

Introduction

Deck West steel floor and roof decks have been updated to meet the needs of the construction industry for the next millennium. The profiles manufactured by Deck West have been used with great success throughout the United States for over four decades. The manufacturing plant is centrally located in Stockton, California.

The deck west roof and floor panels are cold-rolled from steel sheets conforming to ASTM A653 SS Grade 40 (modified 46,000 psi yield). Deck West products are available with mill finish, flashcoat galvanized, G-15, G-30, G-40, G-60, G-90, G-120, G-210 AND G-235 galvanized finishes. Panels are available in four thicknesses as shown in the tables.

Deck West deck products are listed by the International Code Council Evaluation Service (ICC-ES) Report #ESR-2050, registered with the Underwriters Laboratories certification body, and Factory Mutual approved.

This catalog contains information about Deck West roof and floor deck profiles;

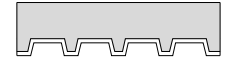
Section properties
Vertical load tables
Allowable diaphragm shears
Standard metal deck details

Additional information may be obtained by contacting Deck West, Inc.

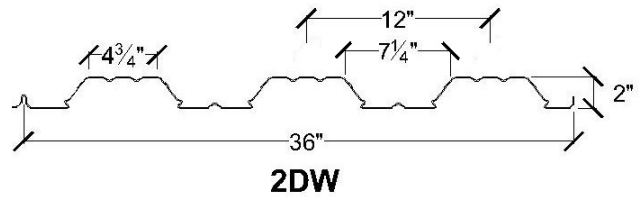
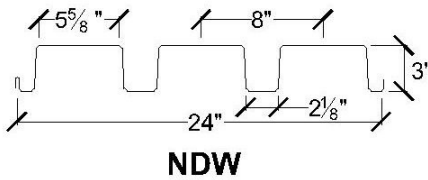
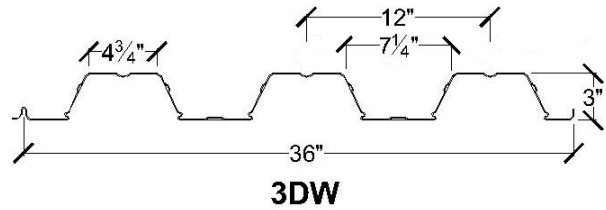
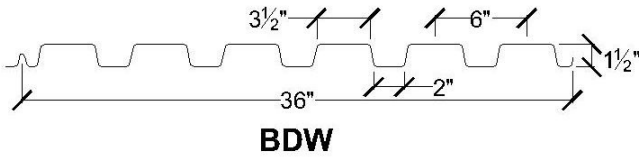
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PRODUCT PROFILES



SECTION PROPERTIES

DECK TYPE	GAUGE	WT (psf)	DESIGN METAL THICKNESS (INCHES)	MOMENT OF INERTIA (in4/ft)			SECTION MODULUS (in3/ft)	
				Ix	Ieff +	Ieff -	Seff +	Seff -
BDW	22	1.8	0.0299	0.1996	0.1461	0.1813	0.1808	0.1947
	20	2.1	0.0359	0.2399	0.1854	0.2263	0.2361	0.2464
	18	2.9	0.0478	0.3200	0.2723	0.3184	0.3382	0.3536
	16	3.6	0.0598	0.4012	0.3685	0.4012	0.4318	0.4437
BADW	22	1.8	0.0299	0.1989	0.1462	0.1757	0.1821	0.1851
	20	2.1	0.0359	0.2390	0.1855	0.2204	0.2377	0.2359
	18	2.9	0.0478	0.3189	0.2728	0.3125	0.3358	0.3421
	16	3.6	0.0598	0.3998	0.3687	0.3957	0.4281	0.4326
NDW	22	2.2	0.0299	0.9363	0.5978	0.8653	0.3593	0.4636
	20	2.6	0.0359	1.1253	0.7543	1.0654	0.4631	0.5761
	18	3.5	0.0478	1.5010	1.0910	1.4752	0.6961	0.8089
	16	4.2	0.0598	1.8812	1.4651	1.8785	0.9463	1.0345
2DW	22	1.7	0.0299	0.3526	0.3109	0.2789	0.2639	0.2650
	20	2.1	0.0359	0.4237	0.3854	0.3481	0.3316	0.3204
	18	2.6	0.0478	0.5654	0.5384	0.4884	0.4725	0.4308
	16	3.3	0.0598	0.7088	0.6975	0.6298	0.6207	0.5418
3DW	22	1.8	0.0299	0.8348	0.7042	0.7718	0.3940	0.5086
	20	2.2	0.0359	1.0028	0.8939	0.9550	0.5128	0.6167
	18	2.9	0.0478	1.3362	1.2890	1.3156	0.7688	0.8283
	16	3.6	0.0598	1.6728	1.6648	1.6936	1.0078	1.0508

PROPERTIES BASED ON MINIMUM 46,000 psi YIELD STRENGTH

BDW Vertical Load Table

DECK GAUGE			DECK SPAN (FT)											
			5' - 0"	5' - 6"	6' - 0"	6' - 6"	7' - 0"	7' - 6"	8' - 0"	8' - 6"	9' - 0"	9' - 6"	10' - 0"	10' - 6"
SIMPLE SPAN	22 (0.0299")	S	113	110	9	79	68	59	52	46	41	37	33	30
		Δ	112	84	65	51	41	33	28	23	20	16	14	12
	20 (0.0359")	S	173	144	121	103	89	78	68	60	54	48	43	40
		Δ	142	107	83	65	52	42	35	29	24	21	18	15
	18 (0.0478")	S	245	203	170	145	125	109	96	85	76	68	61	56
		Δ	205	154	119	94	75	61	50	42	35	30	26	22
	16 (0.0598")	S	316	261	220	187	161	141	124	109	97	88	79	72
		Δ	227	208	160	126	101	82	68	56	48	40	35	30
TWO SPAN	22 (0.0299")	S	141	117	98	84	72	63	55	49	44	39	35	32
		Δ	269	202	155	122	98	80	66	55	46	39	34	29
	20 (0.0359")	S	178	148	124	106	91	80	70	62	56	50	45	41
		Δ	343	257	198	156	125	102	84	70	59	50	43	37
	18 (0.0478")	S	251	209	176	150	130	113	99	88	79	71	64	58
		Δ	495	372	287	225	181	147	121	101	85	72	62	54
	16 (0.0598")	S	319	264	222	190	164	143	126	112	100	90	81	74
		Δ	667	501	386	304	243	198	163	136	114	97	83	72
THREE OR MORE SPANS	22 (0.0299")	S	175	145	122	104	90	79	69	61	55	49	44	40
		Δ	211	159	122	96	77	63	52	43	36	31	26	23
	20 (0.0359")	S	221	183	154	132	114	99	87	78	69	62	56	51
		Δ	269	202	156	123	98	80	66	55	46	39	34	29
	18 (0.0478")	S	312	259	218	187	161	141	124	110	98	88	80	72
		Δ	389	292	225	177	142	115	95	79	67	57	49	42
	16 (0.0598")	S	395	328	277	236	204	178	157	139	124	112	101	92
		Δ	524	394	303	239	191	155	128	107	90	77	66	57

S (STRESS), Δ DEFLECTION

HOW TO USE TABLE:

1. DETERMINE THE TOTAL LOAD AND THE LIVE LOAD FOR THE ROOF DECK
2. USING THE TOTAL LOAD, SELECT THE GAUGE AND THE SPAN WITH EQUAL OR GREATER VALUES FOR S (STRESS)
3. USING THE LIVE LOAD, SELECT THE GAUGE AND THE SPAN WITH EQUAL OR GREATER VALUE FOR Δ (DEFLECTION) OF L/180.
4. SELECT THE GAUGE AND THE SPAN FOR THE ROOF DECK THAT MEETS BOTH STEPS 2 & 3.

BDW Shear Table – Button Punch

ALLOWABLE DIAPHRAGM SHEARS (q), lbs/ft

DECK GAUGE	SEAM ATTACHMENT SPACING (INCHES) BUTTON PUNCH	NO. PUDDLE WELDS/UNIT	FACTOR	DECK SPAN (FEET)					
				5	6	7	8	9	10
22 (0.0299")	12	5	q	316	305	298	287	279	253
			G	14	16	19	21	24	26
	24		q	250	247	240	229	225	220
			G	12	15	17	21	24	25
	12	7	q	469	454	443	408	361	313
			G	82	91	96	98	102	105
24	q		488	458	446	397	351	281	
	G		75	84	91	94	99	101	
20 (0.0359")	12	5	q	445	431	416	394	356	316
			G	14	16	19	21	24	28
	24		q	384	356	330	320	284	251
			G	11	15	17	20	22	26
	12	7	q	746	710	628	541	462	425
			G	26	30	33	40	46	51
24	q		655	580	511	458	414	387	
	G		16	19	22	26	31	35	
18 (0.0478")	12	5	q	910	776	681	603	561	507
			G	31	37	42	47	52	55
	24		q	745	639	569	517	494	470
			G	28	33	38	42	46	50
	12	7	q	1093	1000	880	805	722	670
			G	112	122	129	137	141	145
24	q		1090	920	800	710	640	590	
	G		88	95	101	108	114	120	
16 (0.0598")	12	5	q	1012	982	870	780	710	660
			G	49	57	65	74	84	95
	24		q	1050	900	790	700	630	580
			G	42	51	59	68	77	88
	12	7	q	1518	1308	1205	1094	966	829
			G	140	151	164	177	190	203
24	q		1407	1207	1095	978	910	740	
	G		99	110	122	135	148	161	

NOTES

FOR SI 1 INCH=25.4mm, lbf = 4.45N, 1psi = 6.9 kpa, 1ft = 304.8mm

1. SPACING OF BOUNDARY OR PARALLEL SHEAR COLLECTOR ATTACHMENTS SHALL BE BASED ON MAXIMUM ALLOWABLE SHEAR FOR PUDDLE WELDS PER WELDING SCHEDULE.
2. TYPICAL INTERIOR END LAP SHALL BE TWO INCHES (NO LAP WELD IS REQUIRED, ARC SPOT WELD IS REQUIRED)
3. TO CONVERT SHEAR STIFFNESS (G) TO THE FLEXIBILITY FACTOR (F), $F = 1000/G$
4. THESE SHEAR VALUES MEET THE FACTOR OF SAFETY REQUIREMENTS LISTED IN 2007 AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, TABLE D5. THE FACTOR HAS BEEN UNIFORMLY APPLIED TO VALUES AS 3.00.
5. THE SPACING OF BOUNDARY SPOT WELDS AT PERIMETER SUPPORTS SHALL BE DETERMINED IN ACCORDANCE WITH THE VALUES SET FORTH IN THE WELDING SCHEDULE TABLE.

BDW Shear Table – Top Seam Weld

ALLOWABLE DIAPHRAGM SHEARS (q), lbs/ft

DECK GAUGE	SEAM ATTACHMENT SPACING (INCHES) TOP SEAM WELD	NO. PUDDLE WELDS/UNIT	FACTOR	DECK SPAN (FEET)						
				5	6	7	8	9	10	
22 (0.0299")	12	5	q	666	622	589	566	546	530	
			G	15	17	20	22	25	27	
	24		q	501	459	408	385	356	346	
			G	14	17	19	21	24	26	
	12		7	q	831	762	709	666	635	610
				G	78	85	91	96	100	104
24	q	639		571	504	471	432	415		
	G	72		78	81	85	87	90		
20 (0.0359")	12	5		q	794	743	705	676	660	634
				G	22	26	30	33	37	40
	24		q	598	551	489	465	427	415	
			G	22	25	29	32	35	38	
	12		7	q	993	908	848	799	760	731
				G	106	114	121	126	130	134
24	q	764		684	604	564	516	495		
	G	96		102	105	109	110	113		
18 (0.0478")	12	5		q	1180	110	1020	960	910	870
				G	42	49	55	61	66	71
	24		q	910	830	740	680	630	590	
			G	37	43	49	54	58	63	
	12		7	q	1400	1340	1220	1130	1050	1000
				G	162	170	176	181	185	188
24	q	1140		1070	940	850	780	720		
	G	141		147	147	151	151	153		
16 (0.0598")	12	5		q	1680	1520	1410	1330	1280	1230
				G	70	79	88	96	104	110
	24		q	1230	1170	1060	980	910	860	
			G	65	74	80	87	92	97	
	12		7	q	1750	1605	1382	1422	1360	1240
				G	220	228	233	237	240	243
24	q	1668		1424	1297	1187	1086	1022		
	G	187		192	190	193	190	193		

NOTES

FOR SI 1 INCH=25.4mm, lbf = 4.45N, 1psi = 6.9 kpa, 1ft = 304.8mm

1. SPACING OF BOUNDARY OR PARALLEL SHEAR COLLECTOR ATTACHMENTS SHALL BE BASED ON MAXIMUM ALLOWABLE SHEAR FOR PUDDLE WELDS PER WELDING SCHEDULE.
2. TYPICAL INTERIOR END LAP SHALL BE TWO INCHES (NO LAP WELD IS REQUIRED, ARC SPOT WELD IS REQUIRED)
3. TO CONVERT SHEAR STIFFNESS (G) TO THE FLEXIBILITY FACTOR (F), $F = 1000/G$
4. THESE SHEAR VALUES MEET THE FACTOR OF SAFETY REQUIREMENTS LISTED IN 2007 AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, TABLE D5. THE FACTOR HAS BEEN UNIFORMLY APPLIED TO VALUES AS 3.00.
5. THE SPACING OF BOUNDARY SPOT WELDS AT PERIMETER SUPPORTS SHALL BE DETERMINED IN ACCORDANCE WITH THE VALUES SET FORTH IN THE WELDING SCHEDULE TABLE

BDW Shear Table – Side Seam Weld

ALLOWABLE DIAPHRAGM SHEARS (q), lbs/ft

DECK	SEAM ATTACHMENT SPACING (INCHES) SIDE SEAM WELD	NO. PUDDLE WELDS/UNIT	FACTOR	DECK SPAN (FEET)					
				5	6	7	8	9	10
22 (0.0299")	12	5	q	583	558	540	525	514	505
			G	15	17	20	22	25	27
	24	5	q	423	402	369	360	338	333
			G	14	17	19	21	24	26
	12	7	q	729	684	651	626	606	589
			G	78	85	91	96	100	104
24	7	q	532	494	443	422	391	382	
		G	72	78	81	85	87	90	
20 (0.0359")	12	5	q	696	667	646	628	615	603
			G	22	26	30	33	37	40
	24	5	q	505	480	441	430	404	398
			G	22	25	29	32	35	38
	12	7	q	872	818	779	749	725	706
			G	106	114	121	126	130	134
24	7	q	635	590	530	468	468	457	
		G	96	102	105	109	110	113	
18 (0.0478")	12	5	q	1400	1290	1225	1196	1168	1145
			G	42	49	55	61	66	71
	24	5	q	1113	1044	985	823	767	756
			G	40	46	51	56	60	65
	12	7	q	1652	1552	1479	1423	1369	1315
			G	162	170	176	181	185	188
24	7	q	1201	1117	1011	967	967	967	
		G	141	147	147	151	151	153	
16 (0.0598")	12	5	q	1920	1760	1660	1580	1530	1490
			G	70	79	88	96	104	110
	24	5	q	1440	1290	1180	1100	1040	990
			G	65	74	80	87	92	97
	12	7	q	2057	1941	2005	1929	1722	1678
			G	220	228	233	237	240	243
24	7	q	1905	1680	1390	1444	1253	1187	
		G	187	192	190	193	190	193	

NOTES

FOR SI 1 INCH=25.4mm, lbf = 4.45N, 1psi = 6.9 kpa, 1ft = 304.8mm

1. SPACING OF BOUNDARY OR PARALLEL SHEAR COLLECTOR ATTACHMENTS SHALL BE BASED ON MAXIMUM ALLOWABLE SHEAR FOR PUDDLE WELDS PER WELDING SCHEDULE.
2. TYPICAL INTERIOR END LAP SHALL BE TWO INCHES (NO LAP WELD IS REQUIRED, ARC SPOT WELD IS REQUIRED)
3. TO CONVERT SHEAR STIFFNESS (G) TO THE FLEXIBILITY FACTOR (F), $F = 1000/G$
4. THESE SHEAR VALUES MEET THE FACTOR OF SAFETY REQUIREMENTS LISTED IN 2007 AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, TABLE D5. THE FACTOR HAS BEEN UNIFORMLY APPLIED TO VALUES AS 3.00.
5. THE SPACING OF BOUNDARY SPOT WELDS AT PERIMETER SUPPORTS SHALL BE DETERMINED IN ACCORDANCE WITH THE VALUES SET FORTH IN THE WELDING SCHEDULE TABLE

BDW Shear Table – SHEARCORR

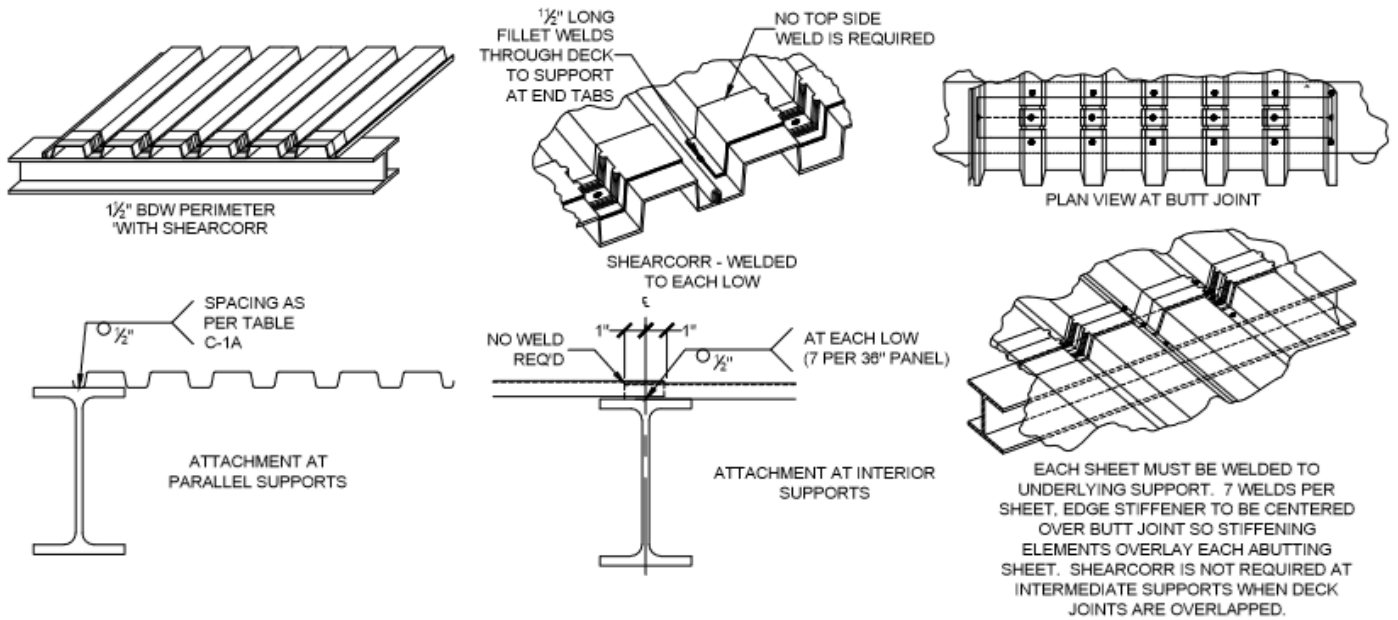
ALLOWABLE DIAPHRAGM SHEARS (q), lbs/ft

DECK GAUGE	SEAM ATTACHMENT SPACING (INCHES) TOP SEAM WELD	NO. PUDDLE WELDS/UNIT	FACTOR	DECK SPAN (FEET)					
				5	6	7	8	9	10
22 (0.0299")	12	7	q	1117	1014	943	890	855	819
			G	218	210	200	195	185	178
	18		q	975	890	825	753	700	653
			G	200	192	182	170	164	159
	24		q	872	793	720	668	633	610
			G	198	184	170	162	154	150
20 (0.0359")	12	7	q	1646	1523	1423	1342	1281	1144
			G	280	260	250	245	230	225
	18		q	1437	1273	1121	1170	1105	1052
			G	256	244	227	200	181	175
	24		q	1290	1137	1025	963	901	832
			G	232	212	192	182	167	153
18 (0.0478")	12	7	q	2206	2044	1935	1854	1644	1444
			G	385	370	355	345	330	319
	18		q	1895	1711	1716	1536	1471	1415
			G	360	340	320	311	290	283
	24		q	1748	1562	1416	1303	1229	1167
			G	352	332	310	290	275	265
16 (0.0598")	12	7	q	2895	2665	2504	2357	2220	1865
			G	510	480	458	430	426	411
	18		q	2502	2274	2073	1916	1793	1719
			G	479	455	436	406	392	365
	24		q	2296	1986	1842	1652	1557	1472
			G	450	437	405	385	365	346

NOTES

FOR SI 1 INCH=25.4mm, lbf = 4.45N, 1psi = 6.9 kpa, 1ft = 304.8mm

1. SHEARCORR IS USED AT END OF DECK UNITS OR AT INTERIOR LINES FOR SHEAR TRANSFER PERPENDICULAR TO THE METAL DECK. SHEARCORR CAN BE PRE-PUNCHED WITH ½" DIAMETER HOLES TO FACILITATE WELDING. (UNPUNCHED SHEARCORR CAN ACCOMMODATE PIN OR SCREW INSTALLATIONS)
2. SEE SHEARCORR EDGE STIFFENING SYSTEM DETAILS
3. SPACING OF BOUNDARY OR PARALLEL SHEAR COLLECTOR ATTACHMENTS SHALL BE BASED ON MAXIMUM ALLOWABLE SHEAR FOR PUDDLE WELDS PER WELDING SCHEDULE.
4. TYPICAL INTERIOR END LAP SHALL BE TWO INCHES (NO LAP WELD IS REQUIRED, ARC SPOT WELD IS REQUIRED)
5. TO CONVERT SHEAR STIFFNESS (G) TO THE FLEXIBILITY FACTOR (F), F = 1000/G
6. THESE SHEAR VALUES MEET THE FACTOR OF SAFETY REQUIREMENTS LISTED IN 2007 AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, TABLE D5. THE FACTOR HAS BEEN UNIFORMLY APPLIED TO VALUES AS 3.00.
7. THE SPACING OF BOUNDARY SPOT WELDS AT PERIMETER SUPPORTS SHALL BE DETERMINED IN ACCORDANCE WITH THE VALUES SET FORTH IN THE WELDING SCHEDULE TABLE



SHEARCORR Edge Stiffening System

BDW Shear Table – Side Lap Screw

ALLOWABLE DIAPHRAGM SHEARS (q), lbs/ft FLEXIBILITY FACTOR (f) IN INCHES PER POUND x 10⁻⁶

DECK GAUGE	SEAM ATTACHMENT SPACING (INCHES)	NO. PUDDLE WELDS/UNIT	Factor	DECK SPAN (FEET)						
				5	6	7	8	9	10	
22 (0.0299")	24	5	q	659	569	505	457	420	391	
			f	5.44+161.6R	6.21+134.7R	6.96+115.4R	7.7+101R	8.43+89.8R	9.14+80.8R	
	18		q	696	606	542	495	458	429	
			f	5.23+161.6R	5.92+134.7R	6.58+115.4R	7.22+101R	7.84+89.8R	8.44+80.8R	
	12		q	768	679	616	570	534	506	
			f	4.87+161.6R	5.43+134.7R	5.96+115.4R	6.46+101R	6.93+89.8R	7.37+80.8R	
	8		q	876	789	727	682	648	622	
		f	4.44+161.6R	4.87+134.7R	5.26+115.4R	5.62+101R	5.95+89.8R	6.26+80.8R		
	6	q	984	898	838	795	762	737		
		f	4.1+161.6R	4.44+134.7R	4.75+115.4R	5.02+101R	5.27+89.8R	5.5+80.8R		
	4	q	1,201	1,118	1,061	1,020	990	968		
		f	3.62+161.6R	3.84+134.7R	4.04+115.4R	4.22+101R	4.38+89.8R	4.52+80.8R		
		24	7	q	791	680	602	543	498	462
				f	5.26+82.5R	6.02+68.7R	6.77+58.9R	7.51+51.5R	8.24+45.8R	8.96+41.2R
18		q		830	720	642	583	539	503	
		f		5.08+82.5R	5.76+68.7R	6.43+58.9R	7.09+51.5R	7.72+45.8R	8.34+41.2R	
12		q		908	798	721	664	621	586	
		f		4.76+82.5R	5.33+68.7R	5.88+58.9R	6.39+51.5R	6.89+45.8R	7.36+41.2R	
8		q		1,024	916	841	786	744	711	
		f	4.38+82.5R	4.83+68.7R	5.24+58.9R	5.62+51.5R	5.98+45.8R	6.31+41.2R		
6		q	1,140	1,034	961	907	866	835		
		f	4.08+82.5R	4.43+68.7R	4.76+58.9R	5.06+51.5R	5.33+45.8R	5.58+41.2R		
4		q	1,373	1,271	1,200	1,149	1,112	1,084		
		f	3.62+82.5R	3.87+68.7R	4.09+58.9R	4.28+51.5R	4.46+45.8R	4.62+41.2R		
20 (0.0359")		24	5	q	1,043	893	786	706	645	596
				f	4.18+93.4R	4.8+77.8R	5.42+66.7R	6.05+58.4R	6.67+51.9R	7.28+46.7R
	18	q		1,086	937	831	751	691	642	
		f		4.06+93.4R	4.63+77.8R	5.2+66.7R	5.75+58.4R	6.3+51.9R	6.84+46.7R	
	12	q		1,172	1,024	919	841	782	735	
		f		3.85+93.4R	4.34+77.8R	4.81+66.7R	5.26+58.4R	5.7+51.9R	6.13+46.7R	
	8	q		1,302	1,155	1,052	976	919	874	
		f	3.58+93.4R	3.98+77.8R	4.35+66.7R	4.7+58.4R	5.03+51.9R	5.34+46.7R		
	6	q	1,431	1,286	1,185	1,111	1,056	1,012		
		f	3.37+93.4R	3.69+77.8R	3.99+66.7R	4.27+58.4R	4.52+51.9R	4.77+46.7R		
	4	q	1,689	1,548	1,451	1,381	1,329	1,290		
		f	3.03+93.4R	3.26+77.8R	3.47+66.7R	3.66+58.4R	3.83+51.9R	3.99+46.7R		
		24	7	q	1,246	1,063	934	837	762	703
				f	4.04+47.6R	4.64+39.7R	5.25+34R	5.87+29.8R	6.49+26.5R	7.11+23.8R
18		q		1,292	1,110	981	885	811	753	
		f		3.93+47.6R	4.5+39.7R	5.06+34R	5.61+29.8R	6.17+26.5R	6.72+23.8R	
12		q		1,384	1,204	1,076	982	909	852	
		f		3.75+47.6R	4.24+39.7R	4.71+34R	5.18+29.8R	5.63+26.5R	6.07+23.8R	
8		q		1,522	1,344	1,219	1,127	1,056	1,001	
		f	3.51+47.6R	3.92+39.7R	4.3+34R	4.66+29.8R	5.01+26.5R	5.34+23.8R		
6		q	1,660	1,484	1,362	1,272	1,203	1,150		
		f	3.32+47.6R	3.65+39.7R	3.97+34R	4.26+29.8R	4.54+26.5R	4.8+23.8R		
4		q	1,936	1,765	1,647	1,561	1,497	1,449		
		f	3.01+47.6R	3.26+39.7R	3.48+34R	3.69+29.8R	3.87+26.5R	4.05+23.8R		

SEAM ATTACHMENT, (2) HILTI S-SLC 01 M HWH SELF DRILLING SCREWS
 SEE BDW NOTES- Page 10

BDW Shear Table – Side Lap Screw Continued

ALLOWABLE DIAPHRAGM SHEARS (q), lbs/ft FLEXIBILITY FACTOR (f) IN INCHES PER POUND x 10⁻⁶

DECK GAUGE	SEAM ATTACHMENT SPACING (INCHES)	NO. PUDDLE WELDS/UNIT	Factor	DECK SPAN (FEET)					
				5	6	7	8	9	10
18 (0.0478")	24	5	q	1,469	1,246	1,088	970	878	805
			f	2.75+39.6R	3.18+33R	3.62+28.3R	4.06+24.7R	4.52+22R	4.99+19.8R
	q		1,509	1,287	1,129	1,011	921	848	
	f		2.7+39.6R	3.11+33R	3.52+28.3R	3.93+24.7R	4.35+22R	4.78+19.8R	
	q		1,588	1,367	1,211	1,095	1,006	935	
	f		2.61+39.6R	2.97+33R	3.34+28.3R	3.7+24.7R	4.06+22R	4.42+19.8R	
	q		1,706	1,488	1,334	1,220	1,133	1,065	
	f		2.49+39.6R	2.8+33R	3.11+28.3R	3.41+24.7R	3.7+22R	3.98+19.8R	
	q		1,825	1,609	1,457	1,346	1,261	1,194	
	f		2.39+39.6R	2.66+33R	2.92+28.3R	3.17+24.7R	3.41+22R	3.65+19.8R	
	q		2,062	1,851	1,704	1,597	1,516	1,454	
	f		2.21+39.6R	2.42+33R	2.62+28.3R	2.81+24.7R	2.99+22R	3.15+19.8R	
	q	2,028	1,717	1,497	1,332	1,204	1,103		
	f	2.66+20.2R	3.07+16.8R	3.49+14.4R	3.93+12.6R	4.38+11.2R	4.85+10.1R		
	q	2,077	1,768	1,548	1,384	1,258	1,157		
	f	2.61+20.2R	3.01+16.8R	3.41+14.4R	3.82+12.6R	4.24+11.2R	4.66+10.1R		
	q	2,176	1,869	1,651	1,489	1,364	1,266		
	f	2.54+20.2R	2.89+16.8R	3.25+14.4R	3.61+12.6R	3.97+11.2R	4.34+10.1R		
	q	2,324	2,020	1,805	1,646	1,524	1,429		
	f	2.43+20.2R	2.74+16.8R	3.05+14.4R	3.35+12.6R	3.65+11.2R	3.94+10.1R		
	q	2,472	2,171	1,959	1,803	1,684	1,591		
	f	2.34+20.2R	2.61+16.8R	2.88+14.4R	3.13+12.6R	3.38+11.2R	3.63+10.1R		
	q	2,768	2,473	2,268	2,118	2,005	1,917		
	f	2.18+20.2R	2.4+16.8R	2.6+14.4R	2.8+12.6R	2.98+11.2R	3.16+10.1R		
16 (0.0598")	24	5	q	1,609	1,359	1,181	1,049	946	864
			f	1.99+20.2R	2.3+16.8R	2.62+14.4R	2.96+12.6R	3.31+11.2R	3.67+10.1R
	q		1,641	1,392	1,215	1,083	981	900	
	f		1.96+20.2R	2.26+16.8R	2.57+14.4R	2.89+12.6R	3.22+11.2R	3.55+10.1R	
	q		1,706	1,458	1,283	1,152	1,051	971	
	f		1.91+20.2R	2.19+16.8R	2.47+14.4R	2.76+12.6R	3.05+11.2R	3.34+10.1R	
	q		1,802	1,557	1,384	1,255	1,157	1,079	
	f		1.85+20.2R	2.09+16.8R	2.34+14.4R	2.59+12.6R	2.83+11.2R	3.08+10.1R	
	q		1,899	1,656	1,485	1,359	1,262	1,186	
	f		1.79+20.2R	2.01+16.8R	2.23+14.4R	2.44+12.6R	2.65+11.2R	2.86+10.1R	
	q		2,092	1,854	1,687	1,565	1,473	1,401	
	f		1.69+20.2R	1.87+16.8R	2.04+14.4R	2.21+12.6R	2.37+11.2R	2.53+10.1R	
	q	2,224	1,876	1,629	1,445	1,301	1,188		
	f	1.92+10.3R	2.22+8.6R	2.54+7.4R	2.86+6.4R	3.21+5.7R	3.56+5.2R		
	q	2,264	1,918	1,672	1,488	1,346	1,233		
	f	1.9+10.3R	2.19+8.6R	2.49+7.4R	2.8+6.4R	3.12+5.7R	3.46+5.2R		
	q	2,346	2,002	1,758	1,576	1,436	1,325		
	f	1.86+10.3R	2.12+8.6R	2.4+7.4R	2.68+6.4R	2.97+5.7R	3.27+5.2R		
	q	2,469	2,127	1,886	1,707	1,570	1,462		
	f	1.8+10.3R	2.04+8.6R	2.28+7.4R	2.53+6.4R	2.78+5.7R	3.03+5.2R		
	q	2,591	2,253	2,015	1,839	1,704	1,598		
	f	1.75+10.3R	1.97+8.6R	2.18+7.4R	2.4+6.4R	2.61+5.7R	2.83+5.2R		
	q	2,836	2,504	2,272	2,101	1,973	1,872		
	f	1.66+10.3R	1.84+8.6R	2.02+7.4R	2.19+6.4R	2.35+5.7R	2.52+5.2R		

NOTES

FOR SI 1 INCH=25.4mm, lbf = 4.45N, 1psi = 6.9 kpa, 1ft = 304.8mm

1. SPACING OF BOUNDARY OR PARALLEL SHEAR COLLECTOR ATTACHMENTS SHALL BE BASED ON MAXIMUM ALLOWABLE SHEAR FOR PUDDLE WELDS PER WELDING SCHEDULE.
2. TYPICAL INTERIOR END LAP SHALL BE TWO INCHES (NO LAP WELD IS REQUIRED, ARC SPOT WELD IS REQUIRED)
3. TO CONVERT SHEAR STIFFNESS (G) TO THE FLEXIBILITY FACTOR (F), F = 1000/G
4. THESE SHEAR VALUES MEET THE FACTOR OF SAFETY REQUIREMENTS LISTED IN 2007 AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, TABLE D5. THE FACTOR HAS BEEN UNIFORMLY APPLIED TO VALUES AS 3.00.
5. THE SPACING OF BOUNDARY SPOT WELDS AT PERIMETER SUPPORTS SHALL BE DETERMINED IN ACCORDANCE WITH THE VALUES SET FORTH IN THE WELDING SCHEDULE TABLE

BDW Shear Table – Side Lap Screw + SHEARCORR

ALLOWABLE DIAPHRAGM SHEARS (q), lbs/ft FLEXIBILITY FACTOR (f) IN INCHES PER POUND x 10⁻⁶

DECK GAUGE	SEAM ATTACHMENT SPACING (INCHES)	NO. PUDDLE WELDS/UNIT	Factor	DECK SPAN (FEET)					
				5	6	7	8	9	10
22 (0.0299")	24	7	q	555	520	494	475	461	449
			f	6.69+164.9R	6.31+137.4R	6+117.8R	5.75+103.1R	5.54+91.6R	5.36+82.5R
	18		q	674	638	613	594	579	568
			f	6+164.9R	5.65+137.4R	5.38+117.8R	5.16+103.1R	4.98+91.6R	4.82+82.5R
	12		q	911	876	850	832	817	805
			f	5.16+164.9R	4.88+137.4R	4.66+117.8R	4.48+103.1R	4.34+91.6R	4.22+82.5R
	8	q	1,267	1,232	1,206	1,188	1,032	836	
		f	4.49+164.9R	4.27+137.4R	4.1+117.8R	3.97+103.1R	3.87+91.6R	3.78+82.5R	
	6	q	1,623	1,588	1,563	1,306	1,032	836	
		f	4.12+164.9R	3.94+137.4R	3.8+117.8R	3.7+103.1R	3.62+91.6R	3.55+82.5R	
	4	q	2,336	2,300	1,706	1,306	1,032	836	
		f	3.71+164.9R	3.58+137.4R	3.49+117.8R	3.41+103.1R	3.35+91.6R	3.31+82.5R	
20 (0.0359")	24	7	q	876	810	764	729	702	681
			f	5.53+95.3R	5.26+79.4R	5.04+68.1R	4.85+59.5R	4.69+52.9R	4.55+47.6R
	18		q	1,047	981	935	900	873	852
			f	5.01+95.3R	4.75+79.4R	4.54+68.1R	4.37+59.5R	4.23+52.9R	4.1+47.6R
	12		q	1,389	1,324	1,277	1,243	1,216	1,072
			f	4.35+95.3R	4.12+79.4R	3.95+68.1R	3.81+59.5R	3.69+52.9R	3.59+47.6R
	8	q	1,902	1,837	1,791	1,675	1,323	1,072	
		f	3.8+95.3R	3.62+79.4R	3.48+68.1R	3.37+59.5R	3.28+52.9R	3.21+47.6R	
	6	q	2,416	2,350	2,188	1,675	1,323	1,072	
		f	3.49+95.3R	3.33+79.4R	3.22+68.1R	3.13+59.5R	3.06+52.9R	3.01+47.6R	
	4	q	3,442	2,978	2,188	1,675	1,323	1,072	
		f	3.14+95.3R	3.03+79.4R	2.94+68.1R	2.88+59.5R	2.83+52.9R	2.79+47.6R	
18 (0.0478")	24	7	q	1,355	1,240	1,152	1,087	1,037	997
			f	4.02+40.4R	3.87+33.6R	3.74+28.8R	3.63+25.2R	3.54+22.4R	3.45+20.2R
	18		q	1,584	1,471	1,383	1,318	1,268	1,100
			f	3.69+40.4R	3.54+33.6R	3.41+28.8R	3.3+25.2R	3.21+22.4R	3.13+20.2R
	12		q	2,043	1,932	1,845	1,719	1,358	1,100
			f	3.25+40.4R	3.11+33.6R	2.99+28.8R	2.9+25.2R	2.82+22.4R	2.75+20.2R
	8	q	2,731	2,625	2,245	1,719	1,358	1,100	
		f	2.87+40.4R	2.75+33.6R	2.65+28.8R	2.57+25.2R	2.51+22.4R	2.46+20.2R	
	6	q	3,420	3,056	2,245	1,719	1,358	1,100	
		f	2.64+40.4R	2.54+33.6R	2.46+28.8R	2.39+25.2R	2.34+22.4R	2.3+20.2R	
	4	q	4,401	3,056	2,245	1,719	1,358	1,100	
		f	2.39+40.4R	2.3+33.6R	2.24+28.8R	2.2+25.2R	2.16+22.4R	2.13+20.2R	
16 (0.0598")	24	7	q	1,734	1,598	1,506	1,442	1,395	1,353
			f	3.07+20.6R	2.99+17.2R	2.91+14.7R	2.84+12.9R	2.78+11.5R	2.73+10.3R
	18		q	2,000	1,871	1,786	1,727	1,670	1,353
			f	2.85+20.6R	2.75+17.2R	2.67+14.7R	2.6+12.9R	2.54+11.5R	2.49+10.3R
	12		q	2,533	2,417	2,345	2,114	1,670	1,353
			f	2.54+20.6R	2.45+17.2R	2.37+14.7R	2.3+12.9R	2.25+11.5R	2.2+10.3R
	8	q	3,333	3,236	2,761	2,114	1,670	1,353	
		f	1.13+10.3R	1.09+8.6R	1.06+7.4R	1.03+6.4R	1+5.7R	0.99+5.2R	
	6	q	4,132	3,757	2,761	2,114	1,670	1,353	
		f	1.05+10.3R	1.01+8.6R	0.98+7.4R	0.96+6.4R	0.94+5.7R	0.92+5.2R	
	4	q	5,411	3,757	2,761	2,114	1,670	1,353	
		f	0.95+10.3R	0.92+8.6R	0.9+7.4R	0.88+6.4R	0.86+5.7R	0.85+5.2R	

SEAM ATTACHMENT, (2) HILTI S-SLC 01 M HWH SELF DRILLING SCREWS
 SEE BDW NOTES- Page 7

BADW Shear Table – Side Seam Weld

ALLOWABLE DIAPHRAGM SHEARS (q), lbs/ft FLEXIBILITY FACTOR (f) IN INCHES PER POUND x 10⁻⁶

DECK	SEAM ATTACHMENT SPACING (INCHES) SIDE SEAM WELD	NO. PUDDLE WELDS/UNIT	FACTOR	DECK SPAN (FEET)				
				6	7	8	9	10
22 (0.0299")	12	4	q	410	400	380	370	360
			F	7.4 + 187.0R	6.9 + 160.0R	6.6 + 140.0R	6.3 + 125.0R	6.1 + 112.0R
	18		q	320	300	280	270	270
			F	8.8 + 187.0R	8.3 + 160.0R	7.9 + 140.0R	7.5 + 125.0R	7.3 + 112.0R
	24		q	270	250	240	230	220
	F		9.8 + 187.0R	9.3 + 160.0R	8.9 + 140.0R	8.6 + 125.0R	8.3 + 112.0R	
12	18	7	q	480	450	430	410	400
			F	7.2 + 46.8R	6.9 + 40.1R	6.6 + 35.1R	6.3 + 31.2R	6.1 + 28.1R
	18		q	380	350	330	310	300
			F	8.3 + 46.8R	8.0 + 40.1R	7.7 + 35.1R	7.4 + 31.2R	7.2 + 28.1R
	24		q	330	300	280	260	250
			F	9.1 + 46.8R	8.8 + 40.1R	8.5 + 35.1R	8.3 + 31.2R	8.1 + 28.1R
20 (0.0359")	12	4	q	640	610	580	560	550
			F	6.1 + 108.0R	5.8 + 92.6R	5.5 + 81.0R	5.3 + 72.0R	5.1 + 64.8R
	18		q	500	470	440	420	410
			F	7.2 + 108.0R	6.8 + 92.6R	6.6 + 81.0R	6.3 + 72.0R	6.1 + 64.8R
	24		q	440	400	370	350	340
	F		8.0 + 108.0R	7.6 + 92.6R	7.3 + 81.0R	7.1 + 72.0R	6.9 + 64.8R	
12	18	7	q	750	700	660	630	610
			F	5.9 + 27.0R	5.7 + 23.2R	5.5 + 20.3R	5.3 + 18.0R	5.1 + 16.2R
	18		q	610	560	520	490	470
			F	6.7 + 27.0R	6.5 + 23.2R	6.3 + 20.3R	6.1 + 18.0R	6.0 + 16.2R
	24		q	540	490	450	420	400
			F	7.3 + 27.0R	7.1 + 23.2R	6.9 + 20.3R	6.8 + 18.0R	6.6 + 16.2R
18 (0.0478")	12	4	q	1290	1200	1140	1090	1050
			F	4.5 + 45.8R	4.3 + 39.2R	4.1 + 34.3R	4.0 + 30.5R	3.9 + 27.5R
	18		q	1040	950	890	840	800
			F	5.1 + 45.8R	4.9 + 39.2R	4.8 + 34.3R	4.6 + 30.5R	4.5 + 27.5R
	24		q	920	830	770	720	680
	F		5.6 + 45.8R	5.4 + 39.2R	5.3 + 34.3R	5.1 + 30.5R	5.0 + 27.5R	
12	18	7	q	1520	1400	1300	1230	1170
			F	4.2 + 11.4R	4.1 + 9.8R	4.0 + 8.6R	3.9 + 7.6R	3.8 + 6.9R
	18		q	1270	1150	1060	980	930
			F	4.7 + 11.4R	4.6 + 9.8R	4.5 + 8.6R	4.4 + 7.6R	4.4 + 6.9R
	24		q	1150	1030	930	860	800
			F	5.0 + 11.4R	5.0 + 9.8R	4.9 + 8.6R	4.8 + 7.6R	4.8 + 6.9R
16 (0.0598")	12	4	q	1760	1660	1580	1530	1490
			F	3.4 + 23.4R	3.3 + 20.0R	3.2 + 17.5R	3.1 + 15.6R	3.0 + 14.0R
	18		q	1450	1340	1260	1200	1160
			F	3.8 + 23.4R	3.7 + 20.0R	3.6 + 17.5R	3.6 + 15.6R	3.5 + 14.0R
	24		q	1290	1180	1100	1040	990
	F		4.1 + 23.4R	4.0 + 20.0R	4.0 + 17.5R	3.9 + 15.6R	3.8 + 14.0R	
12	18	7	q	2230	2070	1970	1890	1820
			F	3.2 + 5.8R	3.1 + 5.0R	3.1 + 4.4R	3.0 + 3.9R	3.0 + 3.5R
	18		q	1890	1730	1620	1530	1460
			F	3.5 + 5.8R	3.5 + 5.0R	3.4 + 4.4R	3.4 + 3.9R	3.4 + 3.5R
	24		q	1730	1560	1440	1350	1280
			F	3.7 + 5.8R	3.7 + 5.0R	3.7 + 4.4R	3.7 + 3.9R	3.6 + 3.5R

SEE BDW NOTES- Page 4

NDW Vertical Load Table

DECK			DECK SPAN (FT)											
			7' - 0"	7' - 6"	8' - 0"	8' - 6"	9' - 0"	9' - 6"	10' - 0"	10' - 6"	11' - 0"	12' - 0"	13' - 0"	13' - 6"
SIMPLE SPAN	22 (0.0299")	S	114	106	100	91	81	73	66	60	55	46	39	36
		Δ	162	132	108	90	76	65	56	48	42	32	25	23
	20 (0.0359")	S	158	148	132	117	104	94	84	77	70	59	50	47
		Δ	205	167	138	115	97	82	70	61	53	41	32	29
	18 (0.0478")	S	259	226	198	176	157	141	127	115	105	88	75	70
		Δ	299	243	200	167	141	120	102	89	77	59	47	42
	16 (0.0598")	S	344	300	264	234	208	187	169	153	140	117	100	93
		Δ	404	328	271	226	190	162	139	120	104	80	63	56
TWO SPAN	22 (0.0299")	S	129	121	113	106	100	90	81	74	68	57	49	45
		Δ	390	317	261	218	183	156	134	116	101	77	61	54
	20 (0.0359")	S	192	179	158	141	125	113	102	93	84	71	61	56
		Δ	494	402	331	276	233	198	170	147	128	98	77	69
	18 (0.0478")	S	291	254	223	198	176	159	144	130	119	100	86	79
		Δ	721	586	483	402	339	288	274	213	186	143	113	101
	16 (0.0598")	S	370	323	285	252	225	203	183	166	152	127	109	101
		Δ	973	791	652	543	458	389	334	288	251	193	152	136
THREE OR MORE SPANS	22 (0.0299")	S	142	133	124	117	110	105	100	92	84	71	65	56
		Δ	306	249	205	171	144	123	105	91	79	61	54	43
	20 (0.0359")	S	198	184	173	163	154	140	127	115	105	89	76	70
		Δ	388	315	260	217	183	155	133	115	100	88	60	54
	18 (0.0478")	S	360	315	278	246	220	198	179	163	148	125	106	99
		Δ	566	460	379	316	266	226	194	168	146	112	99	79
	16 (0.0598")	S	458	401	353	313	280	252	228	207	189	159	136	126
		Δ	765	622	512	427	360	306	262	227	198	152	119	107

S (STRESS), Δ DEFLECTION

HOW TO USE TABLE:

1. DETERMINE THE TOTAL LOAD AND THE LIVE LOAD FOR THE ROOF DECK
2. USING THE TOTAL LOAD, SELECT THE GAUGE AND THE SPAN WITH EQUAL OR GREATER VALUES FOR S (STRESS)
3. USING THE LIVE LOAD, SELECT THE GAUGE AND THE SPAN WITH EQUAL OR GREATER VALUE FOR Δ (DEFLECTION) OF L/180.
4. SELECT THE GAUGE AND THE SPAN FOR THE ROOF DECK THAT MEETS BOTH STEPS 2 & 3.

NDW Shear Table – Button Punch

ALLOWABLE DIAPHRAGM SHEARS (q), lbs/ft FLEXIBILITY FACTOR (f) IN INCHES PER POUND x 10⁻⁶

DECK GAUGE	SEAM ATTACHMENT SPACING (INCHES) BUTTON PUNCH	NO. PUDDLE WELDS/UNIT	FACTOR	DECK SPAN (FEET)			
				8	10	12	14
22 (0.0299")	12	2	q	170	160	150	140
			F	30.1 + 1112R	32.7 + 889R	34.8 + 741R	36.5 + 635R
	18		q	130	120	110	110
			F	37.2 + 1112R	41.4 + 889R	44.8 + 741R	47.7 + 635R
	24		q	120	100	100	90
	F		42.4 + 1112R	48.0 + 889R	32.6 + 741R	56.7 + 635R	
20 (0.0359")	12	4	q	240	220	200	190
			F	21.3 + 123R	24.4 + 98.8R	27.1 + 82.3R	29.5 + 70.5R
	18		q	210	180	160	150
			F	24.4 + 123R	28.5 + 98.8R	32.4 + 82.3R	35.9 + 70.5R
	24		q	190	160	150	130
			F	26.3 + 123R	31.3 + 98.8R	38.0 + 82.3R	40.5 + 70.5R
18 (0.0478")	12	2	q	230	210	200	190
			F	25.8 + 643R	28.6 + 514R	30.5 + 428R	32.7 + 367R
	18		q	190	170	160	150
			F	30.7 + 643R	34.8 + 514R	37.6 + 428R	40.7 + 367R
	24		q	170	150	130	120
	F		39.1 + 643R	45.8 + 514R	45.8 + 428R	50.4 + 367R	
16 (0.0598")	12	4	q	340	300	280	260
			F	17.6 + 71.6R	20.4 + 57.2R	22.7 + 47.6R	25.2 + 40.8R
	18		q	300	260	230	220
			F	19.6 + 71.6R	23.3 + 57.2R	27.1 + 47.6R	29.4 + 40.8R
	24		q	280	240	210	190
			F	20.8 + 71.6R	25.0 + 57.2R	29.5 + 47.6R	33.6 + 40.8R
18 (0.0478")	12	2	q	370	330	310	290
			F	19.1 + 271R	21.8 + 217R	23.7 + 181R	25.8 + 155R
	18		q	320	280	250	230
			F	21.8 + 271R	25.3 + 217R	28.7 + 181R	32.1 + 155R
	24		q	290	250	230	210
	F		23.9 + 271R	28.2 + 217R	31.3 + 181R	35.1 + 155R	
16 (0.0598")	12	4	q	580	490	440	400
			F	12.0 + 30.1R	14.6 + 24.1R	16.9 + 20.1R	19.4 + 17.2R
	18		q	530	440	390	350
			F	15.0 + 30.1R	16.1 + 24.1R	18.9 + 20.1R	21.9 + 17.2R
	24		q	500	420	380	320
			F	13.6 + 30.1R	16.8 + 24.1R	20.3 + 20.1R	23.8 + 17.2R
16 (0.0598")	12	2	q	520	460	410	390
			F	14.2 + 139R	16.4 + 111R	18.8 + 92.5R	20.3 + 79.4R
	18		q	460	400	360	330
			F	15.9 + 139R	18.7 + 111R	21.2 + 92.5R	23.8 + 79.4R
	24		q	430	360	320	290
	F		16.9 + 139R	20.6 + 111R	23.7 + 92.5R	26.8 + 79.4R	
16 (0.0598")	12	4	q	800	680	600	550
			F	8.9 + 15.4R	10.8 + 12.3R	12.3 + 10.3R	14.7 + 8.8R
	18		q	740	620	540	490
			F	9.5 + 15.4R	11.7 + 12.3R	14.1 + 10.3R	16.3 + 8.8R
	24		q	720	590	510	460
			F	9.7 + 15.4R	12.3 + 12.3R	14.8 + 10.3R	17.3 + 8.8R

SEE NDW SIDE SEAM NOTES- Page 16

NDW Shear Table – Top Seam Weld

ALLOWABLE DIAPHRAGM SHEARS (q), lbs/ft FLEXIBILITY FACTOR (f) IN INCHES PER POUND x 10⁻⁶

DECK GAUGE	SEAM ATTACHMENT SPACING (INCHES) TOP SEAM WELDS	NO. PUDDLE WELDS/UNIT	FACTOR	DECK SPAN (FEET)			
				8	10	12	14
22 (0.0299")	12	2	q	260	250	250	240
			F	11.6 + 1112R	10.1 + 889R	9.1 + 741R	8.4 + 635R
	18		q	190	180	170	160
			F	15.0 + 1112R	13.2 + 889R	11.8 + 741R	10.8 + 635R
	24	q	150	140	140	130	
		F	18.0 + 1112R	15.8 + 889R	14.2 + 741R	13.0 + 635R	
22 (0.0299")	12	4	q	310	290	280	270
			F	10.1 + 123R	9.1 + 98.8R	8.4 + 82.3R	7.9 + 70.5R
	18		q	240	220	210	200
			F	11.7 + 123R	11.2 + 98.8R	10.4 + 82.3R	9.8 + 70.5R
	24	q	200	180	170	160	
		F	14.0 + 123R	12.9 + 98.8R	12.1 + 82.3R	11.4 + 70.5R	
20 (0.0359")	12	2	q	390	380	370	360
			F	9.9 + 643R	8.7 + 514R	7.9 + 429R	7.2 + 367R
	18		q	290	270	260	250
			F	12.7 + 643R	11.2 + 514R	10.1 + 429R	9.3 + 367R
	24	q	240	220	210	200	
		F	15.0 + 643R	13.3 + 514R	12.1 + 429R	11.1 + 367R	
20 (0.0359")	12	4	q	480	450	420	400
			F	8.4 + 71.4R	7.7 + 57.2R	7.2 + 47.6R	6.8 + 40.8R
	18		q	380	340	320	300
			F	10.1 + 71.4R	9.4 + 57.2R	8.8 + 47.6R	8.6 + 40.8R
	24	q	330	290	260	250	
		F	11.3 + 71.4R	10.6 + 57.2R	10.1 + 47.6R	9.6 + 40.8R	
18 (0.0478")	12	2	q	840	800	780	770
			F	7.4 + 271R	6.7 + 217R	6.1 + 181R	5.6 + 155R
	18		q	630	590	570	570
			F	9.3 + 271R	8.5 + 217R	7.7 + 181R	7.0 + 155R
	24	q	530	490	460	450	
		F	10.7 + 271R	9.8 + 217R	9.1 + 181R	8.4 + 155R	
18 (0.0478")	12	4	q	1020	960	920	890
			F	6.1 + 30.1R	5.7 + 24.1R	5.3 + 20.1R	5.1 + 17.2R
	18		q	770	680	620	580
			F	7.5 + 30.1R	7.3 + 24.1R	7.1 + 20.1R	6.9 + 17.2R
	24	q	720	650	600	570	
		F	7.9 + 30.1R	7.5 + 24.1R	7.3 + 20.1R	7.1 + 17.2R	
16 (0.0598")	12	2	q	1130	1080	1050	1050
			F	3.8 + 139R	5.2 + 111R	4.8 + 92.5R	4.4 + 79.4R
	18		q	870	810	770	770
			F	7.2 + 139R	6.5 + 111R	6.0 + 92.5R	5.5 + 79.4R
	24	q	740	680	640	620	
		F	8.2 + 139R	7.5 + 111R	7.0 + 92.5R	6.5 + 79.4R	
16 (0.0598")	12	4	q	1380	1280	1220	1190
			F	4.6 + 15.4R	4.4 + 12.3R	4.2 + 10.3R	4.0 + 8.8R
	18		q	1090	980	920	880
			F	5.5 + 15.4R	5.3 + 12.3R	5.1 + 10.3R	4.9 + 8.8R
	24	q	1000	890	820	770	
		F	5.9 + 15.4R	5.7 + 12.3R	5.6 + 10.3R	3.4 + 8.8R	

SEE NDW SIDE SEAM NOTES- Page 16

NDW Shear Table – Side Seam Weld

ALLOWABLE DIAPHRAGM SHEARS (q), lbs/ft FLEXIBILITY FACTOR (f) IN INCHES PER POUND x 10⁻⁶

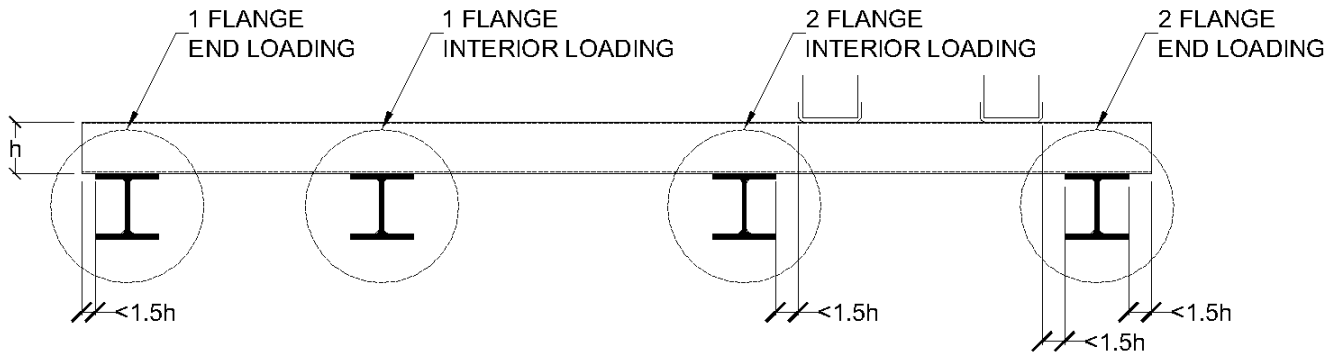
DECK	SEAM ATTACHMENT SPACING (INCHES) SIDE SEAM WELD	NO. PUDDLE WELDS/UNIT	FACTOR	DECK SPAN (FEET)			
				8	10	12	14
20 (0.0359")	12	2	q	600	590	580	570
			F	7.27 + 643R	6.43 + 515R	5.83 + 429R	5.42 + 367R
	18		q	430	410	400	390
			F	9.3 + 643R	8.19 + 515R	7.4 + 429R	6.79 + 367R
	24		q	340	320	310	300
	F		11.1 + 643R	9.76 + 515R	8.81 + 429R	8.07 + 376R	
18 (0.0478")	12	4	q	690	650	630	610
			F	6.54 + 71.5R	5.97 + 57.2R	5.56 + 47.6R	5.23 + 40.8R
	18		q	520	480	460	440
			F	7.96 + 71.5R	7.30 + 57.2R	6.81 + 47.6R	6.39 + 40.8R
	24		q	430	390	370	350
			F	9.10 + 71.5R	8.41 + 57.2R	7.36 + 47.6R	7.41 + 40.8R
16 (0.0598")	12	2	q	1130	1090	1060	1040
			F	5.58 + 271R	4.97 + 217R	4.55 + 181R	4.23 + 155R
	18		q	820	780	750	730
			F	7.05 + 271R	6.27 + 217R	5.71 + 181R	5.29 + 155R
	24		q	660	620	590	570
	F		8.29 + 271R	7.40 + 271R	6.76 + 181R	6.25 + 155R	
18 (0.0478")	12	4	q	1330	1240	1190	1140
			F	4.89 + 30.1R	4.51 + 24.1R	4.24 + 20.1R	4.02 + 17.2R
	18		q	1020	930	870	830
			F	5.84 + 30.1R	5.43 + 24.1R	5.13 + 20.1R	4.88 + 17.2R
	24		q	870	780	720	680
			F	6.58 + 30.1R	6.17 + 24.1R	5.86 + 20.1R	5.60 + 17.2R
16 (0.0598")	12	2	q	1650	1610	1590	1580
			F	4.2 + 139R	3.95 + 111R	3.64 + 92.5R	3.4 + 79.4R
	18		q	1220	1160	1130	1120
			F	5.50 + 139R	4.94 + 111R	4.54 + 92.5R	4.23 + 79.4R
	24		q	1000	940	910	880
	F		6.40 + 139R	5.79 + 111R	5.33 + 92.5R	4.96 + 79.4R	
18 (0.0478")	12	4	q	1880	1800	1760	1740
			F	3.78 + 15.4R	3.54 + 12.3R	3.52 + 10.3R	3.20 + 8.8R
	18		q	1460	1370	1310	1280
			F	4.46 + 15.4R	4.22 + 12.3R	4.0 + 10.4R	3.8 + 8.8R
	24		q	1260	1150	1090	1050
			F	4.97 + 15.4R	4.74 + 12.3R	4.54 + 10.4R	4.38 + 8.8R

Notes:

FOR SI 1 INCH=25.4mm, lbf = 4.45N, 1psi = 6.9 kpa, 1ft = 304.8mm

1. SPACING OF BOUNDARY OR PARALLEL SHEAR COLLECTOR ATTACHMENTS SHALL BE BASED ON MAXIMUM ALLOWABLE SHEAR FOR PUDDLE WELDS PER WELDING SCHEDULE.
2. TYPICAL INTERIOR END LAP SHALL BE TWO INCHES (NO LAP WELD IS REQUIRED, ARC SPOT WELD IS REQUIRED)
3. TO CONVERT SHEAR STIFFNESS (G) TO THE FLEXIBILITY FACTOR (F), F = 1000/G
4. THESE SHEAR VALUES MEET THE FACTOR OF SAFETY REQUIREMENTS LISTED IN 2007 AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, TABLE D5. THE FACTOR HAS BEEN UNIFORMLY APPLIED TO VALUES AS 3.00.
5. THE SPACING OF BOUNDARY SPOT WELDS AT PERIMETER SUPPORTS SHALL BE DETERMINED IN ACCORDANCE WITH THE VALUES SET FORTH IN THE WELDING SCHEDULE TABLE

Web Crippling Tables



DECK TYPE	GAUGE	BASE METAL THICKNESS (INCHES)	ALLOWABLE REACTIONS BASED ON WEB CRIPPLING						
			POUNDS PER FOOT OF DECK WIDTH (1 FLANGE LOADING / END BEARING)						
			1.0 in	1.5 in	2.0 in	2.5 in	3.0 in	3.5 in	4.0 in
3DW	22	0.0299	510	578	635	685	731	773	812
	20	0.0359	716	807	884	952	1014	1070	1123
	18	0.0478	1228	1375	1499	1609	1708	1799	1883
	16	0.0598	1873	2086	2266	2424	2567	2698	2820
2DW	22	0.0299	463	524	576	622	663	701	736
	20	0.0359	646	728	798	859	915	966	1013
	18	0.0478	1101	1233	1344	1442	1531	1612	1688
	16	0.0598	1672	1862	2022	2163	2290	2408	2517
BDW	22	0.0299	1203	1363	1498	1616	1724	1822	1914
	20	0.0359	1673	1887	2067	2226	2369	2501	2624
	18	0.0478	2836	3176	3462	3715	3943	4153	4348
	16	0.0598	4291	4779	5190	5552	5879	6180	6461
BADW	22	0.0299	1203	1363	1498	1616	1724	1822	1914
	20	0.0359	1673	1887	2067	2226	2369	2501	2624
	18	0.0478	2836	3176	3462	3715	3943	4153	4348
	16	0.0598	4291	4779	5190	5552	5879	6180	6461
NDW	22	0.0299	840	952	1046	1129	1204	1273	1337
	20	0.0359	1180	1330	1458	1570	1671	1764	1851
	18	0.0478	2025	2268	2473	2653	2816	2966	3106
	16	0.0598	3091	3442	3738	3999	4235	4452	4653

TABLES REFLECT NOMINAL STRENGTH, R_n
 ASD ALLOWABLE, R_n / Ω_w , $\Omega_w = 1.70$
 LRFD, ϕR_n , $\phi = 0.90$

Web Crippling Tables

DECK TYPE	GAUGE	BASE METAL THICKNESS (INCHES)	ALLOWABLE REACTIONS BASED ON WEB CRIPPLING						
			POUNDS PER FOOT OF DECK WIDTH (1 FLANGE LOADING / INTERIOR BEARING)						
			1.0 in	1.5 in	2.0 in	2.5 in	3.0 in	3.5 in	4.0 in
3DW	22	0.0299	853	948	1028	1099	1163	1222	1276
	20	0.0359	1215	1344	1453	1549	1636	1716	1790
	18	0.0478	2108	2315	2490	2644	2783	2911	3030
	16	0.0598	3239	3537	3789	4011	4211	4396	4567
2DW	22	0.0299	743	825	895	956	1012	1063	1111
	20	0.0359	1057	1169	1264	1347	1423	1492	1557
	18	0.0478	1832	2012	2164	2298	2419	2530	2633
	16	0.0598	2813	3073	3291	3484	3658	3818	3967
BDW	22	0.0299	1868	2076	2252	2406	2546	2674	2794
	20	0.0359	2658	2940	3178	3388	3578	3752	3914
	18	0.0478	4604	5056	5438	5774	6078	6357	6617
	16	0.0598	7067	7718	8267	8751	9188	9590	9965
BADW	22	0.0299	1868	2076	2252	2406	2546	2674	2794
	20	0.0359	2658	2940	3178	3388	3578	3752	3914
	18	0.0478	4604	5056	5438	5774	6078	6357	6617
	16	0.0598	7067	7718	8267	8751	9188	9590	9965
NDW	22	0.0299	1411	1568	1701	1817	1923	2020	2110
	20	0.0359	2009	2223	2403	2562	2705	2837	2960
	18	0.0478	3486	3829	4118	4372	4602	4814	5011
	16	0.0598	5357	5850	6266	6633	6965	7269	7553

TABLES REFLECT NOMINAL STRENGTH, R_n
 ASD ALLOWABLE, R_n / Ω_w , $\Omega_w = 1.75$
 LRFD, ϕR_n , $\phi = 0.85$

DECK TYPE	GAUGE	BASE METAL THICKNESS (INCHES)	ALLOWABLE REACTIONS BASED ON WEB CRIPPLING						
			POUNDS PER FOOT OF DECK WIDTH (2 FLANGE LOADING / END BEARING)						
			1.0 in	1.5 in	2.0 in	2.5 in	3.0 in	3.5 in	4.0 in
3DW	22	0.0299	534	588	633	673	709	743	773
	20	0.0359	808	885	950	1007	1059	1106	1151
	18	0.0478	1515	1648	1760	1859	1948	2030	2107
	16	0.0598	2452	2652	2821	2970	3105	3229	3344
2DW	22	0.0299	506	557	600	638	672	704	733
	20	0.0359	756	828	889	943	991	1036	1078
	18	0.0478	1398	1521	1625	1716	1798	1874	1944
	16	0.0598	2242	2426	2580	2717	2840	2953	3059
BDW	22	0.0299	1359	1496	1611	1713	1805	1889	1968
	20	0.0359	2014	2206	2368	2511	2640	2759	2870
	18	0.0478	3684	4008	4280	4520	4737	4937	5123
	16	0.0598	5867	6348	6752	7109	7431	7728	8004
BADW	22	0.0299	1359	1496	1611	1713	1805	1889	1968
	20	0.0359	2014	2206	2368	2511	2640	2759	2870
	18	0.0478	3684	4008	4280	4520	4737	4937	5123
	16	0.0598	5867	6348	6752	7109	7431	7728	8004
NDW	22	0.0299	877	965	1039	1105	1164	1219	1269
	20	0.0359	1327	1453	1560	1654	1739	1817	1890
	18	0.0478	2492	2711	2895	3058	3205	3340	3465
	16	0.0598	4036	4366	4645	4890	5112	5315	5505

TABLES REFLECT NOMINAL STRENGTH, R_n
 ASD ALLOWABLE, R_n / Ω_w , $\Omega_w = 1.80$
 LRFD, ϕR_n , $\phi = 0.85$

Web Crippling Tables

DECK TYPE	GAUGE	BASE METAL THICKNESS (INCHES)	ALLOWABLE REACTIONS BASED ON WEB CRIPPLING						
			POUNDS PER FOOT OF DECK WIDTH (2 FLANGE LOADING / INTERIOR BEARING)						
			1.0 in	1.5 in	2.0 in	2.5 in	3.0 in	3.5 in	4.0 in
3DW	22	0.0299	974	1094	1195	1284	1365	1439	1508
	20	0.0359	1409	1575	1715	1839	1951	2054	2149
	18	0.0478	2492	2767	2998	3202	3386	3555	3713
	16	0.0598	3876	4279	4618	4917	5187	5436	5667
2DW	22	0.0299	873	981	1072	1152	1224	1291	1352
	20	0.0359	1259	1407	1533	1643	1743	1835	1920
	18	0.0478	2215	2459	2665	2846	3010	3161	3301
	16	0.0598	3435	3791	4092	4357	4596	4816	5021
BDW	22	0.0299	2251	2528	2762	2968	3155	3326	3486
	20	0.0359	3234	3616	3938	4222	4478	4714	4934
	18	0.0478	5670	6294	6821	7284	7704	8089	8448
	16	0.0598	8767	9677	10445	11121	11732	12294	12818
BADW	22	0.0299	2251	2528	2762	2968	3155	3326	3486
	20	0.0359	3234	3616	3938	4222	4478	4714	4934
	18	0.0478	5670	6294	6821	7284	7704	8089	8448
	16	0.0598	8767	9677	10445	11121	11732	12294	12818
NDW	22	0.0299	1606	1804	1971	2118	2251	2374	2487
	20	0.0359	2324	2599	2830	3034	3218	3388	3546
	18	0.0478	4113	4566	4948	5284	5588	5868	6128
	16	0.0598	6400	7064	7625	8118	8564	8975	9357

TABLES REFLECT NOMINAL STRENGTH, R_n
 ASD ALLOWABLE, R_n / Ω_w , $\Omega_w = 1.75$
 LRFD, ϕR_n , $\phi = 0.85$

BDW Light Weight Concrete

TOTAL SLAB DEPTH & CONCRETE WEIGHT (psf)	GAUGE	MAX UNSHORED SPAN (FEET - INCHES)			(l) Total Allowable Superimposed Load (psf)												
		SINGLE	DOUBLE	THREE OR MORE	(q ₄) DIAPHRAGM SHEAR VALUES (plf) - 4 ENDS WELDS/SHEET (q ₇) DIAPHRAGM SHEAR VALUES (plf) - 7 ENDS WELDS/SHEET												
					SPAN (FEET)												
						6'-0"	6'-6"	7'-0"	7'-6"	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	
3 1/2" (23.2 psf)	22 (0.0299")	6-6	8-6	8-3	l	310	262	223	192	166	145	127	112	100	89	79	
					q4	1338	1316	1299	1282	1270	1256	1248	1236	1230	1221	1214	
					q7	1451	1418	1396	1372	1355	1335	1322	1308	1296	1284	1275	
	20 (0.0359")	8-0	9-6	8-9	l	357	301	257	221	192	168	148	131	116	104	93	
					q4	1404	1375	1357	1336	1322	1305	1296	1282	1274	1262	1256	
					q7	1538	1499	1472	1443	1424	1400	1385	1366	1355	1339	1329	
	18 (0.0478")	10-0	10-6	10-0	l	338	285	243	209	182	159	139	123	109	97	87	
					q4	1541	1502	1478	1450	1433	1409	1396	1378	1368	1352	1344	
					q7	1715	1663	1629	1589	1564	1533	1513	1489	1473	1452	1440	
	16 (0.0598")	11-3	11-6	10-9	l	323	273	232	200	173	151	132	117	103	92	82	
					q4	1684	1634	1608	1570	1551	1521	1507	1482	1472	1451	1442	
					q7	1899	1833	1793	1742	1712	1673	1650	1617	1600	1573	1560	
4" (27.8 psf)	22 (0.0299")	6-3	8-3	7-6	l	397	335	286	246	213	186	164	145	128	114	102	
					q4	1598	1574	1559	1541	1529	1516	1507	1495	1489	1479	1474	
					q7	1710	1678	1655	1632	1613	1595	1582	1567	1556	1543	1535	
	20 (0.0359")	7-6	9-0	8-6	l	400	385	329	284	247	216	190	168	150	134	120	
					q4	1664	1635	1617	1595	1582	1565	1555	1541	1533	1522	1516	
					q7	1797	1759	1732	1703	1682	1660	1645	1625	1613	1598	1589	
	18 (0.0478")	9-3	10-3	9-6	l	400	366	312	269	234	204	180	159	141	126	113	
					q4	1799	1760	1738	1708	1691	1668	1656	1637	1628	1612	1604	
					q7	1975	1923	1888	1849	1823	1791	1773	1747	1733	1712	1699	
	16 (0.0598")	10-9	11-3	10-3	l	400	350	299	257	223	195	171	151	134	120	107	
					q4	1944	1894	1867	1829	1810	1780	1765	1741	1730	1710	1702	
					q7	2158	2092	2051	2002	1972	1932	1910	1877	1859	1833	1819	
4 3/4" (34.7 psf)	22 (0.0299")	5-9	7-9	7-3	l	400	400	385	332	289	252	222	197	175	156	140	
					q4	1986	1963	1947	1931	1919	1905	1895	1885	1877	1868	1863	
					q7	2098	2067	2044	220	2002	1984	1971	1955	1945	1932	1924	
	20 (0.0359")	7-0	8-6	8-0	l	400	400	400	384	334	293	258	229	204	182	164	
					q4	2053	2024	2006	1984	1971	1954	1944	1931	1921	1911	1905	
					q7	2185	2148	2120	2092	2071	2049	2033	2015	2002	1986	1977	
	18 (0.0478")	8-6	9-6	9-0	l	400	400	400	366	318	279	246	218	194	173	155	
					q4	2188	2149	2127	2097	2080	2058	2045	2025	2016	2001	1993	
					q7	2363	2311	2276	2237	2211	2181	2162	2136	2122	2101	2088	
	16 (0.0598")	9-9	10-3	9-6	l	400	400	400	351	305	267	235	208	185	165	148	
					q4	2332	2283	2256	2218	2200	2168	2154	2129	2119	2098	2090	
					q7	2548	2482	2440	2391	2361	2321	2298	2266	2248	2222	2207	
5 3/4" (43.9 psf)	22 (0.0299")	5-6	7-3	6-6	l	400	400	400	400	394	345	304	269	240	214	192	
					q4	2505	2482	2466	2448	2436	2423	2414	2402	2396	2387	2380	
					q7	2617	2586	2562	2539	2521	2503	2488	2474	2564	2451	2443	
	20 (0.0359")	6-6	7-9	7-3	l	400	400	400	400	400	400	354	314	280	251	226	
					q4	2570	2543	2525	2503	2490	2473	2462	2448	2440	2430	2423	
					q7	2704	2666	2639	2609	2590	2566	2551	2532	2521	2505	2496	
	18 (0.0478")	7-9	8-9	8-3	l	400	400	400	400	400	385	339	301	268	240	216	
					q4	2707	2668	2646	2616	2599	2575	2564	2544	2535	2519	2512	
					q7	2882	2830	2795	2756	2730	2699	2681	2655	2639	2620	2601	
	16 (0.0598")	9-3	9-9	8-9	l	400	400	400	400	400	370	327	289	258	230	207	
					q4	2851	2802	2774	2737	2717	2687	2673	2648	2638	2617	2609	
					q7	3065	2999	2959	2908	2880	2839	2817	2785	2766	2740	2726	

SEE BDW NORMAL WEIGHT TABLE NOTES- Page 22

BDW Normal Weight Concrete

TOTAL SLAB DEPTH & CONCRETE WEIGHT (psf)	GAUGE	MAX UNSHORED SPAN (FEET - INCHES)			(l) Total Allowable Superimposed Load (psf)											
					(q ₄) DIAPHRAGM SHEAR VALUES (plf) - 4 ENDS WELDS/SHEET (q ₇) DIAPHRAGM SHEAR VALUES (plf) - 7 ENDS WELDS/SHEET											
		SINGLE	DOUBLE	THREE OR MORE	SPAN (FEET)											
	6'-0"				6'-6"	7'-0"	7'-6"	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"		
3 1/2" (30.5 psf)	22 (0.0299")	6-0	8-0	7-6	l	325	273	232	199	172	150	131	115	101	90	80
					q4	1871	1847	1832	1815	1803	1789	1780	1769	1762	1752	1747
					q7	2028	1996	1972	1948	1930	1911	1898	1882	1871	1858	1850
	20 (0.0359")	7-3	8-8	8-0	l	376	317	270	232	201	175	153	135	120	106	94
					q4	1937	1908	1890	1868	1855	1838	1828	1815	1806	1795	1789
					q7	2117	2079	2050	2022	2000	1978	1962	1943	1930	1914	1905
	18 (0.0478")	9-0	10-0	9-0	l	359	302	257	221	191	166	145	128	113	100	89
					q4	2072	2033	2011	1981	1964	1942	1929	1910	1901	1885	1877
					q7	2300	2246	2210	2171	2144	2113	2093	2067	2052	2031	2018
	16 (0.0598")	10-3	10-8	10-0	l	345	290	247	212	183	159	139	122	108	95	85
					q4	2217	2167	2140	2102	2084	2053	2038	2014	2003	1983	1975
					q7	2488	2421	2378	2328	2297	2256	2230	2200	2181	2155	2140
4" (36.6 psf)	22 (0.0299")	5-8	7-6	7-3	l	400	348	296	254	220	191	168	147	130	115	103
					q4	2263	2240	2224	2206	2194	2181	2172	2161	2154	2145	2139
					q7	2430	2398	2374	2349	2331	2313	2298	2284	2273	2260	2252
	20 (0.0359")	6-8	8-3	7-9	l	400	400	344	296	256	224	196	173	154	137	122
					q4	2328	2301	2283	2261	2248	2231	2220	2206	2198	2187	2180
					q7	2519	2480	2453	2422	2402	2378	2362	2343	2331	2316	2306
	18 (0.0478")	8-6	9-6	8-9	l	400	386	329	282	245	213	187	165	146	129	115
					q4	2465	2426	2404	2374	2357	2334	2322	2302	2293	2278	2270
					q7	2701	2647	2612	2572	2546	2514	2494	2468	2453	2431	2419
	16 (0.0598")	9-9	10-3	9-6	l	400	371	316	271	235	204	179	158	139	124	110
					q4	2609	2560	2532	2495	2475	2445	2431	2406	2396	2375	2367
					q7	2889	2821	2780	2728	2699	2657	2635	2601	2583	2556	2542
4 1/2" (42.7 psf)	22 (0.0299")	5-6	7-6	7-0	l	400	400	363	312	270	235	206	182	161	143	127
					q4	2656	2633	2617	2599	2587	2574	2565	2553	2547	2538	2531
					q7	2832	2800	2776	2750	2733	2715	2700	2685	2675	2661	2652
	20 (0.0359")	6-6	8-0	7-3	l	400	400	400	363	315	275	242	213	189	169	151
					q4	2721	2694	2674	2653	2640	2623	2613	2599	2591	2579	2573
					q7	2921	2881	2854	2824	2804	2780	2764	2745	2733	2717	2707
	18 (0.0478")	8-0	9-0	8-3	l	400	400	400	348	301	263	231	204	181	161	143
					q4	2857	2818	2794	2766	2750	2726	2714	2695	2685	2669	2661
					q7	3102	3048	3014	2974	2947	2915	2895	2870	2854	2833	2821
	16 (0.0598")	9-3	10-0	9-0	l	400	400	390	335	290	253	222	196	173	154	137
					q4	3002	2951	2925	2887	2868	2838	2824	2799	2789	2768	2759
					q7	3290	3223	3182	3129	3099	3059	3035	3003	2985	2957	2943

SEE BDW NORMAL WEIGHT TABLE NOTES- Page 22

BDW Normal Weight Concrete – Continued

TOTAL SLAB DEPTH & CONCRETE WEIGHT (psf)	GAUGE	MAX UNSHORED SPAN (FEET - INCHES)			(I) Total Allowable Superimposed Load (psf) (q ₄) DIAPHRAGM SHEAR VALUES (plf) - 4 ENDS WELDS/SHEET (q ₇) DIAPHRAGM SHEAR VALUES (plf) - 7 ENDS WELDS/SHEET												
		SINGLE	DOUBLE	THREE OR MORE	SPAN (FEET)												
					6'-0"	6'-6"	7'-0"	7'-6"	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"		
5" (48.7 psf)	22 (0.0299")	5-0	6-6	7-0	I	400	400	400	371	322	280	246	217	192	171	152	
					q4	3047	3025	3008	2991	2980	2965	2956	2946	2939	2929	2924	
					q7	3160	3128	3176	3152	3135	3115	3102	3087	3075	3063	3054	
	20 (0.0359")	6-0	8-0	8-0	I	400	400	400	400	376	328	289	255	227	202	181	
					q4	3114	3085	3067	3046	3032	3015	3004	2991	2984	2972	2965	
					q7	3322	3282	3255	3225	3205	3181	3165	3147	3147	3133	3119	3108
	18 (0.0478")	7-0	9-6	9-6	I	400	400	400	400	361	315	277	244	217	193	172	
					q4	3250	3211	3188	3159	3142	3119	3106	3088	3077	3062	3054	
					q7	3503	3450	3415	3374	3349	3317	3297	3272	3256	3235	3223	
	16 (0.0598")	8-6	10-6	10-6	I	400	400	400	400	348	304	267	235	208	185	165	
					q4	3393	3344	3318	3280	3260	3231	3216	3192	3181	3160	3151	
					q7	3692	3624	3583	3531	3501	3461	3437	3403	3386	3358	3344	
5 1/2" (54.8 psf)	22 (0.0299")	4-6	6-6	6-6	I	400	400	400	400	374	327	287	253	224	199	178	
					q4	3440	3416	3401	3384	3372	3358	3349	3338	3331	3322	3316	
					q7	3635	3602	3578	3554	3536	3517	3503	3487	3477	3465	3455	
	20 (0.0359")	5-6	7-6	8-0	I	400	400	400	400	400	383	337	298	265	236	211	
					q4	3506	3478	3459	3439	3424	3407	3397	3384	3376	3364	3358	
					q7	3724	3684	3656	3627	3606	3583	3567	3548	3535	3521	3510	
	18 (0.0478")	7-0	9-0	9-0	I	400	400	400	400	400	368	324	286	254	226	202	
					q4	3641	3604	3580	3550	3535	3511	3498	3480	3470	3454	3446	
					q7	3817	3765	3731	3691	3666	3635	3615	3591	3575	3554	3541	
	16 (0.0598")	8-0	10-0	10-0	I	400	400	400	400	400	356	313	276	245	218	195	
					q4	3786	3736	3710	3671	3653	3623	3609	3584	3572	3552	3544	
					q7	4001	3935	3895	3844	3814	3774	3752	3719	3702	3675	3661	
6" (60.8 psf)	22 (0.0299")	4-6	6-0	6-0	I	400	400	400	400	400	374	328	290	257	229	204	
					q4	3832	3809	3793	3777	3765	3751	3741	3731	3723	3714	3709	
					q7	3944	3913	3890	3866	3848	3830	3817	3801	3791	3778	3770	
	20 (0.0359")	5-6	7-0	7-6	I	400	400	400	400	400	400	386	341	303	271	243	
					q4	3899	3870	3852	3830	3817	3800	3790	3777	3767	3757	3751	
					q7	4031	3994	3966	3938	3917	3895	3879	3861	3848	3832	3823	
	18 (0.0478")	6-6	8-6	9-0	I	400	400	400	400	400	400	372	329	292	261	233	
					q4	4034	3995	3973	3943	3926	3904	3891	3871	3862	3847	3839	
					q7	4209	4157	4122	4083	4057	4027	4008	3982	3968	3947	3934	
	16 (0.0598")	7-6	9-6	10-0	I	400	400	400	400	400	400	360	318	282	252	225	
					q4	4178	4129	4103	4064	4046	4014	4000	3975	3965	3944	3936	
					q7	4394	4328	4286	4237	4207	4167	4144	4112	4094	4068	4053	

Notes:

1. ALLOWABLE SUPERIMPOSED VERTICAL LOAD (psf) FOR SIMPLY SUPPORTED DECK SECTIONS. THESE LOADS MAY BE USED WITH STEEL FLOOR UNITS ON SIMPLE OR CONTINUOUS SPANS. COMPOSITE SLAB DESIGN TO BE BASED ON SIMPLE SPAN ANALYSIS. THE CONCRETE SHALL BE REINFORCED IN ACCORDANCE APPLICABLE CODE REQUIREMENTS.
2. THE DIAPHRAGM SHEAR VALUES (plf) LISTED IN THE TABLES ARE FOR STEEL DECKS WITH FOUR PUDDLE WELDS PER PANEL JOINT, SUBJECT TO SEISMIC LOADS OR COMBINATIONS OF LOADS WHICH INCLUDE SEISMIC LOADS WHICH ARE WELDED TO SUPPORTING MEMBERS AND WELDED OR MECHANICALLY FASTENED TOGETHER TO FORM A ROOF OR FLOOR DIAPHRAGM. THESE VALUES MEET THE FACTOR OF SAFETY REQUIREMENTS LISTED IN 2007 AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, TABLE D5. THE FACTOR HAS BEEN UNIFORMLY APPLIED TO ALL VALUES AS 3.00.
3. SUFFICIENT SEAM ATTACHMENT FOR COMPOSITE DECK SECTIONS CONSIST OF FASTENING AT A 30" ON CENTER MAXIMUM, BETWEEN SUPPORTS. FASTENING MAY BE #10 SELF PIERCING/TAPPING SCREW BY 3/4" LONG, WELDS 1 1/2" LONG, OR BUTTON PUNCHES. ATTACHMENTS ARE ILLUSTRATED IN THE WELDING SCHEDULE FIGURE.
4. THE SPACING OF BOUNDARY SPOT WELDS AT PERIMETER SUPPORTS SHALL BE DETERMINED IN ACCORDANCE WITH THE VALUES SET FORTH IN THE WELDING SCHEDULE TABLE.
5. FOR DECK WITH NO TRENCH HEADER AND WITH CONCRETE FILL, THE FLEXIBILITY FACTOR MAY BE TAKEN AS 1.

2DW Light Weight Concrete

TOTAL SLAB DEPTH & CONCRETE WEIGHT (psf)	GAUGE	MAX UNSHORED SPAN (FEET - INCHES)			(l) Total Allowable Superimposed Load (psf) (q) DIAPHRAGM SHEAR VALUES (plf) - 4 ENDS WELDS/SHEET																											
		SINGLE	DOUBLE	THREE OR MORE	SPAN (FEET)																											
					7'-0"	7'-6"	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"	12'-0"	12'-6"	13'-0"															
4" (27.5 psf)	22 (0.0299")	8-0	10-3	10-0	l	264	227	192	172	151	133	118	105	94	84	75	68	61	q	1348	1327	1321	1304	1299	1284	1281	1269	1266	1263	1260	1254	1248
	20 (0.0359")	9-3	11-3	10-6	l	312	269	234	205	180	159	142	127	113	102	92	83	75	q	1409	1385	1375	1356	1349	1333	1329	1314	1312	1300	1295	1290	1284
	18 (0.0478")	10-9	12-6	11-6	l	400	348	303	266	235	209	186	167	150	135	122	111	101	q	1522	1490	1479	1453	1444	1424	1418	1400	1395	1389	1382	1376	1370
	16 (0.0598")	13-6	13-6	12-6	l	400	400	364	320	283	252	225	202	182	164	149	136	124	q	1642	1602	1589	1556	1546	1520	1512	1490	1485	1480	1473	1465	1459
4 1/2" (32.1 psf)	22 (0.0299")	8-0	9-6	10-0	l	320	276	239	209	184	162	144	128	114	103	92	83	75	q	1608	1587	1580	1563	1557	1544	1541	1529	1526	1516	1509	1500	1492
	20 (0.0359")	9-0	10-6	11-0	l	379	327	284	249	219	194	173	154	138	124	112	101	92	q	1668	1643	1634	1615	1608	1593	1587	1573	1570	1559	1550	1541	1533
	18 (0.0478")	10-6	12-6	12-6	l	400	400	369	324	286	254	227	203	183	165	150	136	124	q	1782	1750	1738	1712	1704	1682	1677	1659	1655	1639	1630	1621	1612
	16 (0.0598")	11-6	13-0	13-6	l	400	400	400	390	345	307	275	247	222	201	183	166	152	q	1902	1862	1847	1815	1806	1778	1772	1749	1743	1724	1712	1699	1687
5 1/4" (39.0 psf)	22 (0.0299")	7-6	9-0	9-0	l	400	353	306	268	236	208	185	165	148	132	119	107	97	q	1997	1976	1968	1951	1946	1933	1929	1918	1915	1905	1902	1894	1893
	20 (0.0359")	8-6	10-0	10-6	l	400	400	365	320	282	250	222	199	178	161	145	131	119	q	2057	2032	2023	2003	1997	1981	1976	1963	1959	1947	1945	1934	1933
	18 (0.0478")	10-0	12-0	12-0	l	400	400	400	400	369	328	293	263	237	214	194	176	161	q	2171	2139	2127	2101	2093	2071	2060	2048	2044	2028	2025	2011	2010
	16 (0.0598")	11-0	13-0	13-0	l	400	400	400	400	400	397	355	319	288	261	237	216	197	q	2291	2250	2236	2204	2194	2167	2161	2137	2132	2114	2110	2093	2090
6 1/4" (48.2 psf)	22 (0.0299")	7-0	8-0	8-6	l	400	400	400	351	309	273	243	217	194	175	157	142	128	q	2500	2495	2487	2470	2465	2452	2448	2436	2434	2423	2421	2413	2412
	20 (0.0359")	7-6	9-0	9-6	l	400	400	400	400	370	328	292	262	235	212	192	174	158	q	2560	2551	2542	2522	2516	2500	2495	2480	2478	2466	2464	2453	2452
	18 (0.0478")	9-0	11-0	11-6	l	400	400	400	400	400	400	387	347	313	283	257	234	213	q	2668	2657	2646	2620	2612	2590	2584	2566	2562	2547	2544	2530	2527
	16 (0.0598")	10-0	12-0	12-0	l	400	400	400	400	400	400	400	400	400	382	346	315	287	263	q	2783	2769	2755	2722	2713	2686	2679	2656	2651	2631	2629	2612

SEE 2DW NORMAL WEIGHT TABLE NOTES- Page 25

2DW Normal Weight Concrete

TOTAL SLAB DEPTH & CONCRETE WEIGHT (psf)	GAUGE	MAX UNSHORED SPAN (FEET - INCHES)			(l) Total Allowable Superimposed Load (psf) (q) DIAPHRAGM SHEAR VALUES (plf) - 4 ENDS WELDS/SHEET										
					SPAN (FEET)										
		SINGLE	DOUBLE	THREE OR MORE		7'-0"	7'-6"	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"
4" (36.3 psf)	22 (0.0299")	7-6	9-6	9-3	l	271	232	201	175	153	134	118	105	93	82
					q	1881	1860	1853	1836	1830	1817	1814	1802	1799	1790
	20 (0.0359")	8-6	10-3	10-0	l	323	278	240	210	184	162	144	128	114	101
					q	1941	1916	1907	1888	1881	1866	1860	1847	1843	1837
	18 (0.0478")	10-0	11-9	10-9	l	400	363	316	276	243	215	191	171	153	138
					q	2055	2023	2011	1985	1977	1955	1950	1932	1928	1921
	16 (0.0598")	12-3	12-6	11-6	l	400	400	382	335	296	262	234	209	188	170
					q	2175	2135	2120	2088	2079	2051	2045	2022	2016	2009
4 1/2" (42.3 psf)	22 (0.0299")	7-0	8-6	9-0	l	328	281	243	212	185	163	144	127	113	101
					q	2274	2253	2245	2228	2223	2209	2206	2194	2192	2181
	20 (0.0359")	8-0	9-6	10-0	l	391	336	292	255	223	197	175	155	139	124
					q	2334	2309	2300	2280	2274	2257	2253	2239	2236	2224
	18 (0.0478")	9-6	11-6	12-0	l	400	400	383	336	296	262	233	208	187	168
					q	2448	2415	2404	2378	2370	2348	2343	2324	2321	2305
	16 (0.0598")	10-6	12-0	12-6	l	400	400	400	400	360	320	285	255	230	207
					q	2568	2527	2513	2480	2471	2444	2438	2414	2409	2389
5" (48.4 psf)	22 (0.0299")	7-0	8-0	8-6	l	387	333	288	251	219	193	171	151	134	120
					q	2665	2644	2638	2621	2616	2601	2597	2586	2583	2574
	20 (0.0359")	7-6	9-0	9-6	l	400	398	345	302	265	234	207	185	165	148
					q	2726	2701	2692	2673	2666	2649	2646	2631	2629	2617
	18 (0.0478")	9-0	11-0	11-6	l	400	400	400	398	351	311	277	248	222	200
					q	2839	2808	2796	2270	2763	2740	2735	2717	2713	2698
	16 (0.0598")	10-0	12-0	12-0	l	400	400	400	400	400	380	339	304	274	247
					q	2959	2919	2906	2873	2863	2837	2829	2807	2802	2782
5 1/2" (54.4 psf)	22 (0.0299")	6-6	7-6	8-0	l	400	386	334	291	255	224	198	176	157	140
					q	3058	3037	3030	3013	3008	2994	2990	2978	2976	2965
	20 (0.0359")	7-6	9-0	9-0	l	400	400	400	350	308	272	241	215	192	172
					q	3119	3094	3085	3065	3059	3042	3038	3024	3020	3008
	18 (0.0478")	8-6	10-6	11-0	l	400	400	400	400	400	362	323	289	259	234
					q	3232	3199	3189	3163	3154	3133	3127	3110	3104	3089
	16 (0.0598")	9-6	12-0	12-0	l	400	400	400	400	400	400	396	355	320	289
					q	3351	3311	3298	3266	3255	3229	3221	3199	3194	3175

SEE 2DW NORMAL WEIGHT TABLE NOTES- Page 25

2DW Normal Weight Concrete - Continued

TOTAL SLAB DEPTH & CONCRETE WEIGHT (psf)	GAUGE	MAX UNSHORED SPAN (FEET - INCHES)			(l) Total Allowable Superimposed Load (psf) (q) DIAPHRAGM SHEAR VALUES (plf) - 4 ENDS WELDS/SHEET										
		SINGLE	DOUBLE	THREE OR MORE	SPAN (FEET)										
					7'-0"	7'-6"	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"	
6" (60.4 psf)	22	6-0	7-0	7-6	l	400	400	381	332	291	257	227	202	179	160
					q	3437	3429	3422	3406	3400	3387	3383	3371	3368	3358
	20	7-0	8-6	9-0	l	400	400	400	400	352	311	276	246	220	197
					q	3493	3485	3478	3457	3452	3435	3429	3416	3413	3401
	18	8-0	10-0	10-6	l	400	400	400	400	400	400	370	331	298	268
					q	3599	3592	3580	3554	3546	3526	3519	3501	3497	3481
	16	9-0	11-0	11-6	l	400	400	400	400	400	400	400	400	367	332
					q	3712	3704	3689	3657	3648	3621	3614	3592	3587	3567
6 1/2" (66.4 psf)	22	6-0	6-6	7-0	l	400	400	400	374	328	289	256	227	203	181
					q	3830	3822	3814	3797	3792	3779	3775	3764	3761	3751
	20	7-0	8-0	8-6	l	400	400	400	400	397	351	223	246	220	197
					q	3885	3878	3869	3849	3843	3827	3822	3809	3805	3793
	18	8-0	9-6	10-0	l	400	400	400	400	400	400	400	374	336	303
					q	3992	3985	3973	3974	3939	3918	3912	3894	3890	3874
	16	9-0	11-0	11-0	l	400	400	400	400	400	400	400	400	400	376
					q	4104	4096	4082	4050	4040	4013	4007	3983	3979	3960
7 1/4" (75.4 psf)	22	5-6	6-0	6-6	l	400	400	400	400	384	339	300	267	238	213
					q	4420	4411	4403	4386	4381	4367	4363	4352	4349	4339
	20	6-6	7-6	8-0	l	400	400	400	400	400	400	366	326	292	262
					q	4473	4467	4458	4438	4432	4415	4411	4397	4394	4380
	18	7-6	9-0	9-6	l	400	400	400	400	400	400	400	400	396	357
					q	4580	4573	4562	4536	4528	4506	4501	4482	4479	4463
	16	8-6	10-0	10-6	l	400	400	400	400	400	400	400	400	400	400
					q	4691	4685	4671	4638	4628	4602	4594	4572	4567	4547

NOTES:

1. ALLOWABLE SUPERIMPOSED VERTICAL LOAD (psf) FOR SIMPLY SUPPORTED DECK SECTIONS. THESE LOADS MAY BE USED WITH STEEL FLOOR UNITS ON SIMPLE OR CONTINUOUS SPANS. COMPOSITE SLAB DESIGN TO BE BASED ON SIMPLE SPAN ANALYSIS. THE CONCRETE SHALL BE REINFORCED IN ACCORDANCE APPLICABLE CODE REQUIREMENTS.
2. THE DIAPHRAGM SHEAR VALUES (plf) LISTED IN THE TABLES ARE FOR STEEL DECKS WITH FOUR PUDDLE WELDS PER PANEL JOINT, SUBJECT TO SEISMIC LOADS OR COMBINATIONS OF LOADS WHICH INCLUDE SEISMIC LOADS WHICH ARE WELDED TO SUPPORTING MEMBERS AND WELDED OR MECHANICALLY FASTENED TOGETHER TO FORM A ROOF OR FLOOR DIAPHRAGM. THESE VALUES MEET THE FACTOR OF SAFETY REQUIREMENTS LISTED IN 2007 AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, TABLE D5. THE FACTOR HAS BEEN UNIFORMLY APPLIED TO ALL VALUES AS 3.00.
3. SUFFICIENT SEAM ATTACHMENT FOR COMPOSITE DECK SECTIONS CONSIST OF FASTENING AT A 30" ON CENTER MAXIMUM, BETWEEN SUPPORTS. FASTENING MAY BE #10 SELF PIERCING/TAPPING SCREW BY 3/4" LONG, WELDS 1 1/2" LONG, OR BUTTON PUNCHES. ATTACHMENTS ARE ILLUSTRATED IN THE WELDING SCHEDULE FIGURE.
4. THE SPACING OF BOUNDARY SPOT WELDS AT PERIMETER SUPPORTS SHALL BE DETERMINED IN ACCORDANCE WITH THE VALUES SET FORTH IN THE WELDING SCHEDULE TABLE.
5. FOR DECK WITH NO TRENCH HEADER AND WITH CONCRETE FILL, THE FLEXIBILITY FACTOR MAY BE TAKEN AS 1.

3DW Light Weight Concrete

TOTAL SLAB DEPTH & CONCRETE WEIGHT (psf)	GAUGE	MAX UNSHORED SPAN (FEET - INCHES)			(l) Total Allowable Superimposed Load (psf) (q) DIAPHRAGM SHEAR VALUES (plf) - 4 ENDS WELDS/SHEET																																	
		SINGLE	DOUBLE	THREE OR MORE	SPAN (FEET)																																	
					8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"	12'-0"	12'-6"	13'-0"	13'-6"	14'-0"	14'-6"	15'-0"																			
5 (32.1 psf)	22 (0.0299")	9-9	10-9	11-3	l	248	217	190	168	149	133	119	107	96	86	78	71	64	58	52	q	1321	1304	1299	1284	1281	1269	1266	1256	1245	1244	-	-	-	-			
	20 (0.0359")	11-6	14-3	13-3	l	297	260	229	203	181	162	145	131	118	107	97	88	80	73	66	q	1375	1356	1349	1333	1329	1314	1312	1299	1297	1287	1284	-	-	-	-		
	18 (0.0478")	14-3	15-9	14-6	l	389	342	302	269	240	215	194	175	159	144	131	120	110	100	92	q	1479	1453	1444	1424	1418	1400	1395	1379	1377	1364	1361	-	-	-	-		
	16 (0.0598")	16-3	17-0	15-9	l	400	400	367	326	292	262	237	214	195	177	162	148	136	125	115	q	1589	1556	1546	1520	1512	1490	1485	1465	1461	1444	1442	-	-	-	-		
5 1/2" (36.7 psf)	22 (0.0299")	9-3	10-0	10-3	l	291	254	224	198	176	157	140	126	113	102	92	83	76	68	62	q	1580	1563	1557	1544	1541	1529	1526	1516	1513	1505	1504	1496	1495	-	-		
	20 (0.0359")	10-9	13-9	12-9	l	348	305	269	238	212	190	170	153	139	125	114	103	94	86	78	q	1634	1615	1608	1593	1587	1573	1570	1559	1556	1546	1544	1535	1534	-	-	-	
	18 (0.0478")	13-6	15-3	14-0	l	400	400	354	314	281	252	227	205	186	169	154	141	129	118	108	q	1738	1712	1704	1682	1677	1659	1655	1639	1637	1622	1620	1608	1607	-	-	-	
	16 (0.0598")	15-6	16-3	15-3	l	400	400	400	385	345	310	280	253	230	210	192	176	161	148	136	q	1847	1815	1806	1778	1772	1749	1743	1724	1721	1704	1702	1686	1685	-	-	-	
6 1/4" (43.5 psf)	22 (0.0299")	8-6	8-9	9-3	l	360	315	277	246	218	195	175	157	141	127	115	104	95	86	78	q	1956	1951	1946	1933	1929	1918	1915	1905	1902	1894	1893	1885	1884	1877	-	-	
	20 (0.0359")	10-0	12-3	12-0	l	400	378	333	295	263	236	212	191	172	156	142	129	118	107	98	q	2008	2003	1997	1981	1976	1963	1959	1947	1945	1934	1933	1924	1923	1915	1915	-	-
	18 (0.0478")	12-6	14-6	13-3	l	400	400	400	389	348	312	281	255	231	210	192	175	160	147	135	q	2107	2101	2093	2074	2066	2048	2047	2028	2025	2011	2010	1997	1996	1985	1985	-	-
	16 (0.0598")	14-6	15-6	14-3	l	400	400	400	400	400	384	347	314	286	261	238	219	201	185	170	q	2210	2204	2194	2167	2161	2137	2132	2114	2110	2093	2090	2075	2074	2061	2061	-	-
7 1/4" (52.7 psf)	22 (0.0299")	8-0	7-9	8-3	l	400	400	354	314	279	249	224	201	181	164	149	135	122	111	101	q	2476	2470	2465	2452	2448	2436	2434	2423	2421	2413	2412	2404	2402	2396	2396	2395	
	20 (0.0359")	9-3	10-9	11-3	l	400	400	400	377	336	301	271	244	221	201	182	166	152	138	127	q	2531	2522	2516	2500	2495	2480	2478	2466	2464	2453	2452	2443	2441	2432	2432	2432	
	18 (0.0478")	11-6	13-6	12-6	l	400	400	400	400	400	400	360	326	296	270	246	225	207	190	175	q	2629	2620	2612	2590	2584	2566	2562	2547	2544	2530	2527	2516	2514	2504	2504	2503	
	16 (0.0598")	13-6	14-6	13-6	l	400	400	400	400	400	400	400	400	400	367	335	307	281	259	238	220	q	2733	2720	2713	2686	2679	2656	2651	2631	2629	2612	2609	2594	2592	2579	2578	2578

SEE 3DW NORMAL WEIGHT TABLE NOTES- Page 28

3DW Normal Weight Concrete

TOTAL SLAB DEPTH & CONCRETE WEIGHT (psf)	GAUGE	MAX UNSHORED SPAN (FEET - INCHES)			(l) Total Allowable Superimposed Load (psf) (q) DIAPHRAGM SHEAR VALUES (plf) - 4 ENDS WELDS/SHEET												
					SPAN (FEET)												
		SINGLE	DOUBLE	THREE OR MORE		8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"	12'-0"	12'-6"	13'-0"	13'-6"
5 (42.3 psf)	22 (0.0299")	8-9	9-3	9-9	l	252	220	193	169	150	133	118	105	94	84	75	67
					q	1853	1836	1830	1817	1814	1802	1799	1789	1786	1778	1777	-
	20 (0.0359")	10-3	12-9	12-6	l	305	266	234	206	183	163	146	130	117	105	95	85
					q	1907	1888	1881	1860	1847	1843	1832	1829	1819	1817	-	-
	18 (0.0478")	13-0	14-9	13-6	l	400	353	312	276	246	220	197	178	160	145	132	120
					q	2011	1985	1977	1955	1950	1932	1928	1912	1910	1895	1894	-
	16 (0.0598")	15-0	15-9	14-6	l	400	400	385	342	305	273	246	222	201	183	166	152
					q	2120	2088	2079	2051	2045	2022	2016	1998	1994	1977	1975	-
5 1/2" (48.4 psf)	22 (0.0299")	8-6	8-6	9-0	l	296	258	226	199	176	156	139	124	111	99	89	80
					q	2245	2228	2223	2209	2206	2194	2192	2181	2179	2171	2168	2162
	20 (0.0359")	9-9	11-9	11-9	l	357	312	274	242	215	191	171	153	138	124	112	101
					q	2300	2280	2274	2257	2253	2239	2236	2224	2222	2211	2210	2201
	18 (0.0478")	12-3	14-3	13-3	l	400	400	365	324	288	258	232	209	188	171	155	141
					q	2404	2378	2370	2348	2343	2324	2321	2305	2301	2288	2285	2274
	16 (0.0598")	14-3	15-3	14-0	l	400	400	400	400	357	320	289	261	236	215	196	179
					q	2513	2480	2471	2444	2438	2414	2409	2389	2387	2370	2367	2352
6" (54.4 psf)	22 (0.0299")	8-0	8-0	8-0	l	343	299	262	231	204	181	161	144	129	115	104	93
					q	2638	2621	2616	2601	2597	2586	2583	2574	2571	2562	2561	2553
	20 (0.0359")	9-3	10-9	11-3	l	400	261	317	280	249	222	198	178	160	144	130	117
					q	2692	2673	2666	2649	2646	2631	2629	2617	2614	2604	2601	2592
	18 (0.0478")	11-6	13-6	12-6	l	400	400	400	374	333	298	268	242	218	198	180	164
					q	2796	2770	2763	2740	2735	2717	2713	2698	2694	2681	2678	2666
	16 (0.0598")	13-6	14-6	13-6	l	400	400	400	400	400	371	334	302	274	249	227	207
					q	2906	2873	2863	2837	2829	2807	2802	2782	2778	2761	2759	2744
6 1/2" (60.4 psf)	22 (0.0299")	7-9	7-3	7-6	l	391	341	299	264	233	207	185	165	148	132	119	107
					q	3030	3013	3008	2994	2990	2978	2976	2965	2964	2955	2954	2946
	20 (0.0359")	9-0	10-3	10-6	l	400	400	362	320	284	253	227	203	183	165	149	135
					q	3085	3065	3059	3042	3038	3024	3020	3008	3007	2997	2994	2985
	18 (0.0478")	11-3	13-3	12-3	l	400	400	400	400	381	341	306	276	250	227	206	187
					q	3189	3163	3154	3133	3127	3110	3104	3089	3086	3073	3071	3059
	16 (0.0598")	12-9	14-3	13-3	l	400	400	400	400	400	400	382	346	313	285	260	238
					q	3298	3266	3255	3229	3221	3199	3194	3175	3171	3154	3151	3137

SEE 3DW NORMAL WEIGHT TABLE NOTES- Page 28

3DW Normal Weight Concrete-Continued

TOTAL SLAB DEPTH & CONCRETE WEIGHT (psf)	GAUGE	MAX UNSHORED SPAN (FEET - INCHES)			(l) Total Allowable Superimposed Load (psf) (q) DIAPHRAGM SHEAR VALUES (plf) - 4 ENDS WELDS/SHEET													
					SPAN (FEET)													
		SINGLE	DOUBLE	THREE OR MORE	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"	12'-0"	12'-6"	13'-0"	13'-6"		
7 (66.4 psf)	22 (0.0299")	7-6	6-9	7-0	l q	400 3422	384 3406	337 3400	297 3387	263 3383	234 3371	209 3368	187 3358	167 3357	150 3348	135 3346	121 3338	
	20 (0.0359")	8-9	9-6	9-6	l q	400 3478	400 3457	400 3452	361 3435	321 3429	286 3416	256 3413	230 3401	207 3398	187 3388	169 3387	153 3377	
	18 (0.0478")	10-9	12-9	11-0	l q	400 3580	400 3554	400 3546	400 3526	400 3519	385 3501	346 3497	312 3481	283 3479	256 3465	233 3463	212 3452	
	16 (0.0598")	12-6	13-9	12-9	l q	400 3689	400 3657	400 3648	400 3621	400 3614	400 3592	400 3587	391 3567	355 3563	323 3546	294 3544	269 3528	
7 1/2" (72.5 psf)	22 (0.0299")	7-3	6-3	6-6	l q	400 3806	400 3797	377 3792	332 3779	294 3775	262 3764	234 3761	209 3751	187 3749	168 3740	151 3739	136 3601	
	20 (0.0359")	8-6	8-9	9-3	l q	400 3855	400 3849	400 3843	400 3827	358 3822	320 3809	287 3805	257 3793	232 3791	209 3780	189 3779	171 3770	
	18 (0.0478")	10-6	12-6	11-6	l q	400 3958	400 3947	400 3939	400 3918	400 3912	400 3894	387 3890	349 3874	316 3871	287 3857	261 3856	238 3843	
	16 (0.0598")	12-3	13-6	12-6	l q	400 4062	400 4050	400 4040	400 4013	400 4007	400 3983	400 3979	400 3960	397 3956	362 3939	330 3936	302 3921	
8 1/4" (81.4 psf)	22 (0.0299")	7-0	6-0	6-0	l q	400 4398	400 4386	400 4381	386 4367	342 4363	304 4352	272 4349	243 4339	218 4337	196 4329	177 4326	159 4320	
	20 (0.0359")	8-3	8-0	8-3	l q	400 4456	400 4438	400 4435	400 4418	400 4411	372 4397	333 4394	300 4382	270 4380	244 4369	221 4368	200 4358	
	18 (0.0478")	10-3	12-0	11-3	l q	400 4548	400 4536	400 4528	400 4506	400 4501	400 4482	400 4479	400 4463	368 4459	335 4446	305 4443	278 4432	
	16 (0.0598")	11-9	13-0	12-0	l q	400 4650	400 4638	400 4628	400 4602	400 4594	400 4572	400 4567	400 4547	400 4545	400 4527	385 4525	353 4510	

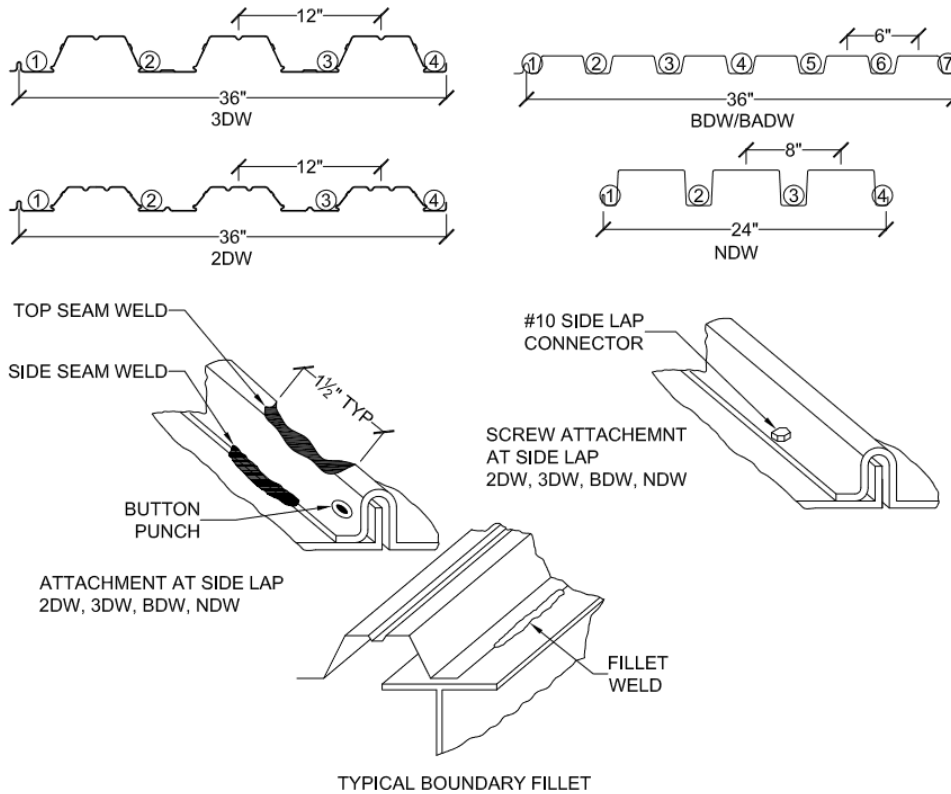
Notes:

1. ALLOWABLE SUPERIMPOSED VERTICAL LOAD (psf) FOR SIMPLY SUPPORTED DECK SECTIONS. THESE LOADS MAY BE USED WITH STEEL FLOOR UNITS ON SIMPLE OR CONTINUOUS SPANS. COMPOSITE SLAB DESIGN TO BE BASED ON SIMPLE SPAN ANALYSIS. THE CONCRETE SHALL BE REINFORCED IN ACCORDANCE APPLICABLE CODE REQUIREMENTS.
2. THE DIAPHRAGM SHEAR VALUES (plf) LISTED IN THE TABLES ARE FOR STEEL DECKS WITH FOUR PUDDLE WELDS PER PANEL JOINT, SUBJECT TO SEISMIC LOADS OR COMBINATIONS OF LOADS WHICH INCLUDE SEISMIC LOADS WHICH ARE WELDED TO SUPPORTING MEMBERS AND WELDED OR MECHANICALLY FASTENED TOGETHER TO FORM A ROOF OR FLOOR DIAPHRAGM. THESE VALUES MEET THE FACTOR OF SAFETY REQUIREMENTS LISTED IN 2007 AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, TABLE D5. THE FACTOR HAS BEEN UNIFORMLY APPLIED TO ALL VALUES AS 3.00.
3. SUFFICIENT SEAM ATTACHMENT FOR COMPOSITE DECK SECTIONS CONSIST OF FASTENING AT A 30" ON CENTER MAXIMUM, BETWEEN SUPPORTS. FASTENING MAY BE #10 SELF PIERCING/TAPPING SCREW BY 3/4" LONG, WELDS 1 1/2" LONG, OR BUTTON PUNCHES. ATTACHMENTS ARE ILLUSTRATED IN THE WELDING SCHEDULE FIGURE.
4. THE SPACING OF BOUNDARY SPOT WELDS AT PERIMETER SUPPORTS SHALL BE DETERMINED IN ACCORDANCE WITH THE VALUES SET FORTH IN THE WELDING SCHEDULE TABLE.
5. FOR DECK WITH NO TRENCH HEADER AND WITH CONCRETE FILL, THE FLEXIBILITY FACTOR MAY BE TAKEN AS 1.

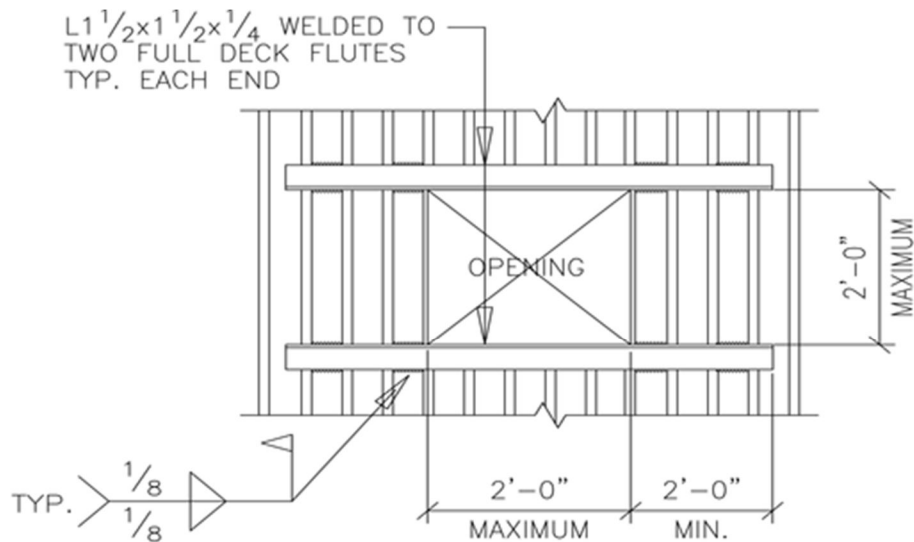
WELD TABLE

DECK TYPE	TRANSVERSE SUPPORTS			PARRALELL TO DECKING		SIDELAP CONNECTION					
	SPOT WELDS			SPOT WELDS		WELD 1 1/2" TOP OR SIDE SEAM	BUTTON PUNCH OR SCREWS				
	EFFECTIVE DIAMETER	# OF WELDS PER UNIT	LOCATIONS	EFFECTIVE DIAMETER	SPACING						
3DW	1/2"	4	1,2,3,4	1/2"	CALCULATE PER BELOW	12, 24, 30	12, 24, 30				
2DW	1/2"	4	1,2,3,4	1/2"	CALCULATE PER BELOW	12, 24, 30	12, 24, 30				
NDW	1/2"	2	1,4	1/2"	CALCULATE PER BELOW	12, 18, 24	12, 18, 24				
BDW	1/2"	7	1,2,3,4,5,6,7	1/2"	CALCULATE PER BELOW	12, 24, 30" MAX FOR COMPOSITE CONSTRUCTION	12, 24, 30" MAX FOR COMPOSITE CONSTRUCTION				
		5	1,2,4,6,7								
BADW	1/2"	7	1,2,3,4,5,6,7	1/2"	CALCULATE PER BELOW	12, 18, 24	12, 18, 24 SCREWS ONLY				
		5	1,3,4,5,7								
NOMINAL SHEAR (q) ON BOUNDARY PUDDLE WELDS (lbs/ft)											
		Qf	SPACING (FEET)								
			6 in	8 in	12 in	16 in	18 in	24 in	30 in	33 in	36
16	0.06	4051	8101	6076	4051	3038	2700	2025	1620	1473	1350
18	0.048	3168	6337	4753	3168	2376	2112	1584	1267	1152	1056
20	0.036	2328	4656	3492	2328	1746	1552	1164	931	847	776
22	0.03	1917	3834	2876	1917	1438	1278	959	767	697	639
NOMINAL SHEAR (q) ON BOUNDARY 1 1/2 INCH FILLET WELDS (lbs/ft)											
		Qf	SPACING (FEET)								
			6 in	8 in	12 in	16 in	18 in	24 in	30 in	33 in	36
16	0.06	3252	6504	4878	3252	2439	2168	1626	1301	1183	1084
18	0.048	2465	4929	3697	2465	1849	1643	1232	986	896	822
20	0.036	1851	3702	2777	1851	1388	1234	926	740	673	617
22	0.03	1542	3083	2313	1542	1156	1028	771	617	561	514

- SEE WELD FIGURE FOR SUPPORT CONNECTIONS AND CONNECTIONS AT SIDELAPS
- FOR STRUCTURAL CONCRETE FILL, SIDE LAP CONNECTIONS MAY BE #10 SELF PIERCING/SELF TAPPING SCREWS, WELDS 1 1/2" LONG OR BUTTON PUNCH AT 30" MAX.
- VALUES ARE BASED ON NORTH AMERICAN STANDARD SPECIFICATION OF COLD-FORMED STEEL 2007.
- TABLES REFLECT NOMINAL STRENGTH, Rn
- ASD ALLOWABLE, R_n / Ω_w , $\Omega_w = 2.20_{\text{PUDDLE WELDS}}$, $\Omega_w = 3.05_{\text{FILLET WELDS}}$
- LRFD, ϕR_n , $\phi = 0.70_{\text{PUDDLE WELDS}}$, $\phi = 0.50_{\text{FILLET WELDS}}$
- FOR ALTERNATE SPACING, CONNECTIONS AND ATTACHMENTS AT PERIMETER/INTERMEDIATE CONNECTIONS
GENERALLY, $SPACING (e) = < 1/\Omega \times (Q_f/S_{ASD})$ OR $\phi \times (Q_f/S_{LRFD})$



WELD FIGURE



NOTE: 1. NO REINFORCING ANGLE IS REQUIRED WHEN OPENING IS LESS THAN 6".

DECK OPENING BEFORE POUR

DECK OPENING

Diaphragm Flexibility

F	MAXIMUM SPAN IN FET FOR MASONRY OR CONCRETE WALLS	SPAN-DEPTH LIMITATION			
		ROTATION NOT CONSIDERED IN DIAPHRAGM		ROTATION CONSIDERED IN DIAPHRAGM	
		MASONRY OR CONCRETE WALLS	FLEXIBLE WALLS	MASONRY OR CONCRETE WALLS	FLEXIBLE WALLS
MORE THAN 150	NOT USED	NOT USED	2:1	NOT USED	1 1/2 :1
70 - 150	200	2:1 OR AS REQUIRED FOR DEFLECTION	3:1	NOT USED	2:1
10 - 70	400	2 1/2:1 OR AS REQUIRED FOR DEFLECTION	4:1	AS REQUIRED FOR DEFLECTION	2 1/2:1
1 - 10	NO LIMITATION	3:1 OR AS REQUIRED FOR DEFLECTION	5:1	AS REQUIRED FOR DEFLECTION	3:1
LESS THAN 1	NO LIMITATION	AS REQUIRED FOR DEFLECTION	NO LIMITATION	AS REQUIRED FOR DEFLECTION	3 1/2:1

1. ROOF DIAPHRAGMS SHALL BE INVESTIGATED REGARDING THEIR FLEXIBILITY AND SUGGESTED SPAN-DEPTH RATIO. REFER TO THE ABOVE TABLES FOR DETERMINATION OF F.
2. WHEN APPLYING THESE LIMITATION TO CANTILEVER DIAPHRAGMS, THE SUGGESTED SPAN-DEPTH RATIO SHALL BE ½ OF THOSE SHOWN IN TABLE.
3. DIAPHRAGM CLASSIFICATION (FLEXIBLE OR RIGID) AND DEFLECTION LIMITS SHALL COMPLY WITH CURRENT CODE.
4. WHEN DIAPHRAGMS ARE SUPPORTING MASONRY OR CONCRETE WALLS, THE MAXIMUM DEFLECTION SHALL BE LIMITED TO **EQUATION A**.
5. THE TOTAL DEFLECTION, Δ OF THE DIAPHRAGM IN INCHES SHALL BE COMPUTED FROM **EQUATION B**.

EQUATION A

$$\Delta_{WALL} = (h^2)(f_c) \times [0.01(E)(t)]^{-1} = \frac{(h)^2(f_c)}{0.01(E)(t)}$$

WHERE

h	=	UNSUPPORTED HEIGHT OF WALL (ft)
t	=	THICKNESS OF WALL, (in)
E	=	MODULUS OF ELASTICITY OF WALL MATERIAL FOR DEFLECTION DETERMINATION, (psi)
f _c	=	ALLOWABLE COMPRESSIVE STRENGTH OF WALL MATERIAL IN FLEXURE, (psi)
		$f_c \quad 0.45 \times f'_c \quad$ (CONCRETE)
		$f_c \quad 0.33 \times f'_m \quad$ (MASONRY)

EQUATION B

$$\Delta = \Delta_f + \Delta_w$$

WHERE

Δ_f = FLEXURAL DEFLECTION OF THE DIAPHRAGM IN INCHES DETERMINED IN THE SAME MANNERS AS DEFLECTION FOR BEAMS, (Md² OF CHORDS)

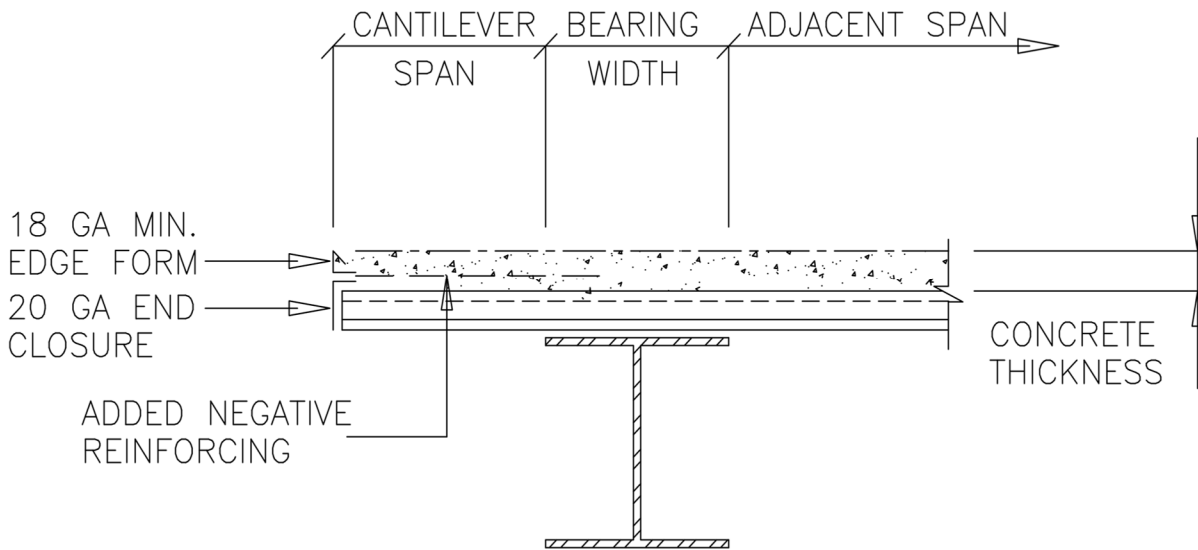
Δ_f = THE WEB DEFLECTION IN INCHES AS DETERMINED BY THE EQUATION, $\frac{(L)(q_{ave})(F)}{10^6}$

L	SPAN BETWEEN POINTS FOR WHICH DEFLECTION IS DETERMINED, (ft)
q _{ave}	AVERAGE SHEAR DEMAND #/ft OVER SPAN OF L
F	FLEXIBILITY FACTOR (MICRO INCHES A DIAPHRAGM WILL DEFLECT ON A SPAN OF 1 ft UNDER A SHEAR LOAD OF 1 #/ft

CANTILEVER SPANS

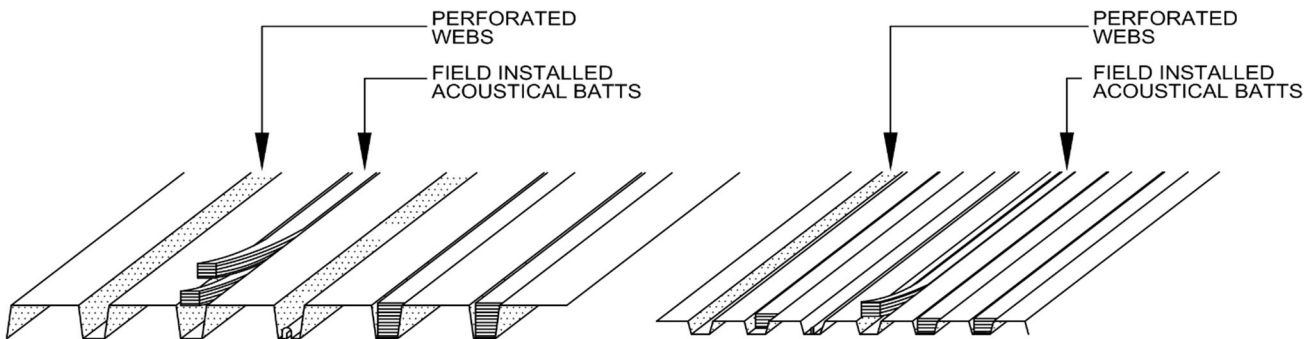
DECK TYPE		CONCRETE THICKNESS					
		2"	2 1/2"	3"	3 1/2"	4"	4 1/2"
BDW	22 (0.0299")	2-0	1-11	1-11	1-10	1-10	1-9
	20 (0.0359")	2-6	2-5	2-4	2-3	2-2	2-2
	18 (0.0478")	3-2	3-1	2-11	2-10	2-9	2-9
	16 (0.0598")	3-9	3-8	3-6	3-5	3-3	3-2
2DW	22 (0.0299")	2-8	2-7	2-6	2-6	2+5	2-4
	20 (0.0359")	3-4	3-2	3-1	3-0	2-11	2-10
	18 (0.0478")	4-4	4-2	4-0	3-11	3-9	3-8
	16 (0.0598")	5-1	4-11	4-9	4-7	4-5	4-3
3DW	22 (0.0299")	3-9	3-7	3-6	3-5	3-4	3-3
	20 (0.0359")	4-6	4-4	4-2	4-1	3-11	3-10
	18 (0.0478")	5-8	5-6	5-3	5-1	4-11	4-10
	16 (0.0598")	6-8	6-4	6-1	5-11	5-9	5-7

1. NEGATIVE REINFORCING STEEL IS REQUIRED AT CANTILEVER AREAS.
2. CANTILEVER SPAN IS BASED ON NORMAL WEIGHT CONCRETE.
3. CANTILEVER SPAN IS NOT TO EXCEED 1/3 OF ADJACENT SPAN.
4. CONCRETE THICKNESS IS FROM TOP OF CONCRETE TO DECK TOP FLUTE
5. TABLE IS PROVIDED AS REFERENCE ONLY AND NOT INTENDED TO REPLACE ENGINEERS REQUIREMENTS



BDW/NDW Acoustical Deck

Where noise is an issue Deck West offers an acoustical decking in both the NDW and BDW profiles. Vertical webs are perforated and low flutes are filled with insulation strips, (field installed) to provide one of the quietest decks available, absorbing up to 65 to 80% of the sound striking the deck.



Acoustical Information

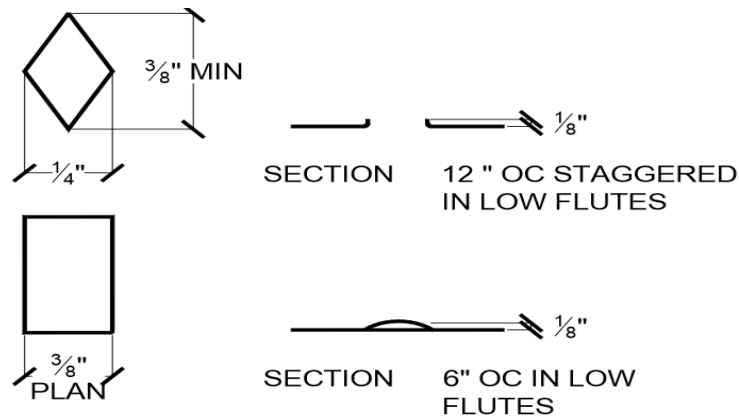
DECK TYPE	ABSORPTION COEFFICIENTS						NRC
	125	250	500	1000	2000	4000	
BDW	0.18	0.56	0.78	0.88	0.39	0.22	0.8
NDW	0.29	0.75	0.88	0.9	0.48	0.29	0.85

NRC DETERMINED BY TESTS IN COMPLIANCE WITH ANSI C423 84a AND ASTM E795 TEST CRITERIA

SECTION MODULUS (S) VALUES ARE UNCHANGED AND MOMENT OF INERTIA (I) VALUES ARE CHANGED LESS THAN 5%

Venting

Venting is often used to provide ventilation for structural concrete and insulating concrete. Decking should be vented when the slab may be exposed to water after construction or covered with a membrane. Venting allows for the dissipation of residual water.



Underwriters' Laboratories, Inc.- Fire Ratings

All of Deck West deck units have been tested by Underwriters Laboratories, Inc. and are tested in a wide variety of designs in the U.L. Fire Resistance Directory. The following is a partial list of Deck West's Floor and ceiling approvals involving only the most economical and popular assemblies.

Fire Resistance – Underwriters' Laboratories Listings



Table of Restraint Ratings

FIRE RATED	U.L. DESIGN #	LISTED	CONCRETE TYPE AND THICKNESS	FIREPROOFING FOR SLAB
1 - HOUR	D743	2DW, 3DW	2" NW/LW	CEMENTITIOUS
	D916	BDW, 2DW, 3DW, INV BDW	3 1/2" NW, 2 1/2" LW	NONE
	D858	2DW, 3DW	2 1/2" NW/LW	FIBER
	D859	2DW, 3DW	2" NW/LW	FIBER
	D902	BDW, 2DW, 3DW, 1 1/2 INV BDW	3 1/2" NW, 2 1/2" LW	NONE
	D914	BADW, BDW, 2DW, 3DW	2 1/2" NW/LW	NONE
2 - HOUR	D712	2DW, 3DW	2 1/2" NW/LW	CEMENTITIOUS
	D722	2DW, 3DW	2 1/2" NW/LW	CEMENTITIOUS
	D739	BDW, 2DW, 3DW	2 1/2" NW/LW	CEMENTITIOUS
	D743	2DW, 3DW	2" NW/LW	CEMENTITIOUS
	D826	BDW, 2DW, 3DW	3 1/4" LW	FIBER
	D832	BDW, 2DW, 3DW	2 1/2" NW/LW	FIBER
	D840	BDW, 2DW, 3DW	3 1/4" LW	NONE
	D858	2DW, 3DW	2 1/2" NW/LW	FIBER
	D859	2DW, 3DW	2" NW/LW	FIBER
	D916	BDW, 2DW, 3DW	4 1/2" NW, 3 1/4" LW	NONE
	D902	BDW, 2DW, 3DW, INV BDW	4 1/2" NW, 3 1/4" LW	NONE
	D907	2DW, 3DW	3 1/4" LW	NONE
3 - HOUR	D703	2DW, 3DW	2 1/2" NW/LW	CEMENTITIOUS
	D708	BDW, 2DW, 3DW	2 1/2" NW/LW	CEMENTITIOUS
	D832	BDW, 2DW, 3DW	2 1/2" NW/LW	FIBER
	D858	2DW, 3DW	2 1/2" NW/LW	FIBER
	D859	2DW, 3DW	2" NW/LW	FIBER
	D916	BDW, 2DW, 3DW	5 1/4" NW, 4 7/16" LW	NONE
	D902	BDW, 2DW, 3DW, INV BDW	5 1/4" NW, 4 7/16" LW	NONE

ALWAYS REFER TO THE LATEST EDITION OF THE U.L. FIRE RESISTANCE INDEX AND/OR LATEST INDIVIDUAL DESIGN CARDS FOR FIREPROOFING THICKNESS' AND/OR OTHER REQUIREMENTS.

Factory Mutual Approvals

Deck West 1 ½" BDW and 3" NDW decks are Factory Mutual (FM) approved as metal roof decks under FM class 1 fire and I-90 windstorm rating. See the table below for FM approved spans.



DECK TYPE	DECK GAUGE			
	22	20	18	16
BDW	5'-9"	6'-6"	7'-9"	9'-0"
NDW	x'-xx"	x'-xx"	x'-xx"	x'-xx"

Concrete Volumes

VOLUME OF CONCRETE, CUBIC FEET PER AREA (ft ³ /ft ²)								
DECK TYPE	THICKNESS OVER TOP OF DECK							
	2"	2 1/2"	3"	3 1/4"	3 1/2"	4"	4 3/16"	4 1/2"
BDW	-	0.255	0.297	0.318	0.339	0.38	0.396	0.422
2DW	0.25	0.292	0.333	0.354	0.375	0.417	0.432	0.458
3DW	0.292	0.333	0.375	0.396	0.417	0.458	0.474	0.5
NDW	-	0.279	0.32	0.314	0.362	0.404	0.419	0.445
FLAT SLAB	0.166	0.208	0.25	0.271	0.292	0.333	0.349	0.375

1. THE CONCRETE VOLUMES LISTED ARE BASED ON THE DEPTHS SHOWN. NO ALLOWANCE HAS BEEN MADE FOR FRAME OR DECK DEFLECTION.
2. TO CALCULATE VOLUMES OF SLABS THICKER THAN SHOWN IN TABLE, ADD VOLUME OF FLAT SLAB OF SAME THICKNESS AS THE DIFFERENCE BETWEEN THE THICKNESS OF SLAB DESIRED AND ON GIVEN IN TABLE

Structural Concrete Minimum Reinforcing

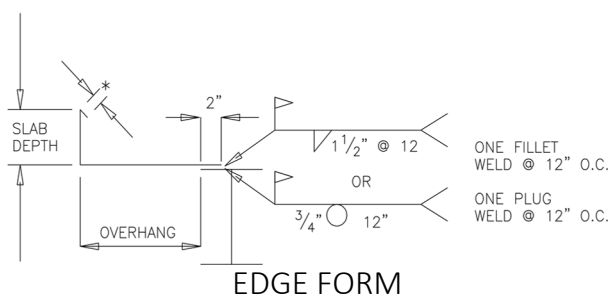
SLAB THICKNESS (INCHES ABOVE DECK TOP FLUTE)	6 BY 6 WELDED WIRE FABRIC (WIRE SIZE)
3 1/4" OR LESS	6x6 W1.4xW1.4
3 1/2	6x6 W1.7x1.7 OR 6x6 W2.1xW2.1
4	6x6 W2.1x2.1
4 1/2	6x6 W2.5x2.5 OR 6x6 W2.9xW2.9
5	6x6 W2.9xW2.9
5 1/2	6x6 W2.9xW2.9
6	6x6 W3.4xW3.4 OR 6x6 W/D4xW/D4

1. A 6x6 WELDED WIRE FABRIC SHRINKAGE MESH, LOCATED 1" FROM THE TOP OF THE CONCRETE SLAB, IS RECOMMENDED FOR ALL SLAB THICKNESSES. APPLY TO BOTH STONE AND LIGHTWEIGHT CONCRETE.
2. OTHER REINFORCEMENT HAVING AN EQUIVALENT AREA AND MAXIMUM SPACING OF 18" MAY BE SUBMITTED.
3. FIBERMESH CAN BE USED AS AN ALTERNATE TO WELDED WIRE FABRIC, SEE ICBO # 4811.

Edge Forms

SLAB DEPTH	OVERHANG INCHES										
	0	1	2	3	4	5	6	7	8	9	10
4"	20	20	20	20	18	18	16	14	12	12	12
4 1/2"	20	20	20	18	18	16	16	14	12	12	12
5"	20	20	18	18	16	16	14	14	12	12	10
5 1/2"	20	18	18	16	16	14	14	12	12	12	10
6	18	18	16	16	14	14	12	12	12	10	10
6 1/2"	18	16	16	14	14	12	12	12	12	10	10
7	16	16	14	14	12	12	12	12	10	10	10
7 1/2"	16	16	14	12	12	12	12	10	10	10	1/4 PLATE

1. 1/2" RETURN LIP REQUIRED FOR 16 GAUGE AND LIGHTER FORM.
2. TABLES BASED ON NORMAL WEIGHT CONCRETE WITH 1/2" MAXIMUM HORIZONTAL AND VERTICAL DEFLECTIONS.
3. TABLE IS PROVIDED AS REFERENCE ONLY AND NOT INTENDED TO REPLACE THE ENGINEERS REQUIREMENTS.



GENERAL NOTES

The following notes apply to the load tables and diaphragm shear tables.

1. Section properties have been computed in accordance with the American Iron and Steel Institute (AISI), North American Specification for the Design of Cold-Formed Steel Structural Members. All materials are subject to AISI tolerances. Weights shown are approximations for design purposes.
2. Arch spot or arc seam (puddle welds) shall have an effective fusion area, to supporting members equivalent to at least 3/8"x1" or 1/2" in diameter. Arch spot welds may be eliminated where they coincide with shear studs.
3. Deck should bear on support a minimum of 2" at perpendicular bearing and 1/2" minimum at parallel bearing conditions.
4. Seam attachment for composite deck profiles to be a minimum button punch at 30" OC or a #10 side lap "stitch" screw.
5. The minimum 28 day compressive strength for structural concrete shall be 3000 psi. The minimum depth of concrete shall be 2" over the top flute and is to be reinforced with a minimum 6x6 W1.4xW1.4 welded wire fabric or fibermesh.
6. All decks with structural concrete fill may be considered rigid diaphragms, flexibility $f < 1.0$.
7. Seam welds for non-composite profiles are to be a minimum of 1 1/2" long.
8. Where individual panels are cut, the partial panel shall be fasted as to fully transfer the shear at the point of the diaphragm to the adjacent full panels for the values specified in the tables.
9. No suspended ceiling, light fixtures, ducts, or utilities shall be supported by the metal deck without approval from the Engineer of Record (EOR)
10. Minimum E60XX or E70XX filler metal complying with the appropriate AWS standard must be used. Weld requirements must comply with the American Welding Society (AWS) D1.3. Prior to proceeding with the welding the welder shall demonstrate his/her ability to produce the prescribed weld satisfactory to the welding inspector.
11. Diaphragm shear values are the working values and are not to be increased for short duration lateral loading.
12. Typical deck manufacturing tolerances:
Panel length $\pm 1/2"$
Panel Width $-3/8" +3/4"$
Panel camber or sweep $1/4"$ per 10' length
Panel end square $1/8"$ per foot of panel width
13. Weld washers are not required for fastening Deck West products.
14. Where underside areas of floors or roofs will be left exposed, deck of 20 ga or heavier should be considered. Construction traffic, where excessive, can cause unsightly conditions that result in customer dissatisfaction. As a further precaution, design drawings should advise the contractor of exposed underside areas, on the structural drawing so that suitable precautions can be taken.
15. Admixtures containing chlorides must not be used with galvanized deck.
16. End labs of BDW and BDW+SHEARCORR should be lapped a minimum of 2" at interior supports. Composite decks should be butted to allow for stud welding.
17. Alternative Fastenign Methods:

For attachment of decking other than welding refer to the following technical information
Steel Deck Institute Dipharagm Design Manual DDM03

<u>ICC-ES Report #</u>	<u>Company</u>
2197	HILTI Fastening systems
2776	HILTI Fastening systems
4373	HILTI Fastening systems
3829	PNEUTECK
3056	ITW BUIDEX
18. Definition of symbols
BP = Button Punch
TSW = Top Seam Weld
SSW = Side Seam Weld
S = Carrying capacity at allowable design stress (lbs/ft²)
 Δ = Carrying capacity at L/180, deflection limitations (lbs/ft²)
L = Span (ft)
W = Uniform vertical load (lbs/ft²)
S± = Section modulus (in³/ft)
I = Moment of Inertia (in⁴/ft)
E = Modulus of Elasticity of steel 29,500 ksi
q = allowable shear (lbs/ft)
F = Flexibility Factor": The Average micro-inches a diaphragm web will deflect in a span of 1 foot under a shear of 1 pound per foot.
R = Ratio of intermediate panel supports to overall panel length, Lv/Ltotal. (R for three span deck = 1/3)



STEEL DECK SPECIFICATIONS

PART 1 GENERAL

1.01 WORK INCLUDED

FURNISH AND INSTALL STEEL DECKING AS DEFINED BY THE CONTRACT DOCUMENTS, INDICATING BASIC LAYOUT AND DECK UNIT TYPES.

1.02 WORK INCLUDED

1. SECTION 3200 – CONCRETE REINFORCEMENT
2. SECTION 3300 – CONCRETE
3. SECTION 5100 – STRUCTURAL STEEL FRAMING
4. SECTION 5200 – STEEL JOISTS
5. SECTION 7250 – FIREPROOFING
6. SECTION 9900 – PAINTING

1.03 WORK INCLUDED

1. SECTION 7500 – ACOUSTICAL BATTS

1.04 QUALITY ASSURANCE

CODES/STANDARDS – THE WORK AND MATERIALS OF THIS SECTION SHALL COMPLY WITH:

1. AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.
2. AWS D1.1 & D1.3 "SPECIFICATION
3. ASTM REFERENCE
 - a. ASTM A653 – STEEL SHEET/ZINC COATED
4. ICC-ES EVALUATION REPORT ESR-2050
5. UNDERWRITERS LABORATORY – FIRE RESISTANCE DIRECTORY

1.05 QUALITY ASSURANCE

1. SHOP DRAWINGS: INDICATE DECK PLAN, SUPPORT LOCATIONS, OPENINGS, DIMENSIONS, ACCESSORIES AND TYPE OF LOCATION OF WELDS.

1.06 PRODUCT HANDLING

1. METAL DECKING IS TO BE DELIVERED, STORED AND HANDLED IN A MANNER THAT WILL ASSURE ITS SHAPE AND SURFACE INTEGRITY.
2. STORE DECK OFF GROUND WITH ONE END ELEVATED FOR DRAINAGE.
3. COVER DECK WITH A WATER RESISTANT MATERIAL TO PROTECT AGAINST THE ELEMENTS.

PART 2 PRODUCTS

2.01 MATERIALS AND FINISHES

COMPOSITE FLOOR DECK SHALL BE PROVIDED IN CONFIGURATION AS MANUFACTURED BY DECK WEST, INC., AND SHALL HAVE DEFORMATION TO PROVIDE A MECHANICAL BOUND WITH THE CONCRETE.

1. 3DW (16,18,20,22) GA 3" DEEP x 36"
2. 2DW (16,18,20,22) GA 2" DEEP x 36"
3. BDW (16,18,20,22) GA 1 1/2" DEEP x 36"

STEEL ROOF DECK SHALL BE PROVIDED IN CONFIGURATION AS MANUFACTURED BY DECK WEST

1. BDW (16,18,20,22) GA 1 1/2" DEEP x 36"
(SPECIFY ACOUSTICAL IF REQUIRED)
2. BDW (16,18,20,22) GA 1 1/2" DEEP x 36" W/ 18 GA SHEARCORR STIFFENER. (SPECIFY ACOUSTICAL IF REQUIRED)
3. NDW (16,18,20,22) GA 3" DEEP x 24"
(SPECIFY ACOUSTICAL IF REQUIRED)

ACOUSTICAL DECK TO HAVE VERTICAL WEBS PERFORATED WITH 1/8" DIAMETER HOLES STAGGERED ON 3/8" CENTERS.
(SPECIFY DECK TYPES 1,2,3 ABOVE IF REQUIRED)

2.02 DECK UNIT COIL MATERIAL

1. SHEET STEEL CONFORMING TO ASTM A653, Fy=46000 psi WITH GALVANIZED COATING (AS PER REQUEST)

2.03 FABRICATION

1. MANUFACTURE DECK UNITS TO SUIT JOB APPLICATION. IN GENERAL DECKS FORMED TO MEET 3-SPAN SUPPORT CONDITION. DECKS FABRICATED WITH STANDARD INTERLOCKING SEAM, ALLOWING BUTTON PUNCH CONNECTION, TOP SEAM WELD OR SCREW SIDE SEAM CONNECTION IN BDW, NDW, 2DW AND 3DW.
2. FABRICATE STEEL DECK ACCESSORIES FROM THE SAME GAUGE AND MATERIAL AS ADJACENT STEEL DECK.



PART 3 EXECUTION

3.01 INSPECTION

VERIFY SUPPORTING MEMBER ARE CORRECTLY LAID OUT AND ALIGNED AND FREE OF DEBRIS. DO NOT PROCEED WITH INSTALLATION UNTIL UNSATISFACTORY CONDITIONS HAVE BEEN CORRECTED.

3.02 INSTALLATION

1. INSTALL DECK UNITS AND ACCESSORIES IN ACCORDANCE WITH APPROVED SHOP DRAWINGS.
2. PLACE DECK UNITS OR SUPPORTING MEMBERS AND ADJUST TO PROPER ALIGNMENT. VERIFY PROPER BEARING ON SUPPORTING MEMBERS AN ACCURATE ALIGNMENT OF END LAPS AND SIDE LAPS PRIOR TO PERMANENT ATTACHMENT.
3. FASTEN STEEL DECK SUPPORTING MEMBERS WITH ½" EFFECTIVE DIAMETER FUSION WELDS. IF STUDS ARE WELDED THROUGH DECK (UN-SHORED CONSTRUCTION ONLY), THE STUDS CAN REPLACE FUSION WELDS.
4. FASTEN SIDE LAPS BY BUTTON PUNCHING, TOP SEAM WELD, SIDE SEAM WELD OR USE OF SCREW TYPE FASTENERS.
5. ALL WELDING SHALL CONFORM TO AWS D1.1 AND D1.3. WELDERS SHALL BE CERTIFIED PRIOR TO COMMENCING WORK.