day Capacity (MGD)
Filters	10	<ul> <li>(4) - 726 sq ft (1921)</li> <li>(2) - 710 sq ft (1946)</li> <li>(4) - 721 sq ft (1969)</li> <li>Total Area = 7,208 sq ft</li> </ul>	37.3	Max-day 1.0	37.3
Clearwells	3	CW#1: 128' x 62.5 x 14.5 Min SWD (V=837,800 gal) CW#2: 127.25' x 60.9' x 14.5 Min SWD (V=816,350 gal) CW#3: 127.5'x72 x 14.5 Min SWD (V=961,380 gal) Total Area = 25,756 sq ft Approved EVF for all 3 CWs = 0.2	25.6	Peak Hour Treatment 1.3	19.3
Finished Water Pumps	4	HS Pump #1 - 9.9 MGD HS Pump #2 - 12.4 MGD HS Pump #3 - 16.2 MGD HS Pump #4 - 13.8 MGD	36.1	Peak Hour Production 1.5	24.1

# Present Plant Capacity – Essential Chemicals

Component		Existing System	Component Capacity (MGD)	Flow Basis of Component Capacity/Ratio	Equivalent Max- Day Capacity (MGD)
Coagulant Alum.	Feed	Three @ 60 gph	50.7	Max-day 1.0	50.7
Chlorohydrate	Storage	One 10,000 gal tank	22.2	Avg-day 1.5	33.3
Chlorine Gas	Feed	Three @ 1,000 lbs/day	55.0	Max-day 1.0	55.0
	Storage	Sixteen (16) - 1 ton cylinders	45.2	Avg-day 1.5	67.8
Phosphoric Acid	Feed	One @ 1.0 gph One @ 1.6 gph	20.2	Max-day 1.0	20.2
	Storage	112 - 50 lb pails	37.3	Avg-day	56.0
Sodium Fluorosilicate	Feed	Feed One @ 60 cfh or 4,500 lbs/hr		Max-day 1.0	>>>
	Storage	112 - 50 lb bags	27.3	Avg-day	41.0

## Present Plant Capacity – 22 MGD

- Key components limiting capacity:
  - Clearwell capacity is being limited by Effective Volume Factor (EVF) of 0.2, reducing capacity to 19.3 MGD for Equivalent Max-Day Capacity
  - 2. Finished Water Pump capacity is limited by the firm capacity (24.1 MGD)
  - 3. Settling Basin capacity is limited by total weir length (23.1 MGD)
  - 4. Phosphoric Acid Feed Pumps can deliver chemical for 20.2 MGD

Why Increase the Capacity?

# The City of Elyria has a goal to increase plant capacity to 30 MGD!



Why Increase the Capacity?

Increase in area demand to meet water quality and system management regulations



City of Elyria WTP - Proposed Improvements for Increasing Plant Capacity

- OPEA "Guideline for Clarifier and Granular Media Filter Ratings at Surface Water Treatment Plants" stipulates approval criteria for high-rate demonstration studies
- Two main criteria:
  - 1. Turbidity from High-rate filter shall:
    - Be less than 0.3 NTU for >95% of the time for 6-week period in which test is conducted
    - Be less than 1 NTU at all times
    - A surface WTP that can meet above will receive credit for 2.5 log removal of Giardia lamblia and 2.0 log removal of Cryptosporidium
  - 2. Total Organic Carbon (TOC) Removal Ratio
    - Bi-weekly source-water and filtered-water TOC to be averaged to determine if TOC removal ratio is >1, otherwise additional techniques

- One of the following three criteria must also be met:
  - Average production efficiency for six-week period should be at least 90 percent (i.e. no more than 10% finished water should be used to backwash the filter)

$$PE = \frac{GUFRV - UBWV}{GUFRV} \times 100$$

#### where PE = Production Efficiency, GUFRV = Gross Unit Filter Run Volume UBWV = Unit Backwash Volume

UBWV is calculated by multiplying optimum backwash rate by optimum backwash time. Optimum backwash rate is determined based on filter media characteristics and water temperature. Time is determined by:

- a) Backwashing filter for at least 10 mins at optimum rate
- b) Collecting a sample every 15 seconds of spend backwash water from filter trough
- c) Measuring turbidity of these samples



- Filter should be run to terminal head loss of 0.2 to 0.3 NTU . The turbidity values should be plotted on the Y-axis against time (X-axis).
- The optimum backwash time is defined as the time when the turbidity decreases to less than 10 to 15 NTU



Time (min.)

- One of the following three criteria must also be met:
  - 2. Average value of clarified water turbidity for all runs during six week period should not exceed 2 NTU, and the 95th percentile value of clarified water turbidity data points should not exceed 5 NTU.
  - 3. Gross water production should be at least 5000 gal/sq. ft of filter area for each filter run. Backwashing should be initiated (or extrapolated) based on the lesser of:
    - a) Reaching total head loss available for filtration at fullscale
    - b) Turbidity breakthrough of 0.2 to 0.3 NTU through the filters

Criteria 3 chosen for Elyria's Demonstration Study

- Demonstration procedure:
  - Goal is to demonstrate successful high-rating by diverting 6 MGD of flow to Settling Basin 1. Any remaining flow will be conveyed equally to Basins 3 and 4.
  - Basin 3 and 4 effluent will be conveyed to Filters 6-10 through a 36-inch main with flow measured using a strap-on flowmeter.
  - Filter 1 will be used as the high-rate filter (4 gpm per sq. ft or 4 MGD), as measured with a rate-of-flow controller
  - Filter 3 will be the "control filter" with remaining 2 MGD flow from Basin 1 diverted to it.







- Demonstration capacity goals:
  - Rapid Mix Tanks: total component capacity of 78.2 MGD at >30 sec detention time
  - Flocculation Tanks: total component capacity of 60 MGD at 30 min detention time
  - Settling Basins: demonstrate capacity of 30 MGD
  - Filters: total component capacity of 37.3 MGD at 4 gpm/ft<sup>2</sup> with largest filter out of service

#### High Rate Demonstration Study Preliminary Results

- Testing underway at the Elyria WTP has revealed the following preliminary results:
  - Filter Effluent is less than 0.3 NTU 95% of the time, and always less than 1.
  - Sedimentation Basin effluent average hasn't exceeded 2 NTU and the 95<sup>th</sup> percentile hasn't exceeded 5 NTU.
  - Filters 1 and 2
     capable of achieving
     4 MGD each





City of Elyria WTP - Proposed Improvements for Increasing Plant Capacity Plant Capacity Improvements Clearwell Baffles

• Clearwell capacity is limited by Effective Volume Factor (EVF) of 0.2, reducing capacity to 19.3 MGD for Equivalent Max-Day Capacity

	36° CWE (DID1)					38° CWE (DI01)			38° CWE (DI01)			
			***************************************									
	6					XX						
	5:							1211.000 1011.000 1012.000 1010.000				
129'6"±								· 전망· 전망· 전망· 전망· 전망· 전망· 전망· 전망· 전망· 전망		-		
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11-0° (TYP) •		74'-0"±	t (, ), e' e (M. * - f, ee (V)) e	1'-8"	1	65'-6"±				65'-6"±		
30° CWH (DI01)	Clearwell 3			24" CWI (DI01)	C	learwe	11 2		Cle	arwell	1	24" CWI (DI01)

Adding baffles (in red) increases clear well capacity by increasing EVF factor





- Including baffles increases clear well capacity by increasing EVF factors:
  - New CW1 EVF  $\rightarrow$  0.43
  - New CW2 EVF  $\rightarrow$  0.44
  - New CW3 EVF  $\rightarrow$  0.4
- Clearwell component capacity increases to 54.0 MGD with Equivalent Max-Day Capacity of 41.5 MGD (peaking factor of 1.3)



Figure B-1

Clearwell 1				Clearwell 2				Clearwell 3				
Segment	L(ft)	W(ft)	Product	Segment	L	W	Product	Segment	L	W	Product	
А	95.42	20.83	1988	A	116.25	20.83	2421	A	115.5	24	2772	
В	20.83	20.83	434	В	20.83	20.83	434	В	24	24	576	
С	85	20.83	1771	С	105.83	20.83	2204	С	103.5	24	2484	
D	20.83	20.83	434	D	20.83	20.83	434	D	24	24	576	
E	105.833	20.83	2205	E	116.25	20.83	2421	E	115.5	24	2772	
F	41.66	20.83	868									
Total	Length	369.573		Total I	Length	379.99		Total L	ength	382.5		
Weighte	ed Width	20.83		Weighte	ed Width	20.83		Weighte	d Width	24		
L:W	Ratio	17.7		L:W	Ratio	18.2		L:W F	Ratio	15.9		
E	VF	0.43		E	/F	0.44		EV	/F	0.4		

City of Elyria WTP - Proposed Improvements for Increasing Plant Capacity Plant Capacity Improvements Finished Water Pumps

# Plant Capacity Improvements

Finished Water Pumps

- Limited by firm capacity of existing pumps: HS Pump 1: 9.9 MGD HS Pump 2: 16.2 MGD HS Pump 3: 12.4 MGD HS Pump 4: 13.8 MGD
- Increase size of smallest pump to increase overall firm capacity (without largest pump)
- Relocate Pump 4 to Pump 1 and replace Pump 1



City of Elyria WTP - Proposed Improvements for Increasing Plant Capacity Plant Capacity Improvements Chlorine Scrubber

#### Plant Capacity Improvements Dry Chlorine Scrubber

- Chlorine Scrubber
  - Used to control gaseous chlorine emissions from Chemical Building
  - Dry media removes Cl<sub>2</sub>/SO<sub>2</sub> by means of adsorption, absorption and chemical reaction
  - Cl<sub>2</sub>/SO<sub>2</sub> are trapped within the pellets where an irreversible chemical reaction changes the gases into harmless solids









#### Plant Capacity Improvements Dry Chlorine Scrubber



City of Elyria WTP - Proposed Improvements for Increasing Plant Capacity Plant Capacity Improvements Sludge Collection System

- Sedimentation Basins
  - Chain & Flight system used to remove settled solids in the Sedimentation Basins
  - Sludge conveyed to Basin sump and pumped for downstream processing
  - All 4 basins have two longitudinal collectors each, connected to the same motor







- Sludge Collection
  - Chain & Flight system comprises sprockets, bearings, guide rails, chain, flights, drive chain, drive
  - Material selection is critical
    - FRP, Stainless Steel typical, UHMW PE guide rails (yellow)







- Sludge Collection
  - Different flight designs recommended for different applications and budgets
  - (ranked in order of quality)
    - 1. Diamond Flight
    - 2. Sigma Plus —
    - 3. Channel Flight







- Sludge Mixing System
  - Head house has two Sludge Storage Tanks with horizontal submersible mixers. Sludge pumps draw from the bottom of the tank and pump to the Sludge Storage Tank which feeds the centrifuge
  - Sludge Storage Tank mixing system to be included.

Goal is to provide consistent homogeneous mixture to the centrifuge

- Alternative technologies:
  - Submersible Mixers
  - Vertical Shaft Mixers
  - Pumped Hydraulic Mixing Systems



- Alternative technologies:
  - Submersible Mixers
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- Alternative technologies:
  - Submersible Mixers
  - Vertical Shaft Mixers
  - Pumped Hydraulic Mixing Systems
    - e.g. Vaughan Rotamix, Hayward Gordon Hydromix Evoqua JetMix







#### Plant Capacity Improvements Sludge Pump Replacement

- Sludge Holding Tank Sludge Pumps and Sedimentation Basin Sludge Pumps are in need of replacement
- Existing pumps are Moyno progressing cavity pumps with 10 or 15 HP motors
  - 15 HP for Holding Tank Pumps, 10 HP for Sed Basin pumps
- Pumps to be replaced "in-kind" (same pump)

#### Sedimentation Basin Sludge Pump Motors





#### Sludge Holding Tank Sludge Pumps

