# <u>GENERAL</u>

- 1. STRUCTURAL NOTES ARE NOT INTENDED TO REPLACE SPECIFICATIONS. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. FOR INCONSISTENCIES BETWEEN STRUCTURAL DRAWINGS, THE SPECIFICATIONS, AND ANY CODE OF STANDARD PRACTICE, THE STRICTER REQUIREMENT SHALL APPLY, AND THE ENGINEER SHALL BE NOTIFIED PRIOR TO PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.
- 2. STRUCTURAL CONSTRUCTION DOCUMENTS SHALL BE USED WITH OTHER CONSTRUCTION DOCUMENTS, INCLUDING ARCHITECTURAL, M/E/P, AND SITE DOCUMENTS. COORDINATE WITH THESE DOCUMENTS FOR LOCATIONS AND DIMENSIONS OF OPENINGS, CHASES, INSERTS, REGLETS, SLEEVES, DEPRESSIONS, ETC., NOT INDICATED ON THE STRUCTURAL DOCUMENTS. ALL DIMENSIONS AND CONDITIONS, EXISTING AND NEW, SHALL BE FIELD VERIFIED. THE ENGINEER SHALL BE NOTIFIED OF DISCREPANCIES PRIOR TO PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.
- 3. THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE STABILITY AND SAFETY DURING CONSTRUCTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF SHEETING, SHORING, TEMPORARY BRACING, GUYS, AND TIEDOWNS. THE CONTRACTOR SHALL PROVIDE SHORING AND BRACING NECESSARY TO PROTECT EXISTING AND ADJACENT STRUCTURES.
- 4. SECTIONS AND DETAILS SHOWN ON ANY STRUCTURAL DOCUMENTS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDITIONS THAT DO NOT HAVE A SPECIFIC SECTION INDICATED, AND SHALL BE PROVIDED AT NO ADDITIONAL COST TO THE OWNER.
- 5. APPLICABLE FEDERAL, STATE AND MUNICIPAL REGULATIONS SHALL BE FOLLOWED, INCLUDING THE FEDERAL DEPARTMENT OF LABOR OSHA
- 6. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED ON THE STRUCTURE. CONSTRUCTION LOADS SHALL NOT EXCEED THE SPECIFIED DESIGN LIVE LOADS. CONCRETE SLABS AND TOPPINGS SHALL NOT BE LOADED UNTIL THE CONCRETE HAS REACHED AT LEAST 75% OF THE SPECIFIED DESIGN COMPRESSIVE STRENGTH
- 7. THE CONTRACTOR'S CONSTRUCTION SEQUENCES SHALL ALLOW FOR THE EFFECTS OF THERMAL MOVEMENTS DURING THE CONSTRUCTION PERIOD, PRIOR TO THE BUILDING BEING ENCLOSED AND TEMPERATURE CONTROLLED. NEGATIVE EFFECTS OF SUCH THERMAL MOVEMENTS, SUCH AS MATÉRIAL CRACKING, FROST HEAVE, ETC.
- SHALL BE CORRECTED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER 8. IN THE ABSENCE OF SPECIFIC INSTRUCTIONS TO THE CONTRARY IN THE CONTRACT DOCUMENTS, THE TRADE PRACTICES THAT ARE DEFINED IN ANY CODE OF STANDARD PRACTICE SHALL GOVERN.
- 9. DO NOT SCALE DRAWINGS TO DETERMINE DIMENSIONS, LOCATIONS, OR SIZES OF ANY ELEMENT.

#### STRUCTURAL DESIGN CRITERIA

- 1. DESIGN LOADS ARE IN ACCORDANCE WITH THE 2015 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC) INCLUDING LOCAL CODES, WHERE APPLICABLE, AND THE FOLLOWING STANDARDS REFERENCED IN IBC 2015:
  - ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE ACI 530 - BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES ACI 530.1 - SPECIFICATIONS FOR MASONRY STRUCTURES AF&PA NDS - NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION AISC 360 - SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS ASCE 7 - MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
- 2. RISK CATEGORY / OCCUPANCY CATEGORY OF BUILDING : II 3. LIVE LOADS ARE AS FOLLOWS. LIVE LOAD REDUCTIONS HAVE BEEN TAKEN WHERE APPLICABLE, UNO.
- ROOF LIVE LOAD 30 PSF ASSEMBLY AREAS 100 PSF 100 PSF CORRIDORS 1*00* PSF 50 PSF + 20 PSF PARTITION OFFICE STAIRS 1*00* PSF
- STORAGE, LIGHT 125 PSF 4. SNOW LOADING IS BASED ON THE FOLLOWING. DRIFTING OR SLIDING SNOW LOADS HAVE BEEN CONSIDERED WHERE APPROPRIATE. RAIN ON SNOW SURCHARGE N/A (Pa > 20 PSF)
- GROUND SNOW LOAD, Pa 25 PSF FLAT-ROOF SNOW LOAD, PF 21 PSF SNOW EXPOSURE FACTOR, Ce 1.0 SNOW THERMAL FACTOR, Ct 1.0 SNOW LOAD IMPORTANCE FACTOR. I 5. WIND LOADING IS BASED ON THE FOLLOWING

BASIC WIND SPEED (3 SEC GUST)

- EXPOSURE CATEGORY BUILDING CATEGORY: SIMPLE DIAPHRAGM, LOW-RISE, ENCLOSED, RIGID INTERNAL PRESSURE COEFF. ±0.18 COMPONENTS & CLADDING
- 10 SF | 20 SF | 50 SF | 100 SF | 500 SF WALLS 32.1, -34.8 | 30.6, -33.3 | 28.8, -31.5 | 27.3, -30.0 | 23.9, -26. WALL CORNER 32.1, -43.1 | 30.6, -40.1 | 28.8, -36.3 | 26.7, -26.7 | 23.9, -26.7 |ROOF ZONE | (27 TO 45)| 29.4,-32.1 | 28.6,-30.5 | 27.5,-28.4 | 26.7,-26.7 ROOF ZONE 2 (27 TO 45) | 29.4, -37.5 | 28.6, -35.9 | 27.5, -33.8 | 26.7, -32.1
- 6. SEISMIC LOADING IS BASED ON THE FOLLOWING: SEISMIC IMPORTANCE FACTOR SEISMIC SITE CLASS SPECTRAL RESPONSE COEFF. (Sps) 0.20<del>4</del>q SPECTRAL RESPONSE COEFF. (Spi) 0.077a LONG PERIOD TRANSITION (TL) SEISMIC DESIGN CATEGORY, ANALYSIS PROCEDURE EQUIVALENT LATERAL FORCE BASIC STRUCTURAL SYSTEM BEARING WALL, BUILDING FRAME SEISMIC FORCE RESISTING SYSTEM STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE LATERALLY SHEATHED SHEAR WALLS RESPONSE MODIFICATION FACTOR (R) 2.0 DESIGN BASE SHEAR

|ROOF ZONE 3 (27 TO 45)||29.4.-37.5 | 28.6.-35.9 | 27.5.-33.8 | 26.7.-32.1

- 8. DESIGN REACTIONS AND SUPPORT DETAILS FOR ARCHITECTURAL MECHANICAL, ELECTRICAL AND PLUMBING EQUIPMENT ARE BASED UPON AVAILABLE MANUFACTURER INFORMATION. SUPPORT CONDITIONS MAY NEED TO BE REVISED BASED UPON ACTUAL SUPPLIED EQUIPMENT AND SUPPORT DETAILS.
- 9. ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING SUPERIMPOSED LOADS: <u>BOTTOM CHORD</u> DEAD = 15 PSFDEAD = 10 PSF

ISSUED FOR PERMIT

LIVE = 30 PSF LIVE = 10 PSF (TYP) LIVE = 40 PSF (ATTIC & CATWALK) SNOW = PER CODE WIND = PER CODE

GABLE END TRUSSES SHALL BE DESIGNED FOR WIND LOAD PERPENDICULAR TO THE PLANE OF THE TRUSS. FOR GABLE END TRUSSES BACKING UP MASONRY VENEER, PROVIDE VERTICAL WEB MEMBERS AT 16" O/C, AND LIMIT OUT-OF-PLANE DEFLECTIONS TO L/600. DESIGN, DETAIL AND PROVIDE ALL TEMPORARY AND PERMANENT BRACING FOR THE TRUSS SYSTEM.

#### EXISTING CONDITIONS

- 1. EXISTING CONDITIONS INDICATED ARE OBTAINED FROM AVAILABLE SOURCES (EXISTING DRAWINGS, FIELD SURVEYS, ETC.) AND ARE NOT GUARANTEED TO BE TRUE AND EXACT. CONTRACTOR(S) SHALL FIELD VERIFY EXISTING CONDITIONS AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES PRIOR TO PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.
- 2. SEE ARCHITECTURAL DRAWINGS FOR EXTENT OF DEMOLITION OF EXISTING CONSTRUCTION. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE DEMOLITION PROCEDURES AND SEQUENCE TO ENSURE STABILITY AND SAFETY DURING DEMOLITION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF TEMPORARY SHORING AND BRACING NECESSARY TO SUPPORT AND PREVENT DAMAGE TO REMAINING CONSTRUCTION.

#### TYPICAL DETAILS

- TYPICAL DETAILS APPLY AT ALL APPROPRIATE LOCATIONS. TYPICAL DETAILS ARE GENERALLY NOT CUT ON THE PLANS.
- CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL TYPICAL DETAIL APPLICATIONS.

# FOUNDATIONS

- 1. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEER'S REPORT PREPARED BY DAVID BLACKMORE & ASSOCIATES INC, POTTSTOWN, PA
- 2. SPREAD FOOTINGS SHALL BEAR ON UNDISTURBED SOIL OR COMPACTED STRUCTURAL FILL HAVING A MINIMUM SAFE BEARING CAPACITY OF 3 KSF.
- 3. THE BOTTOMS OF EXTERIOR FOOTINGS SHALL BE 36 IN. MINIMUM BELOW
- 4. EDGES OF FOOTINGS SHALL NOT BE PLACED AT A GREATER THAN 1 (VERTICAL) TO 2 (HORIZ) SLOPE WITH RESPECT TO ANY ADJACENT FOOTING OR EXCAVATION. ADJACENT COLUMN FOOTINGS THAT ABUT SHALL BE SEPARATED BY A PAPER JOINT.
- 6. FOUNDATION CONCRETE SHALL BE NORMAL WEIGHT HAVING A MINIMUM 28 DAY DESIGN COMPRESSIVE STRENGTH AS FOLLOWS:
- 4000 PSI SPREAD FOOTINGS WALLS & PIERS 4000 PSI SLAB-ON-GRADE (INTERIOR) 3500 PSI
- 7. PROVIDE AIR-ENTRAINMENT IN ALL CONCRETE EXPOSED TO FREEZE-THAW CONDITIONS DURING THE CONSTRUCTION PERIOD AND/OR IN THE COMPLETED

SLAB-ON-GRADE (EXTERIOR) 4500 PSI, 0.45 W/C MAX.

- 8. VERTICAL CRACK CONTROL AND/OR CONSTRUCTION JOINTS IN CONCRETE WALLS SHALL BE PROVIDED AT 30 FT. O/C MAX. CONSTRUCTION JOINTS SHALL BE PROVIDED AT 90 FT. O/C MAX.
- 9. LATERAL EARTH PRESSURES ON RETAINING AND BASEMENT WALLS ARE BASED ON THE FOLLOWING: [PRESUMPTIVE LOADING]: EQUIVALENT AT-REST FLUID PRESSURE (BASEMENT) 60 PCF EQUIVALENT ACTIVE FLUID PRESSURE (CANTILEVERED) 44 PCF EQUIVALENT PASSIVE FLUID PRESSURE 364 PCF

0.5

COEFFICIENT OF FRICTION

#### <u>SUBMITTALS</u>

1. THE APPLICABLE CONTRACTOR SHALL SUBMIT THE FOLLOWING FOR APPROVAL: CONCRETE MIX DESIGNS FOR STRENGTHS INDICATED CONCRETE REINFORCING SHOP DRAWINGS, INCLUDING ELEVATIONS OF ALL WALLS COLDFORMED FRAMING SHOP DRAWINGS & CALCULATIONS W/ PE SEAL STEEL STAIR FRAMING SHOP DRAWINGS & CALCS W/ PE SEAL STRUCTURAL STEEL SHOP DRAWINGS & CONNECTION DESIGN MASONRY REINFORCING SHOP DRAWINGS, INCLUDING ELEVATIONS OF ALL WALLS 6. MASONRY GROUT AND MORTAR MIX DESIGNS WOOD TRUSS SHOP DRAWINGS W/ P.E. SEALED CALCULATIONS PRODUCT DATA & MILL TEST FOR EACH APPLICABLE PRODUCT COLDFORMED TRUSS SHOP DRAWINGS & CALCS w/ PE SEAL STEEL JOIST SHOP DRAWINGS METAL DECK SHOP DRAWINGS

# MECHANICAL ROOFTOP EQPT CURBS, OPENINGS & ROOF ACCESS

- PROVIDE FRAMING FOR ALL ROOFTOP EQUIPMENT CURBS AND OPENINGS, AND ROOF ACCESS, IN NEW AND EXISTING CONSTRUCTION ACCORDING TO TYPICAL DETAILS, UNO
- 2. COORDINATE SIZES AND LOCATIONS OF CURBS AND OPENINGS WITH MECHANICAL DRAWINGS AND MECHANICAL CONTRACTOR AND ROOF ACCESS OPENINGS WITH ARCHITECTURAL DRAWINGS. CURBS AND OPENINGS SHALL BE CENTERED BETWEEN AND ACROSS NEW AND EXISTING ROOF MEMBERS.
- 3. ALL ROOF JOISTS AND TRUSSES (NEW AND EXISTING) SHALL BE REINFORCED FOR OFF-PANEL POINTS LOADS ACCORDING TO TYPICAL DETAIL. 4. RE-USE EXISTING OPENINGS WHERE POSSIBLE.

# DRILLED ANCHORS

- EXPANSION ANCHORS SHALL BE (UNO): HILTI KWIK BOLT III, DEWALT/POWERS POWER-STUD+SD1, OR EQUIVALENT 3/4-INCH DIAMETER
- SUFFICIENT LENGTH TO PROVIDE 6-INCH MINIMUM EMBEDMENT CHEMICAL ADHESIVE ANCHORS SHALL BE (UNO):
- HILTI HIT HYTO OR HY200 AS APPLICABLE, DEWALT/POWERS PURE 110+, OR EQUIVALENT 3/4-INCH DIAMETER
- SUFFICIENT LENGTH TO PROVIDE 6-INCH MINIMUM EMBEDMENT
- GROUT CMU COURSES CONT AT ANCHORS FOR 8" MIN ABOVE & BELOW ANCHOR LINES. 4. ANCHORS IN EXTERIOR APPLICATIONS SHALL BE HOT-DIPPED GALV.

#### SUPPORT AND UNDERPINNING SYSTEM

CONCRETE, GROUT, SHIMS, AND TIEBACKS.

- CONTRACTOR SHALL PROVIDE COMPLETE DESIGN, ERECTION, MAINTENANCE, AND MONITORING OF SUPPORT AND UNDERPINNING SYSTEM TO SAFELY COMPLETE THE PERMANENT WORKS, TO PREVENT DAMAGE TO EXISTING BUILDINGS, AND TO PERMANENTLY SUPPORT EXISTING BUILDING ELEMENTS ADJACENT TO THE NEW WORK.
- 2. CONTRACTOR SHALL ENGAGE AND ASSIGN SUPERVISION OF SUPPORT AND UNDERPINNING SYSTEM TO A QUALIFIED PROFESSIONAL ENGINEER FOUNDATION CONSULTANT.
- THE SUPPORT AND UNDERPINNING SYSTEM SHALL BE SELECTED, DESIGNED AND SUPERVISED BY THE QUALIFIED PROFESSIONAL ENGINEER. ENGINEER SHALL BE LEGALLY AUTHORIZED TO PRACTICE IN JURISDICTION WHERE PROJECT IS LOCATED.
- 4. SUPPORT INCLUDES THE FACILITIES REQUIRED TO PREVENT MOVEMENT OF EXISTING STRUCTURES UNTIL COMPLETION OF THE UNDERPINNING AND NEW FOUNDATIONS. INCLUDE NEEDLING, SHORING, PROPPING, BRACING, CRIBBING
- AND SHEETING. UNDERPINNING INCLUDES THE PERMANENT CONSTRUCTION WHICH DIRECTLY TRANSMITS EXISTING STRUCTURE FOUNDATION LOADS TO A LOWER BEARING ELEVATION, WHICH RESISTS LATERAL EARTH PRESSURES, AND WHICH PRESERVES THE STRUCTURES BEING UNDERPINNED. INCLUDE FORMS, REINFORCEMENT,
- SUBMIT CERTIFICATION LETTER SIGNED AND SEALED BY THE CONSULTANT CERTIFYING THAT THE SUPPORT AND UNDERPINNING SYSTEM AS DESIGNED AND INSTALLED ARE IN COMPLIANCE WITH THE REQUIREMENTS AND GOVERNING CODES AND WILL NOT PRODUCE DAMAGING SETTLEMENTS IN THE EXISTING BUILDING STRUCTURES.

# CONCRETE REINFORCING

- REINFORCED CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 318. CONCRETE REINFORCING SHALL CONFORM TO THE FOLLOWING DESIGNATIONS: DEFORMED BARS ASTM A615, GRADE 60 DEFORMED BARS (WELDABLE) ASTM A706
- WELDED WIRE FABRIC ASTM A1064 3. LAP DEFORMED BARS 40 DIA., UNO. PROVIDE CORNER AND L BARS AT CORNERS AND INTERSECTIONS. REINFORCING INDICATED AS CONTINUOUS SHALL BE LAPPED. HOOKS SHALL BE STANDARD HOOKS, UNO. LAP WELDED WIRE FABRIC SUCH THAT THE OVERLAP OF THE OUTERMOST CROSS-WIRES OF EACH ADJOINING SHEET IS NOT LESS THAN THE SPACING OF THE CROSS-WIRES PLUS TWO IN., UNO. PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE; SPLICE ONLY AS SHOWN OR APPROVED; STAGGER SPLICES WHERE POSSIBLE; USE TENSION SPLICE CLASS "B" UNO. DOWELS SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED REINFORCEMENT
- AND SHALL BE LAPPED WITH TENSION SPLICES. 4. CONCRETE PROTECTION FOR REINFORCEMENT: CONCRETE CAST AGAINST AND PERMANENTLY
  - EXPOSED TO EARTH: CONCRETE EXPOSED TO EARTH OR WEATHER: NO. 6 THROUGH NO. 18 BARS:
  - NO. 5 BAR AND SMALLER: 1-1/2 IN. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS, WALLS, JOISTS: 1-1/2 IN. NO. 14 AND NO. 18 BARS
- NO. 11 BAR AND SMALLER: 3/4 IN. BEAMS, COLUMNS, PIERS: 1-1/2 IN. 5. REINFORCING FOR SLABS ON GRADE, WHERE NOT OTHERWISE SPECIFIED, SHALL
- BE AS FOLLOWS: REINFORCING BARS: SEE FOUNDATION AND TYPICAL DETAILS. AT SLAB
- BLOCKOUT AND RE-ENTRANT CORNERS, PROVIDE 2#5 X 4'-0" DIAGONALS 6x6-W2.9 x W2.9 WWF. REINFORCING SHALL BE
- SUPPORTED AT MID-DEPTH OF SLAB. REINFORCING FOR CONCRETE TOPPING, WHERE NOT OTHERWISE SPECIFIED, SHALL BE AS FOLLOWS:
- REINFORCING BARS: SEE FRAMING AND TYPICAL DETAILS. AT SLAB OPENINGS AND RE-ENTRANT CORNERS, PROVIDE 2#5 X 4'-0" DIAGONALS. WIRE MESH: 6x6-W2.9 x W2.9 WWF. REINFORCING SHALL BE
- SUPPORTED 1 IN. BELOW TOP OF SLAB. WELDING, WELDING ELECTRODES AND FLUXES SHALL CONFORM TO AWS D1.4 "STRUCTURAL WELDING CODE - REINFORCED STEEL". ELECTRODES SHALL
- HAVE A MINIMUM TENSILE STRENGTH OF TO KSI. ASTM ATO6 BARS OR DBA'S SHALL BE USED IN ALL WELDED APPLICATIONS. 8. DETAILING OF CONCRETE REINFORCING AND ACCESSORIES SHALL CONFORM TO ACI DETAILING MANUAL SP-66, AND WITH ACI 315, MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES.

# CONCRETE SLABS ON METAL DECK

- 1. CONCRETE SLABS ON METAL DECK SHALL BE NORMAL WEIGHT WITH A MINIMUM 28-DAY DESIGN COMPRESSIVE STRENGTH OF 3,500 PSI.
- 2. CONCRETE SLABS ON METAL DECK SHALL BE FINISHED LEVEL, UNO, MAINTAINING SPECIFIED MINIMUM SLAB THICKNESS. CONTRACTOR SHALL ALLOW FOR ADDITIONAL CONCRETE OVER THE SPECIFIED MINIMUM TO ACCOUNT FOR METAL DECK AND STRUCTURE DEFLECTIONS.
- 3. CONCRETE SLAB-ON-DECK CONSTRUCTION JOINT (CJ) LOCATION GUIDELINES: A. DO NOT LOCATE CJ ON BEAM OR GIRDER CENTERLINES.
  - B. G. PARALLEL TO FILLER BEAM: LOCATE MIDWAY BETWEEN ADJACENT FILLER BEAMS
  - C. CJ PARALLEL TO GIRDER:
  - LOCATE 1/4 TO 1/3 THE DISTANCE TO ADJACENT GIRDER
  - D. CJ PERPENDICULAR TO BEAMS OR GIRDERS: LOCATE AT 1/4 TO 1/3 THE MEMBER SPAN FROM BEAM OR GIRDER END

# CONCRETE SLABS ON GRADE

- 1. GEOTECHNICAL ENGINEER SHALL OBSERVE AND APPROVE SUBGRADE BEFORE CONCRETE PLACEMENT.
- 2. DO NOT PLACE CONCRETE SLABS ON FROZEN GROUND.
- 3. CONTROL JOINTS ARE REQUIRED IN CONCRETE SLABS. REFER TO PLANS AND TYPICAL DETAILS FOR JOINT CONSTRUCTION AND LOCATIONS.
- 4. INSTALL (2) #4 x 5'-0" LONG BARS DIAGONALLY AT RE-ENTRANT CORNERS AND OPENINGS. 5. COORDINATE LOCATIONS AND DIMENSIONS OF RECESSED SLABS

# STRUCTURAL STEEL

- 1. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING DESIGNATIONS: STRUCTURAL STEEL WE SHAPES ASTM A992 OTHER STRUCTURAL STEEL SHAPES ASTM A36, UNO STEEL BARS, ANGLES AND PLATES ASTM A36, UNO STIFF PLATES IN MOMENT CONNECTIONS ASTM A572, GRADE 50 ROUND PIPE
- SQUARE & RECTANGULAR HSS ASTM A500, GRADE C ROUND HSS ASTM A500, GRADE B 2. BOLTS SHALL BE MINIMUM 3/4 IN. DIA. AND SHALL CONFORM TO THE FOLLOWING DESIGNATIONS, UNO:

ASTM A53, TYPE E OR S

- HIGH STRENGTH BOLTS ASTM A325 [ A490] ANCHOR RODS ASTM F1554, GRADE 36 [55] [105] 3. BOLTED CONNECTIONS SHALL CONFORM TO RCSC'S "SPECIFICATIONS FOR
- STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. 4. WELDING, WELDING ELECTRODES, AND FLUXES SHALL CONFORM TO AWS D1.1 "STRUCTURAL WELDING CODE - STEEL". ELECTRODES SHALL HAVE A MINIMUM TENSILE STRENGTH OF 70 KSI.
- 5. GROUT UNDER STEEL COLUMN OR POST BASE PLATES SHALL BE NONMETALLIC SHRINKAGE-RESISTANT GROUT CONFORMING TO ASTM C1107 HAVING A MINIMUM DESIGN COMPRESSIVE STRENGTH OF 5000 PSI. GROUT UNDER STEEL BEAM BEARING PLATES IN CONCRETE MASONRY WALLS SHALL CONFORM TO ASTM C476.
- 6. HIGH STRENGTH BOLTED CONNECTIONS SHALL BE TIGHTENED TO THE SNUG-TIGHT CONDITION, UNO.
- 7. HIGH-STRENGTH BOLTS IN MOMENT CONNECTIONS (MOMENT PLATES AND SHEAR CONNECTIONS) SHALL BE SLIP CRITICAL, UNO, AND SHALL BE TIGHTENED IN ACCORDANCE WITH ONE OF THE METHODS LISTED IN RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING USING ASTM A325 OR A490 BOLTS"
- 8. HIGH STRENGTH BOLTS IN CONNECTIONS USED FOR KICKERS AND BRACING MEMBERS THAT ARE FABRICATED WITH SLOTTED HOLES SHALL BE SLIP-CRITICAL. IF STANDARD HOLES ARE USED, BOLTS SHALL BE FULLY PRE-
- TENSIONED. 9. THE STEEL FABRICATOR IS RESPONSIBLE FOR CONNECTION DESIGN.
- CONNECTION DESIGN SHALL BE PERFORMED BY A LICENSED PROFESSIONAL ENGINEER 10. MINIMUM CAPACITY OF BEAM CONNECTIONS: DESIGN CONNECTIONS USING THE "MAXIMUM TOTAL UNIFORM LOAD" TABLES IN THE AISC MANUAL. FOR NON-COMPOSITE BEAMS, THE CONNECTION CAPACITY SHALL BE AT LEAST 50% OF THE MAXIMUM TOTAL UNIFORM LOAD, UNO. FOR COMPOSITE BEAMS, THE CONNECTION CAPACITY SHALL BE AT LEAST 80% OF THE MAXIMUM TOTAL UNIFORM LOAD, UNO. REACTIONS NOTED ON DRAWINGS ARE SERVICE LOADS.
- PROVIDE FULL DEPTH CONNECTIONS AT BEAM OR GIRDER TO COLUMN CONNECTION. 12. PROVIDE COLUMN CAP PLATES AS FOLLOWS, UNO:

1/4" THICK (PROVIDE WHERE BEAMS DO NOT FOR DECK BEARING: FRAME INTO BOTH SIDES OF WEB.) 1/2" THICK AT K-SERIES JOISTS. 3/4" THICK AT LH & DLH JOISTS. 1" THICK AT JOIST GIRDERS.

- FOR BEAM BEARING: SEE TYPICAL DETAILS, 3/4" THICK MIN. FOR MOMENT CONNECTIONS: SEE TYPICAL DETAILS. PROVIDE COLUMN CAP PLATES AT ALL SQUARE, RECTANGULAR AND ROUND HSS COLUMNS. 13. WEB STIFFENERS SHALL BE PROVIDED IN WF SHAPES AS FOLLOWS:
- <u>COLUMN WEBS:</u> AT FULLY-DEVELOPED MOMENT CONNECTIONS. STIFFENERS SHALL BE COMPLETE-PENETRATION GROOVE WELDED, SAME THICKNESS AND GRADE AS BEAM FLANGES. WHERE MOMENT CONNECTIONS OCCUR ON COLUMN FLANGES AND COLUMN WEBS, STIFFENER THICKNESS SHALL EQUAL THE VECTOR SUMMATION OF THE RESPECTIVE BEAM FLANGE THICKNESSES BEAM WEBS: WHERE BEAM BEARS ON COLUMN, SAME THICKNESS AND STRENGTH AS COLUMN FLANGES.
- BEAM WEBS: WHERE COLUMN BEARS ON BEAM, SAME THICKNESS AND STRENGTH AS COLUMN FLANGES. 14. PRIOR TO DETAILING CONNECTIONS FOR STRUCTURAL STEEL, THE STEEL FABRICATOR SHALL SUBMIT FOR APPROVAL REPRESENTATIVE DETAILS FOR EACH TYPE OF PROPOSED STRUCTURAL CONNECTION. SUCH DETAILS SHALL INDICATE
- DESIGN CAPACITIES. AFTER APPROVAL, THE CONNECTIONS SHALL BE INCORPORATED INTO SHOP DRAWINGS. 15. ALL EXTERIOR EXPOSED STRUCTURAL STEEL TO BE HOT DIPPED GALVANIZED.

# CONCRETE MASONRY

- 1. CONCRETE MASONRY CONSTRUCTION SHALL CONFORM TO ACI 530 AND 530.1. 2. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE MASONRY, F'M, SHALL BE 1500 PSI. (MIN NET AREA COMPRESSIVE STRENGTH OF UNIT = 1900 PSI.)
- 3. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90. 4. CONCRETE MASONRY REINFORCING SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60. DEFORMED BAR ANCHORS (DBA) SHALL CONFORM TO
- ASTM A496. DBA'S SHALL BE WELDED BY AUTOMATIC EQUIPMENT. 5. GROUT SHALL CONFORM TO THE PROPORTIONAL REQUIREMENTS OF ASTM C476 PROVIDE FINE AND COARSE GROUTS APPROPRIATE FOR SIZE OF VOID SPACE BEING FILLED. GROUT SHALL HAVE A MINIMUM SLUMP OF 8 INCHES PROVIDED BY SUFFICIENT WATER CONTENT. ADMIXTURES ARE NOT PERMITTED IN GROUT.
- GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI. 6. MORTAR SHALL CONFORM TO ASTM C270, TYPE M OR S, PCL OR MORTAR CEMENT. USE OF MASONRY CEMENT IS NOT PERMITTED
- 7. ALL REINFORCED VOIDS SHALL BE GROUTED SOLID. 8. REINFORCED VOIDS, AND NON-REINFORCED VOIDS SPECIFIED TO BE GROUTED, IN CONCRETE MASONRY SHALL BE FILLED SOLID WITH 6ROUT IN 5 FT. MAXIMUM LIFTS
- STOP POURS 1-1/2 INCHES BELOW THE BED JOINT TO FORM A KEY AT POUR JOINTS. 9. REINFORCING BARS SHALL BE TIED TO DOWELS AND HELD IN THE PROPER POSITION BY MECHANICAL BAR POSITIONERS DESIGNED FOR THAT PURPOSE.
- 10. REINFORCING SHALL NOT BE PLUNGED INTO WET GROUT. 11. LAP UNCOATED, DEFORMED BARS 48 BAR DIAMETERS.

AT WALL OPENINGS

- 12. CONCRETE MASONRY SHALL BE LAID IN RUNNING BOND, UNO. PILASTERS SHALL BE BONDED, UNO. 13. LOAD BEARING CMU SHALL HAVE FULL MORTAR BED JOINTS AND SHALL BE SQUARE
- ENDED CONCRETE MASONRY UNITS. 14. PROVIDE LADDER-TYPE, HORIZONTAL JOINT REINFORCEMENT AS FOLLOWS:
- TYPICAL: 16 IN C/C MAX, UNO. AT BELOW GRADE WALLS: PROVIDE AT 8 IN. C/C. AT PARAPETS: PROVIDE AT 8 IN. C/C.
- TERMINATE 2 FT. BEYOND OPENING PROVIDE CONTINUITY AT INTERSECTIONS AND CORNERS USING PREFABRICATED T-SHAPED AND L-SHAPED UNITS, AND LAP ALL CONSECUTIVE SECTIONS OF TRUSS

PROVIDE ADD'L REINF. NOT MORE THAN 8

IN. ABOVE AND BELOW OPENING.

- TYPE REINFORGING A MINIMUM OF 8". 15. PROVIDE VERTICAL CONTROL JOINTS IN WALLS AT 24 FT. O/C MAX, UNO 16. ALL CMU WALLS SHALL BE DOWELED TO SUPPORTING SLABS WITH MINIMUM #4 @ 48 HOOKED DOWELS, UNO. ALL CMU WALLS SUPPORTED DIRECTLY ON STEEL MEMBERS
- X 2'-0" DBA'S AT 48" O/C, UNO. 17. THE TOPS OF ALL NON-LOAD BEARING CMU WALLS SHALL BE BRACED ACCORDING TO SPECIFIC SECTIONS AND / OR TYPICAL DETAILS

SHALL BE ANCHORED WITH 1/2" DIAMETER x 4" STUDS AT 32" O/C, OR WITH #4

UP TO 24 FT.

18. PROVIDE BOND BEAMS FOR WALL THICKNESSES AND HEIGHTS AS FOLLOWS: BOND BEAM UNITS SHALL BE OPEN CELL UNITS THAT PERMIT VERTICAL REINFORCING TO PASS THROUGH.

UP TO 12 FT.: 6" CMU: UP TO 16 FT. 8" CMU: UP TO 16 FT.: 2#5 UP TO 24 FT. UP TO 12 FT. 10" AND 12" CMU: UP TO 20 FT. UP TO 24 FT. 14" AND 16" CMU: UP TO 12 FT.: UP TO 20 FT.

WALL HEIGHT IS DISTANCE FROM FOUNDATION TO TOP OF WALL, OR DISTANCE BETWEEN BOND BEAMS. WHERE BOND BEAM COURSES NEED TO STEP DUE TO SLOPING CONDITIONS, BOND BEAM COURSES SHALL LAP 4 FT. MINIMUM

- STRUCTURAL LUMBER SHALL CONFORM TO AF&PA'S NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION."
- 2. STRUCTURAL LUMBER SHALL BE NO. 2 S-P-F, VISUALLY GRADED, OR BETTER. 3. GLUED LAMINATED TIMBER SHALL CONFORM TO THE CURRENT EDITIONS OF AF&PA'S NDS, AITC 117, "DESIGN STANDARD SPECIFICATIONS FOR STRUCTURAL

GLUED LAMINATED TIMBER FOR SOFTWOOD SPECIES", AND AITC A 190.1,

- "STRUCTURAL GLUED LAMINATED TIMBER" 4. GLUED LAMINATED TIMBER SHALL BE VISUALLY GRADED SOUTHERN PINE.
- COMBINATION 20F-V1 (MIN.). 5. LAMINATED VENEER LUMBER (LVL) SHALL BE AS MANUFACTURED BY ILEVEL, OR EQUIVALENT. LYL'S SHALL NOT BE EXPOSED TO THE ENVIRONMENT IN THE COMPLETED STRUCTURE.
- 6. PARALLEL STRAND LUMBER (PSL) SHALL BE AS MANUFACTURED BY ILEVEL OR EQUIVALENT. PSL'S EXPOSED TO THE ENVIRONMENT SHALL BE WOLMANIZED.
- 7. TJI JOISTS SHALL BE AS MANUFACTURED BY ILEVEL OR APPROVED EQUAL
- TRUS JOIST MACMILLAN OR APPROVED EQUAL. 8. LSL (TIMBERSTRAND) HEADERS SHALL BE AS MANUFACTURED BY ILEVEL 9. OPEN WEB SERIES (TJL, ETC.) JOISTS SHALL BE MANUF. BY REDBUILT
- OR APPROVED EQUAL. 10. PLYWOOD SHALL CONFORM TO APA'S "PANEL DESIGN SPECIFICATION", PDS-04, AND DOC'S PS 1, "CONSTRUCTION AND INDUSTRIAL PLYWOOD". ALL JOINTS SHALL BE STAGGERED. PANELS SHALL BE INSTALLED WITH THE LONG DIMENSION ACROSS SUPPORTS. NAILING SHALL COMPLY WITH MINIMUM APA REQUIREMENTS
- FOR PLYWOOD FLOOR/ROOF DIAPHRAGMS, AND IBC FASTENING SCHEDULE. 11. PLYWOOD ROOF SHEATHING SHALL BE APA STRUCTURAL I RATED SHEATHING, EXPOSURE 1, THICKNESS AS INDICATED. PROVIDE PANEL CLIPS AT UNSUPPORTED EDGES.
- 12. PLYWOOD WALL SHEATHING SHALL BE APA RATED SHEATHING, TYPE CDX, EXPOSURE 1.
- 13. ROOF SHEATHING SHALL BE INSTALLED ON MAIN ROOF MEMBERS PRIOR TO THE INSTALLATION OF OVERFRAMING MEMBERS
- PLYWOOD SUB-FLOORING SHALL BE APA RATED STURD-I-FLOOR, EXPOSURE 1, THICKNESS AS INDICATED, WITH TONGUE AND GROOVE EDGES. FIELD-GLUE USING ADHESIVES MEETING APA SPECIFICATION AFG-01
- CONNECTOR SIZE AND CAPACITY SHALL MATCH MEMBER SIZE AND CAPACITY. TOE-NAILING IS NOT PERMITTED. 16. MINIMUM FASTENING SHALL CONFORM TO IBC TABLE 2304.9.1, "FASTENING

CONNECTIONS SHALL BE MADE USING PREFABRICATED CONNECTORS.

- SCHEDULE". 17. HEADERS AT NON LOAD BEARING CONDITIONS SHALL BE AS FOLLOW: 2X4 WALLS:
- OPENINGS UP TO 9'-0" (2) 2X8 + 1/2" PLYWOOD 2X6 WALLS
- OPENINGS UP TO 9'-0" (3) 2X8 + 1/2" PLYWOOD (2 LAYERS) 18. PROVIDE CONTINUOUS SOLID BLOCKING OR CROSS-BRIDGING LINES AT 8'-0" O/C MAX., ONE LINE MINIMUM. PROVIDE ADDITIONAL BRIDGING FOR MANUFACTURED WOOD PRODUCTS (JOISTS, TRUSSES, ETC.) AS SPECIFIED BY MANUFACTURER.
- 19. REFER TO STRUCTURAL DESIGN CRITERIA FOR TRUSS DESIGN LOADS
- 20. PRESSURE PRESERVATIVE TREATED LUMBER SHALL BE PROVIDED WHERE LUMBER IS IN CONTACT WITH CONCRETE OR MASONRY, OR EXPOSED TO THE WEATHER. 21. ALL FASTENERS AND PREFABRICATED CONNECTORS USED WITH PRESERVATIVE TREATED WOOD SHALL HAVE A HOT-DIP GALVANIZING 6185 COATING ACCORDING TO ASTM A153 AND A123. (TYPE A304 OR 306 STAINLESS STEEL
- FASTENERS AND CONNECTORS) 22. FIRE-TREATED ROOF TRUSSES OR FRAMING SHALL BE PROTECTED FROM MOISTURE BY INSTALLATION OF A VAPOR BARRIER AND BY POSITIVE VENTILATION IN ATTIC. A HUMIDISTAT CONTROLLED BLOWER SHALL BE INSTALLED TO MAINTAIN A RELATIVE HUMIDITY LESS THAN 95% AND A
- TEMPERATURE LESS THAN 150 DEGREES F PRESSURE PRESERVATIVE TREATED GLUE LAMINATED TIMBER SHALL CONFORM TO AMERICAN WOOD PRESERVER'S ASSOCIATION STANDARD C1-00.

24. PROVIDE A SUFFICIENT NUMBER OF STUDS TO ACHIEVE FULL BEARING WIDTH

- FOR ALL BEAMS, TRUSS GIRDERS, AND POINT LOADS. MULTIPLE STUDS SHALL BE NAIL LAMINATED TOGETHER.
- 25. ANCHOR BOLTS SHALL BE SPACED AT 4'-0" c/c.UNO AND 4" TO 12" MAX FROM ENDS OF WALLS WITH 2 BOLTS PER SILL BOARD.

# SYPSUM BOARD/COLD FORMED STEEL SHEAR WALLS

- SEE COLD FORMED STEEL FRAMING NOTES.
- GYPSUM WALLBOARD SHALL CONFORM TO ASTM C36. STEEL SELF DRILLING SCREWS SHALL CONFORM TO ASTM C1002.
- 4. STEEL STUDS SHALL BE MINIMUM 3-5/8 INCH, 16 6AGE, WITH 1-5/8 INCH FLANGES SPACED AT 16 INCHES O/C MAX. PROVIDE DOUBLED STUDS AT WALL ENDS.
- 6YPSUM WALLBOARD SHALL BE MINIMUM 1/2 INCH THICK, PLACED ON BOTH SIDES OF STUD. 6. SCREMS SHALL BE MINIMUM NO. 6 X 1 INCH, SPACED AT 12 INCHES O/C, AT ALL STUDS AND TRACKS.

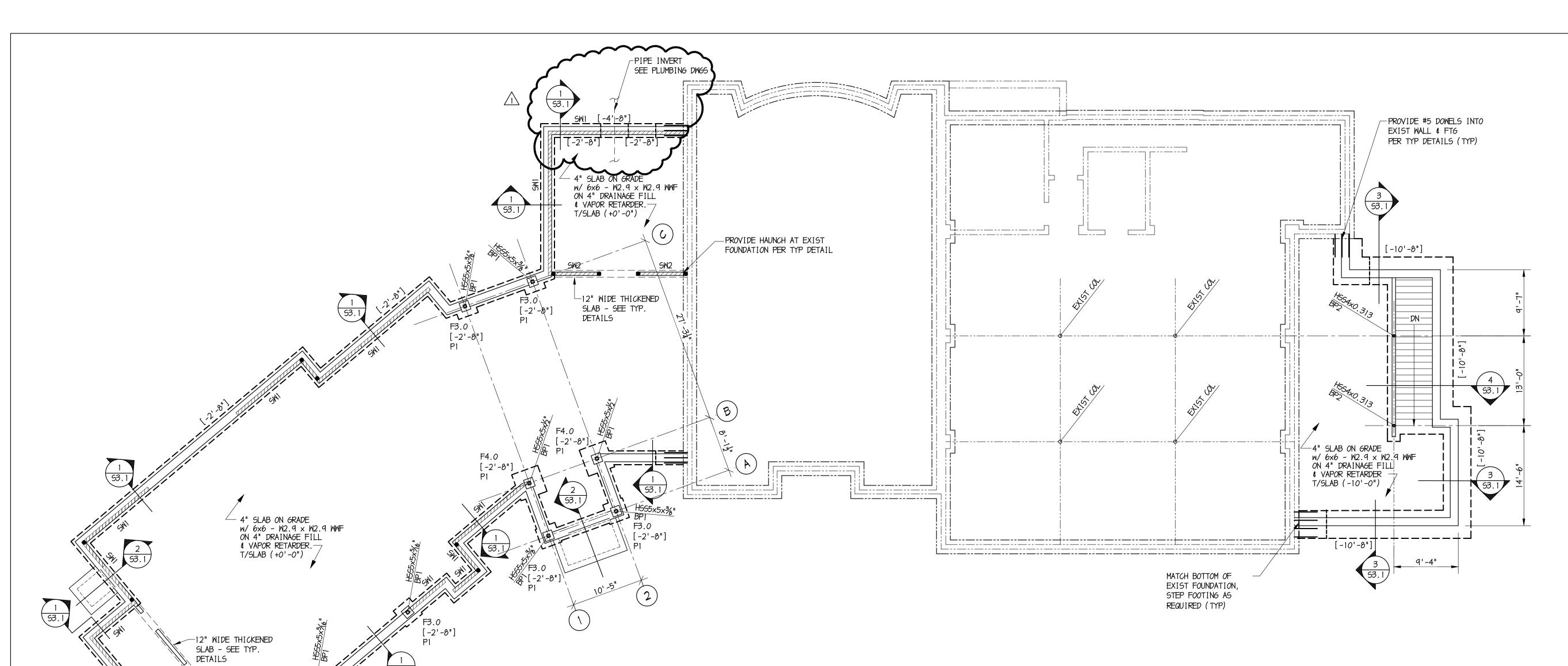
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A PROFESSIONAL A AWRENCE R. BAKER, ENGINEER /

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# FOUNDATION / FIRST FLOOR PLAN NOTES

- . FIRST FLOOR IS REFERENCE ELEVATION (0'-0") = DATEM ELEV. OF 403.50'. 2. ELEVATIONS NOTED AS FOLLOWS ARE WITH RESPECT TO REFERENCE ELEV (0'-0"). [-#'-#] INDICATES TOP OF FOOTING ( -# ' -#) INDICATES TOP OF PIER
- T/SLAB( -# ' -#) INDICATES TOP OF CONCRETE SLAB 3. FOUNDATION MEMBERS SHALL BE AT THE FOLLOWING ELEVATIONS, UNO:
- PIERS ( -0' -8") 4. FOUNDATION MEMBERS ARE DESIGNATED AS FOLLOWS:

PROVIDE HAUNCH AT EXTERIOR WALL

PER TYP DETAIL (TYP)

- F#.# FOOTING MARK - SEE SCHEDULE. PIER MARK - SEE FOUNDATION DETAILS. P#
- COLUMN BASE PLATE SEE FOUNDATION SECTIONS. 5. COORDINATE WITH ARCH, MECH, ELEC, AND PLMB DRAWINGS FOR FLOOR SLOPES, DRAINS, OPENINGS, DEPRESSIONS, ETC., NOT SHOWN ON THIS PLAN, AT ALL
- TOILETS AND OTHER ROOMS. 6 REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT INDICATED. 7. EXISTING CONSTRUCTION SHOWN THUS \_\_\_\_\_\_, TO BE FIELD VERIFIED
- PRIOR TO DETAILING, FABRICATION AND CONSTRUCTION. 8 BOTTOM OF FOOTINGS SHALL MATCH ADJACENT BOTTOM OF EXISTING FOOTINGS.
- STEP FOOTINGS AS REQUIRED.
- 9. REFER TO TYPICAL DETAILS ON DRAWING S6.1. 10. REFER TO STRUCTURAL NOTES ON DRAWING S1.1.
- 11. NEW CONCRETE SHALL BE 4000 PSI PROPORTIONED TO ATTAIN HIGH EARLY STRENGTH, UNO.
- 12. SHOULD EXISTING CONDITION DIFFER DRASTICALLY FROM THOSE SHOWN, CONTACT THE ARCHITECT BEFORE PROCEEDING.

	FOOTING SCHED	
MARK	SIZE	REINFORCING (EACH WAY - BOT)
F3.0	3'-0 x 3'-0 x 1'-0	3#5
F4.0	4'-0 x 4'-0 x 1'-0	4#5

			<u>S</u>	HEA	R WALL	SCHEDU!			
MARK	SHEAT	LINE	NAIL	_ INF	ORMATION	BLOCKED	ום	ACEMENT NOTES	
MARK	SHLAII	HING	EDO	SE	FIELD	EDGES	FL/	ACLIVENT NOTES	
SW1	½" PLYM	NOOD	10d (	9 6"	10d @ 12"	YES	ON EXT	ERIOR FACE OF WALL	
SM2	%" GYP	SUM	6d (	9 7"	6d @ 7"	NO	EACH F	ACE OF INTERIOR WALL	
UN PL		MINIMUM	STUD		ANCHOR F	ROD INFORMA	TION	MINIMUM EMBE	D DEPTH
MARK	'HOLDOWN	MEMB	ER	FOUN:	DATI <i>o</i> n wal	_ SLA	AB/DECK	FOUNDATION WALL	SLAB/DECK
	S/HDU4	(2)20 GA	. STUDS	5/8"	DIA. SSTB24	%" DIA. HI	LTI HIT HY-200	20 %"	5 3/4"

FOUNDATION PLAN

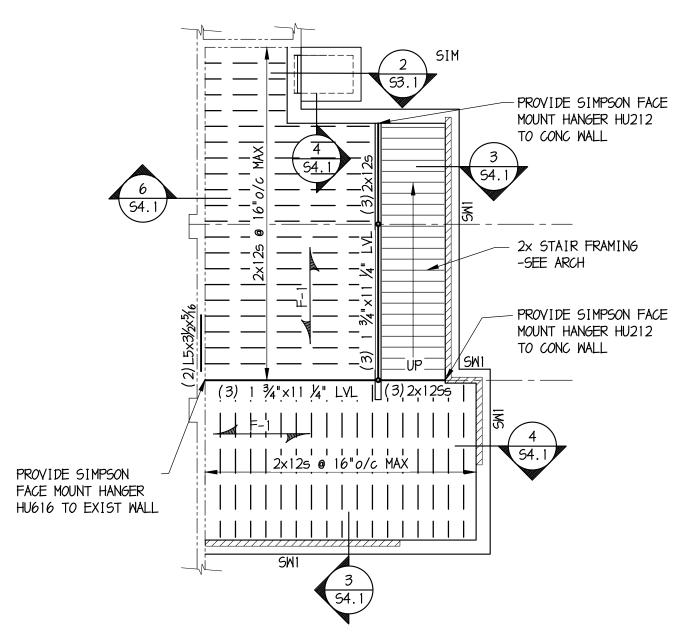
1/8" = 1'-0

# SHEAR WALL NOTES:

52.1

INDICATES SHEAR WALL.

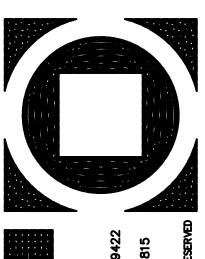
- . SHEAR WALL STUD FRAMING OF COLD-FORMED METAL STUDS. INSTALL HOLD-DOWN ANCHORS IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS AND RECOMMENDATIONS.
- WHERE ONE LAYER OF SHEATHING IS NOTED, SHEATHING IS TO BE APPLIED TO ONE FACE OF SHEAR WALL ONLY.
- WHERE TWO LAYERS OF SHEATHING IS NOTED, SHEATHING IS TO BE APPLIED CONTINUOUSLY TO BOTH FACES OF
- SEE TYPICAL SHEAR WALL DETAILS ON S6.2.

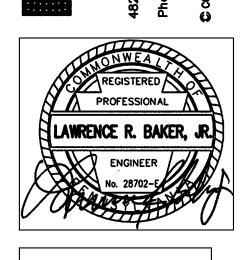


# PARTIAL FLOOR FRAMING PLAN 1/8" = 1'-0

# FLOOR FRAMING PLAN NOTES

- ELEVATIONS ARE NOTED AS (+#'-#") ABOVE THE REFERENCE ELEVATION (0'-0").
- T/FLOOR IS (+0'-0"), TYP UNO. 3. FLOOR CONSTRUCTION TYPE F-1 SHALL BE 3/4" PLYWOOD.
- F. ROOF CONSTRUCTION TYPE R-1 SHALL BE 3/4" PLYWOOD ROOF DECK.
- SYMBOL SHOWN THUS PL# INDICATES BEAM BEARING PLATE SEE TYPICAL DETAIL. 6. EQUALLY SPACE FLOOR FRAMING MEMBERS, UNO, WITH QUANTITY SHOWN ON PLAN.
- REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT INDICATED. 8. EXISTING CONSTRUCTION SHOWN THUS ----- , TO BE FIELD VERIFIED PRIOR TO DETAILING, FABRICATION AND CONSTRUCTION.
- 9. PROVIDE LINTELS ACCORDING TO STRUCTURAL NOTES AND LINTEL SCHEDULE.
- 10. REFER TO TYPICAL DETAILS ON DRAWING S6.1.
- 11. REFER TO STRUCTURAL NOTES ON DRAWINGS S1.1.

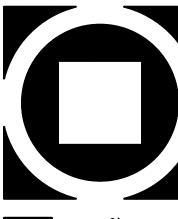




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# ROOF FRAMING PLAN NOTES

- . ROOF REFERENCE DATUM IS ELEVATION 0'-0".
- 2. ELEVATIONS ARE NOTED AS FOLLOWS, REFERENCED FROM THE DATUM ELEVATION: ROOF TRUSS BEARING SHALL BE (+10'-0"), UNO.
- 3. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT INDICATED.
- 4. ROLLED STEEL SHALL BE ASTM A992, UNO.
- 5. ROOF TRUSSES SHALL BE EQUALLY SPACED AT 2'-0" c/c MAX., UNO.
- 6. ROOF CONSTRUCTION SHALL BE 5/8" PLYWOOD DECKING, UNO.
- 7. ATTACH PLYWOOD ROOF DECK w/ 10d NAILS @ 6" o.c. AT ALL SUPPORTED PANEL EDGES. ATTACH w/ 10d NAILS @ 12" o.c. AT ALL INTERMEDIATE FRAMING MEMBERS. PLYWOOD PANEL EDGES SHALL BE STAGGERED 4'-0" BETWEEN ROWS.
- SEE PLAN FOR PANEL SPAN DIRECTION.
  8. LINTELS SHOWN ON PLAN REFER TO THE OPENINGS BELOW THE REFERENCE FLOOR.
  PROVIDE LINTELS ACCORDING TO THE LINTEL SCHEDULE FOR ALL OPENINGS NOT SHOWN
- PRIOR TO DETAILING, FABRICATION AND CONSTRUCTION.
- 10. SYMBOL SHOWN THUS ◀ ▶INDICATES MOMENT CONN SEE TYPICAL DETAILS.
- 11. REFER TO TYPICAL DETAILS ON DRAWING S6.1 & S6.2.
- 12. ALL BUILT UP STUDS SHALL BE CONT TO FND, TYP.
  13. REFER TO STRUCTURAL NOTES ON DRAWING S1.1.



Blue Bell, PA 19422
Facsimile: 610.834.7815

Architecture+S 12 Norristown Road, Suite 200 Blue

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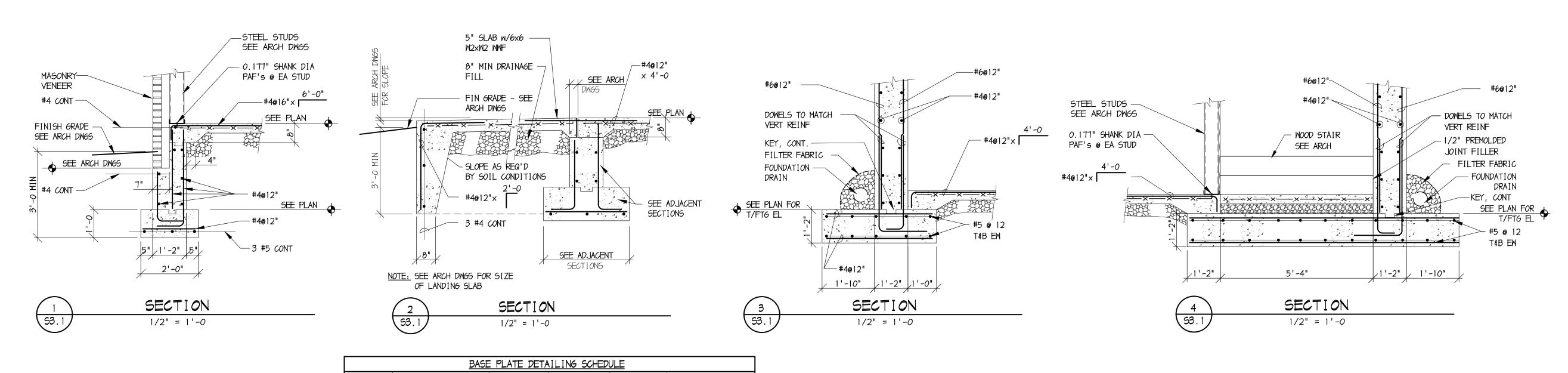
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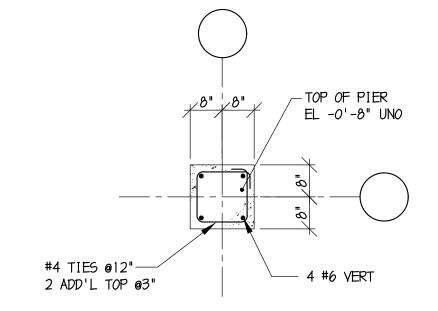
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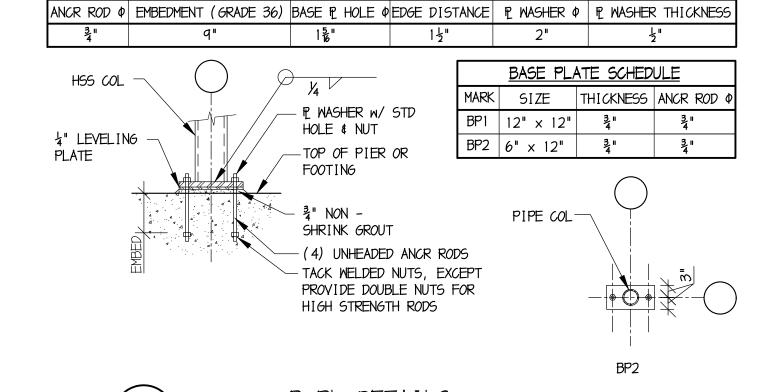
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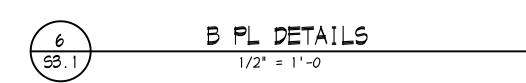
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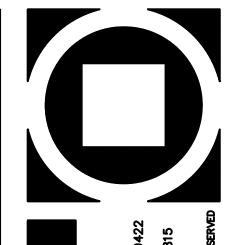




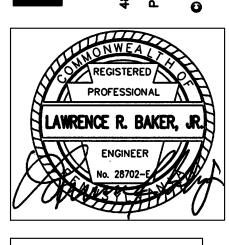








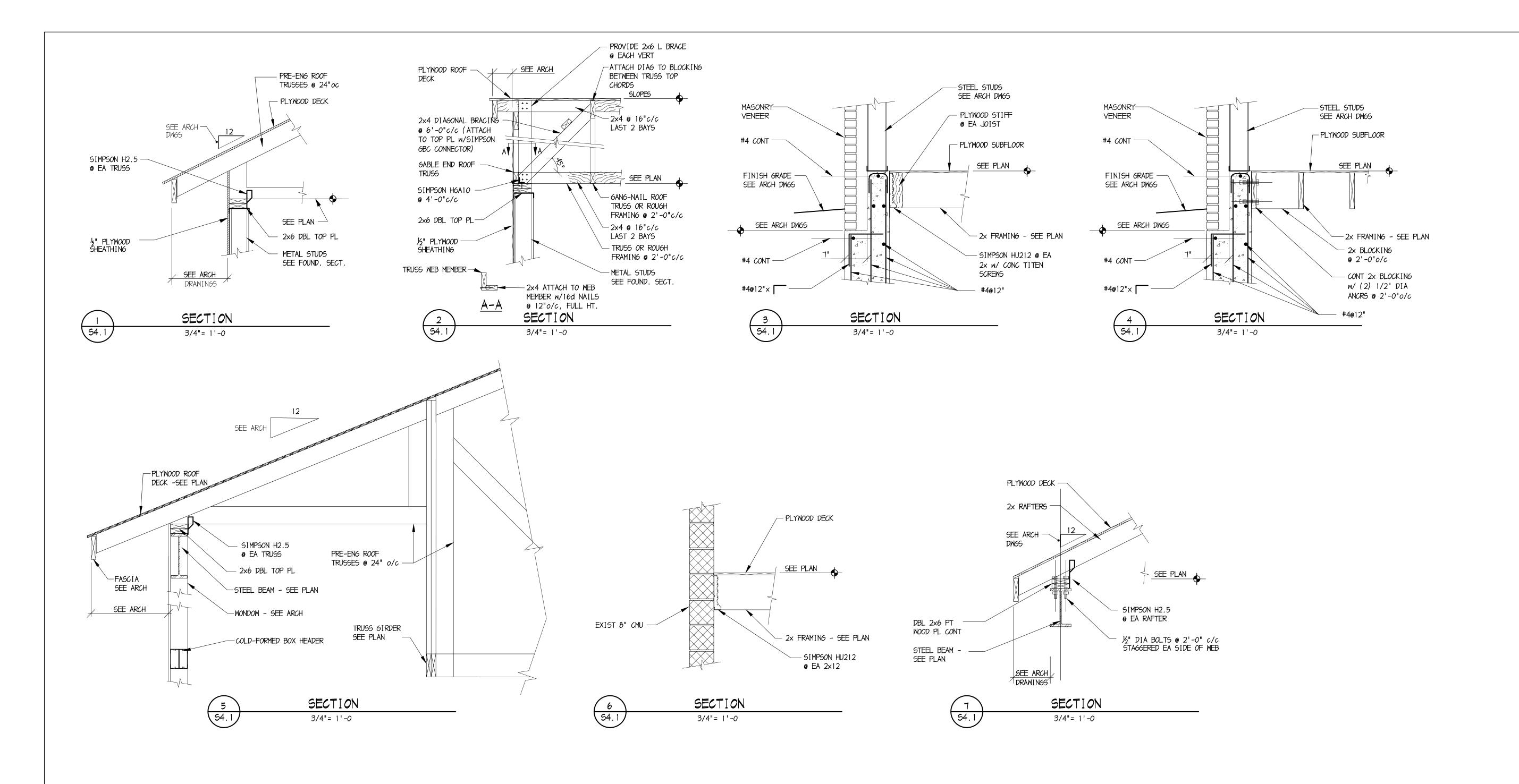
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Archite cture + Site
482 Norristown Road, Suite 200 Blue Bell, PA 1942

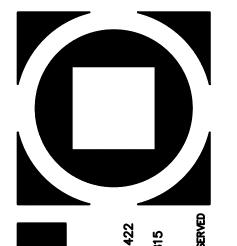


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1385 CAMPUS DRIVE
WEST BRADFORD, PA 19335

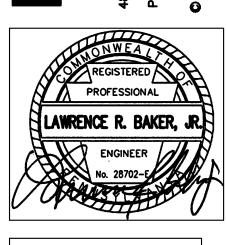
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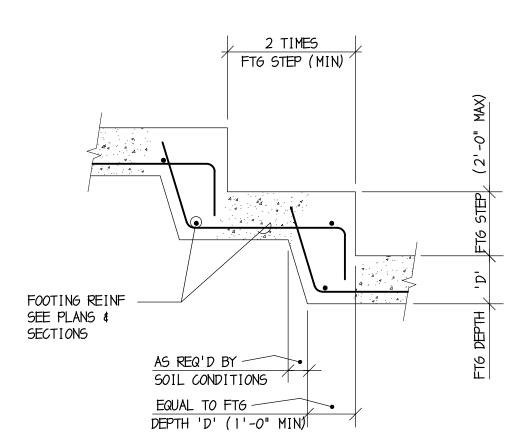
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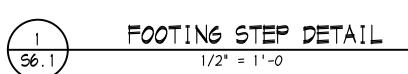
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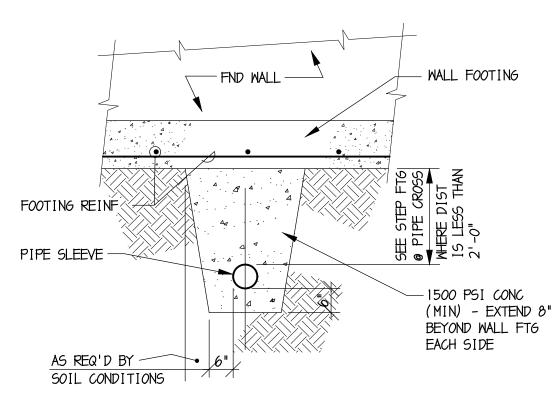
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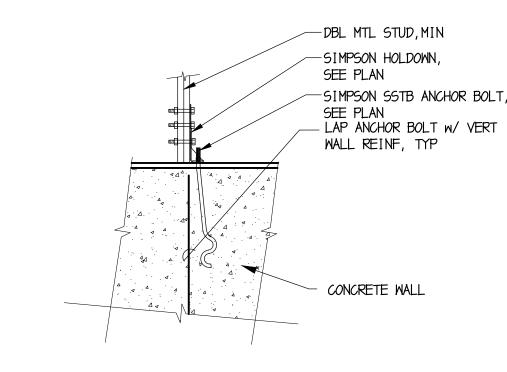
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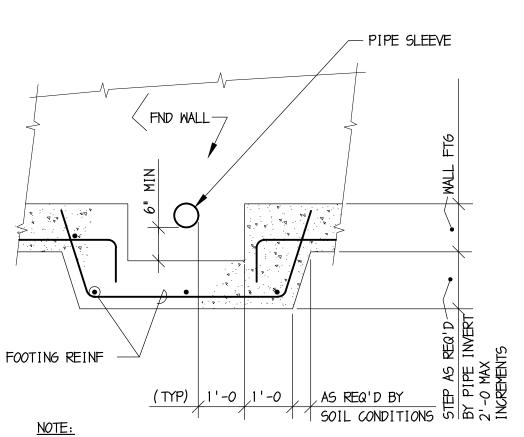




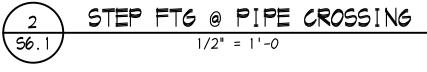


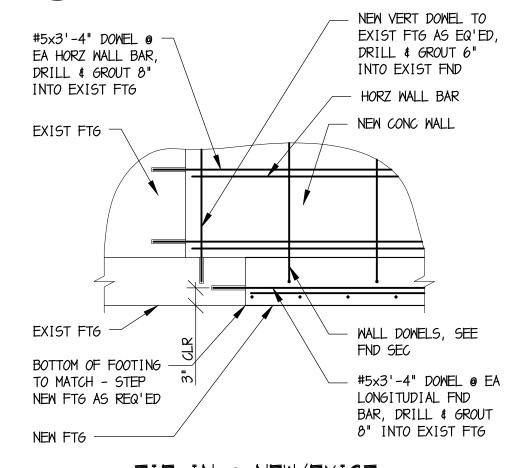
NOTE: ATTACH HOLDOWN W/ MANUF. SPECIFIED HARDWARE



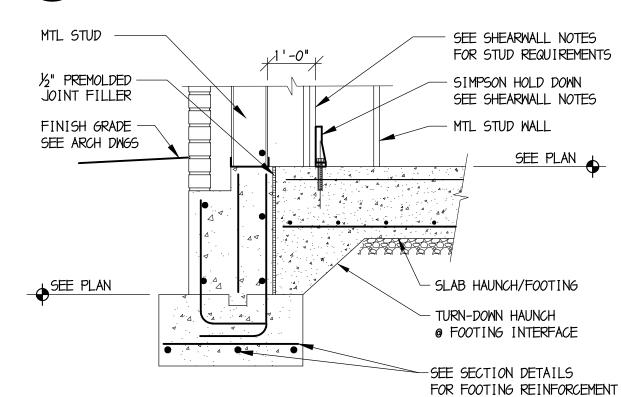


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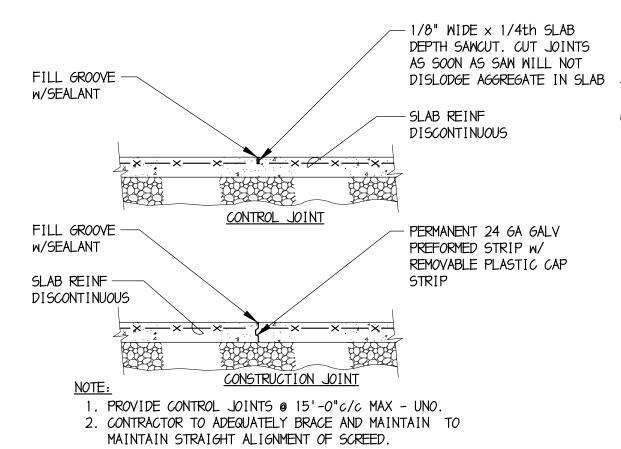




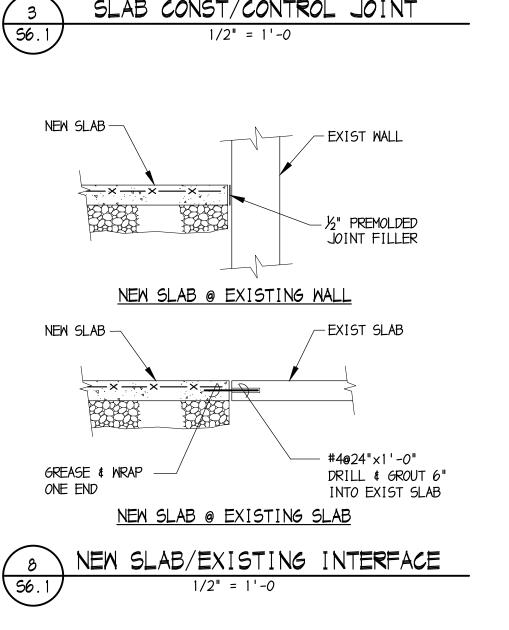
TIE IN @ NEW/EXIST CONCRETE WALL & FOOTING 1/2" = 1'-0

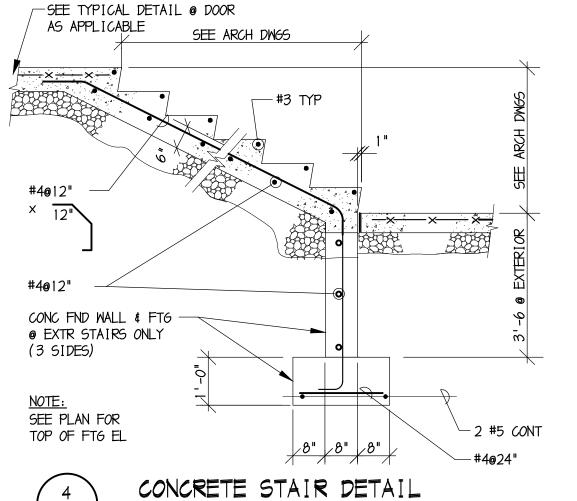




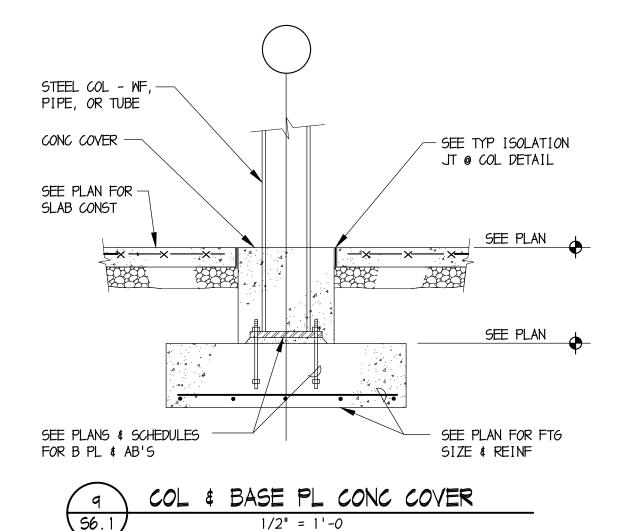


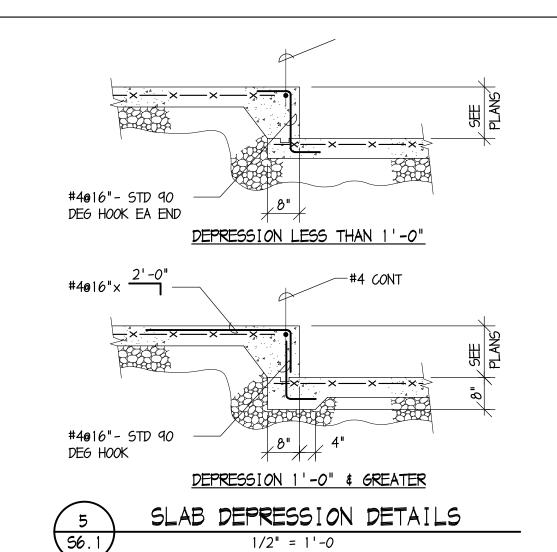
SLAB CONST/CONTROL JOINT

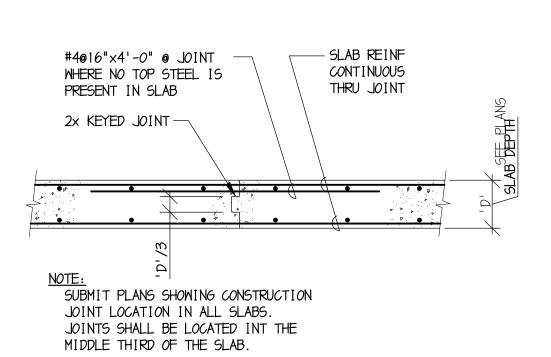




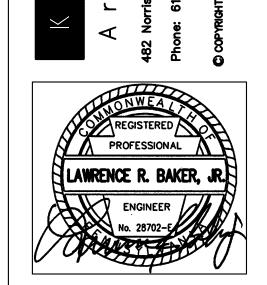
1/2" = 1'-0







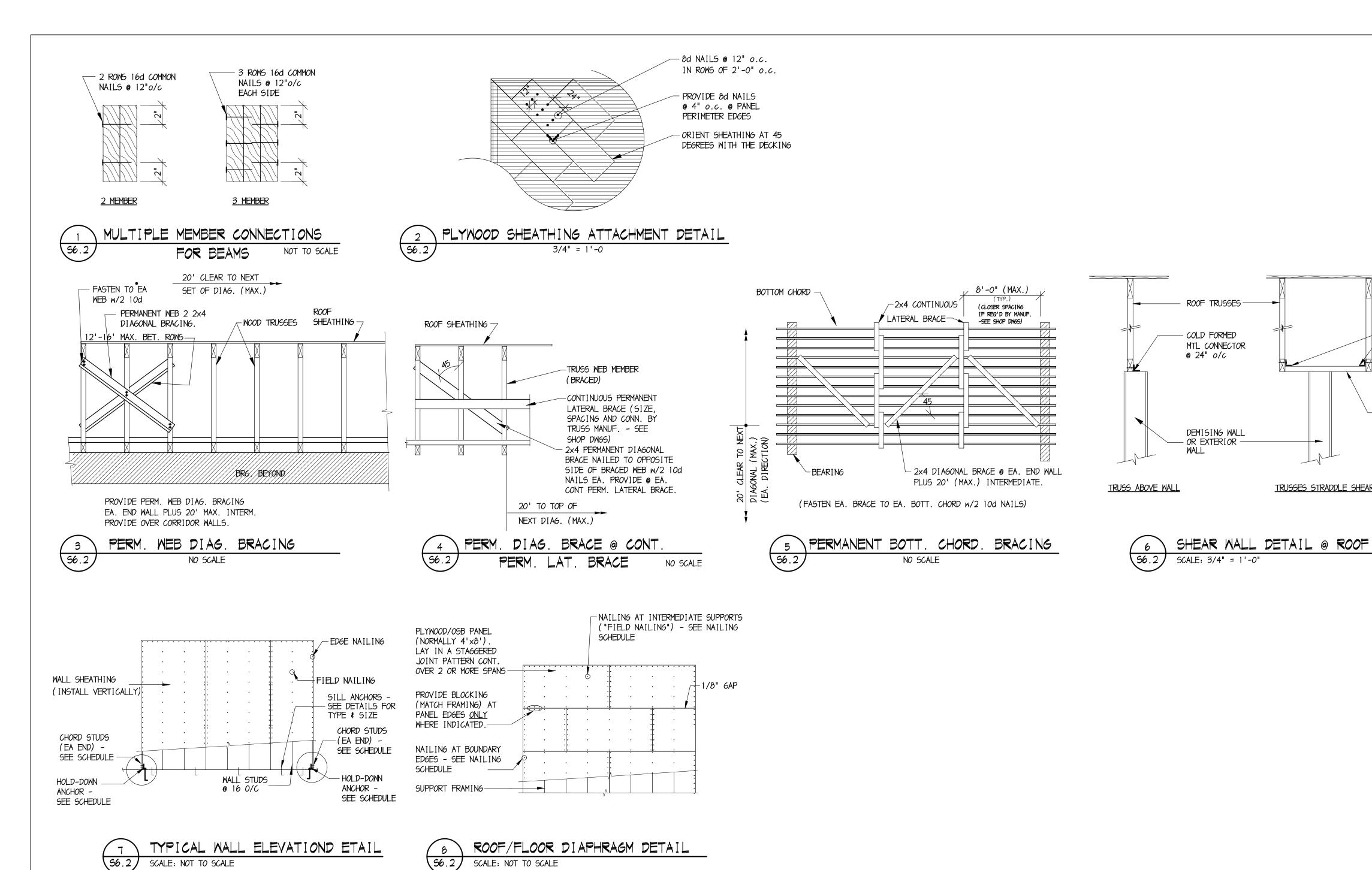


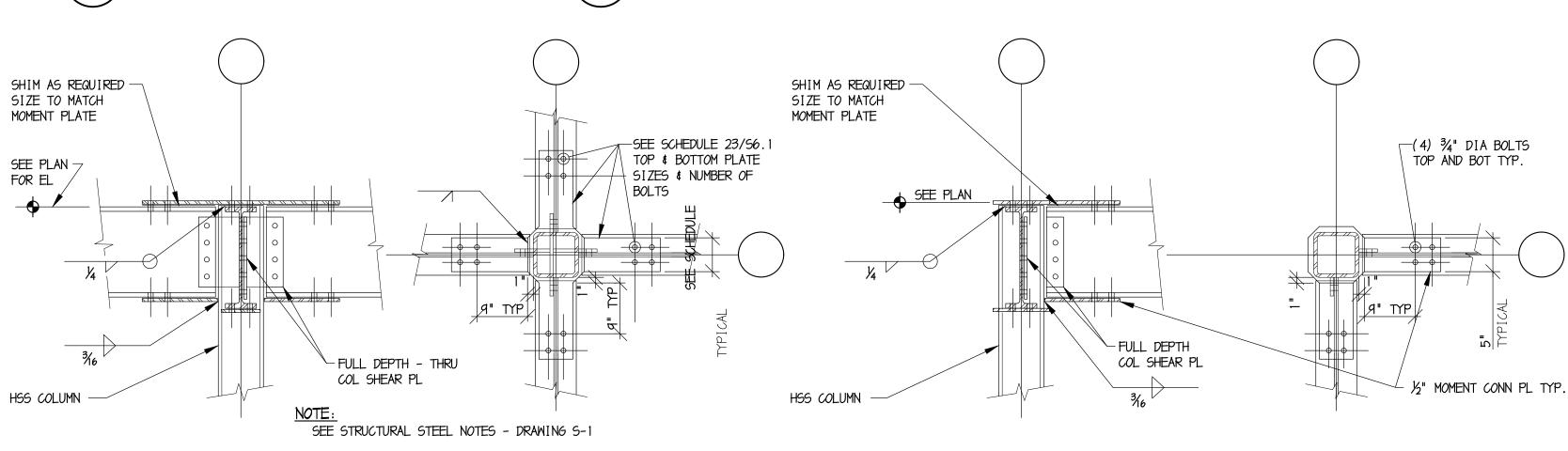


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- COLD FORMED

@ 24" o/c

MTL CONNECTOR

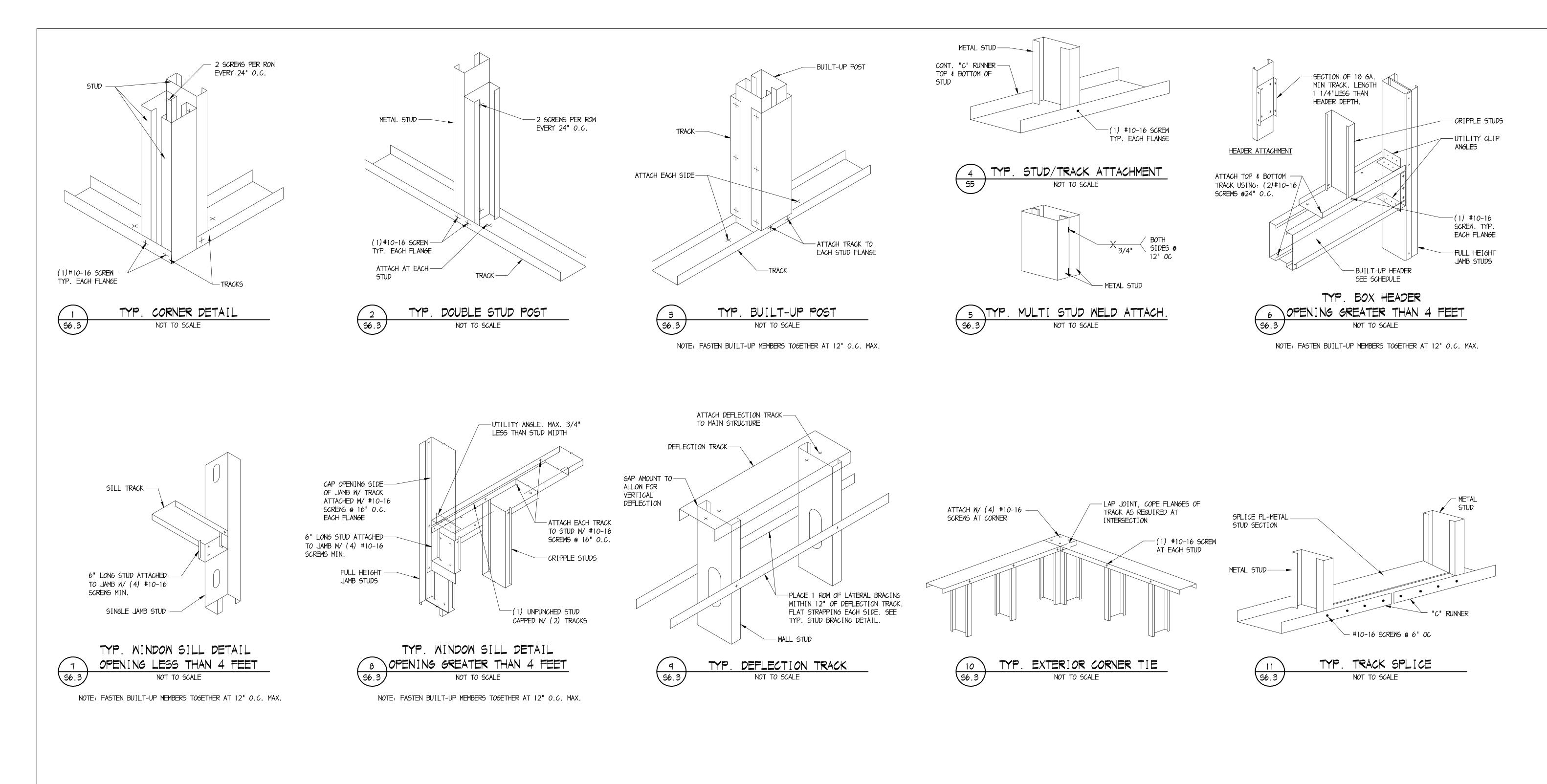
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TRUSSES STRADDLE SHEAR WALL

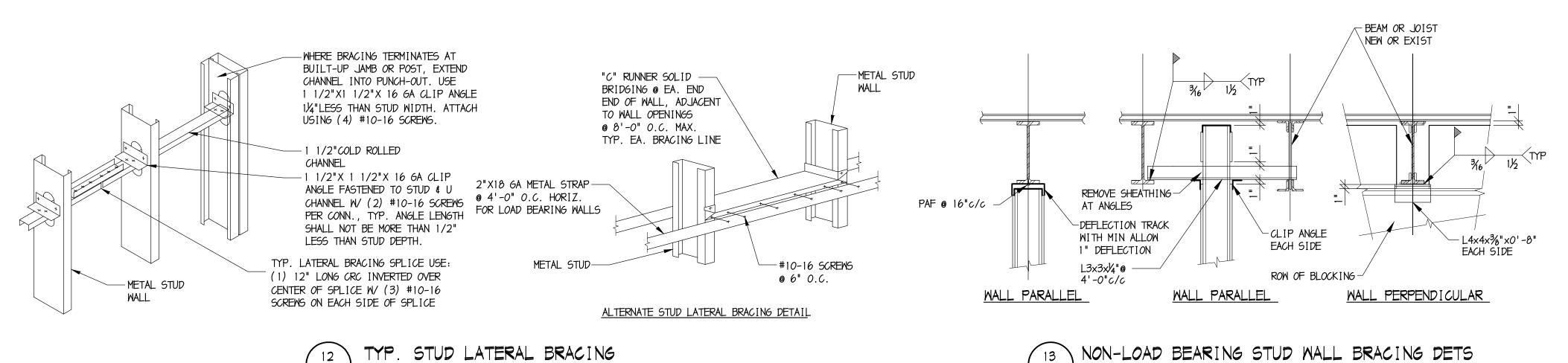
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3/4" = 1'-0



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