

GENERAL	
PARAGRAPH	NOTES
G1	ALL WORK SHALL BE COMPLETED IN ACCORDANCE WITH ALL DRAWINGS AND SPECIFICATIONS CONTAINED HEREIN.
G2	ALL WORK RELATED TO THE STAGING, CONSTRUCTION PRACTICES, AND SAFETY OF THE PROJECT'S WORKERS AND PROPERTY SHALL BE CONSIDERED MEANS AND METHODS AND SHALL BE CONTROLLED BY THE CONTRACTOR IN ACCORDANCE WITH STANDARD INDUSTRY PRACTICE AND ALL CODES AND STANDARDS. VISITS TO THE SITE MADE BY THE ENGINEER ARE FOR THE REVIEW OF THE STRUCTURAL WORK FOR GENERAL CONFORMANCE WITH THE DRAWINGS AND SPECIFICATIONS AND ARE NOT FOR THE REVIEW OF CONTRACTOR RESPONSIBILITIES, INCLUDING BUT NOT LIMITED TO PROJECT SAFETY AND MEANS AND METHODS OF CONSTRUCTION.
G3	ALL DRAWINGS HAVE BEEN PREPARED IN ACCORDANCE WITH THE 2009 INTERNATIONAL BUILDING CODE, PENNSYLVANIA UNIFORM CONSTRUCTION CODE, AS WELL AS ALL REFERENCED STANDARDS CONTAINED THEREIN.
G4	EVALUATION AND COMPLIANCE WITH LOADING RESTRICTIONS FOR MEANS AND METHODS OF CONSTRUCTION AS WELL AS STAGING FOR OTHER TRADES ARE THE RESPONSIBILITY OF THE CONTRACTOR.
G5	ALL WORK SHALL BE INSPECTED IN ACCORDANCE WITH CHAPTER 17 OF THE REFERENCED BUILDING CODE. SUBMIT ALL REPORTS TO THE ENGINEER OF RECORD FOR REVIEW. AT THE COMPLETION OF THE PROJECT, THE SPECIAL INSPECTION REPORT SHALL BE COMPLETED, SIGNED BY THE SPECIAL INSPECTOR, AND SUBMITTED TO THE ENGINEER OF RECORD FOR RECORD PURPOSES.
G6	SCALING OF DRAWINGS TO DETERMINE DIMENSIONS OF ELEMENTS IS NOT PERMITTED. STRUCTURAL DRAWINGS SHALL NOT BE REPRODUCED TO CREATE SHOP DRAWINGS OR WORKING DOCUMENTATION WITHOUT THE EXPRESS WRITTEN CONSENT OF MACINTOSH ENGINEERING.
G7	ALL HORIZONTAL AND VERTICAL DIMENSIONS CONTAINED ON THE STRUCTURAL DRAWINGS WERE DEVELOPED BY OTHER DISCIPLINES FOR THE PURPOSE OF THIS PROJECT. ANY DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHOULD BE COORDINATED WITH THE OTHER DISCIPLINE DRAWINGS.
G8	THE STRUCTURAL DOCUMENTS ARE TO BE USED IN COORDINATION WITH THE ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS AND SPECIFICATIONS AS WELL AS THOSE OF ALL OTHER DISCIPLINES. ANY DISCREPANCIES SHOULD BE BROUGHT TO THE ATTENTION OF THE DESIGN TEAM PRIOR TO THE COMMENCEMENT OF WORK.
G9	ALL REQUESTED CHANGES IN WORK BY THE CONTRACTOR ARE SUBJECT TO THE APPROVAL OF THE DESIGN TEAM AND OWNER AND ARE CONSIDERED TO BE COMPLETED AT NO ADDITIONAL COST UNLESS SPECIFICALLY APPROVED. APPROVAL OF REQUESTED CHANGES DOES NOT CONSTITUTE APPROVAL OF AN INCREASE IN PROJECT COSTS.
G10	REFER TO THE ARCHITECTURAL DOCUMENTATION FOR LOCATION, EXTENT, AND DETAILING OF ALL WATERPROOFING AND FIREPROOFING.
G11	DESIGN LOADS FOR THE PROJECT ARE LISTED IN THE LOAD SCHEDULE ON DRAWING 5003.
G12	SNOW LOADS FOR THE PROJECT ARE LISTED IN THE LOAD SCHEDULE ON DRAWING 5003.
G13	WIND AND SEISMIC LOADS FOR THE PROJECT ARE LISTED IN THE LOAD SCHEDULE ON DRAWING 5003.
G14	

SHOP DRAWING REQUIREMENTS	
PARAGRAPH	NOTES
SD1	SHOP DRAWINGS SHALL BE SUBMITTED FOR THE FOLLOWING ITEMS FOR THIS THE PROJECT:
SD1.1	CONCRETE MIX DESIGNS INCLUDING ALL LABORATORY TESTING, MATERIALS, ETC.
SD1.2	REINFORCING SHOP DRAWINGS
SD1.3	ANCHOR BOLT AND CONCRETE EMBEDDED ASSEMBLIES
SD1.4	STEEL FRAMING
SD1.5	METAL DECK ASSEMBLIES
SD1.6	STAIRS, HANDRAILS AND GUARDRAILS
SD1.7	COLD FORMED METAL FRAMING
SD1.8	WOOD TIMBER FRAMING
SD1.9	MASONRY PRODUCTS
SD1.10	ALL ADMIXTURES, SEALANTS, HARDENERS, COATINGS
SD2	ALL SHOP DRAWINGS NOTED ABOVE SHALL BE SUBMITTED IN A TIMELY MANNER TO ALLOW FOR A 10 BUSINESS DAY REVIEW PERIOD BY THE DESIGN TEAM. ALL SUBMITTED DRAWINGS SHALL CONTAIN THE CONSTRUCTION MANAGER REVIEW STAMP.
SD3	ELECTRONIC SHOP DRAWINGS SHALL BE SUBMITTED AS AN ORGANIZED SINGLE FILE DOCUMENT. DRAWINGS SHALL BE ORGANIZED IN NUMERIC ORDER WITH ALL REFERENCED PLANS LOCATED FIRST IN THE SUBMITTAL.
SD4	SHOP DRAWINGS WILL BE MARKED AS NOTED ON THE REVIEW STAMP. SHOP DRAWINGS MARKED "MAKE CORRECTIONS NOTED" ARE TO BE RE-SUBMITTED FOR RECORD PURPOSES AND WILL NOT BE RE-REVIEWED AS AN ADDITIONAL SUBMITTAL. REVIEW OF "MAKE CORRECTIONS NOTED" SHOP DRAWINGS BEYOND ONE RE-SUBMITTAL WILL REQUIRE ADDITIONAL FEE.
SD5	SUBMITTALS REQUIRING THE SEAL OF A PROFESSIONAL ENGINEER (I.E. UNDERPINNING, PRECAST CONCRETE, ETC.) SHALL BE SUBMITTED WITH CALCULATIONS AND SEALED DRAWINGS PRIOR TO REVIEW.
SD6	CONTRACTOR SHALL PROVIDE DESIGN TEAM WITH A SHOP DRAWING SUBMITTAL SCHEDULE TO ALLOW THE ENGINEERING TEAM APPROPRIATE NOTICE OF SUBMITTALS, DUE DATES, AND ALLOW FOR APPROPRIATE STAFFING. SCHEDULE SHALL BE PROVIDED PRIOR TO THE FIRST SUBMITTAL.

EXISTING CONDITIONS	
PARAGRAPH	NOTES
E1	THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, COORDINATION AND INSTALLATION OF SHORING AND STABILIZATION OF EXISTING CONSTRUCTION AS REQUIRED TO PERFORM THE WORK CONTAINED IN THE DRAWINGS AND SPECIFICATIONS. DIMENSIONS SHOWN REFERRING TO EXISTING STRUCTURES ARE FOR REFERENCE ONLY. ALL DIMENSIONS RELATED TO EXISTING BUILDINGS AND FRAMING SHOULD BE VERIFIED BY THE CONTRACTOR PRIOR TO THE COMMENCEMENT OF WORK.
E2	THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY INFORMATION RELATING TO THE EXISTING STRUCTURE THAT HAS BEEN UNCOVERED DUE TO DEMOLITION AND REMOVAL OF FINISHES.
E3	NEVER CONNECT NEW FRAMING MEMBERS TO EXISTING BRICK OR OTHER MASONRY VENEER WITHOUT APPROVAL FROM THE ENGINEER OF RECORD. REPLACE ANY REMOVED VENEER TO MATCH EXISTING AFTER WORK IS COMPLETE.
E4	

FOUNDATIONS	
PARAGRAPH	NOTES
F1	PERFORM ALL FOUNDATION PREPARATION, EXCAVATION, PLACEMENT OF STRUCTURAL FILL AND / OR SOIL IMPROVEMENT WORK IN STRICT ACCORDANCE WITH THE REPORT OF GEOTECHNICAL INVESTIGATION PREPARED BY EARTH ENGINEERING INCORPORATED (PROJECT NO. 3109100, DATED OCTOBER 1, 2018)
F1.1	EXCAVATE THE BUILDING FOUNDATION AREAS TO THE DEPTH AND EXTENT INDICATED IN THE GEOTECHNICAL REPORT AND FOUNDATION DRAWINGS. ALL FOOTINGS AND SLAB SUBGRADES SHALL BE APPROVED IN WRITING BY THE GEOTECHNICAL ENGINEER PRIOR TO BACKFILLING. SUBMIT ALL REPORTS TO THE ENGINEER OF RECORD FOR RECORD.
F2	BOTTOM OF FOUNDATIONS SHALL BEAR ON SOIL CAPABLE OF SAFELY SUPPORTING 3000 PSF IN ACCORDANCE WITH THE ABOVE REFERENCED GEOTECHNICAL REPORT.
F4	BOTTOM OF FOOTING SUBGRADE MUST BE INSPECTED AND APPROVED BY A REGISTERED GEOTECHNICAL ENGINEER BEFORE PLACING ANY CONCRETE FOUNDATIONS. APPROVAL IN WRITING MUST INDICATE THE SOIL IS ADEQUATE TO SAFELY SUSTAIN THE SPECIFIED BEARING PRESSURE. SUBMIT ALL REPORTS TO THE ENGINEER OF RECORD FOR RECORD.
F5	BOTTOM OF ALL FOOTINGS SUBJECTED TO FREEZE THAN CONDITIONS SHALL BE A MINIMUM 3'-0" FEET BELOW FINISH GRADE OR TOP OF SLAB ELEVATION UNLESS OTHERWISE NOTED.
F6	RETAINING WALLS SHALL BE BACKFILLED AND COMPACTED WITH MATERIAL PRODUCING A MAXIMUM ACTIVE EQUIVALENT FLUID LATERAL EARTH PRESSURE OF 48 PSF.
F7	WALLS RETAINING EARTH SHALL NOT BE BACKFILLED UNTIL A MINIMUM OF 10% OF SPECIFIED COMPRESSIVE STRENGTH IS ACHIEVED. BASEMENT WALLS SHALL NOT BE BACKFILLED UNLESS ADEQUATELY BRACED UNTIL FLOOR SLAB IS IN PLACE AND ATTAINS A MINIMUM OF 10% OF SPECIFIED COMPRESSIVE STRENGTH.
F8	SITE RETAINING WALLS, EXPOSED CONCRETE WALLS SHALL HAVE CONTROL JOINTS A MAXIMUM OF 20 FEET ON CENTER UNLESS OTHERWISE NOTED ON THE DRAWINGS. CONCRETE WALLS WITH INTEGRAL COLUMN PIERS OR PILASTERS SHALL HAVE A FORMED CONTROL JOINT ON ONE SIDE OF EACH PIER ON THE EXPOSED FACE OF THE WALL. JOINTS SHALL BE FILLED WITH SEALANT AS NOTED ON THE ARCHITECTURAL DRAWINGS.

CONCRETE	
PARAGRAPH	NOTES
C1	ALL CONCRETE SHALL BE READY-MIX AND PROPORTIONED ON THE BASIS OF LABORATORY TRIAL MIXTURE OR FIELD TEST DATA OR BOTH ACCORDING TO AC308. DESIGN MIXTURE SHALL MEET THE REQUIREMENTS BELOW.
C1.1	SLABS ON GRADE:
C1.1.1	COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS MINIMUM.
C1.1.2	EXPOSURE CATEGORY: F2
C1.2	FOOTINGS AND FOUNDATION WALLS
C1.2.1	COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS MINIMUM.
C1.2.2	EXPOSURE CATEGORY: F2
C2	ALL CONCRETE EXPOSED TO EXTERIOR CONDITIONS SHALL HAVE CHARACTERISTICS IN ACCORDANCE WITH ACI BUILDING CODE (ACI 318) AND THE 2009 INTERNATIONAL BUILDING CODES AND PENNSYLVANIA UNIFORM CONSTRUCTION CODE.
C3	CONTRACTOR IS RESPONSIBLE FOR THE PREPARATION OF DESIGN MIXTURES FOR EACH APPLICATION/LOCATION USED IN CONSTRUCTION AS NOTED ABOVE AND ON THE DRAWINGS.
C4	ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE LATEST EDITIONS OF THE ACI BUILDING CODE (ACI 318), THE ACI DETAILING MANUAL (318R-88), AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301)
C5	ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL CONFORMING TO ASTM DESIGNATION A615 GRADE 60. LAP ALL BARS MINIMUM 48 BAR DIAMETERS UNLESS OTHERWISE NOTED.
C6	ALL WVF SHALL BE MANUFACTURED FROM HIGH STRENGTH STEEL CONFORMING TO ASTM A193. LAP ALL WVF MINIMUM OF 8 INCHES.
C7	CONCRETE SLAB ON GRADE SHALL BE FINISHED TO TOLERANCE FOR FLOOR FLATNESS (FF) OF 25 AND FLOOR LEVELNESS (FL) OF 20 UNLESS OTHERWISE MANDATED BY ARCHITECTURAL FINISH REQUIREMENTS.
C7.1	ALL CONCRETE SLAB ON GRADE SHALL BE TESTED FOR FLOOR FLATNESS AND LEVELNESS WITHIN 48 HOURS OF THE SLAB ON GRADE PLACEMENT. CONTRACTOR SHALL SUBMIT REPORTS TO THE ENGINEER AND ARCHITECT OF RECORD AND ALL SPECIALTY FLOORING SUB-CONTRACTORS FOR REVIEW.
C7.2	CONTRACTOR SHALL CONDUCT A PRE-INSTALLATION CONFERENCE WITH ALL FLOORING SUB-CONTRACTORS PRIOR TO THE PLACEMENT OF THE SLAB ON GRADE.
C8	PLACE TRANSVERSE REINFORCING (SMB) IN BOTTOM LAYER OF CONTINUOUS FOOTINGS. PROVIDE CORNER BARS IN FOOTINGS TO MATCH CONTINUOUS REINFORCEMENT. EXTEND WALL FOOTING REINFORCING INTO COLUMN FOOTINGS A MINIMUM OF 2 FEET.
C9	PROVIDE KEYS IN CONCRETE WALLS, PIERS, AND FOOTINGS AT INTERSECTIONS UNLESS NOTED OTHERWISE. PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCEMENT AT WALL CORNERS AND TEE INTERSECTIONS.
C10	CONCRETE SHALL ACHIEVE A MINIMUM OF 10 PERCENT OF THE DESIGN STRENGTH PRIOR TO STEEL ERECTION. WRITTEN CONFIRMATION OF THIS STRENGTH SHOULD BE SUBMITTED TO THE ENGINEER OF RECORD PRIOR TO THE COMMENCEMENT OF STEEL ERECTION.
C11	SHOP DRAWINGS FOR CONCRETE MIX DESIGNS SHALL INCLUDE THE FOLLOWING INFORMATION:
C11.1	MIXTURE IDENTIFICATION BY APPLICATION/LOCATION
C11.2	SPECIFIED COMPRESSIVE STRENGTH, FC, THAT IS APPLICABLE FOR THE APPLICATION
C11.3	SPECIFIED EXPOSURE CLASS
C11.4	DOCUMENTATION OF STRENGTH TEST RECORDS OF SIMILAR CLASS OF CONCRETE USED TO ESTABLISH STANDARD DEVIATION IN ACCORDANCE WITH ACI 318, WHEN TEST RECORDS EXIST
C11.5	REQUIRED AVERAGE COMPRESSIVE STRENGTH, FCR, FOR EACH CLASS OF CONCRETE
C11.6	DOCUMENTATION OF REQUIRED AVERAGE COMPRESSIVE STRENGTH, FCR, USED AS THE BASIS FOR SELECTION OF CONCRETE PROPORTIONS
C11.7	INTENDED PLACEMENT METHOD
C11.8	SLUMP OR SLUMP FLOW
C11.9	AIR CONTENT
C11.10	DRY AND NET DENSITY
C11.11	W/C RATIO
C11.12	DOCUMENTATION SUPPORTING OTHER SPECIFIED REQUIREMENTS OF CONCRETE MIXTURES
C11.13	NOMINAL MAXIMUM AGGREGATE SIZE OR SIZE NUMBER
C11.14	TYPE AND INFORMATION ABOUT THE INGREDIENT MATERIALS PROPOSED FOR USE.
C12	CONCRETE TESTING SHALL CONFORM TO THE FOLLOWING:
C12.1	SAMPLES SHALL BE TAKEN AT LEAST ONCE PER DAY AND ONCE FOR EACH 300cy OR 3000sf OF PLACED CONCRETE
C12.2	TAKE SLUMP, AIR, TEMPERATURE FOR EACH CONCRETE CYLINDER SET TAKEN
C12.3	CYLINDER TESTS SHALL BE AS FOLLOWS:
C12.3.1	TEST ONE SET OF CYLINDERS AT 1 DAYS
C12.3.2	TEST ONE SET OF CYLINDERS AT 28 DAYS
C12.3.3	TEST ONE SET OF CYLINDERS AT 56 DAYS

STEEL	
PARAGRAPH	NOTES
S1	ALL STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST AISC CODE. ALL STRUCTURAL STEEL WIDE FLANGE (W) SHAPES SHALL BE ASTM A992 GRADE 50 (F50). ALL STRUCTURAL STEEL S, M, AND HP SHAPES SHALL BE ASTM A572 GRADE 50 (F50). ALL STRUCTURAL STEEL SHALL BE ASTM A58 UNLESS OTHERWISE NOTED.
S2	ALL STEEL RECTANGULAR/SQUARE HOLLOW STRUCTURAL SECTIONS SHALL BE ASTM A500 GRADE B, Fy 46 KSI.
S3	ALL STEEL PIPE SECTIONS SHALL BE ASTM A501 OR ASTM A53, TYPE E OR S GRADE B.
S4	ALL STEEL ROUND HOLLOW STRUCTURAL SECTIONS SHALL BE ASTM A500 GRADE B, Fy 42 KSI.
S5	ALL STEEL SHALL BE THOROUGHLY CLEANED IN ACCORDANCE WITH SSPC-SP8 AND SHALL HAVE A SHOP COAT OF RUST INHIBITIVE PAINT COMPATIBLE TO THE FINISH PAINT PRODUCT.
S6	ALL WELDS SHALL BE GROUND SMOOTH TO THE APPROVAL OF THE ENGINEER OF RECORD AND THE ARCHITECT.
S7	ALL PAINT SHALL BE TOUCHED UP TO THE APPROVAL OF THE ARCHITECT.
S8	SHOP AND FIELD WELDING SHALL BE PERFORMED BY WELDERS CERTIFIED AS DESCRIBED IN LATEST EDITION OF THE AMERICAN WELDING SOCIETY'S STANDARD QUALIFICATION PROCEDURE, AWS D11, TO PERFORM THE TYPE OF WORK REQUIRED.
S9	ALL BOLTS USED FOR THE ANCHORAGE TO CONCRETE AS SPECIFIED ON THE DRAWINGS SHALL CONFORM TO ASTM F1554.
S10	ALL CONNECTIONS SHALL BE BOLTED WITH A MINIMUM OF 3/4" A325N HIGH STRENGTH BOLTS OR WELDED AS DESIGNED BY THE STEEL FABRICATOR.
S11	PROVIDE FULL DEPTH DOUBLE ANGLE CONNECTIONS ON ALL GIRDER AND BEAM CONNECTIONS TO COLUMNS. BOLTS SHALL BE AT 3-INCH O/C VERT.
S12	PROVIDE FULL DEPTH DOUBLE ANGLE CONNECTIONS WITH TOP AND BOTTOM CLIP ANGLES (AISC TYPE 2 PR) ON ALL GIRDER AND BEAM CONNECTIONS TO COLUMNS AS NOTED ON DRAWINGS. BOLTS SHALL BE AT 3-INCH O/C. BOLTS IN CLIP ANGLES SHALL BE AS NOTED IN THE DRAWINGS.
S13	PROVIDE A MINIMUM 3/8" INCH THICK FULL DEPTH THRU-PLATE FOR ALL PIPE AND TUBE COLUMN CONNECTIONS UNLESS OTHERWISE NOTED ON THE DRAWINGS.
S14	ALL BEAM TO GIRDER CONNECTIONS SHALL BE AS DESIGNED BY THE FABRICATOR. SUBJECT TO THE ENGINEER'S APPROVAL, THE FOLLOWING CONNECTIONS ARE PERMITTED:
S14.1	DOUBLE ANGLE
S14.3	FABRICATOR SHALL ADHERE TO ALL OSHA FEDERAL REGISTER STANDARDS SECTION 1926.111 WITH REGARD TO CONNECTION DESIGN.
S15	ALL TENSION CONTROLLED BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F1552 AND F2280.
S16	ALL ALUMINUM AND STEEL MEMBERS SHALL BE TREATED OR PROPERLY SEPARATED TO PREVENT GALVANIC AND CORROSIVE EFFECTS.
S17	ALL STEEL WELDING RODS SHALL BE AS FOLLOWS:
S17.1	E70XX FOR STEEL CONNECTIONS
S18	STEEL FABRICATOR IS SOLELY RESPONSIBLE FOR COORDINATING WITH THE GENERAL CONTRACTOR FOR THE PURPOSE OF SURVEYING AND VERIFICATION OF EXISTING CONDITIONS INCLUDING BUT NOT LIMITED TO THE LOCATION, ELEVATION, AND DIMENSIONS OF WALLS AND FRAMING THAT EXIST AT THE TIME OF THE STEEL ERECTION.
S19	ALL LINTELS AND SHELF ANGLES SHALL BE HOT DIPPEP GALVANIZED
S20	ANY POINTS OF WELDING SHALL BE TOUCHED UP IN THE FIELD WITH A ZINC-RICH PAINT BY THE STEEL ERECTOR.
S21	PERIMETER ANGLE AND BENT PLATE AS NOTED ON THE DRAWINGS SHALL BE ADJUSTABLE. ANGLE AND BENT PLATE SHALL BE WELDED AFTER ADJUSTMENT IN THE FIELD.
S22	SPANDREL ANGLE SHALL BE ADJUSTABLE. SHOP ANGLE LOOSE AND SET WITH STRIKING LINE IN FIELD FOR VERTICAL AND HORIZONTAL ALIGNMENT AFTER STEEL IS FULLY ERECTED TO A MAXIMUM TOLERANCE OF 1/4" HORIZONTAL PER BAY/PER FLOOR AND MUST BE SET PLUMB BY STEEL ERECTOR PRIOR TO STEEL ERECTION. ANGLE MUST BE INSTALLED IN ONE LENGTH PER BAY.

MASONRY	
PARAGRAPH	NOTES
M1	MASONRY UNITS SHALL BE AS OUTLINED BELOW.
M1.1	NORMAL WEIGHT MASONRY UNITS
M1.2	ASTM C90 HOLLOW GROUTED SOLID BELOW GRADE.
M1.3	ASTM C90 HOLLOW ABOVE GRADE.
M1.4	MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI (AVERAGE OF 3 UNITS)
M2	ALL CMU SHALL BE LAID IN A FULL BED OF MORTAR.
M3	THE FOLLOWING BLOCK STRENGTHS ARE REQUIRED:
M3.1	ASTM C90 SOLID 2000 PSI ON 96"X96" AREA OF INDIVIDUAL UNITS.
M3.2	ASTM C90 SOLID 1800 PSI ON NET AREA OF AVERAGE OF 3 UNITS PER ACI-530.
M3.3	ASTM C90 HOLLOW 1700 PSI ON NET AREA OF INDIVIDUAL UNITS.
M3.4	IVANY 3000 PSI ON NET AREA OF INDIVIDUAL UNITS.
M3.5	IVANY 3750 PSI ON NET AREA OF AVERAGE OF 3 UNITS
M4	ALL MORTAR SHALL BE ASTM C270 TYPE S AND LAID USING A WITH A MINIMUM COMPRESSIVE STRENGTH OF 1000 PSI AT 28 DAYS.
M5	ALL MORTAR SHALL BE FIELD OBTAINED MORTAR CUBES TESTED IN ACCORDANCE WITH ASTM C270 AND ASTM C780.
M6	ALL IVANY BLOCK CONSTRUCTION SHALL BE LAID USING ASTM C270 TYPE M MORTAR WITH A MINIMUM COMPRESSIVE STRENGTH OF 2800 PSI AT 28 DAYS.
M7	GROUT SHALL BE A HIGH SLUMP MIX IN ACCORDANCE WITH ASTM SPECIFICATION C476.
M7.1	GROUT SHALL BE A HIGH SLUMP MIX IN WITH A COMPRESSIVE STRENGTH OF 3000PSI AT 28 DAYS.
M8	ALL GROUT SHALL BE FIELD OBTAINED CYLINDERS TESTED IN ACCORDANCE WITH ASTM C476.
M9	ALL CMU UNITS FOR REINFORCED WALLS SHALL BE IVANY BLOCK UNITS AS MANUFACTURED BY PIZZANO BROTHERS OR APPROVED EQUAL.
M10	ALL CONCRETE MASONRY SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST EDITION OF THE BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES ACI 530/ASCE 5/TMS 402 AND THE SPECIFICATION FOR MASONRY STRUCTURES ACI 530/ASCE 6/TMS 602.
M11	ALL MASONRY SHALL BE INSPECTED BY A QUALIFIED ENGINEER AS NOTED IN THE INTERNATIONAL BUILDING CODE SPECIAL INSPECTIONS REQUIREMENTS FOR THE PROJECT.
M12	ALL BRICK MASONRY UNITS SHALL BE GRADE 9A IN ACCORDANCE WITH ASTM C216 WITH A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI, BONDED TOGETHER WITH TYPE S MORTAR.
M13	PROVIDE HOT-DIPPEP GALVANIZED TRUSS TYPE HORIZONTAL JOINT REINFORCEMENT, MIN. 6 GA. AT 16" ON CENTER VERTICAL IN ALL MASONRY WALLS. SPACE HORIZONTAL JOINT REINFORCEMENT AT 8 INCHES ON CENTER IN ALL PARAPETS. USE SHOP FABRICATED SPECIAL PIECES AT ALL CORNERS AND TEES.

METAL FLOOR AND ROOF DECK	
PARAGRAPH	NOTES
D1	STEEL ROOF DECK FOR THE PROJECT SHALL BE AS FOLLOWS:
D1.1	1-1/2" 20 GA TYPE B METAL DECK
D1.2	PAINTED
D2	ALL ROOF DECK SHALL BE AS MANUFACTURED BY VULCRAFT, INC. OR APPROVED EQUAL. MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE. ROOF DECK FABRICATION AND INSTALLATION MUST COMPLY WITH STEEL DECK INSTITUTE STANDARDS. ALL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS.
D3	ALL ROOF DECK SHALL BE DESIGNED, MANUFACTURED, AND INSTALLED IN ACCORDANCE WITH THE LATEST FACTORY MUTUAL STANDARDS.
D4	USE WELDING WASHERS ON ALL CONNECTIONS OF STEEL DECK WITH METAL THICKNESS LESS THAN 22 GA. TO STRUCTURAL STEEL SUPPORTS.
D5	IN AREAS OF WARPED ROOF DECK USE SELF DRILLING SCREWS OR POWDER ACTUATED FASTENERS (PAFs) FOR CONNECTIONS OF STEEL ROOF DECK TO STRUCTURAL STEEL SUPPORTS. SCREW OR PAF SIZES SHALL COMPLY WITH MANUFACTURER AND FACTORY MUTUAL REQUIREMENTS. ATTACH DECK TO ALL SUPPORTING ROOF JOISTS AND BEAMS.
D6	ATTACH METAL ROOF DECK TO STRUCTURAL STEEL SUPPORTS WITH 5/8" DIAMETER RIVUL WELDS. FASTEN SIDE LAPS TOGETHER WITH #12 SELF DRILLING SCREWS AT 36 INCHES ON CENTER. (4/36 FASTENING PATTERN UNLESS OTHERWISE NOTED)
D7	AS AN ALTERNATE ROOF DECK FASTENING METHOD TO STRUCTURAL STEEL UTILIZE HILTI DECK FASTENER X-ENF-19. (4/36 FASTENING PATTERN UNLESS OTHERWISE NOTED) FASTEN SIDE LAPS WITH #12 SELF DRILLING SCREWS AT 36 INCHES ON CENTER.

TIMBER	
PARAGRAPH	NOTES
T1	ALL STRUCTURAL TIMBER FRAMING, WALLS, BLOCKING, ETC. SHALL BE HEM FIR #2 MINIMUM, STRESS GRADE LUMBER OR APPROVED EQUAL.
T2	ALL STRUCTURAL TIMBER FRAMING SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE PROPERTIES - Fb = 850 PSI, Fv = 190 PSI, E = 1,300,000 PSI
T3	ALL STRUCTURAL TIMBER MUST BE STAMPED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION'S "CONSTRUCTION MANUAL"
T4	ALL MICRO-LAM BEAMS SHALL BE AS ENGINEERED AND MANUFACTURED BY NEYERHAEUSER OR APPROVED EQUAL. THE MINIMUM ALLOWABLE PROPERTIES FOR MICRO-LAM BEAMS SHALL BE Fb = 2800 PSI, Fv = 285 PSI, E = 2,000,000 PSI.
T5	ALL PARALLAM BEAMS SHALL BE AS ENGINEERED AND MANUFACTURED BY NEYERHAEUSER OR APPROVED EQUAL. THE MINIMUM ALLOWABLE PROPERTIES FOR PARALLAM BEAMS SHALL BE Fb = 2900 PSI, Fv = 290 PSI, E = 2,000,000 PSI.
T6	ALL TIMBER AND TIMBER CONSTRUCTION SHALL COMPLY WITH LATEST EDITIONS OF THE FOLLOWING STANDARDS:
T6.1	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION: TIMBER CONSTRUCTION MANUAL.
T6.2	NATIONAL FOREST PRODUCTS ASSOCIATION: NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION.
T6.3	AMERICAN PLYWOOD ASSOCIATION: PLYWOOD DESIGN SPECIFICATION.
T6.4	AMERICAN WOOD-PRESERVERS ASSOCIATION STANDARDS.
T6.5	NATIONAL LUMBER MANUFACTURERS ASSOCIATION: NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENINGS.
T7	ALL TIMBER CONNECTIONS SHALL BE MADE USING PREFABRICATED CONNECTORS. TOE-NAILING IS NOT PERMITTED AS THE FINAL CONNECTION UNLESS OTHERWISE APPROVED BY THE ENGINEER. SUBMIT MANUFACTURER'S DATA FOR REVIEW. FASTENERS SHALL BE AS MANUFACTURED BY SIMPSON STRONGTIE OR APPROVED EQUAL.
T8	TREATED LUMBER SHALL BE PROVIDED AT ALL LOCATIONS WHERE LUMBER IS IN CONTACT WITH CONCRETE AND MASONRY FOUNDATION WALLS OR AT EXTERIOR OF BUILDING.
T9	SHEATHING FOR ROOFS SHALL BE 3/4" THICK 32/16 SPAN RATINGS APA STRUCTURAL I RATED PLYWOOD SHEATHING, EXPOSURE 1. ALL JOINTS IN SHEATHING SHALL BE STAGGERED. FOR ROOF SHEATHING, USE PANEL CLIPS, TONGUE & GROOVE OR LUMBER BLOCKED EDGE SUPPORTS AS RECOMMENDED BY APA. NAILING SHALL COMPLY WITH APA REQUIREMENTS FOR PLYWOOD FLOOR/ROOF DIAPHRAGMS.

COLD FORMED STRUCTURAL METAL FRAMING	
PARAGRAPH	NOTES
LT1	COLD FORMED METAL FRAMING DESIGNATIONS SHOWN ON STRUCTURAL DRAWINGS SHALL BE MANUFACTURED BY MARINO KARE OR APPROVED EQUAL. MANUFACTURER MUST SUBMIT LITERATURE INDICATING THAT THE METAL FRAMING SUPPLIER PROVIDES EQUIVALENT STRENGTH AND STIFFNESS. MANUFACTURER AND/OR SUPPLIER MUST PROVIDE INFORMATION INDICATING CAPACITY OF MEMBERS, FRAMING DETAILS, CONNECTIONS, BRACING, AND BRIDGING OF MEMBERS CONFORM TO LOAD CRITERIA. SUBMITTAL OF UNMARKED PRODUCT CATALOG PAGES AND FULL PRODUCT CATALOGS IS NOT PERMITTED.
LT2	ALL COLD FORMED METAL HEADERS INDICATED ON DRAWINGS ARE TO BE PROVIDED BY MANUFACTURER/SUPPLIER UNLESS OTHERWISE NOTED.
LT3	ALL STRUCTURAL METAL STUDS SHALL BE HOT-DIPPEP GALVANIZED (G 60) IN ACCORDANCE WITH ASTM A954. COLD FORMED METAL STUDS SHALL BE DESIGNED, MANUFACTURED AND INSTALLED IN ACCORDANCE WITH THE LATEST EDITIONS OF THE AISC SPECIFICATIONS AND SHALL COMPLY WITH ASTM A STRENGTH AS FOLLOWS:
LT3.1	16 GA AND HEAVIER - Fy = 50KSI.
LT3.2	18 GA, 20 GA - Fy = 39KSI.
LT4	ALL WELDING OF LIGHT GAGE STEEL FRAMING MUST BE DONE UTILIZING E60XX ELECTRODES AND SHALL BE COMPLETED BY CERTIFIED WELDERS IN ACCORDANCE WITH THE LATEST EDITION OF AWS D13 SPECIFICATION FOR WELDING SHEET STEEL IN STRUCTURES.
LT5	ALL CONNECTIONS SHALL BE MADE WITH SELF-TAPPING SCREWS OR WELDING S0 THAT THE CONNECTIONS MEET OR EXCEEDS THE DESIGN LOADS. ALWAYS USE WELDS WHERE SHOWN ON DRAWINGS.
LT6	CUT ALL LIGHT GAGE STEEL FRAMING MEMBERS WITH SABS OR SHEARS. FLAME CUTTING IS NOT PERMITTED.
LT7	THE LIGHT GAGE STEEL FRAMING SUPPLIER AND ERECTOR SHALL HAVE A MINIMUM 5 YEARS EXPERIENCE IN THE FABRICATION AND ERECTION OF LIGHT GAGE STEEL FRAMING SYSTEMS.

POST-INSTALLED ANCHORS	
PARAGRAPH	NOTES
PIA1	EXCEPT WHERE INDICATED ON THE DRAWINGS, POST-INSTALLED ANCHORS SHALL CONSIST OF THE FOLLOWING ANCHOR TYPES AS PROVIDED BY HILTI, INC. CONTACT HILTI FOR ANY PRODUCT RELATED INFORMATION.
PIA2	ADHESIVE/MECHANICAL ANCHORS TO CONCRETE SHALL BE:
PIA 2.1	DIAMETER AND EMBEDMENT AS INDICATED IN THE PLANS AND SECTIONS
PIA 2.2	HILTI Kwik Bolt-T2 EXPANSION ANCHOR
PIA 2.3	HILTI Kwik Bolt-3 EXPANSION ANCHOR
PIA3	REBAR DOWELING INTO CONCRETE
PIA3.1	DIAMETER AND EMBEDMENT AS INDICATED IN THE PLANS AND SECTIONS
PIA3.2	HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT AND HAS-E ROD
PIA4	ANCHORAGE INTO SOLID OR GROUTED SOLID MASONRY
PIA4.1	DIAMETER AND EMBEDMENT AS INDICATED IN THE PLANS AND SECTIONS
PIA4.2	HILTI HIT-HY TO MASONRY ADHESIVE ANCHORING SYSTEM WITH HAS-E ROD OR CONTINUOUSLY DEFORMED REBAR
PIA4.3	HILTI Kwik Bolt-3 EXPANSION ANCHOR
PIA5	ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI, INC. OR SUCH OTHER METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED BY THE ENGINEER OF RECORD PRIOR TO PURCHASE AND/OR USE. CONTRACTOR SHALL PROVIDE MANUFACTURER DATA DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED FOR COMPLIANCE WITH THE PROJECT REQUIREMENTS.
PIA6	INSTALL ALL ANCHORS PER THE MANUFACTURER INSTALLATION GUIDELINES INCLUDED WITH THE PRODUCT.
PIA6.1	A MINIMUM OF 10% OF ANCHOR INSTALLATION SHALL BE VERIFIED BY THIRD PARTY SPECIAL INSPECTION FOR CONFORMANCE WITH INSTALLATION INSTRUCTION. WRITTEN REPORTS FOR INSPECTIONS SHALL BE PROVIDED TO THE ENGINEER OF RECORD.
PIA7	OVERHEAD ADHESIVE ANCHORS SHALL BE INSTALLED USING THE HILTI PROFIS SYSTEM.
PIA8	ANCHOR CAPACITY IS DEPENDENT ON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO THE EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE STRUCTURAL DRAWINGS.
PIA9	EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT BARS MAY BE CUT, THE CONTRACTOR SHALL REVIEW ANY AVAILABLE DOCUMENTS AND SHALL LOCATE ALL REINFORCING PRIOR TO DRILLING FOR ANCHOR INSTALLATION. UTILIZE HILTI FERROSCAN, GPR, X-RAY, CHIPPING OR OTHER MEANS.

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SEAL

PROJECT # 6230.00
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SPECIAL INSPECTIONS REQUIREMENTS

PARAGRAPH	NOTES	FREQUENCY	REFERENCE STANDARD
SP1	ALL SPECIAL INSPECTIONS SHALL BE COMPLETED IN ACCORDANCE WITH THE 2004 INTERNATIONAL BUILDING CODE, PENNSYLVANIA UNIFORM CONSTRUCTION CODE AS WELL AS ALL REFERENCED STANDARDS CONTAINED THEREIN.		
SP2	THE OWNER WILL ENGAGE (PER THE CONTACT REQUIREMENTS) ONE OR MORE SPECIAL INSPECTORS (I.E. 3RD PARTY INSPECTOR) AND INSPECTION AGENCIES TO PROVIDE INSPECTIONS DURING THE CONSTRUCTION OF THE WORK INDICATED ON THE CONSTRUCTION DOCUMENTS TO THE EXTENT OF CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE AND AS OUTLINED BELOW.		
SP3	INSPECTIONS AND TESTING SHALL BE CARRIED OUT BASED ON THE FREQUENCY NOTED WITH THE SPECIAL INSPECTION ITEM.		
SP4	REFER TO THE GENERAL NOTES FOR ADDITIONAL TESTING AND INSPECTION REQUIREMENTS.		
SP5	THE SPECIAL INSPECTOR(S) SHALL KEEP RECORDS OF ALL INSPECTIONS AND TESTING COMPLETED ON THE WORK INDICATED.		
SP6	THE SPECIAL INSPECTOR(S) SHALL FURNISH INSPECTION AND TESTING REPORTS TO THE BUILDING OFFICIAL AND THE ENGINEER OF RECORD. REPORTS SHALL INDICATE THAT THE WORK INSPECTED WAS OR WAS NOT COMPLETED IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. REPORTS SHALL ALSO INDICATE CORRECTED DISCREPANCIES IN THE WORK.		
SP7	DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THEY ARE NOT CORRECTED IN A TIMELY MANNER, THE DISCREPANCIES SHALL BE BROUGHT TO THE BUILDING OFFICIAL AND THE ENGINEER OF RECORD PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK.		
SP8	REPORTS SHALL BE PROVIDED WITHIN 7 DAYS OF ALL INSPECTIONS AND SHALL BE PROVIDED TO ALL PARTIES INVOLVED, INCLUDING BUT NOT LIMITED TO THE CONTRACTOR, ARCHITECT OF RECORD, OWNER AND ENGINEER OF RECORD.		
SP9	EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A MAIN WIND OR SEISMIC FORCE RESISTING SYSTEM COMPONENT LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER PRIOR TO COMMENCEMENT OF WORK. STATEMENT SHALL ACKNOWLEDGE AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS.		

SOILS ELEMENTS

SP10.1	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY INDICATED.	PERIODIC	
SP10.2	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER WATER TABLE.	PERIODIC	
SP10.3	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS TO BE USED.	PERIODIC	
SP10.4	VERY USE OF PROPER MATERIALS DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	CONTINUOUS	
SP10.5	PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT THE SITE HAS BEEN PROPERLY PREPARED.	PERIODIC	

STEEL CONSTRUCTION ELEMENTS

SP11.1	VERIFICATION OF IDENTIFICATION MARKINGS THAT HIGH STRENGTH BOLTS, NUTS AND WASHERS CONFORM TO STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	PERIODIC	ASTM/AISC
SP11.2	MANUFACTURER'S CERTIFICATE OF COMPLIANCE FOR HIGH STRENGTH BOLTS, NUTS AND WASHERS.	PERIODIC	
SP11.3	INSPECTION OF SNUG-TIGHT JOINTS USING HIGH STRENGTH BOLTS IN STANDARD BEARING CONNECTIONS.	PERIODIC	AISC
SP11.6	MATERIAL VERIFICATION OF STRUCTURAL STEEL AND GOLD FORMED STEEL DECK FOR IDENTIFICATION MARKINGS CONFORMING TO AISC AND ASTM STANDARDS AND MANUFACTURER CERTIFIED TEST REPORTS.	PERIODIC	AISC
SP11.7	VERIFICATION OF WELDING PROCEDURES, WELDING ROD MATERIAL AND WELDING CERTIFICATIONS OF WELDERS.	PERIODIC	AISC/ANS
SP11.8	MATERIAL VERIFICATION OF WELD FILLER MATERIALS FOR IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATIONS.	PERIODIC	AISC/ANS
SP11.9	MATERIAL VERIFICATION OF COMPLIANCE MANUFACTURER'S CERTIFICATE OF COMPLIANCE.	PERIODIC	
SP11.10	INSPECTION OF WELDING OF STRUCTURAL STEEL WITH COMPLETE AND PARTIAL JOINT PENETRATION WELDS, MULTI-PASS FILLET WELDS, SINGLE PASS FILLET WELDS GREATER THAN 5/16 INCH IN SIZE, AND PLUG AND SLOT WELDS TO INCLUDE JOINT PREPARATION, DIMENSIONS, CLEANLINESS, BACKING AND TACKLING.	CONTINUOUS	ANS
SP11.11	INSPECTION OF WELDING OF STRUCTURAL STEEL WITH SINGLE PASS FILLET WELDS LESS THAN OR EQUAL TO 5/16 INCH IN SIZE TO INCLUDE JOINTS DIMENSIONS, CLEANLINESS AND TACKLING.	PERIODIC	ANS
SP11.12	ALL INSPECTIONS OF WELDS SHALL INCLUDE SIZE, LENGTH, AND LOCATION, VISUAL ACCEPTANCE CRITERIA, REMOVAL OF BACKING BARS, WELD PROFILES, UNDERCUT, POROSITY, CRACK, CRACK PREVENTION AND FUSION OF MATERIALS.	PERIODIC	AISC/ANS
SP11.13	INSPECTION OF WELDING OF FLOOR AND ROOF DECK TO SUPPORT STRUCTURE AND WELDING OF HEADED SHEAR STUDS TO COMPOSITE FLOOR STRUCTURE.	CONTINUOUS	ANS

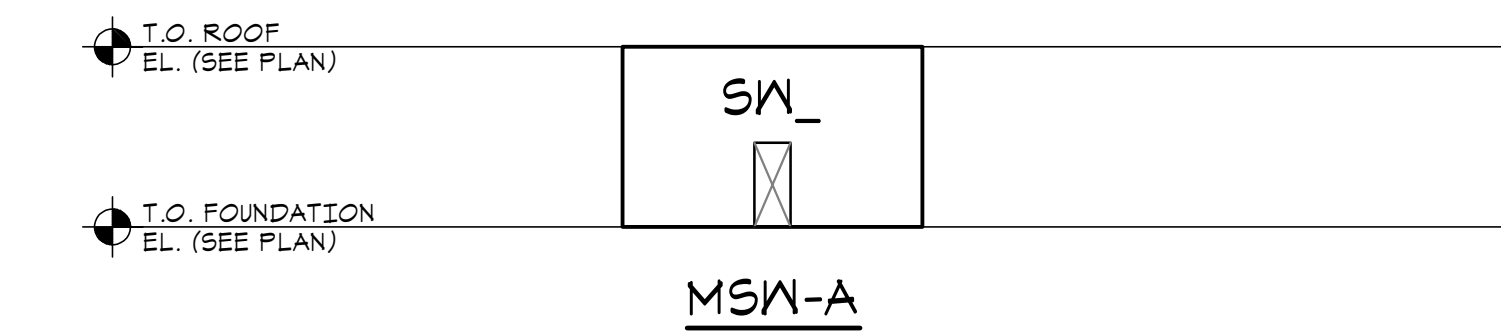
CONCRETE CONSTRUCTION ELEMENTS

SP12.1	INSPECTION OF REINFORCING PLACEMENT, SIZE, LOCATION AND CLEAR COVER (INCLUDING PRESTRESSING TENDONS).	PERIODIC	ACI
SP12.2	INSPECTION OF WELDED REINFORCING STEEL.	PERIODIC	ANS/ACI
SP12.3	INSPECTION OF BOLTS INSTALLED IN CONCRETE PRIOR TO AND DURING THE PLACEMENT OF CONCRETE.	CONTINUOUS	ACI
SP12.4	INSPECTION OF POST INSTALLED ANCHORS IN HARDENED CONCRETE.	PERIODIC	ACI
SP12.5	VERIFICATION OF USE OF APPROVED MIX DESIGN AND IN LOCATION PERMITTED.	PERIODIC	ACI
SP12.6	INSPECTION OF CONCRETE SLUMP, AIR CONTENT AND TEMPERATURE.	CONTINUOUS	ACI
SP12.7	INSPECTION FOR MAINTAINING SPECIFIED CURING TEMPERATURE AND TECHNIQUES (I.E. HOT AND GOLD WEATHER CONCRETE PLACEMENT TECHNIQUES).	PERIODIC	ACI
SP12.8	INSPECTION OF FORMWORK FOR SIZE, SHAPE, LOCATION AND DIMENSIONS FOR THE CONCRETE MEMBERS BEING FORMED.	PERIODIC	ACI

MASONRY CONSTRUCTION ELEMENTS (LEVEL 1)

SP13.1	VERIFICATION OF APPROVED SUBMITTALS AND COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS.	PERIODIC	TMS/ACI
SP13.2	VERIFICATION OF MASONRY STRENGTH.	PERIODIC	TMS/ACI
SP13.3	VERIFICATION OF SLUMP, FLOW AND VSI FOR SELF CONSOLIDATING GROUT.	CONTINUOUS	TMS/ACI
SP13.4	VERIFICATION OF PROPORTIONS OF SITE PREPARED MORTAR, PLACEMENT OF MASONRY UNITS, CONSTRUCTION OF JOINTS, LOCATION OF REINFORCEMENT, CONNECTORS AND ANCHORAGES.	PERIODIC	TMS/ACI

SP13.5	INSPECTION OF SIZE AND LOCATION OF STRUCTURAL ELEMENTS; TYPE, SIZE, AND LOCATION OF ANCHORS INCLUDING ANCHORAGE OF MASONRY TO OTHER STRUCTURAL MEMBERS; SPECIFIED TYPE, SIZE AND GRADE OF REINFORCING, AND BOLTS.	PERIODIC	TMS/ACI
SP13.6	PREPARATION AND CONSTRUCTION OF IN CONFORMANCE WITH HOT AND GOLD WEATHER REQUIREMENTS.	PERIODIC	TMS/ACI
SP13.7	INSPECTION FOR CLEAN GROUT SPACE, PLACEMENT OF REINFORCING AND CONNECTORS, PROPORTIONS OF SITE PREPARED GROUT AND CONSTRUCTION OF MORTAR JOINTS.	PERIODIC	TMS/ACI
SP13.8	VERIFICATION AND INSPECTION OF GROUT PLACEMENT.	CONTINUOUS	TMS/ACI
SP13.9	REVIEW OF PREPARATION OF REQUIRED GROUT SPECIMENS, MORTAR SPECIMENS AND OR PREMS.	PERIODIC	TMS/ACI

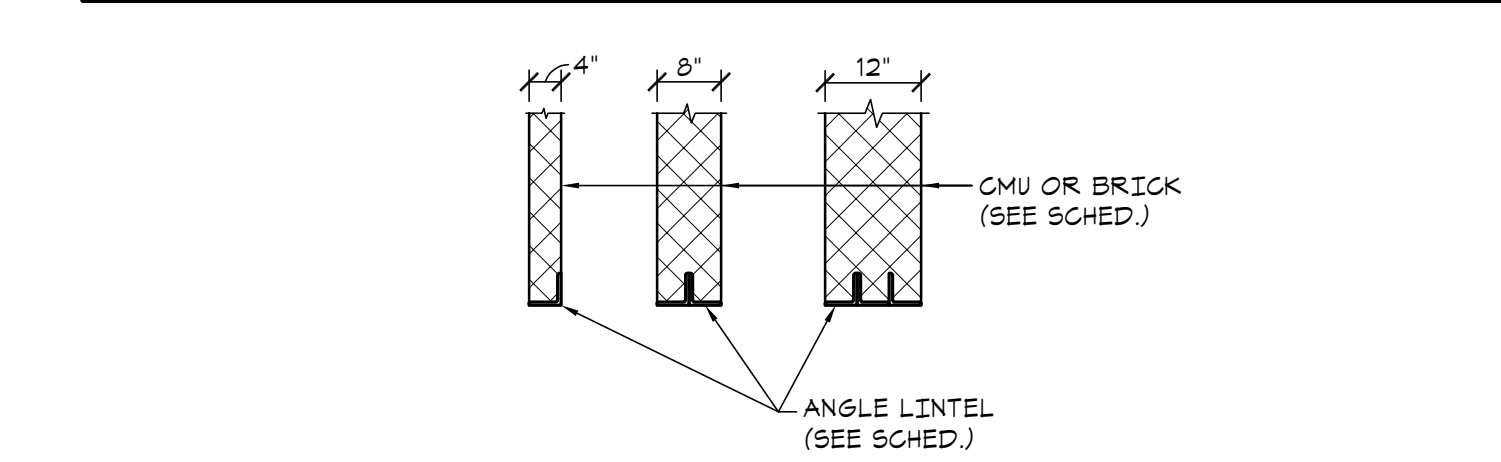


CONCRETE/ MASONRY SHEAR WALL ELEVATIONS

NOTES:
1. 'SW' INDICATES SHEAR WALL TYPE. SEE CONCRETE/ MASONRY SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.

CONCRETE/ MASONRY SHEAR WALL SCHEDULE

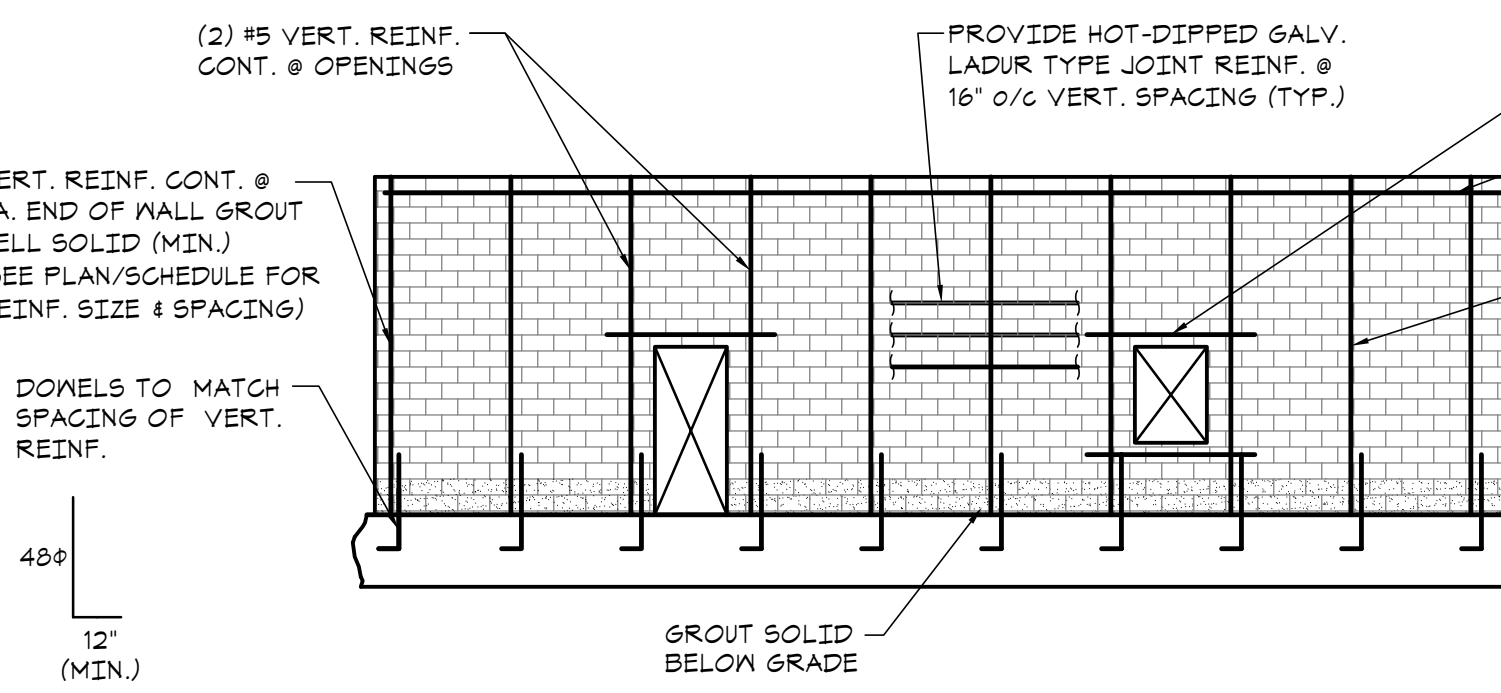
MARK	PLAN VIEW	WALL TYPE	VERTICAL REINFORCEMENT	HORIZONTAL REINFORCEMENT	NOTES
SW1		8" CMU (GROUTED SOLID @ 24" O/C)	#5 @ 16" O/C VEF	#8B LADDER TYPE REINFORCEMENT @ 16" O/C	SEE SHEARWALL ELEVATIONS



STEEL LINTEL SCHEDULE

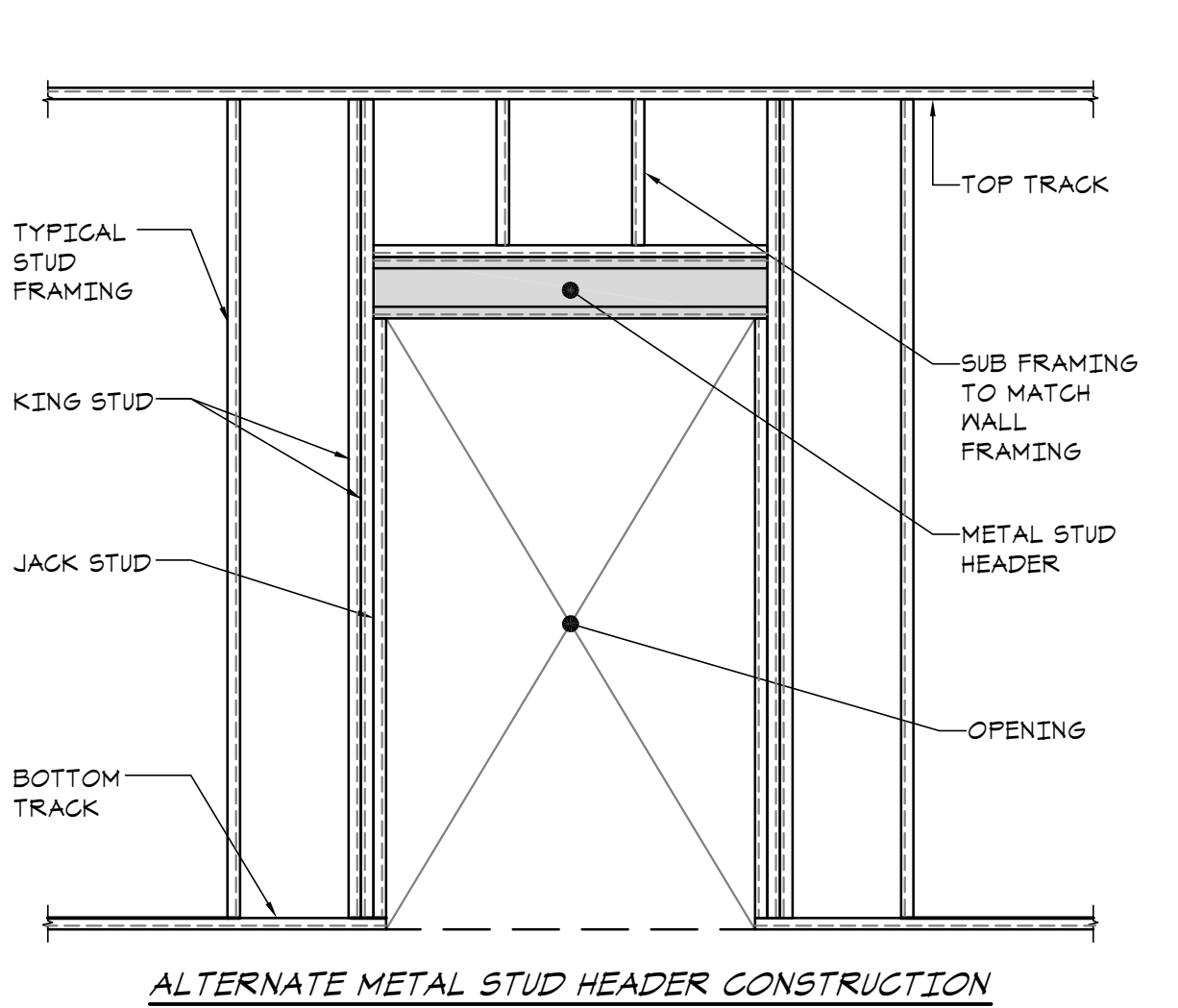
WIDTH OF OPENING	STEEL FOR EACH 4" OF WALL THICKNESS	REMARKS
UP TO 2'-11"	L 3 1/2 X 3 1/2 X 5/16	
3'-0" TO 3'-11"	L 4 X 3 1/2 X 5/16	
4'-0" TO 5'-11"	L 5 X 3 1/2 X 5/16	
6'-0" TO 8'-0"	L 6 X 3 1/2 X 5/16	

NOTES: 1) ALL STEEL LINTELS SHALL BE ASTM A-36.
2) FILL CMU VOIDS SOLID (2) COURSES BELOW LINTEL BEARING.
3) ALL LINTELS SHALL HAVE 8" MINIMUM BEARING UNDO.

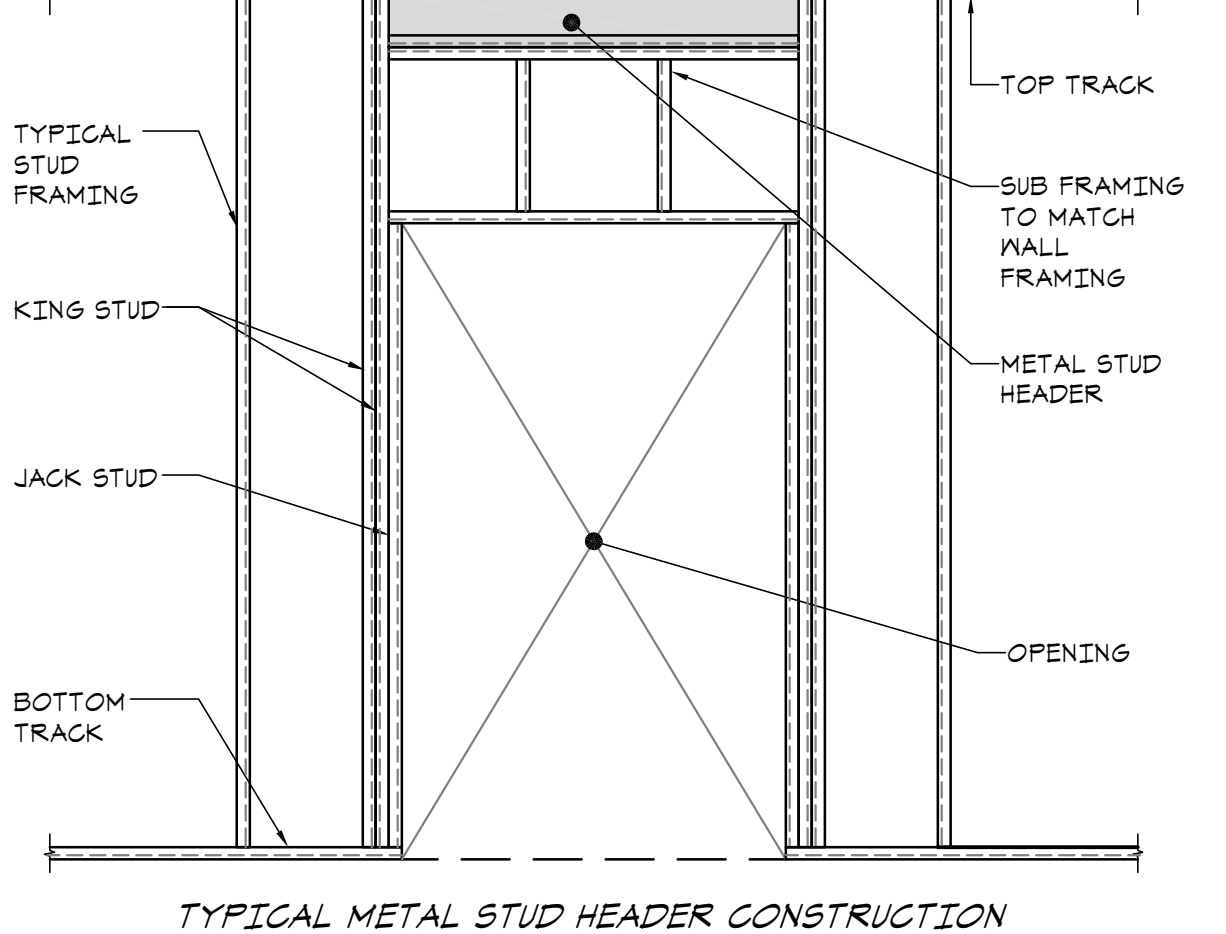


Typical Intermediate Reinforced Masonry Shear Wall

NOTES:
1. ALL VERT. BARS ARE TO BE CONTINUOUS FROM FLOOR TO FLOOR.
2. FILL ALL CORES W/ REINFORCING BARS W/ 3000 PSI GROUT.
3. THIS IS MIN. REINF. REQUIRED. SEE WALL SECTIONS FOR ADDITIONAL TENSION REINFORCING.
4. SEE PLAN FOR REQD LENGTH OF WALL.



ALTERNATE METAL STUD HEADER CONSTRUCTION



METAL STUD HEADER SCHEDULE

MARK	NO. & SIZE	JACK STUDS	KING STUDS	SECTION
MH1	(2) 8005162-54 (2) 6007200-68 BOX BEAM	(2) 6005162-54	(1) 6005162-54	

COLUMN SCHEDULE

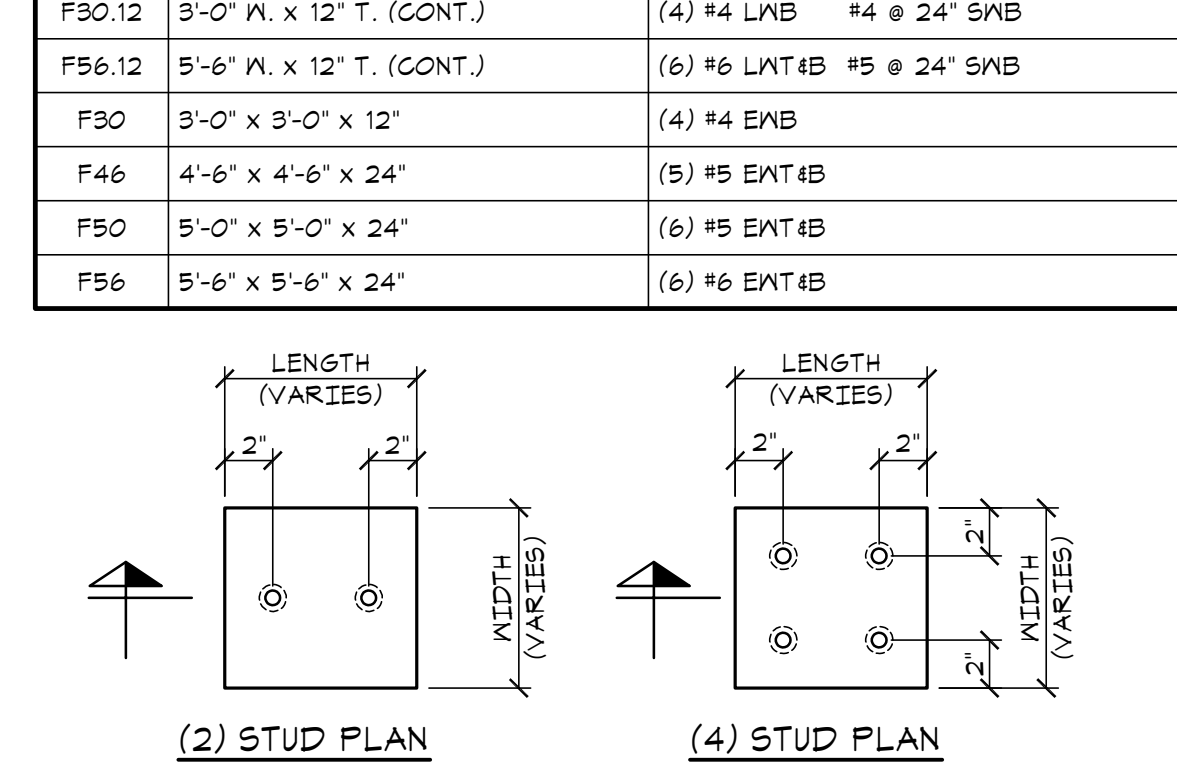
MARK	SIZE	BASE PLATE (A36)	ANCHOR BOLTS
C1	H587X13/8	14"X14"X1"	(4) 3/4"Ø
C2	H587X1/2	14"X14"X1"	(4) 3/4"Ø
C3	H586X11/4	12"X12"X1/2"	(4) 3/4"Ø HLLT KWK 3 W/ 4 3/4" EMBED (COUNTERSUNK)
C4	H587X1/4	14"X14"X3/4"	(4) 3/4"Ø

CONCRETE PIER SCHEDULE

MARK	SIZE	REINFORCING	NOTES
F1	20' X 20' CONG.	(8) #1 VERT. W/ #3 TIES @12" O/C	

FOOTING SCHEDULE

MARK	SIZE	REINFORCING
F20.12	2'-0" W. X 12" T. (CONT.)	(3) #4 LNB #4 @ 24" S/B
F26.12	2'-6" W. X 12" T. (CONT.)	(4) #4 LNB #4 @ 24" S/B
F30.12	3'-0" W. X 12" T. (CONT.)	(4) #4 LNB #4 @ 24" S/B
F56.12	5'-6" W. X 12" T. (CONT.)	(6) #6 LNT4B #5 @ 24" S/B
F30	3'-0" X 3'-0" X 12"	(4) #4 EMB
F46	4'-6" X 4'-6" X 24"	(5) #5 EMT4B
F50	5'-0" X 5'-0" X 24"	(6) #5 EMT4B
F56	5'-6" X 5'-6" X 24"	(6) #6 EMT4B

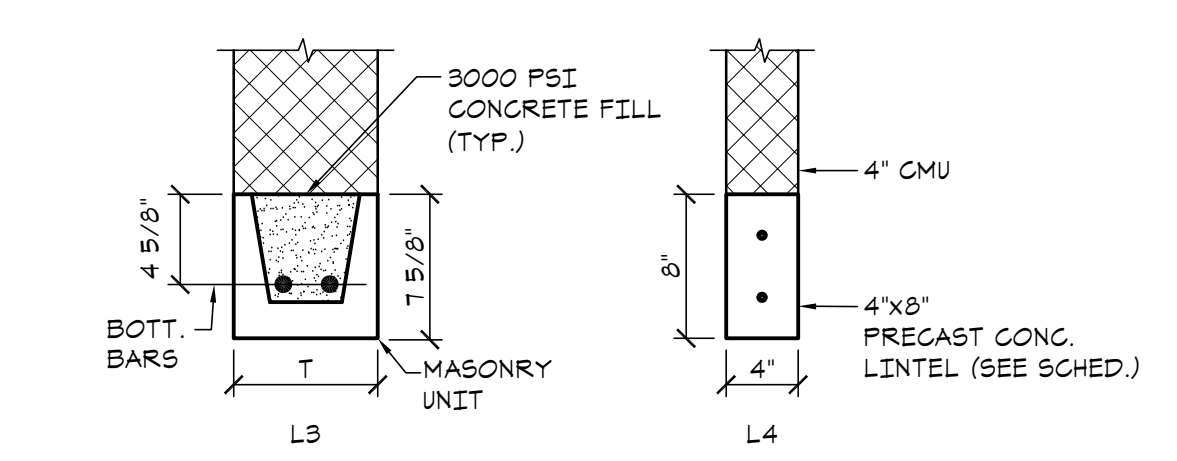


BEARING PLATE SCHEDULE

MARK	THICK	WIDTH	LENGTH	# OF HEADED STUDS
BP1	1/2"	7-1/2"	10"	(2)
BP2	1/2"	7-1/2"	12"	(2)

LINTEL SCHEDULE

MARK	NO. & SIZE	SECTION
L1	L6X3-1/2X1/2 (LLV)	
L2	(2) L6X3-1/2X5/16 (LLV)	
L3	(2) L4X3-1/2X3/8 (LLV)	



MASONRY/ CONCRETE LINTEL SCHEDULE

MARK	W'	T'	REINFORCING				REMARKS
			TOP BARS	BOTTOM BARS	TIES	MID BARS	
L3	7 5/8"	7 5/8"	N/A	(2) #4	N/A	N/A	MASONRY
L4	8"	4"	(1) #4	(1) #4	N/A	N/A	REIN. CONG.

GRADE BEAM SCHEDULE

MARK	SIZE	REINFORCING				STIRRUPS EACH END	REMARKS
		TOP BARS	BOTT. BARS	VID BARS	SIZE & SPACINGS		
GB1	12"X36"	(3) #5	(3) #5	(3) #5	#3 @ 12" O/C		

DESIGN LOAD SCHEDULE
(ALL LOADS SHOWN ARE IN POUNDS PER SQ. FT.)

COMPONENT	AREA		ROOF
	SUB ON GRADE	ROOF	
4" CONCRETE SLAB ON GRADE	50		
STEEL FRAMING	5		
CEILING	2		
COLLATERAL	3		
MECHANICAL	5		
ROOF INSULATION	12		
WOOD FRAMING	5		
TOTAL DEAD LOAD	50	32	
TOTAL LIVE LOAD	120	30	
TOTAL LOAD	150	62	

LATERAL LOAD DESIGN SCHEDULE

2004 INTERNATIONAL BUILDING CODE

ITEM	SYMBOL	VALUE	REFERENCE
BASIC WIND SPEED	V ₃₀	90MPH	FIGURE 1604
OCCUPANCY CATEGORY	-	II	TABLE 1604.2
WIND LOAD IMPORTANCE	I	1.0	TABLE 1601.6
WIND EXPOSURE CATEGORY	-	C	SECTION 1604.3
DESIGN WIND FORCE	-	ANALYTICAL	SECTION 1604.5
MAIN WIND-FORCE PRESSURE	P _s	20 PSF	SECTION 6.2.2 (ASCE-10)
INTERNAL PRESSURE COEFFICIENT	GC _p	0.18	TABLE 6.2.1 (ASCE-10)

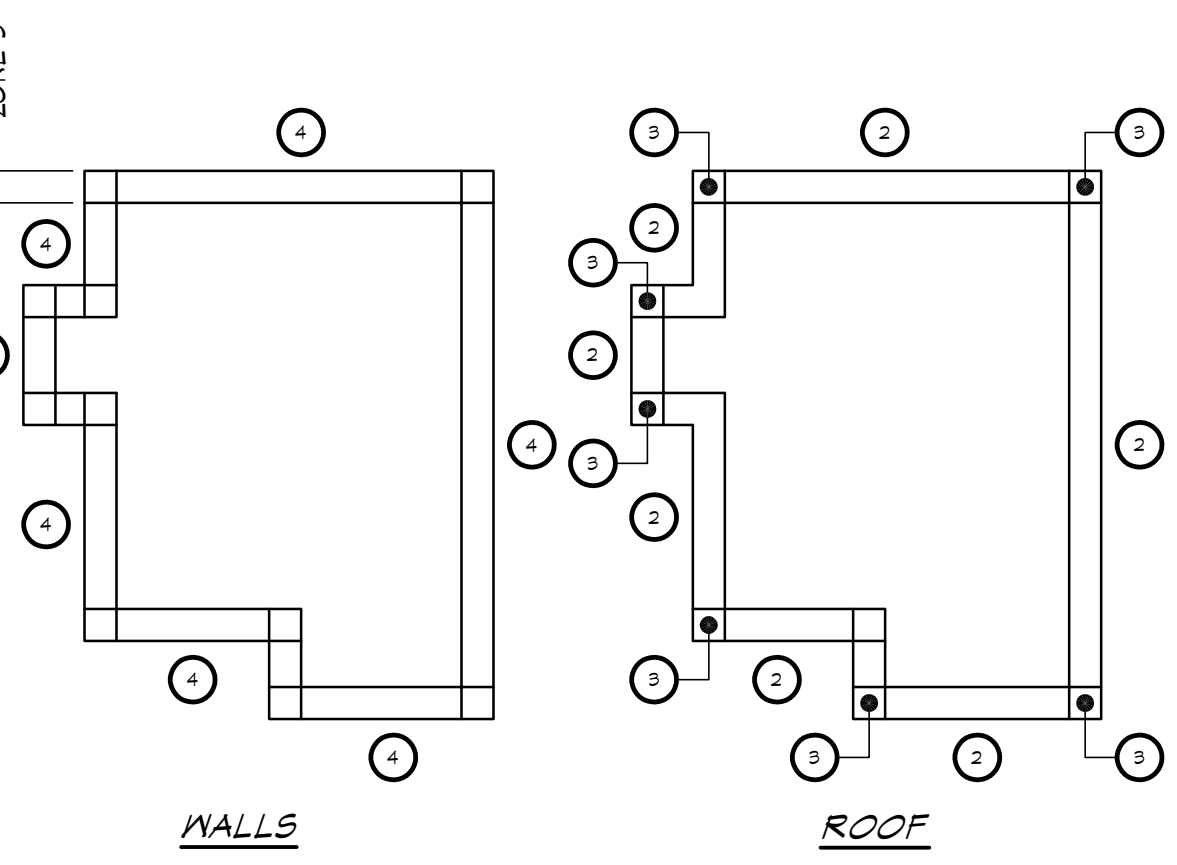
SEISMIC LOAD

ITEM	SYMBOL	VALUE	REFERENCE
SITE CLASS	-	D	SECTION 1601.2
HAPPED SPECTRAL RESPONSE ACCELERATION	S ₁	.186	FIGURE 1601.8 (1)
HAPPED SPECTRAL RESPONSE ACCELERATION (1-SECOND RESPONSE)	S ₁	.048	FIGURE 1601.8 (2)
DESIGN SPECTRAL RESPONSE ACCELERATION	S _{DS}	.186	SECTION 1601.5.4
DESIGN SPECTRAL RESPONSE ACCELERATION (1-SECOND RESPONSE)	S ₁	.077	SECTION 1601.5.4
OCCUPANCY CATEGORY	-	II	SECTION 1604.5
SEISMIC DESIGN CATEGORY	-	B	TABLE 1601.6
SEISMIC IMPORTANCE FACTOR	I _e	1.0	TABLE 1601.6 (1)
DESIGN BASE SHEAR	-	0.7 K	SECTION 12.8.1 (ASCE-10)
ANALYSIS PROCEDURE	-	EQUIVALENT LATERAL FORCE	SECTION 12.8 (ASCE-10)
BASIC STRUCTURAL SYSTEM	-	STEEL FRAME AND MASONRY BEARING WALLS	12.2.1 (ASCE-10)
BASIC BEEMO FORCE REDUCTION SYSTEM	-	STEEL SYSTEM NOT SPECIFICALLY DETAILED AND CONFORMING REINFORCED MASONRY WALLS	TABLE 12.2.1 (ASCE-10)
BASIC BEEMO RESPONSE COEFFICIENT	C _b	0.01	SECTION 12.8.1.1 (ASCE-10)
RESPONSE MOD. FACTOR	R	2	TABLE 12.2-1 (ASCE-10)

SNOW LOAD DESIGN SCHEDULE

2004 INTERNATIONAL BUILDING CODE

ITEM	SYMBOL	VALUE	REFERENCE
GROUND SNOW LOAD	P _g	30 PSF	FIGURE 1602.2
SNOW EXPOSURE FACTOR	C _e	II	TABLE 1602.1 (1)
SNOW LOAD IMPORTANCE FACTOR	I	1.0	TABLE 1602.1 (2)
THERMAL FACTOR	C _t	1.0	TABLE 1602.1 (3)
FLAT-ROOF SNOW LOAD	P _f	21 PSF	SECTION 1602.1 (ASCE-10)



COMPONENTS & CLADDING WIND PRESSURES

TRIBUTARY AREA (SF)	PRESSURE			
	+20	50	200	500+
ROOF INTERIOR ZONE (ZONE 1)	(+)	10.00	10.00	10.00
ROOF END ZONE (ZONE 2)	(-)	-18.12	-17.61	-17.19
ROOF CORNER ZONE (ZONE 3)	(+)	10.00	10.00	10.00
MALL INTERIOR ZONE (ZONE 4)	(+)	11.13	15.37	13.85
MALL END ZONE (ZONE 5)	(-)	-22.84	-14.32	-16.28

NOTES:
1. WORST CASE PRESSURE OF POSITIVE (WINDWARD) AND NEGATIVE (LEEWARD/ SIDE) PRESSURE TO BE USED FOR DESIGN OF GLAZING COMPONENTS AND THEIR CONNECTIONS TO THE MAIN BUILDING STRUCTURE.
2. SEE BUILDING DIAGRAMS FOR LOCATIONS AND DIMENSIONS OF ZONES INDICATED.
3. ALL PRESSURES SHOWN ARE ULTIMATE IN ACCORDANCE W/ ASCE7-05 AND THE 2004 INTERNATIONAL BUILDING CODE.

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DRAWING TITLE

PROJECT SCHEDULES & SPECIAL INSPECTIONS

SCALE: AS NOTED
DRAWN: BAS
CHECKED: BMS
APPROVED: BTM
DATE: SEPTEMBER 28, 2018

REVISIONS

SYMBOL	DATE	DESCRIPTION
	11/20/2018	50% CD SUBMITTAL
	3/8/2019	FOR PERMITS
	6/21/2019	FOR BID AND PERMITS ONLY

DRAWING #



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DRAWING TITLE

FOUNDATION PLAN

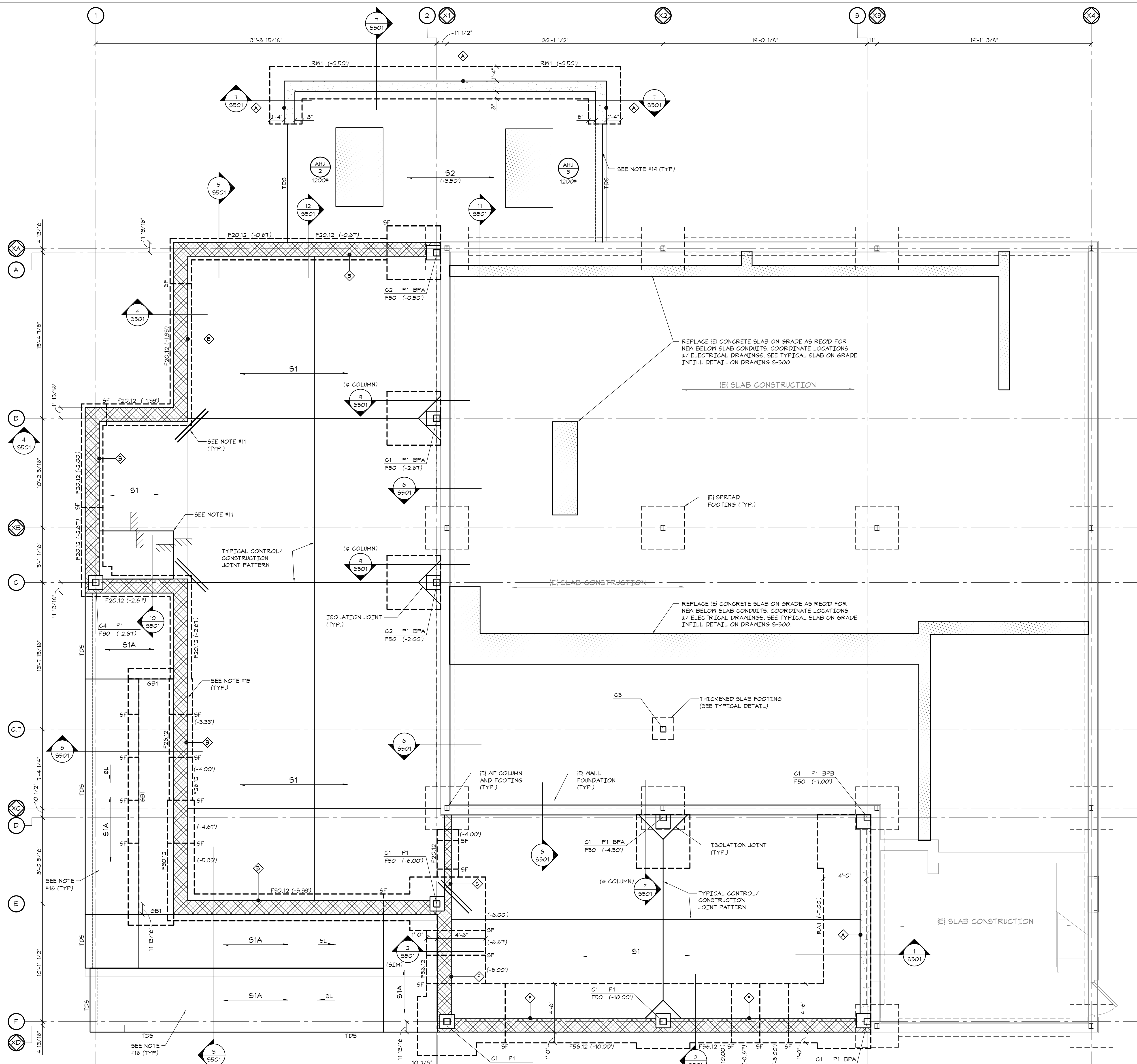
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DATE: SEPTEMBER 28, 2018

REVISIONS

SYMBOL	DATE	DESCRIPTION
	11/20/18	50% CD SUBMITTAL
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5100



- FOUNDATION PLAN**
SCALE: 1/4" = 1'-0"
- NOTES:
- TOP OF SLAB EL. = DATUM EL. (0.00') UNLESS NOTED OTHERWISE THIS (...)
 - TOP OF NEW SLAB ELEVATION TO MATCH TOP OF EXISTING TOP OF SLAB ELEVATION. G.C. TO VERIFY IN FIELD.
 - SEE PLAN FOR TOP OF FOOTING ELEVATION BELOW DATUM ELEVATION.
 - TOP OF PIER (GRADE BEAM/PIER CAP) EL. = (-0.61') BELOW DATUM UNLESS NOTED OTHERWISE (...)
 - S, D, INDICATES FLOOR/ ROOF CONSTRUCTION. SEE FLOOR/ ROOF CONSTRUCTION SCHEDULE ON THIS SHEET FOR ADDITIONAL INFORMATION.
 - COORDINATE ALL UNDER SLAB PIPING WITH ARCHITECTURAL/MECHANICAL DRAWINGS.
 - COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO BUILDING LAYOUT.
 - SF INDICATES STEEPED FOOTINGS. SEE TYPICAL DETAIL ON DRAWING 5500.
 - TS INDICATES THICKENED SLAB. SEE TYPICAL DETAIL ON DRAWING 5500.
 - TDS INDICATES TURNED DOWN SLAB. SEE TYPICAL DETAIL ON DRAWING 5500.
 - PROVIDE (2) #4 x 4'-0" L.S. CONCRETE CORNER BARS IN SLAB.
 - RWL INDICATES RETAINING WALL. SEE SECTIONS INDICATED FOR ADDITIONAL INFORMATION.
 - FD INDICATES FLOOR DRAIN. COORDINATE SIZE & LOCATION W/ ARCH. & MEP DRAWINGS.
 - BOTTOM OF NEW FOOTING TO MATCH BOTTOM OF EXISTING FOOTING. (G.C. TO VERIFY IN FIELD)
 - COORDINATE LOCATIONS OF PENETRATIONS THROUGHOUT WALL FOUNDATION W/ ARCH. & MECH DRAWINGS. SEE TYPICAL FOUNDATION WALL PENETRATION DETAIL ON SHEET 5500 FOR ADDITIONAL INFORMATION.
 - COORDINATE TOP OF SLAB ELEVATION AND SLOPE OF RAMP W/ CIVIL & ARCH DRAWINGS.
 - COORDINATE LOCATION AND SIZE OF DEPRESSED SLAB W/ ARCH DRAWINGS.
 - SB, INDICATES GRADE BEAM. SEE SCHEDULE ON SHEET 5003 FOR ADDITIONAL INFORMATION.
 - SEE TYPICAL EQUIPMENT PAD DETAIL ON SHEET 5500 FOR ADDITIONAL INFORMATION.
 - BP, INDICATES BASEPLATE DESIGNATION; SEE TYPICAL BASEPLATE LAYOUT DETAIL ON SHEET 5500 FOR ADDITIONAL INFO

WALL SCHEDULE

MARK	DESCRIPTION	WIDTH
A	12" REINF. CONCRETE	12"
B	16" CMU (GROUTED SOLID)	15-5/8"
C	8" CMU (GROUTED SOLID)	7-5/8"
D	8" METAL STUD @ 16" O/C	6"
E	4" CMU	3-5/8"
F	12" IVANY 8" CMU (GROUTED SOLID)	14-1/4"

FLOOR/ ROOF CONSTRUCTION SCHEDULE

MARK	SECTION	DESCRIPTION
S1		4" NORMAL WEIGHT CONCRETE SLAB ON GRADE W/ 6% AIR ENTRAINMENT 1/4" W/FF OVER 4" CRUSHED STONE
S1A		4" NORMAL WEIGHT CONCRETE SLAB ON GRADE W/ 6% AIR ENTRAINMENT 1/4" W/FF OVER 4" CRUSHED STONE
S2		6" NORMAL WEIGHT CONCRETE (W/ 6% AIR ENTRAINMENT) SLAB ON GRADE W/ 6% AIR ENTRAINMENT 1/4" W/FF OVER 4" CRUSHED STONE
D1		3/4" APA STRUCTURAL 1 SHEATHING
D2		1 1/2" 22GA WIDE RIB B METAL ROOF DECK (PAINTED)

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JOB NUMBER: 865.081
CONTACT: R. SAVONA

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ROOF FRAMING PLAN

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DATE: SEPTEMBER 28, 2018

REVISIONS

SYMBOL DATE DESCRIPTION

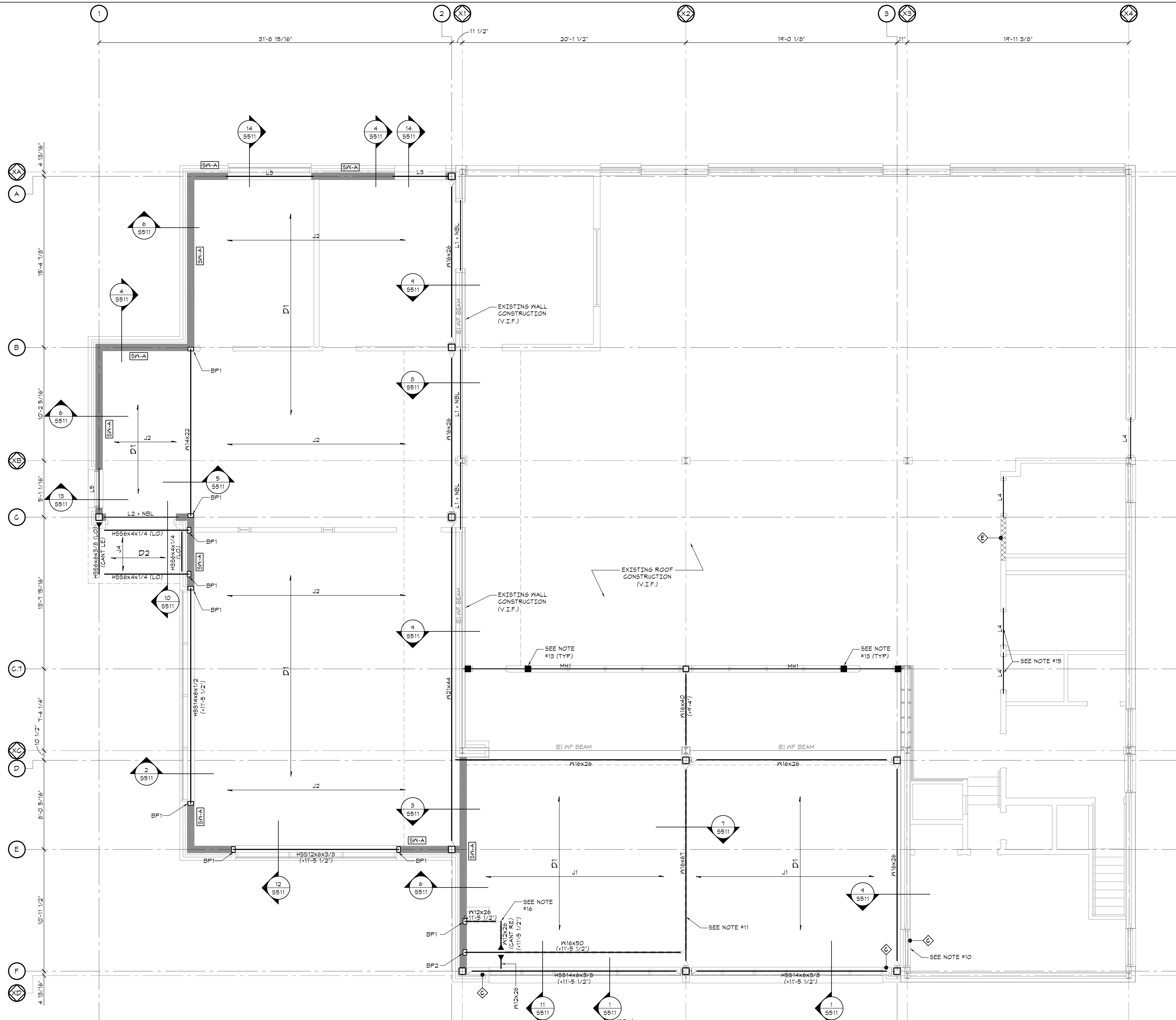
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S101

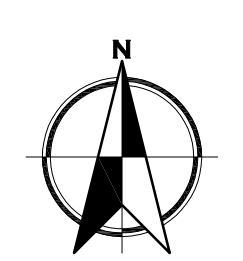


JOIST SCHEDULE		
MARK	DESCRIPTION	SPACING
J1	3 1/2"x11 7/8" PSL	15'-0" O/C*
J2	5 1/4"x11 7/8" PSL	15'-0" O/C*
J3	600S162-54	16" O/C

* SPACING TO MATCH EXISTING FRAMING SPACING & G.C. VERIFY IN FIELD.

WALL SCHEDULE		
MARK	DESCRIPTION	WIDTH
A	12" REINF. CONCRETE	12"
B	16" CMU (GROUTED SOLID)	15'-5/8"
C	8" CMU (GROUTED SOLID)	7'-5/8"
D	6" METAL STUD @ 16" O/C	6"
E	4" CMU	3'-5/8"
F	12" IVANY 1 8" CMU (GROUTED SOLID)	19'-1/4"

FLOOR/ ROOF CONSTRUCTION SCHEDULE		
MARK	SECTION	DESCRIPTION
S1		4" NORMAL WEIGHT CONCRETE SLAB ON GRADE w/ 6x6-M1.4x1.4 MMF OVER 4" CRUSHED STONE
S1A		4" NORMAL WEIGHT CONCRETE (w/ 6% AIR ENTRAINMENT) SLAB ON GRADE w/ 6x6-M1.4x1.4 MMF OVER 4" CRUSHED STONE
S2		6" NORMAL WEIGHT CONCRETE (w/ 6% AIR ENTRAINMENT) SLAB ON GRADE w/ 6x6-M1.4x1.4 MMF OVER 4" CRUSHED STONE
D1		3/4" APA STRUCTURAL 1 SHEATHING
D2		1 1/2" 226A WIDE RIB B METAL ROOF DECK (PAINTED)



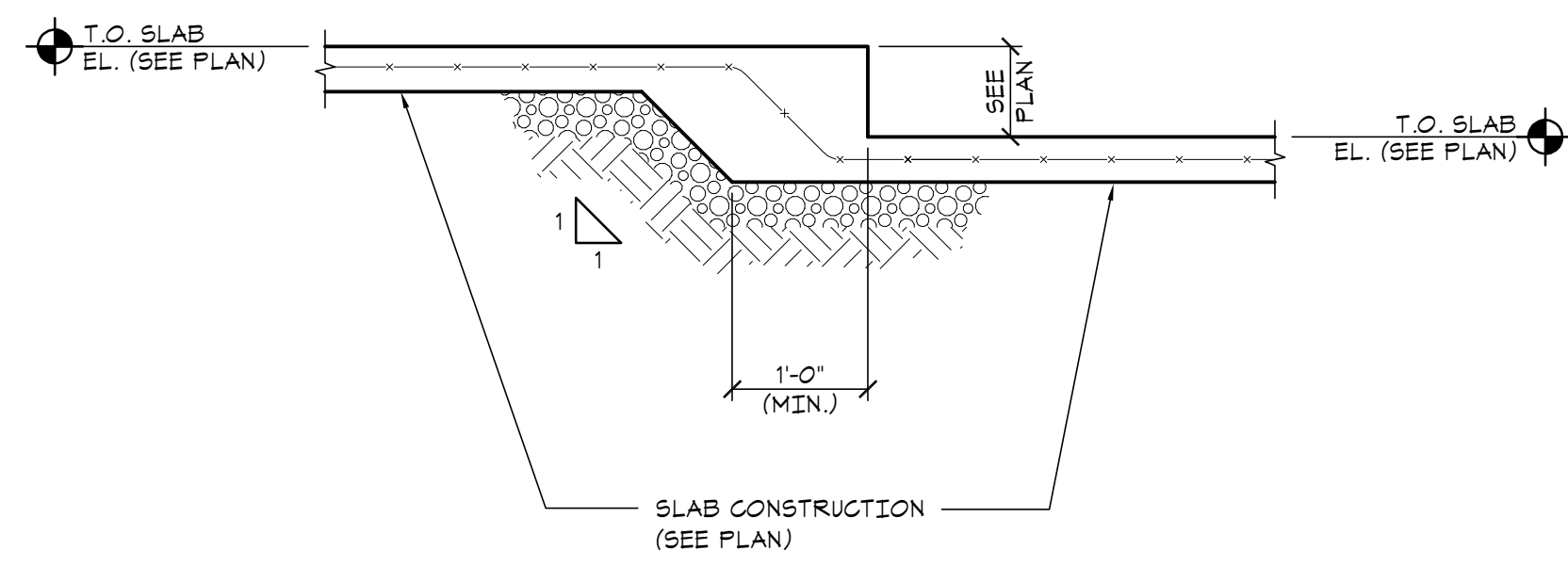
ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

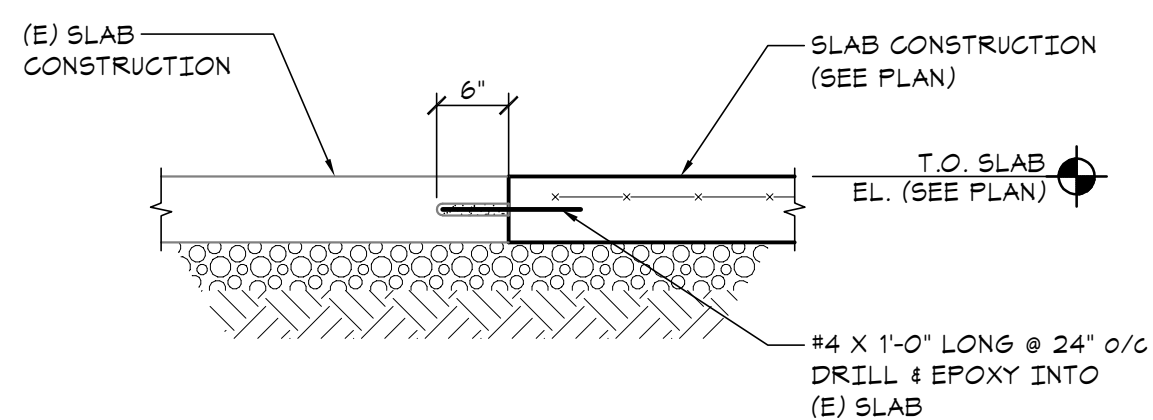
- NOTES:
- DECK BEARING EL. = (+11'-5 1/2") ABOVE DATUM EL.
 - TOP OF STEEL ELEVATION = (+1 1/2") BELOW DECK BEARING ELEVATION UNLESS NOTED OTHERWISE THUS (...).
 - TOP OF EXISTING STEEL ELEVATION = (+11'-10 1/2") ABOVE DATUM (N.O.)
 - (LO) INDICATES TOP OF STEEL EL. = (+8'-4 3/4") ABOVE DATUM EL.
 - (L) INDICATES LINTEL. SEE SCHEDULE ON DRAWING 3003 FOR ADDITIONAL INFORMATION.
 - COORDINATE ROOF TOP PLOT PENETRATIONS WITH ARCHITECTURAL/MECHANICAL DRAWINGS.
 - BP_ INDICATES BEARING PLATE. SEE SCHEDULE ON DRAWING 3003.
 - INDICATES MOMENT CONNECTION - BEAM TO BEAM OR BEAM TO COLUMN - SEE PLAN FOR REQUIRED CONNECTION MOMENT CAPACITY. IF NO LOAD SHOWN, PROVIDE FULL CAPACITY OF BEAM IN ADDITION TO FULL DEPTH SHEAR CONNECTION.
 - INDICATES FLEXIBLE MOMENT CONNECTION (FMC) - BEAM TO COLUMN CONNECTION. SEE PLAN FOR REQUIRED CONNECTION MOMENT. IF NO LOAD SHOWN, SEE TYPICAL DETAILS.
 - EXISTING LOWER TO BE INFILLED; NEW CONSTRUCTION TO MATCH EXISTING WALL CONSTRUCTION.
 - SEE TYPICAL REB PENETRATION DETAIL ON SHEET S511 FOR ADDITIONAL INFORMATION.
 - CANT LE INDICATES CANTILEVERED LEFT END.
 - INDICATES TRIPLE JAMB STUD. SEE SCHEDULE ON SHEET 3003 FOR ADDITIONAL INFORMATION.
 - MH_ INDICATES METAL STUD HEADER. SEE SCHEDULE ON SHEET 3003 FOR ADDITIONAL INFORMATION.
 - G.C. TO SHORE EXISTING WALL DURING DEMO AND INSTALLATION OF NEW PRECAST LINTEL.
 - PROVIDE SOLID CONTINUOUS BLOCKING FOR FOLDING PARTION BELOW.

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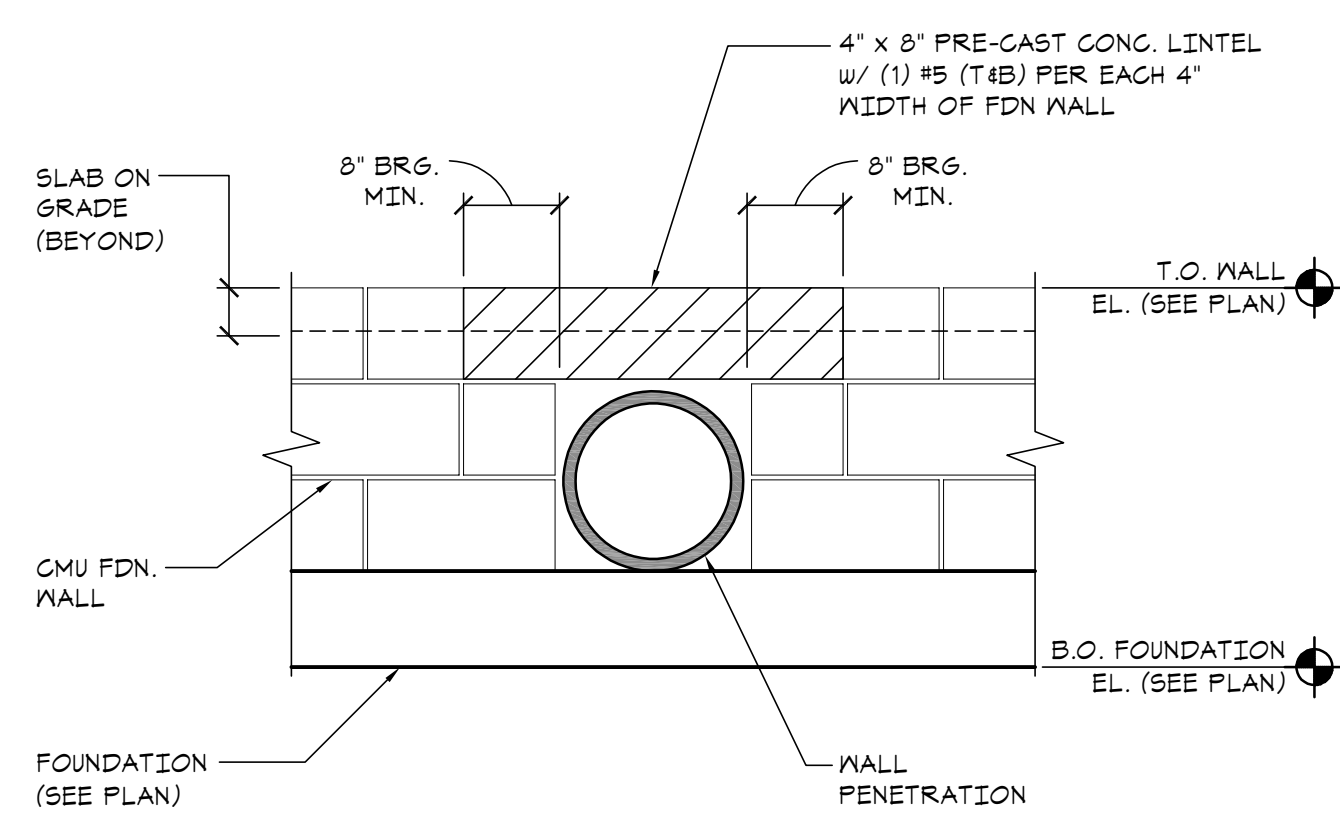
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Typical Depressed Slab Detail

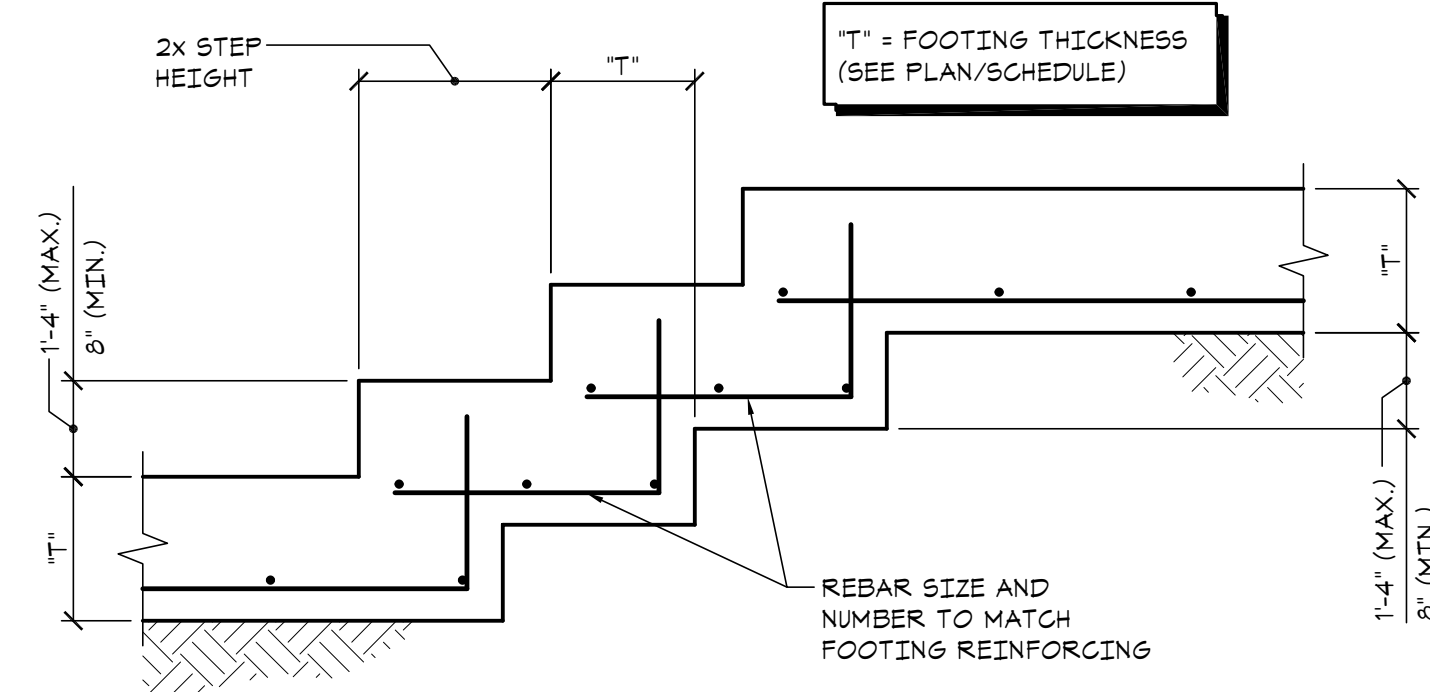


Typical Slab On Grade Infill Detail



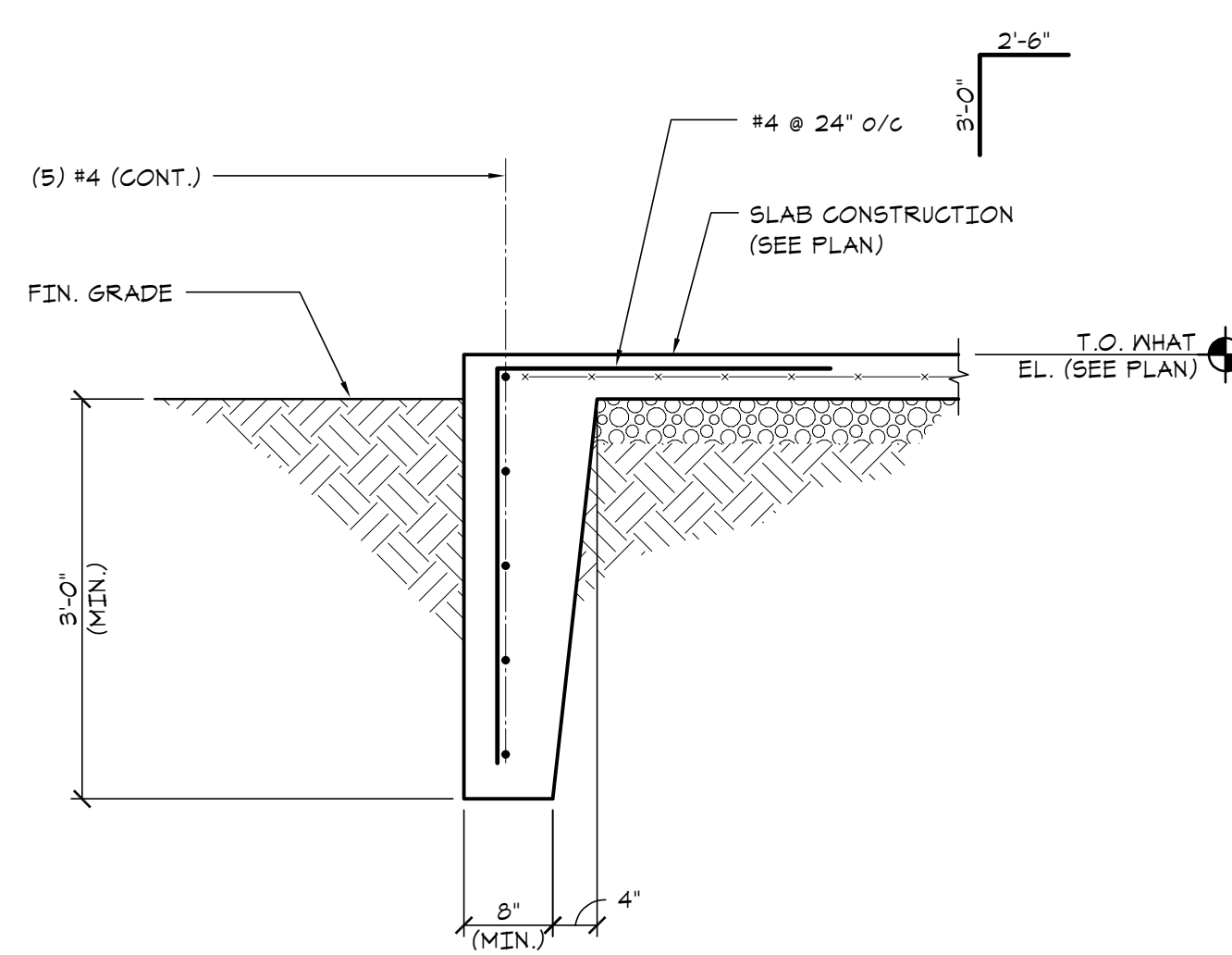
Typical Foundation Wall Penetration Detail

NOTE:
1. STEP FOOTING AS REQUIRED FOR WALL PENETRATION TO OCCUR OVER FOOTING.

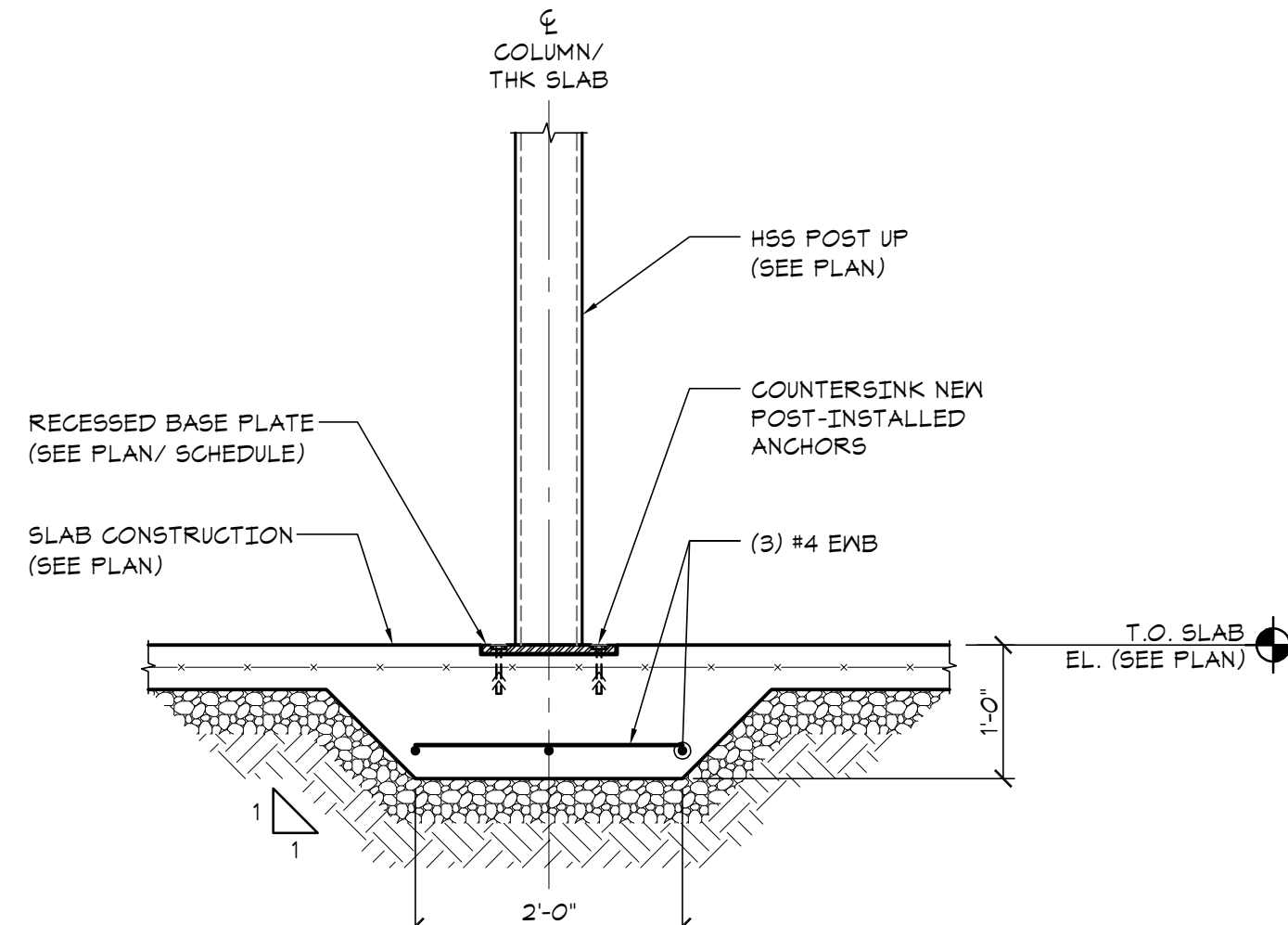


Typical Stepped Footing Detail

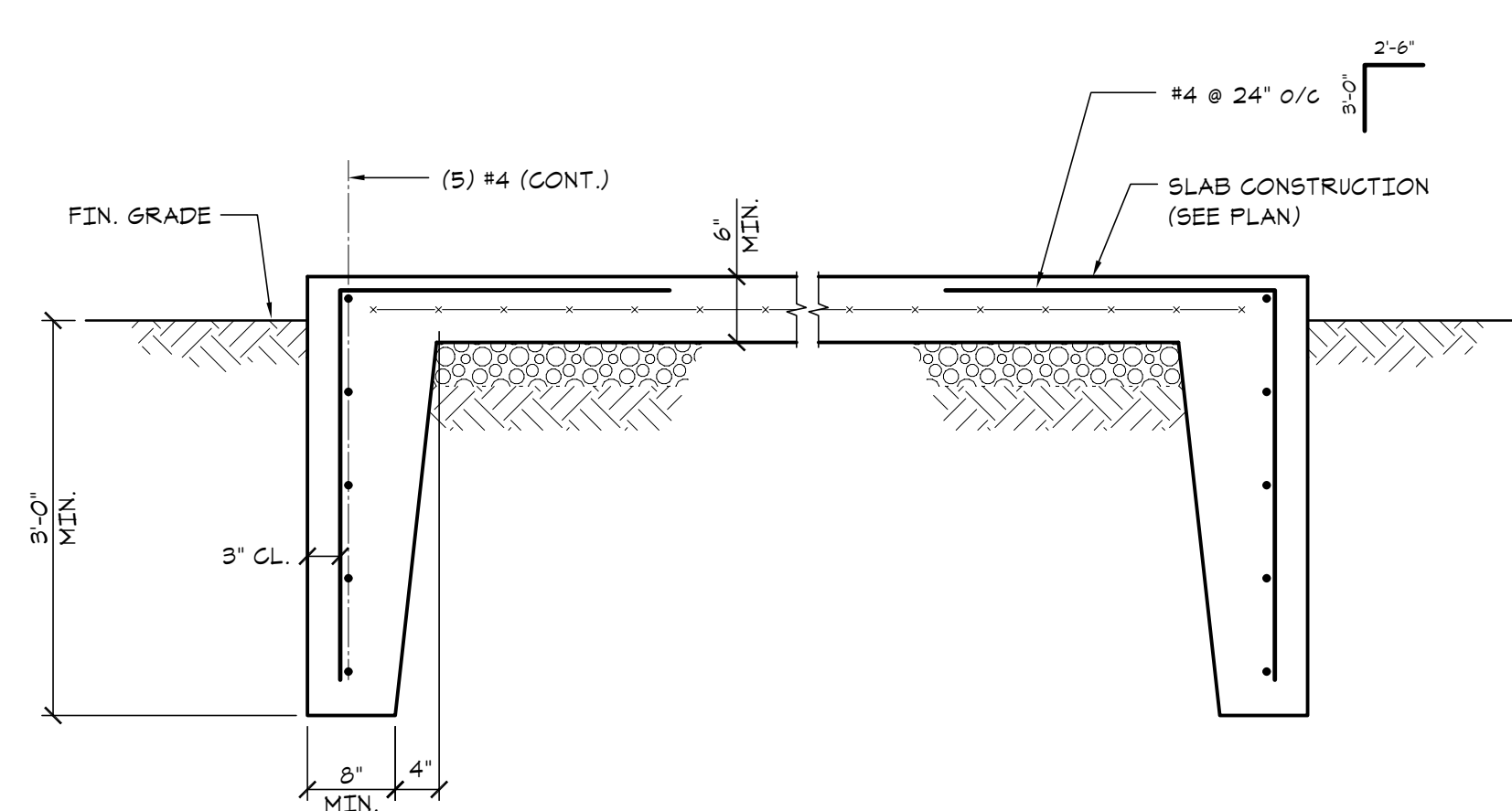
NOTE: SLOPE OF STEPPED FOOTING SHALL NOT EXCEED THE RATIO OF TWO HORIZONTAL TO ONE VERTICAL.



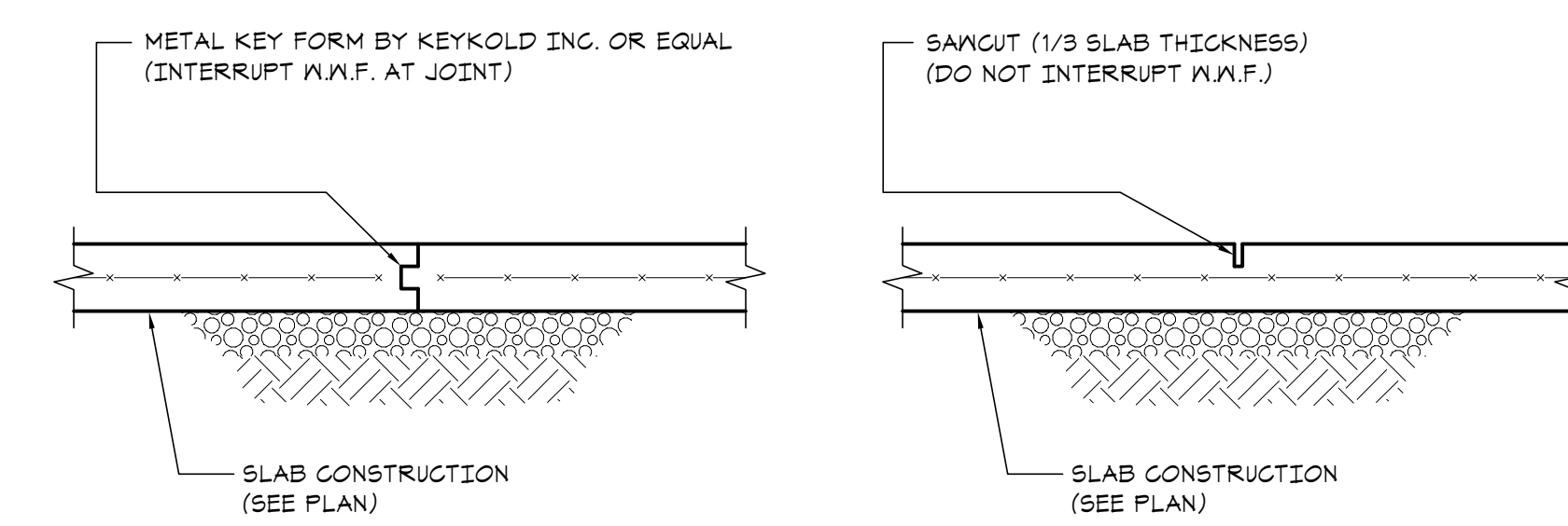
Typical Turned Down Slab Detail



Typical Thickened Footing Slab Detail



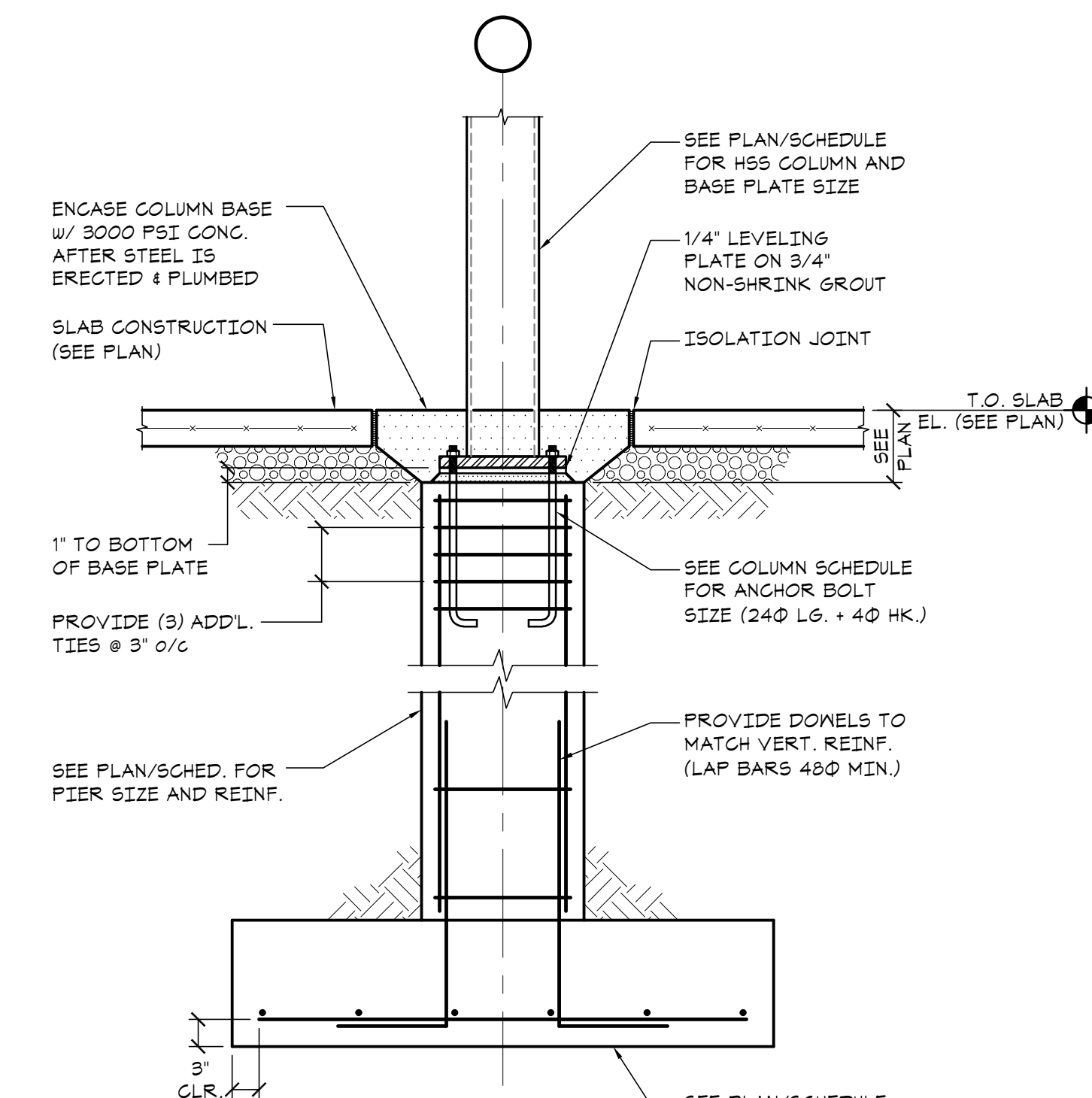
Typical Equipment Pad Detail



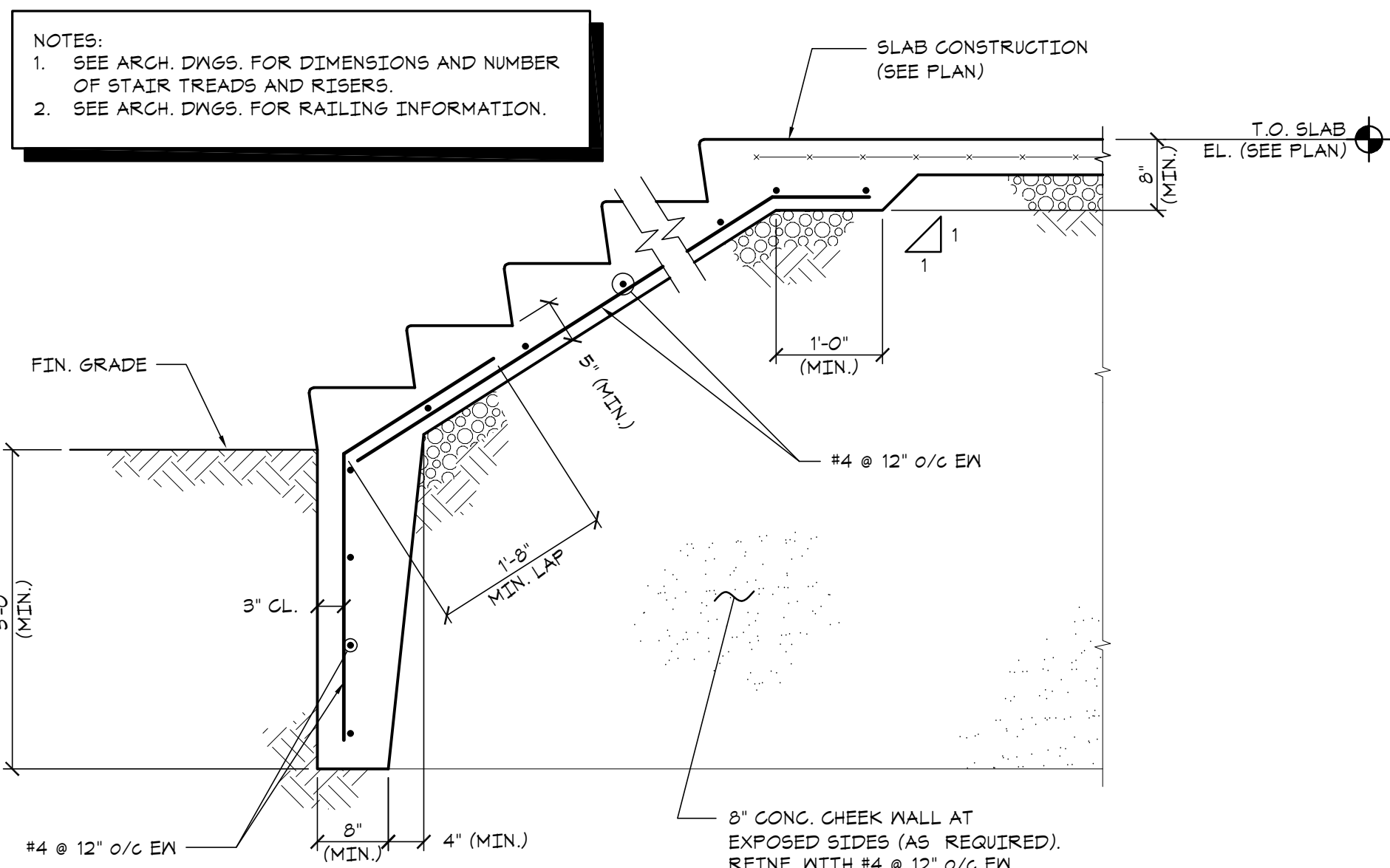
CONSTRUCTION JOINT

CONTROL JOINT

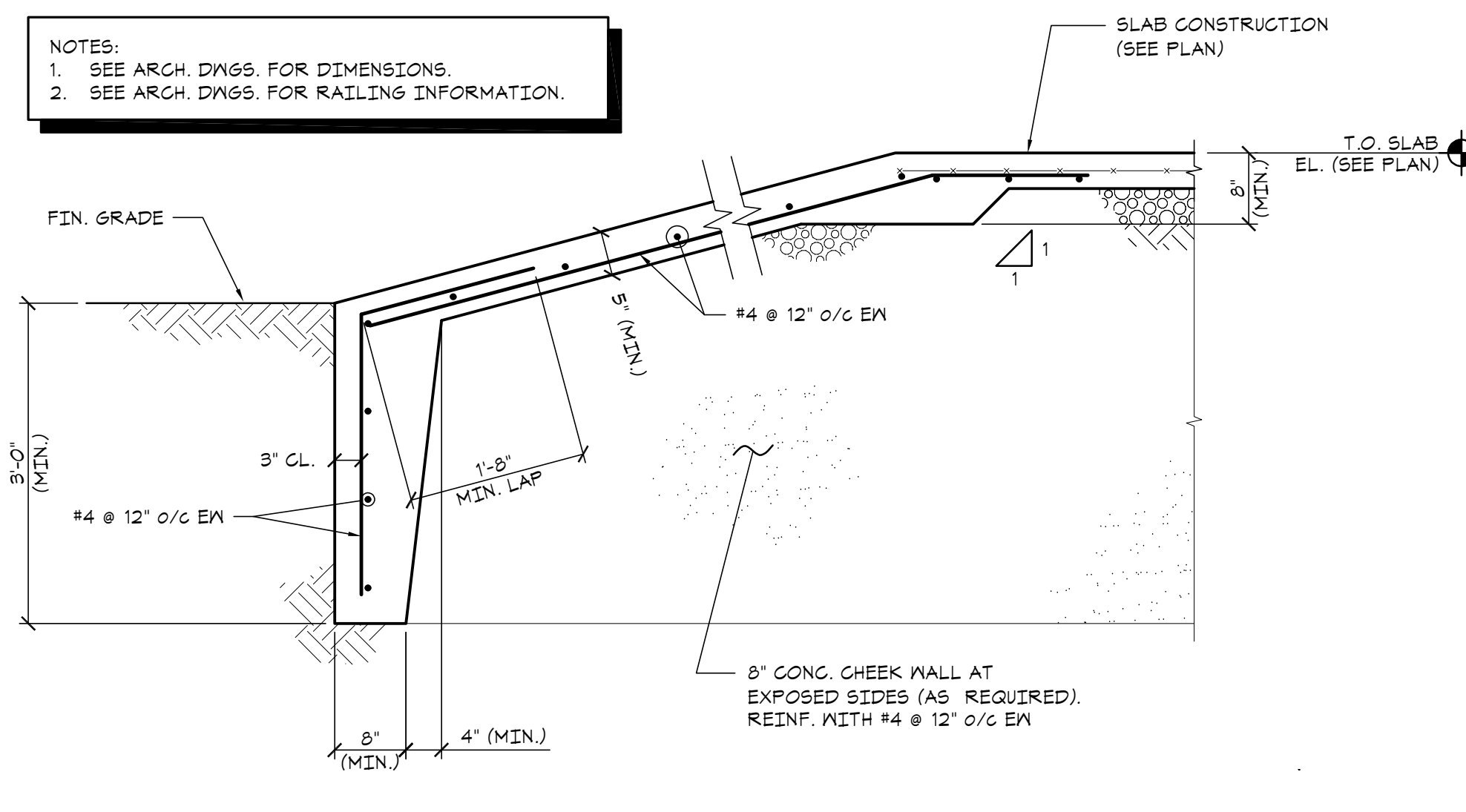
Typical Slab On Grade Details



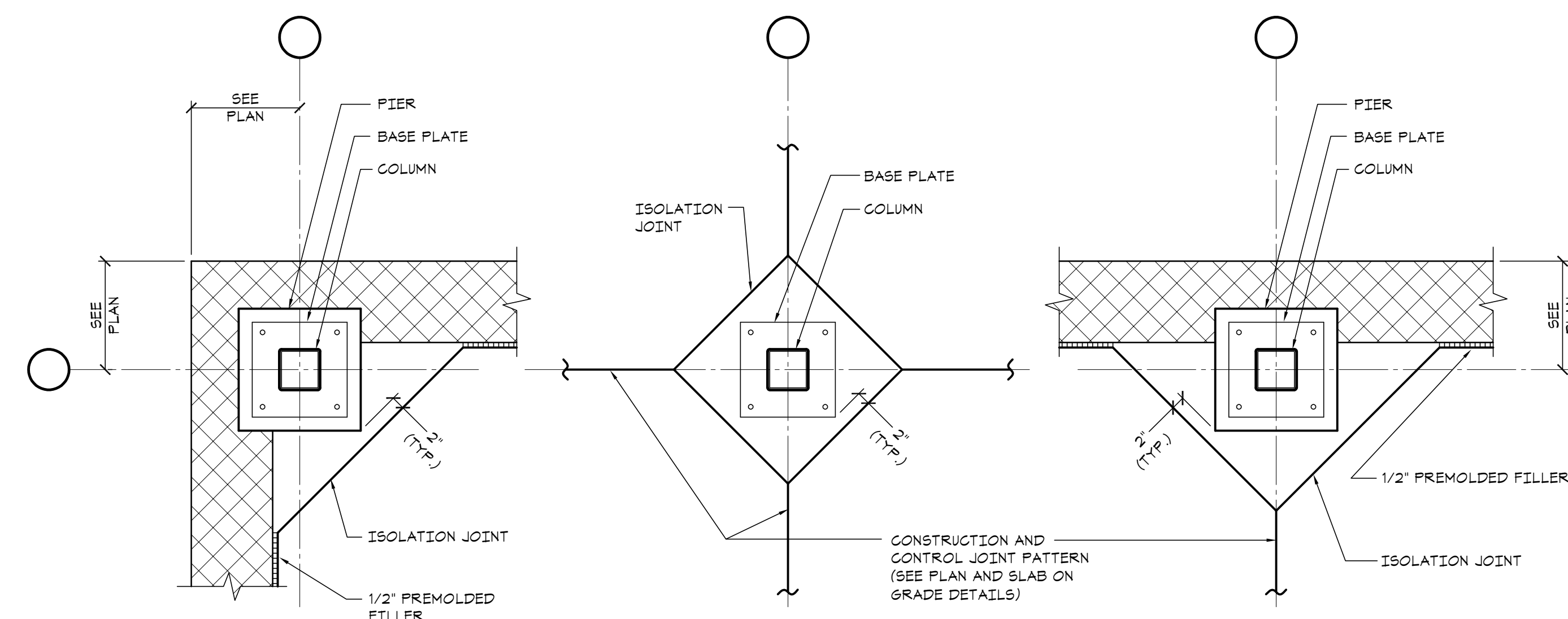
Typical Column Footing w/ Pier Detail



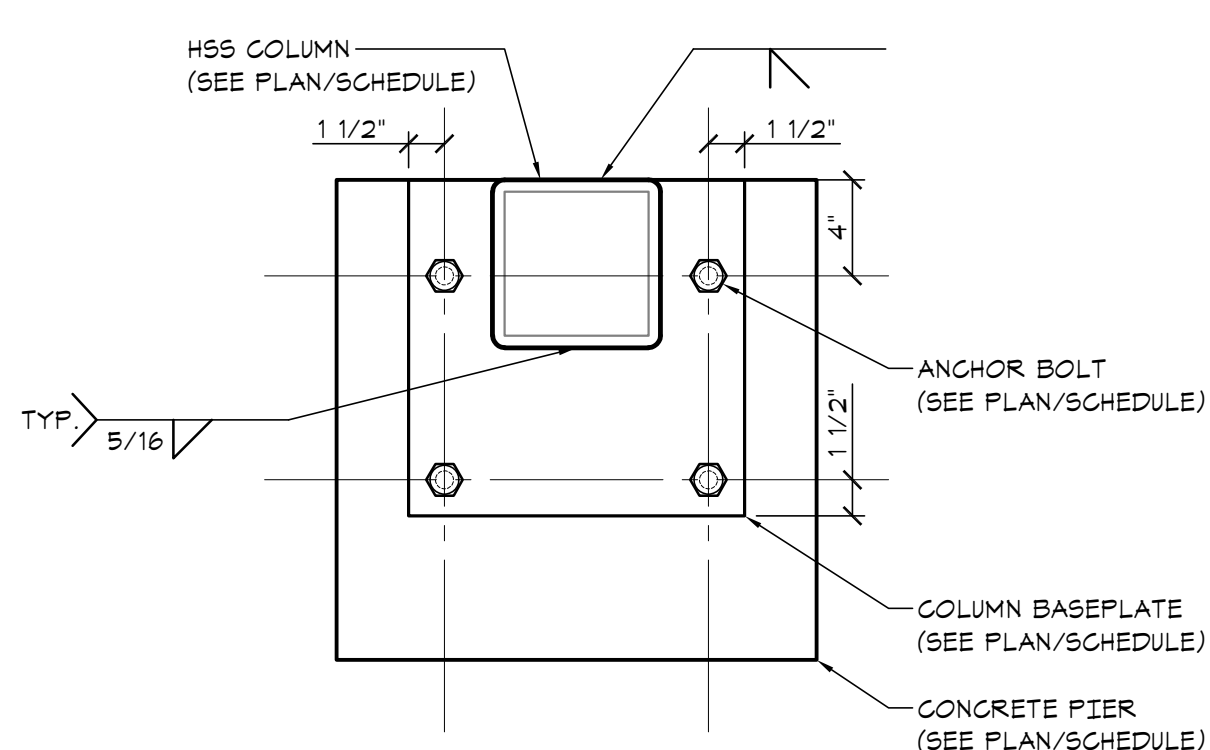
Typical Concrete Stair On Grade Detail



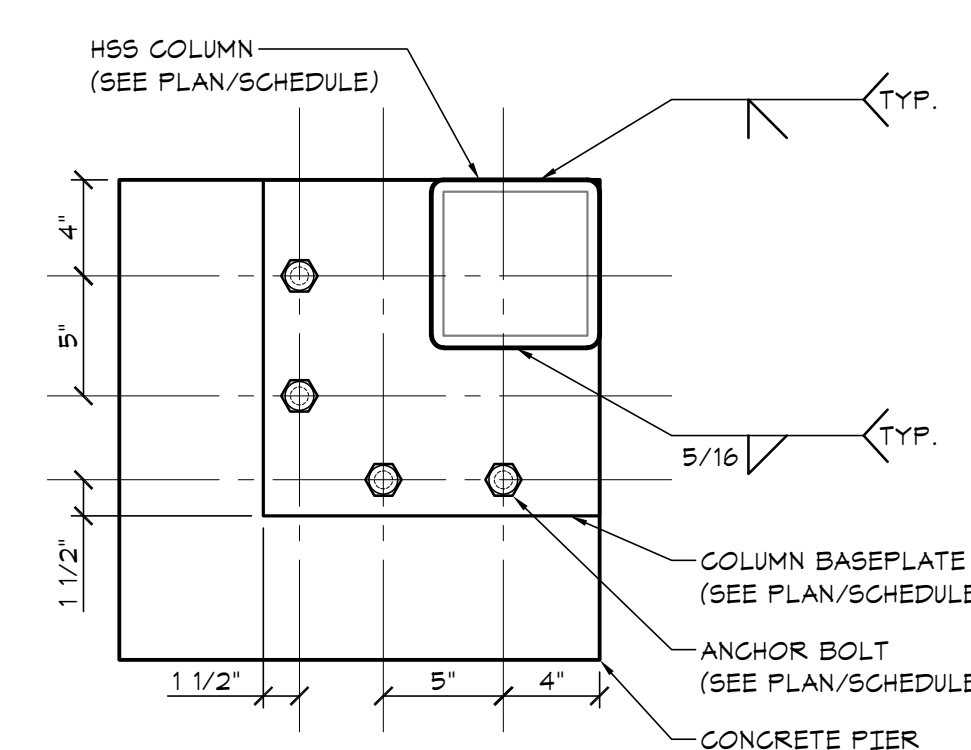
Typical Concrete Ramp On Grade Detail



Typical Column Isolation Joint Details



Baseplate Layout A Detail



Baseplate Layout B Detail



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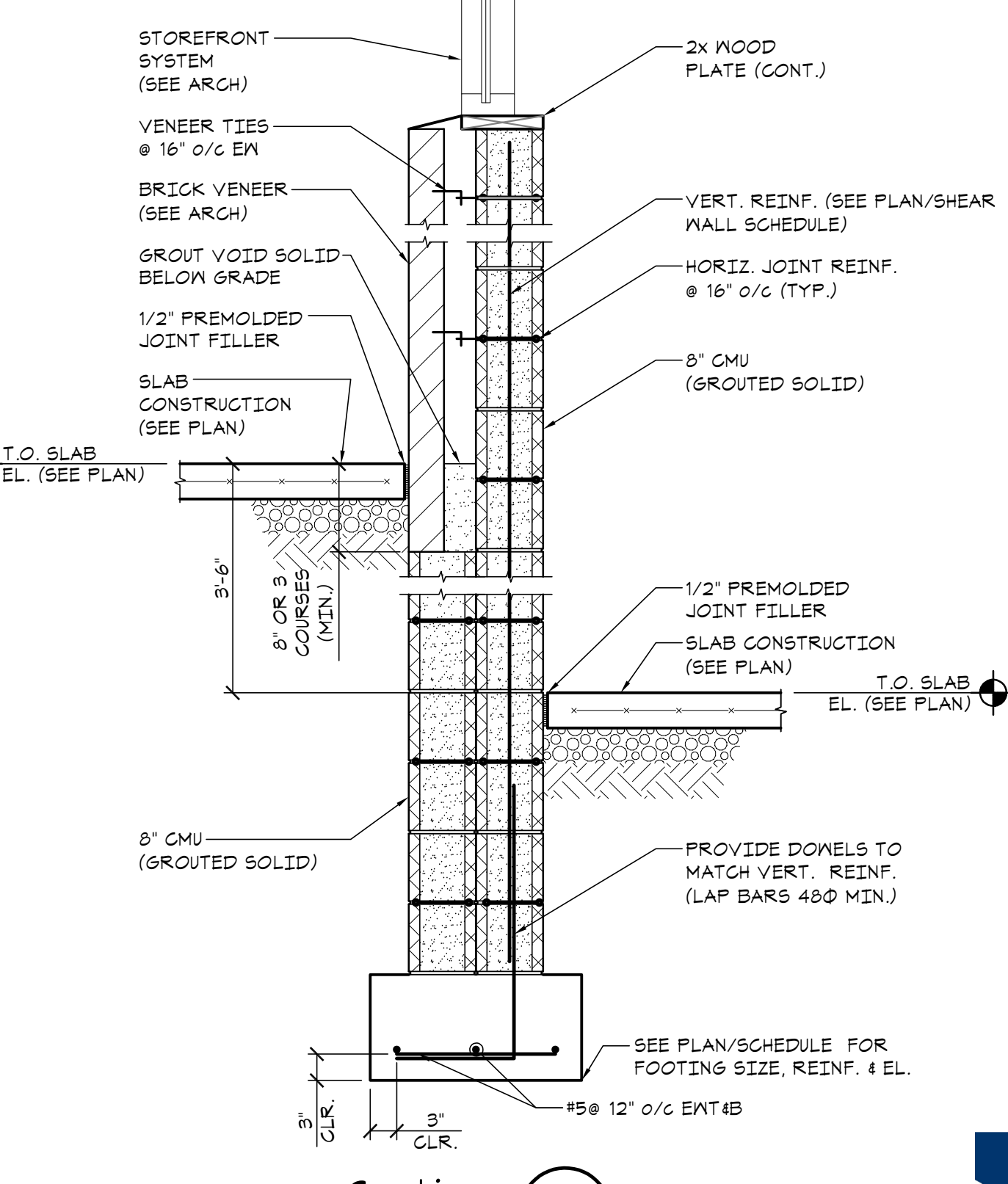
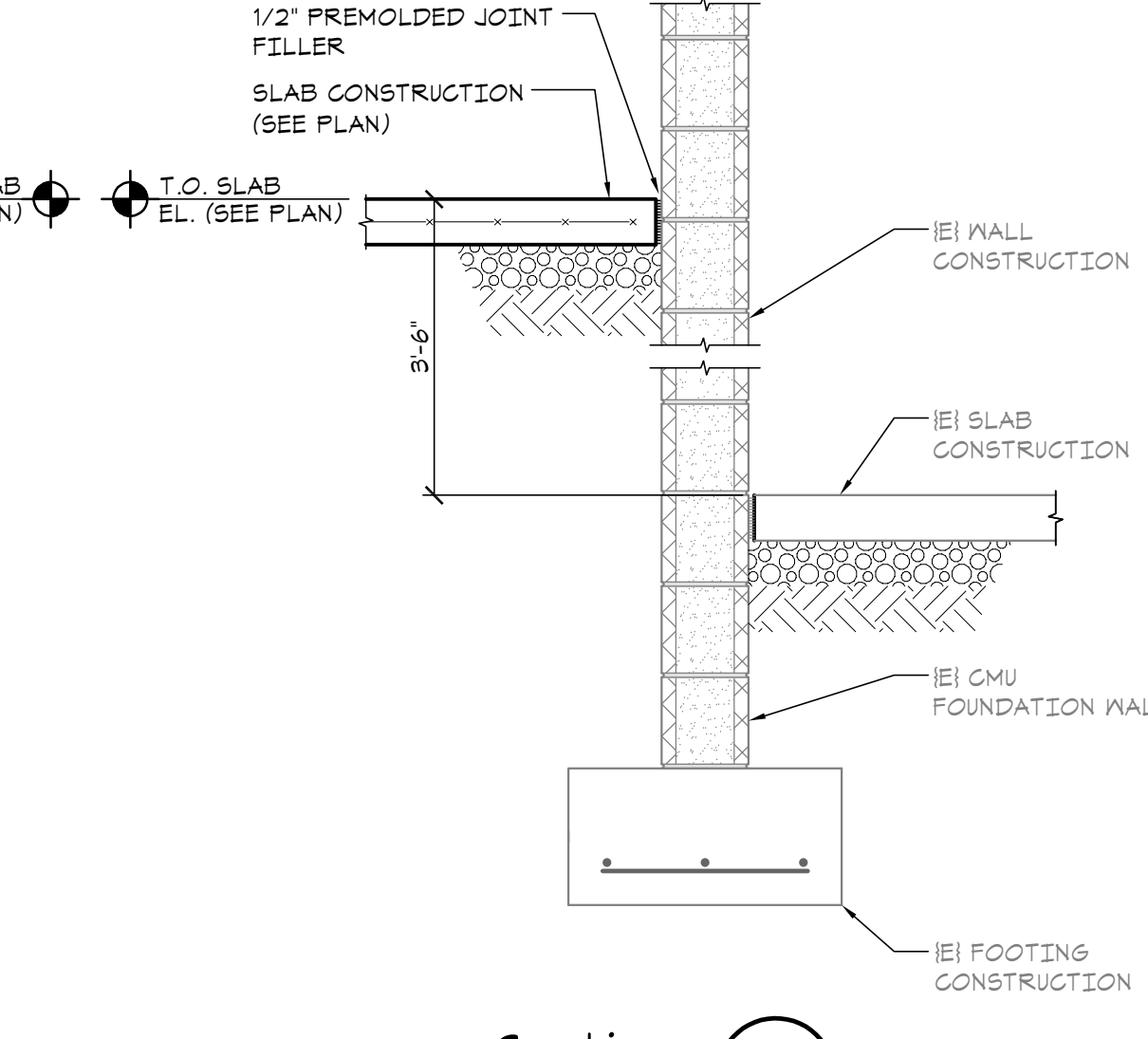
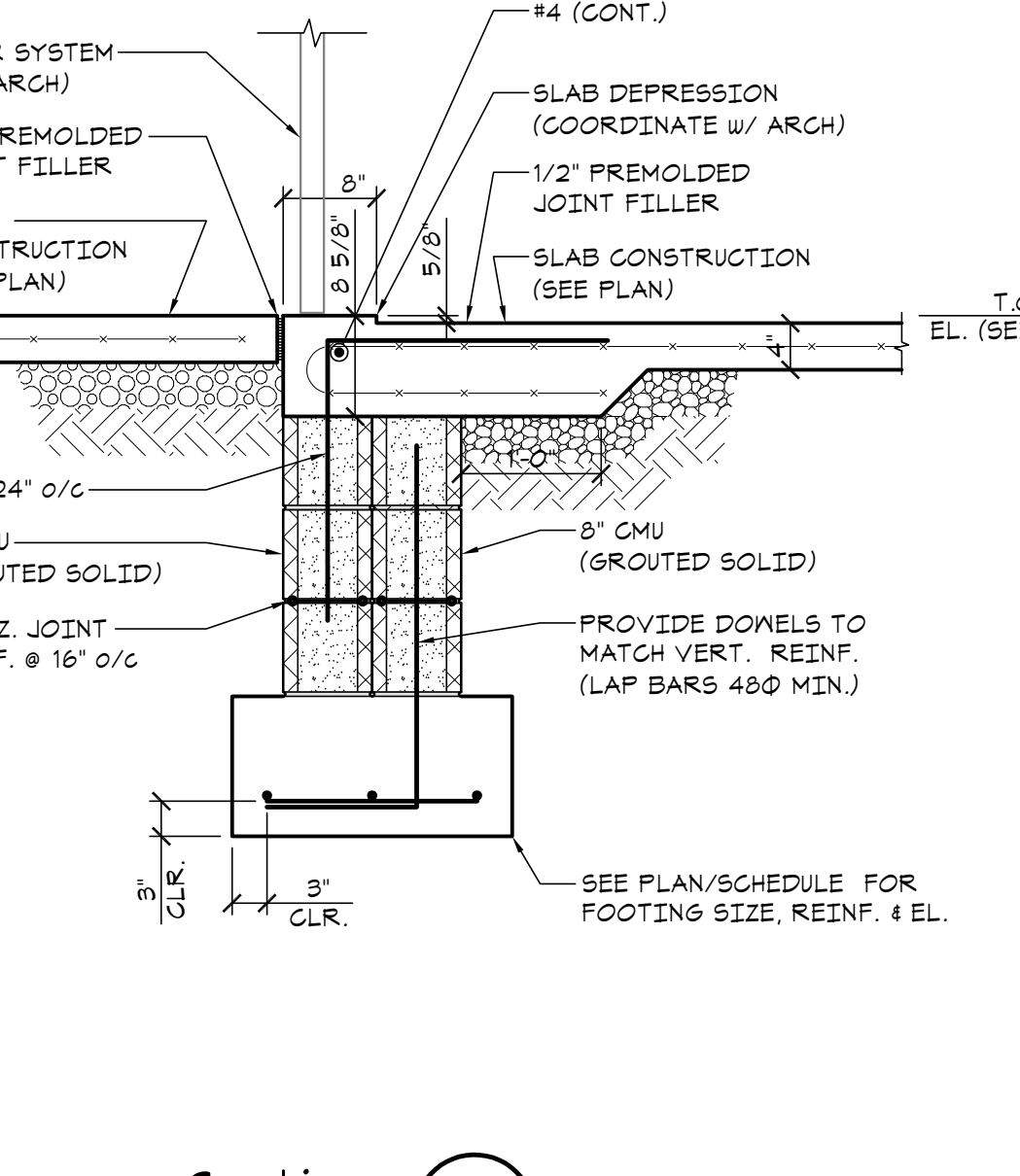
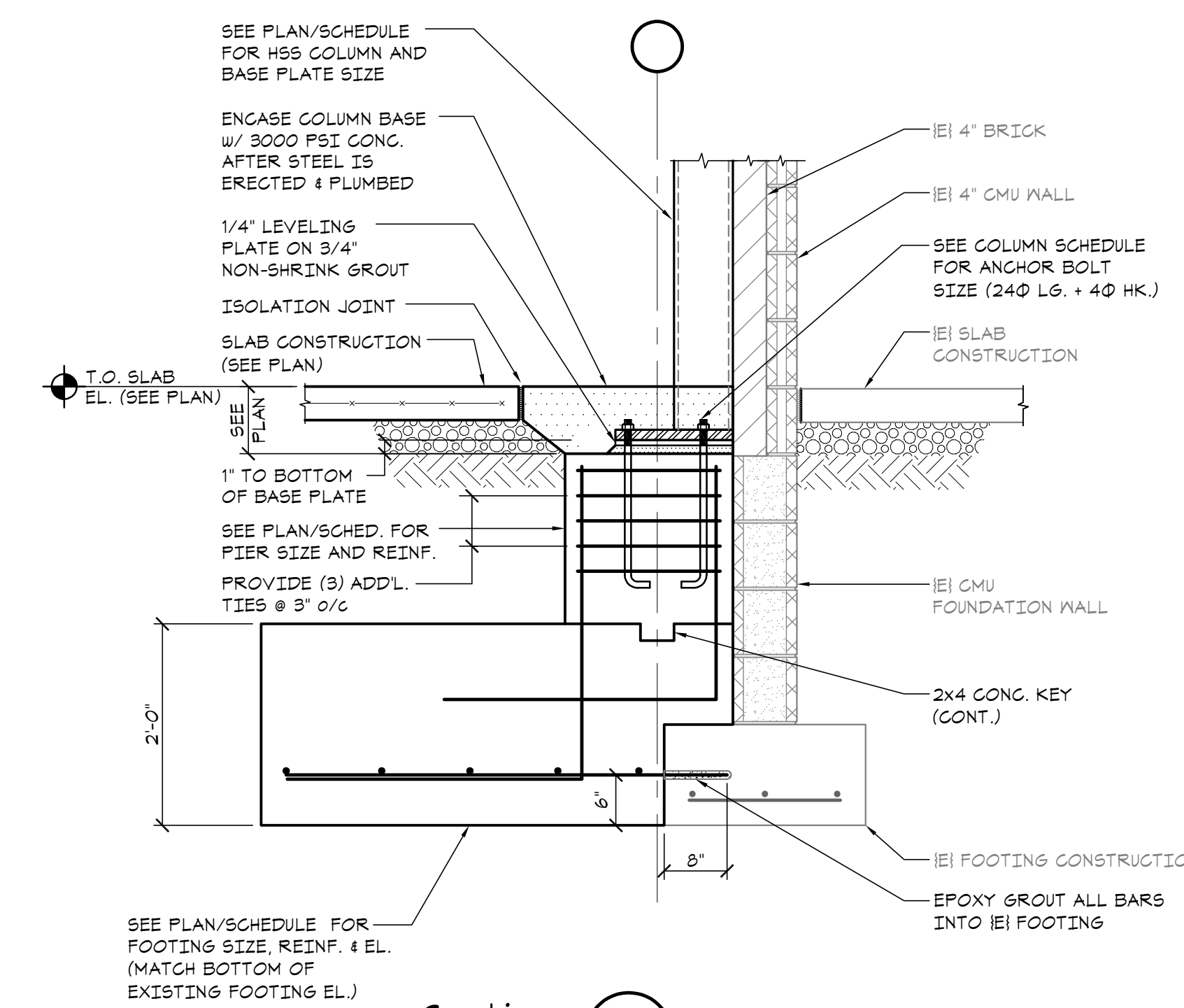
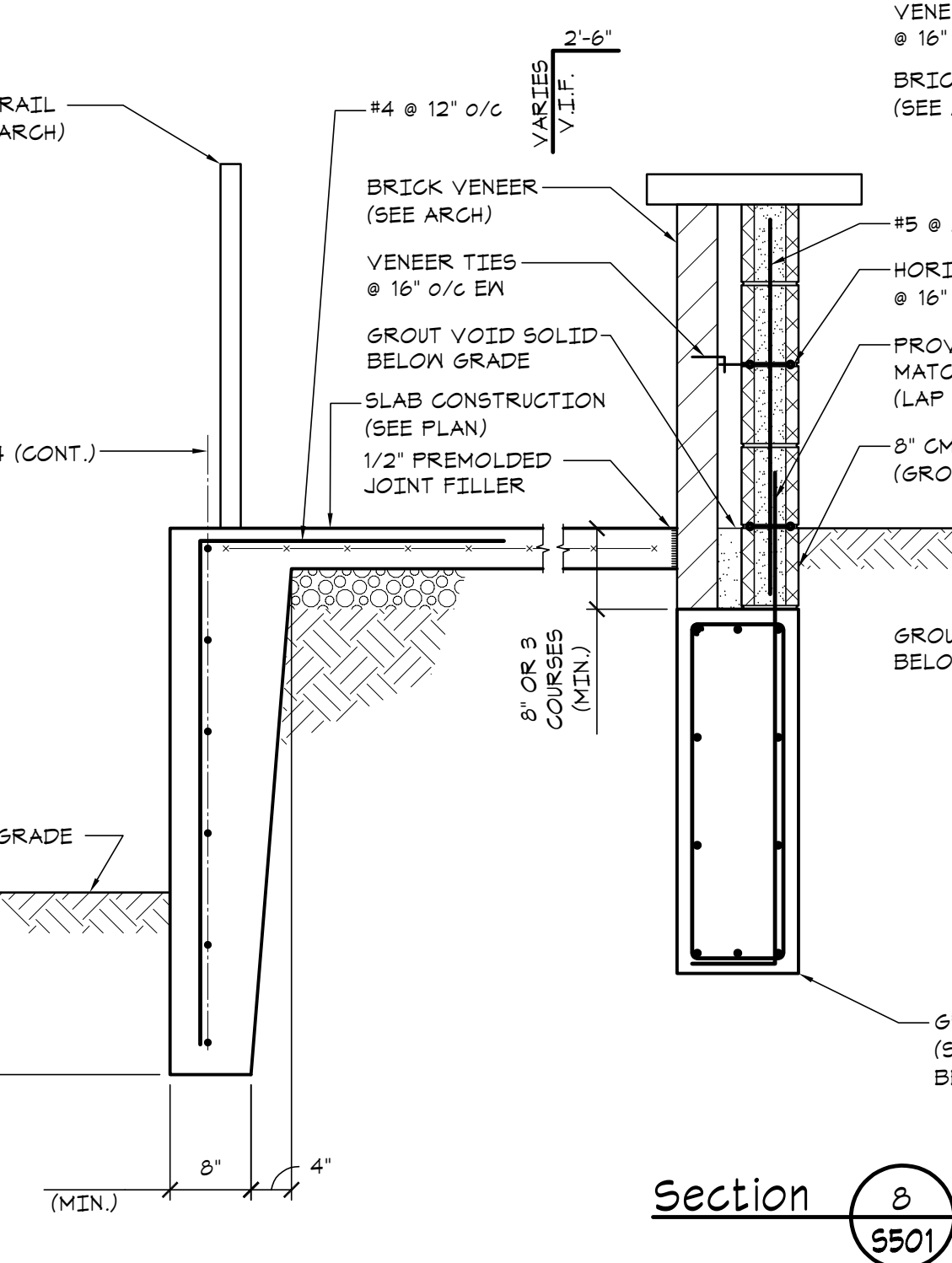
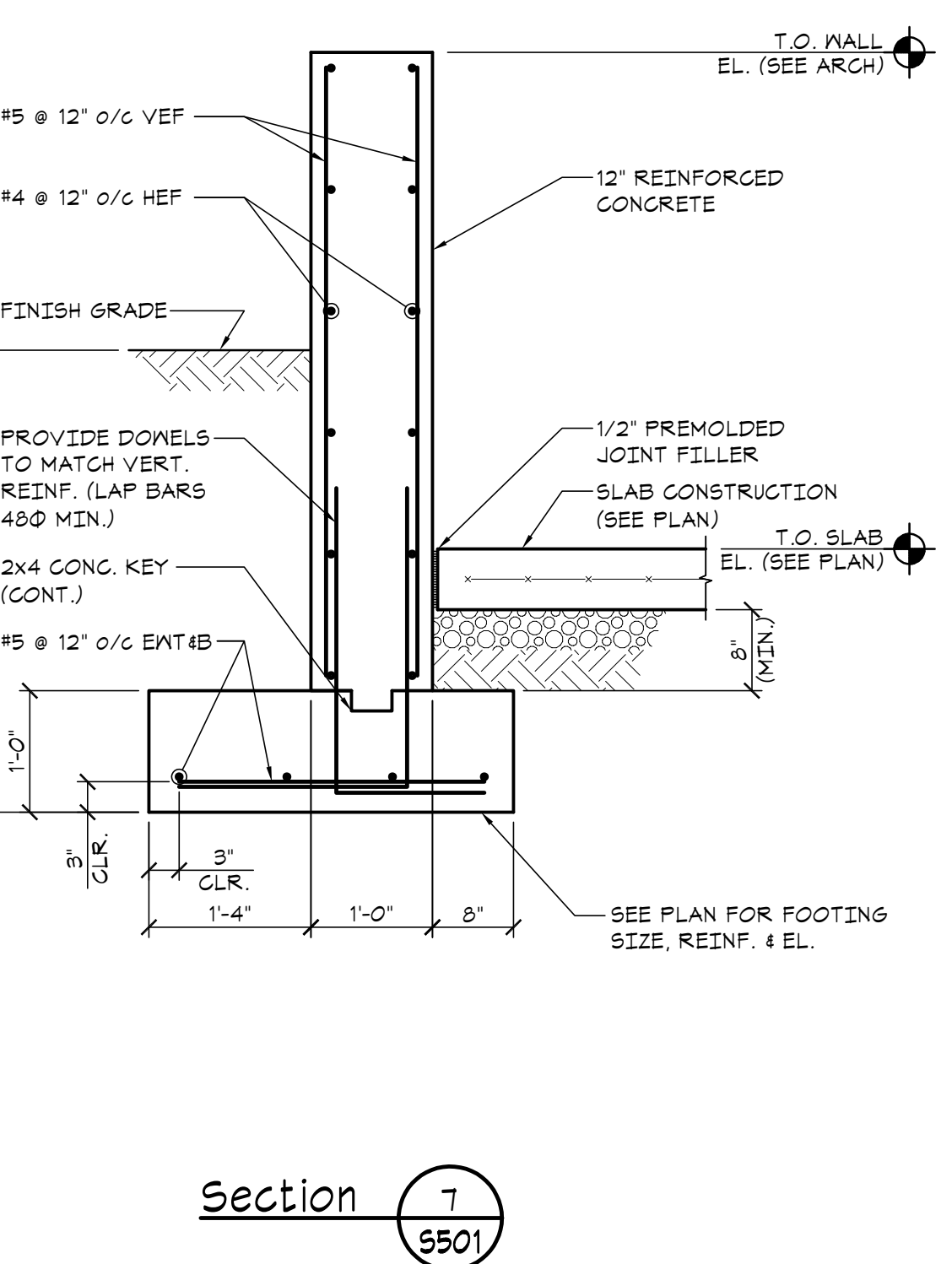
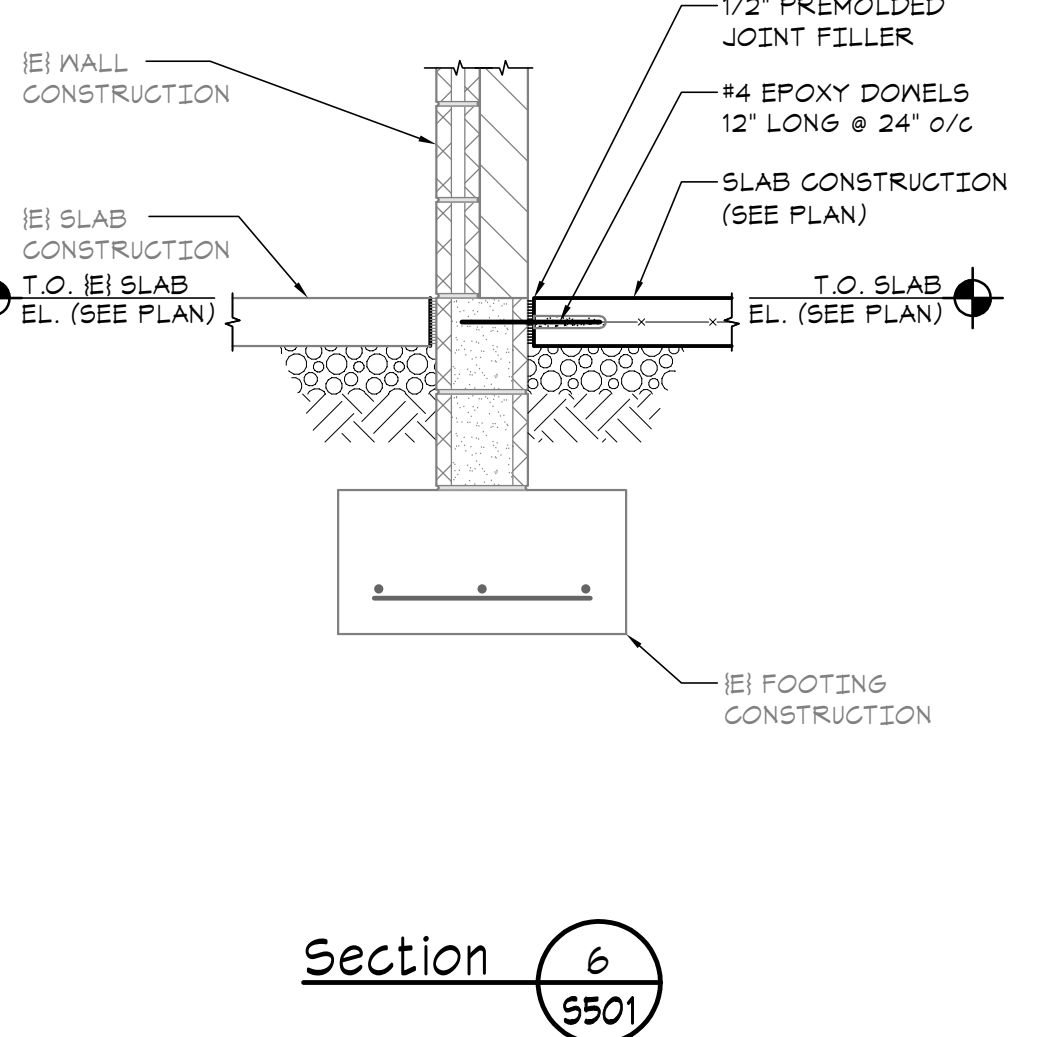
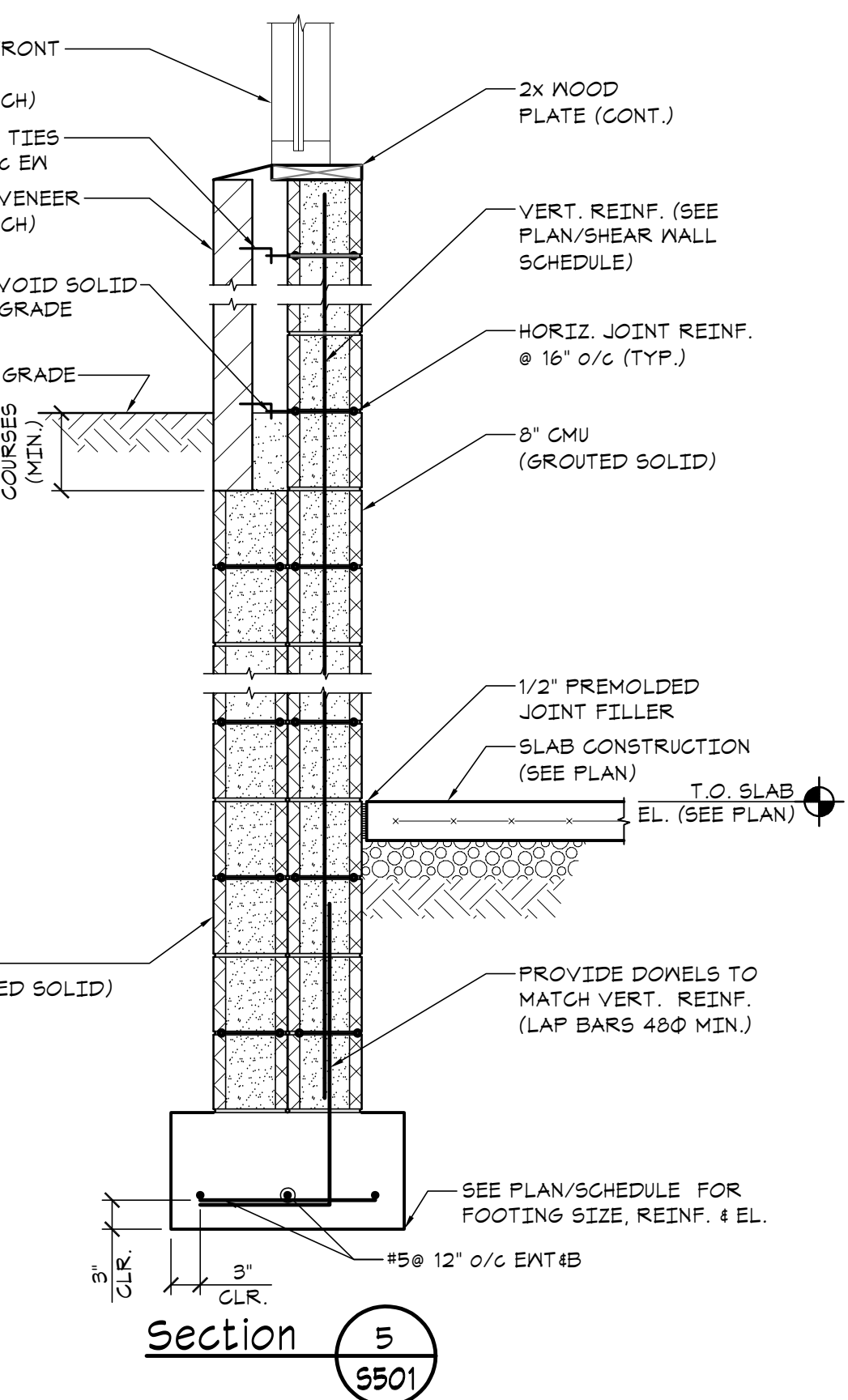
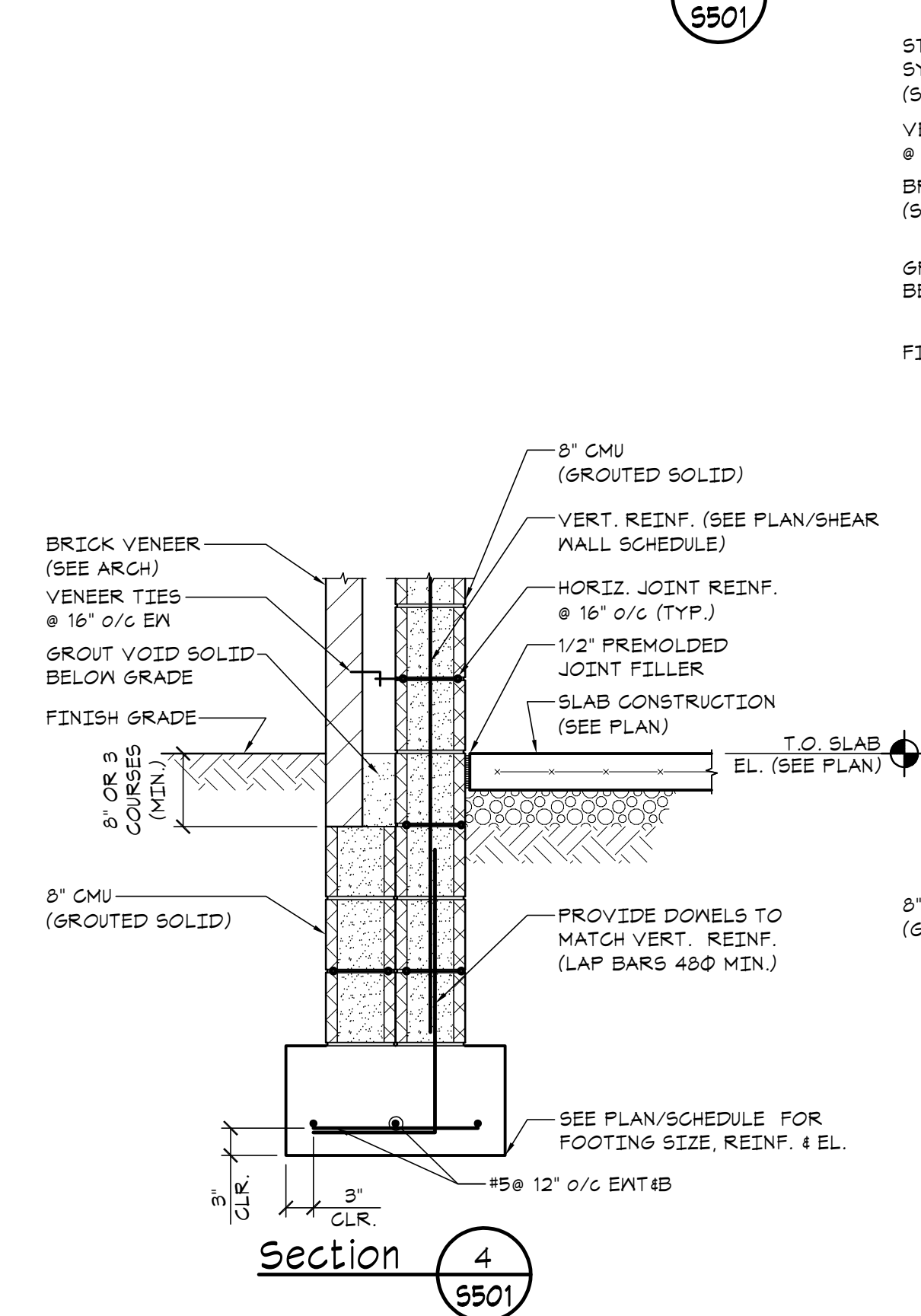
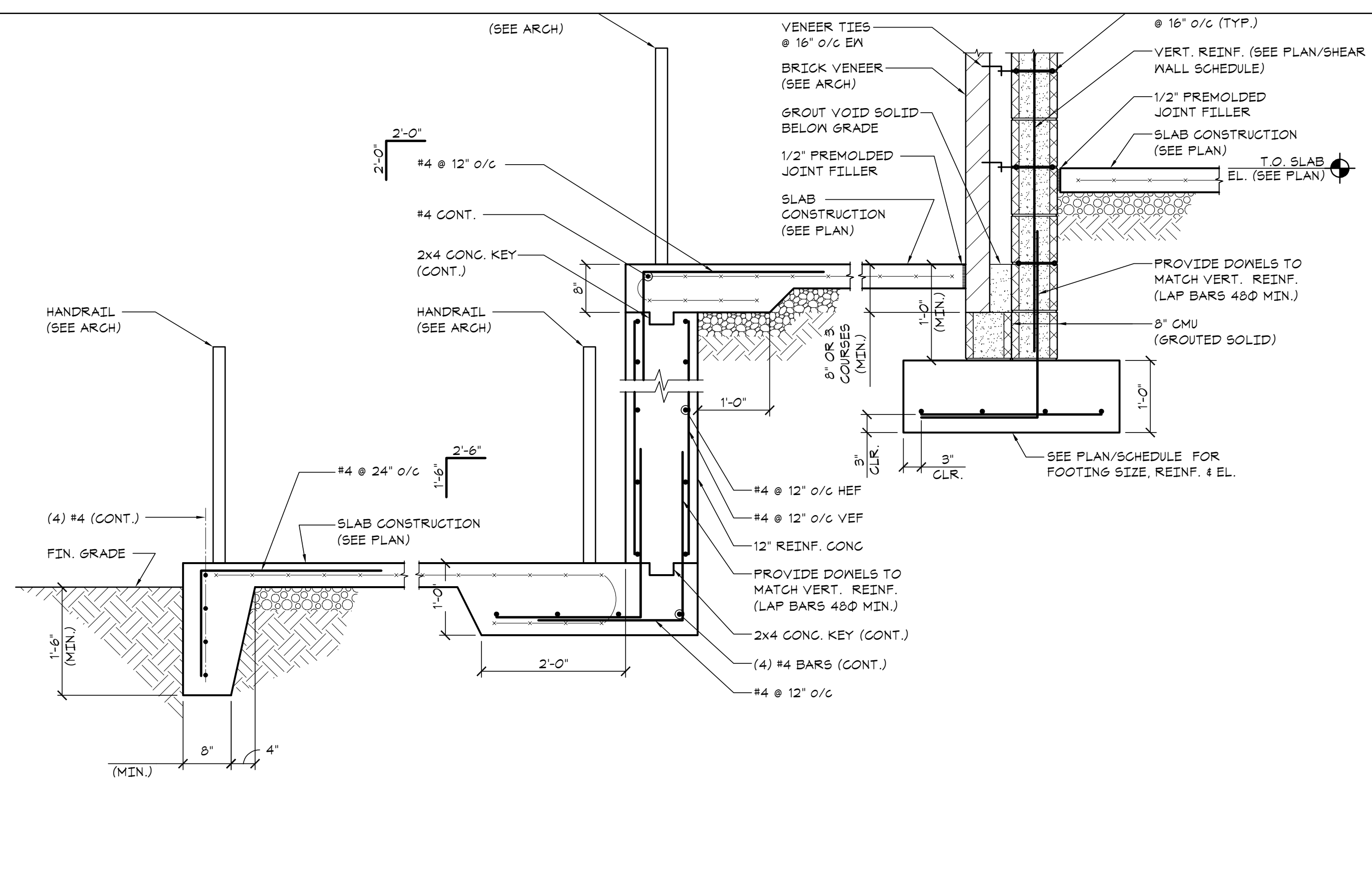
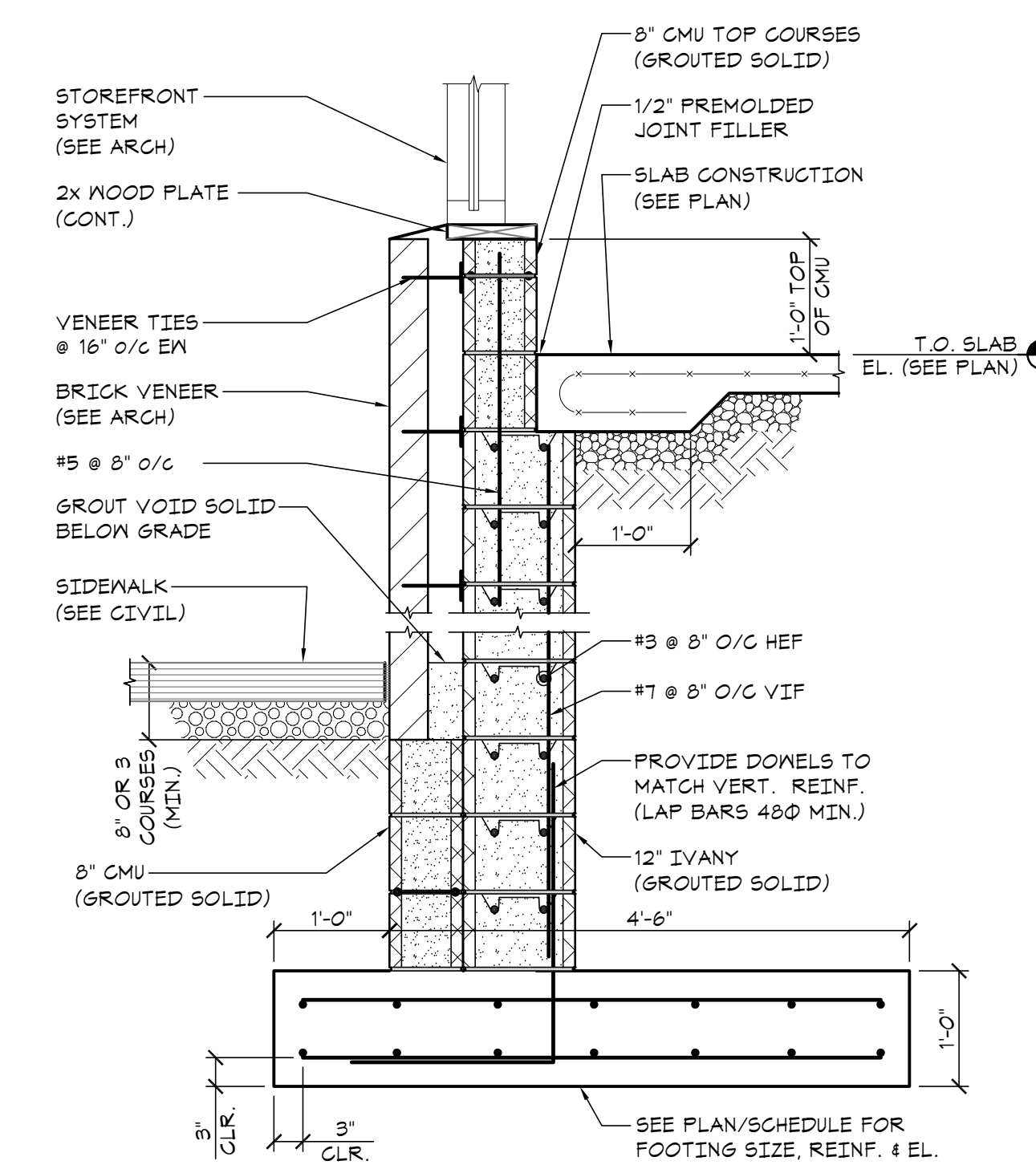
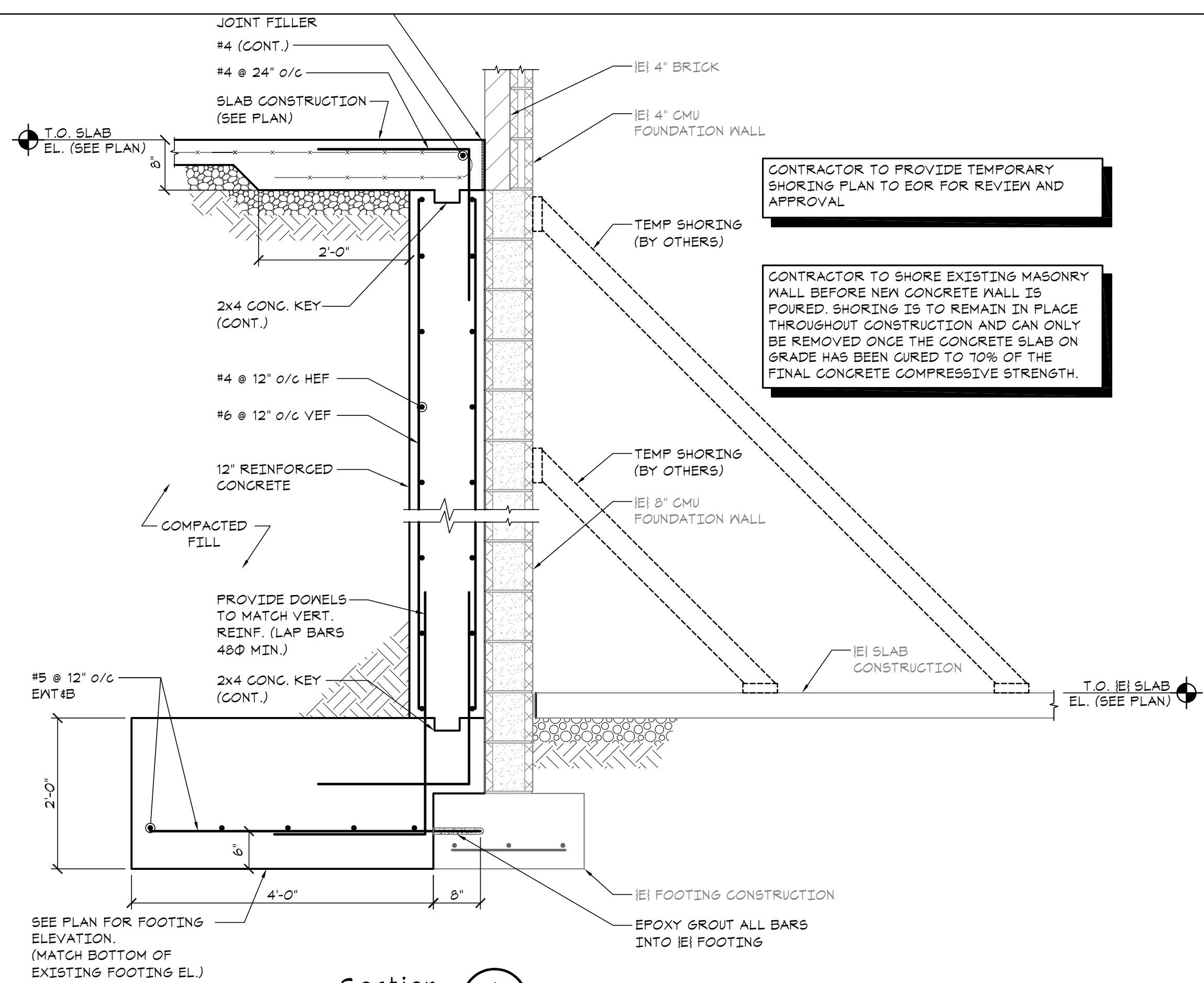
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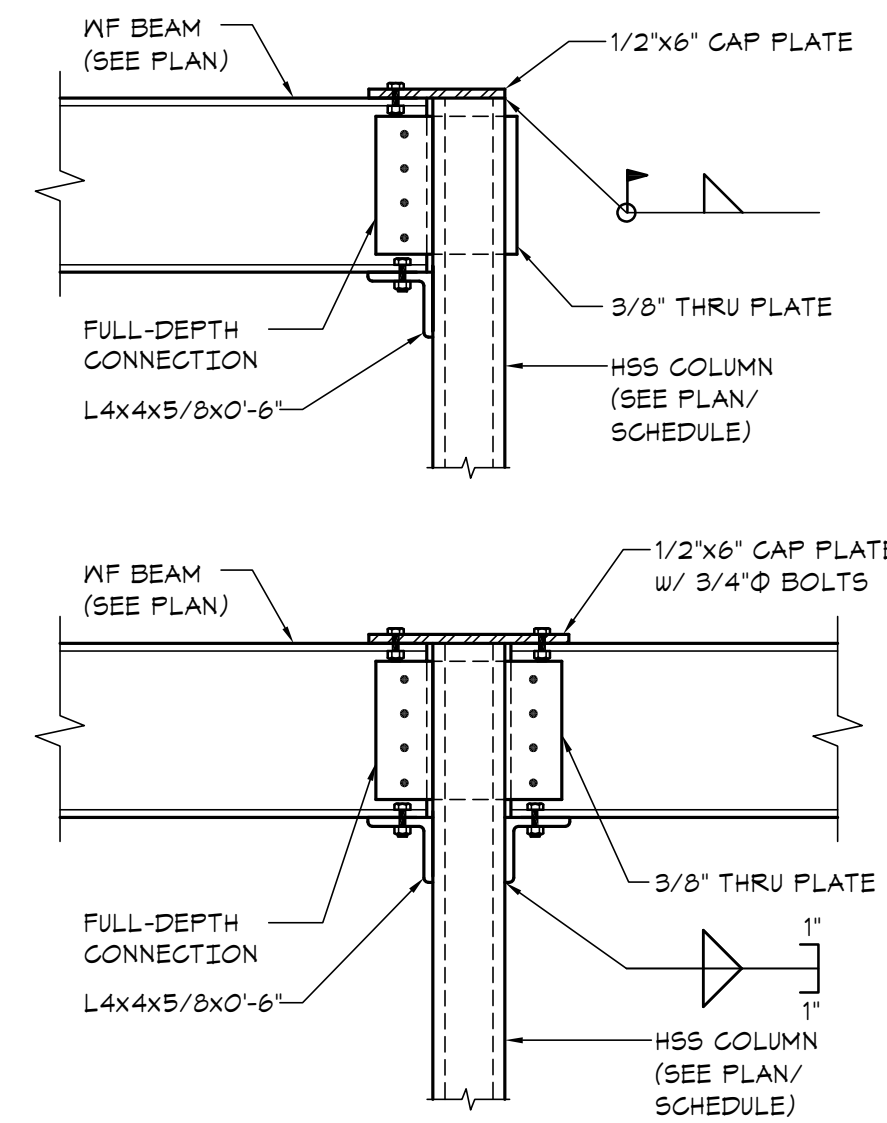
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**TYPICAL
FRAMING
DETAILS**

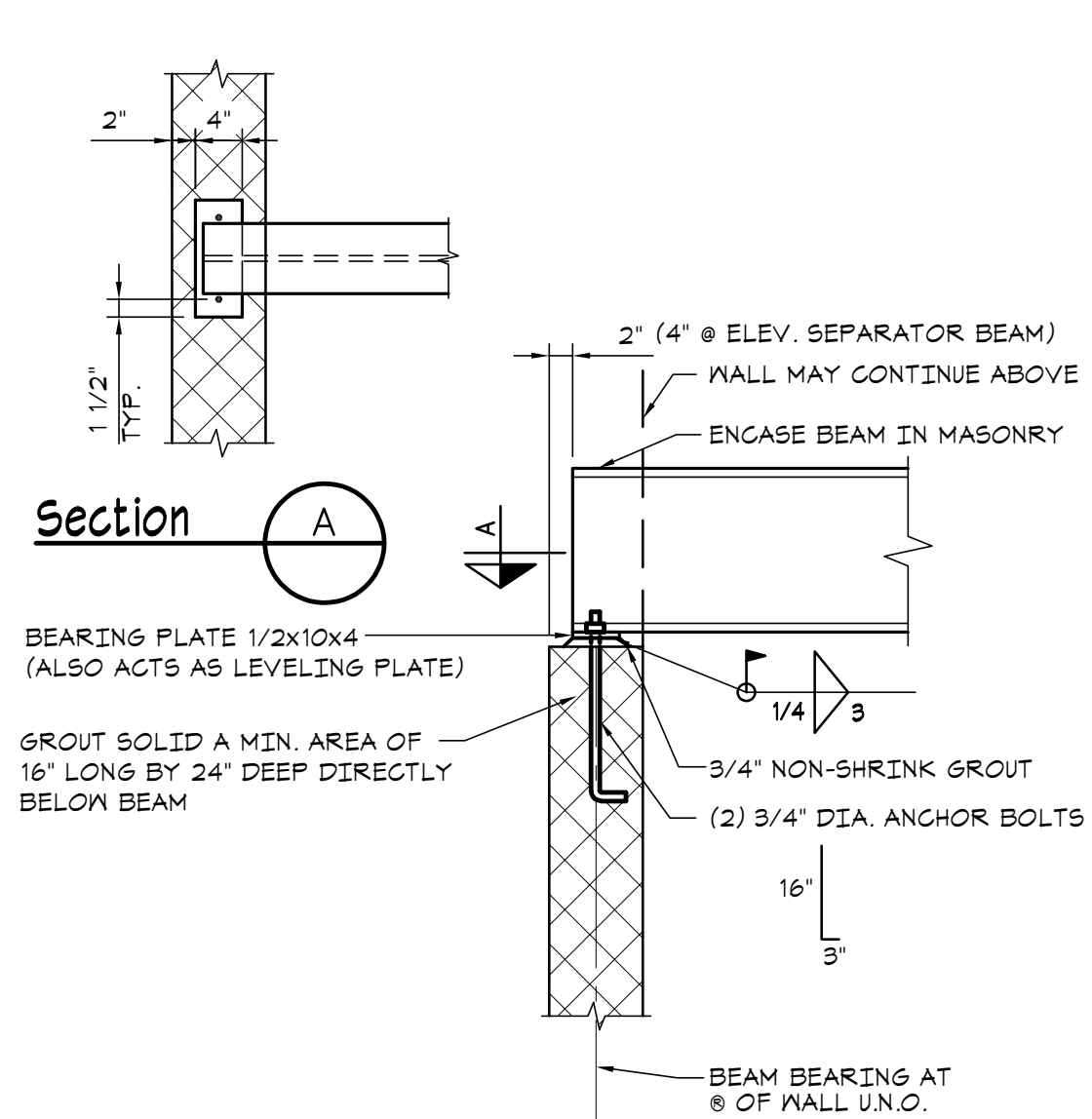
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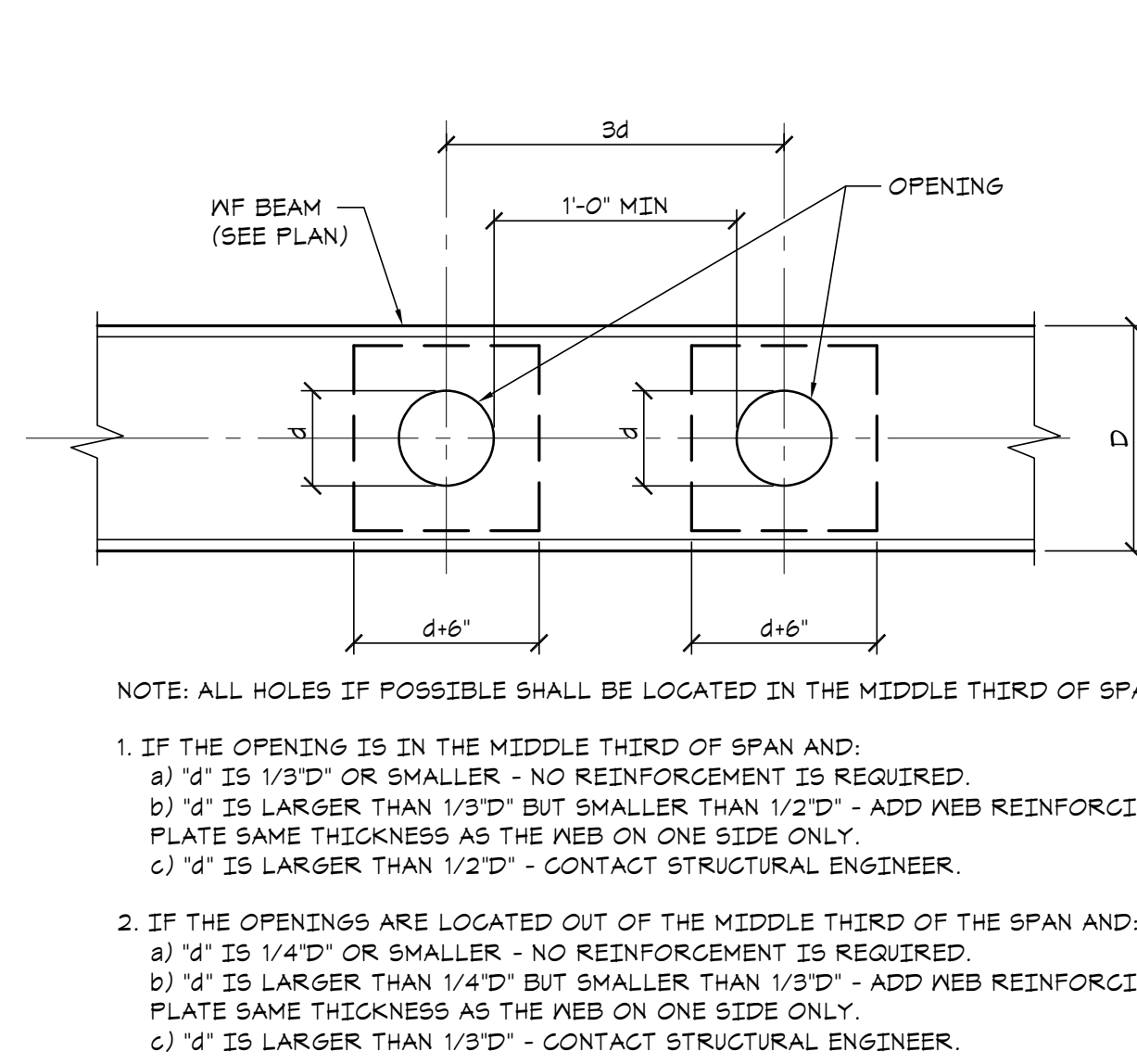
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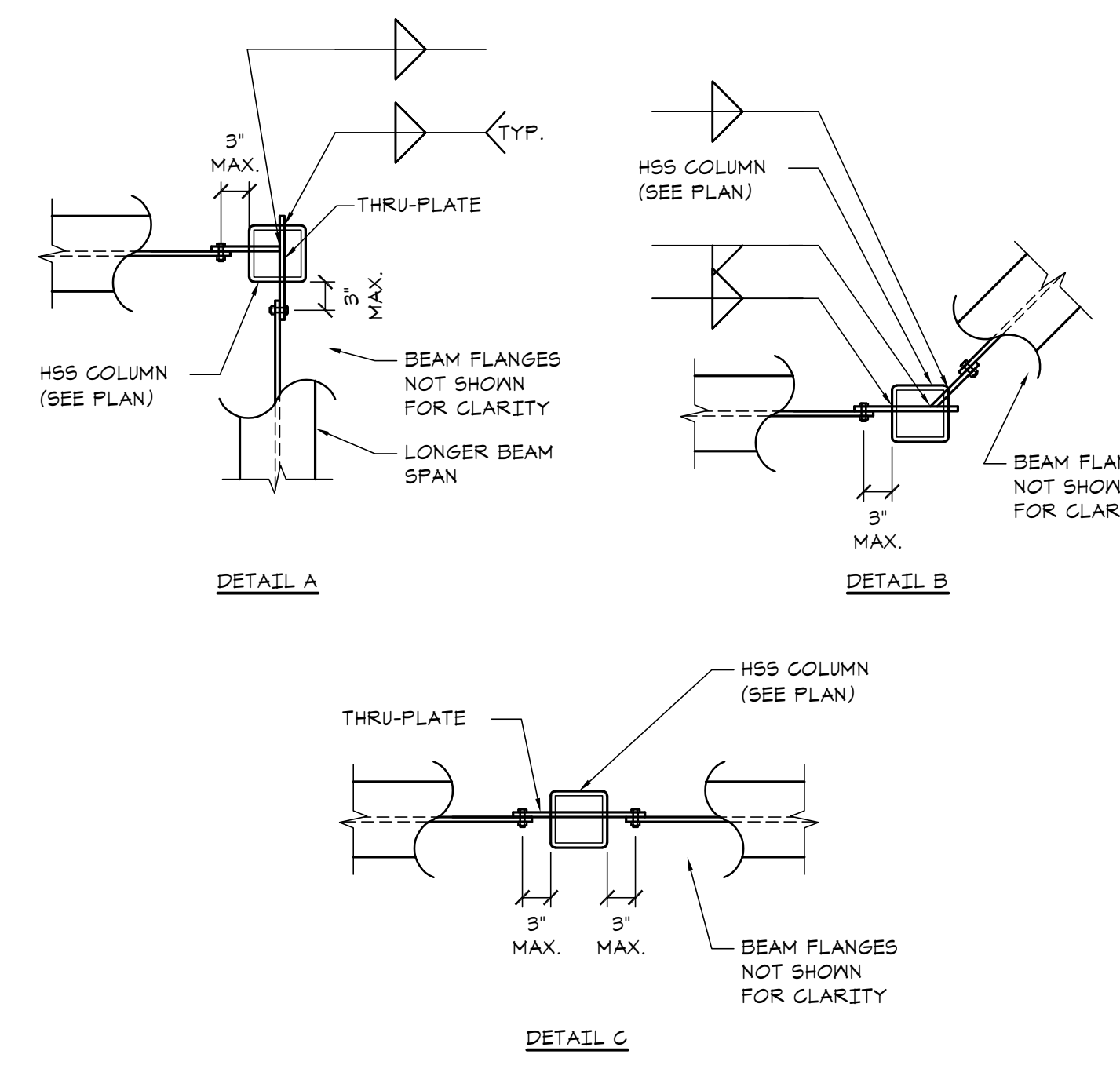
Typical Wind Moment Connection Detail



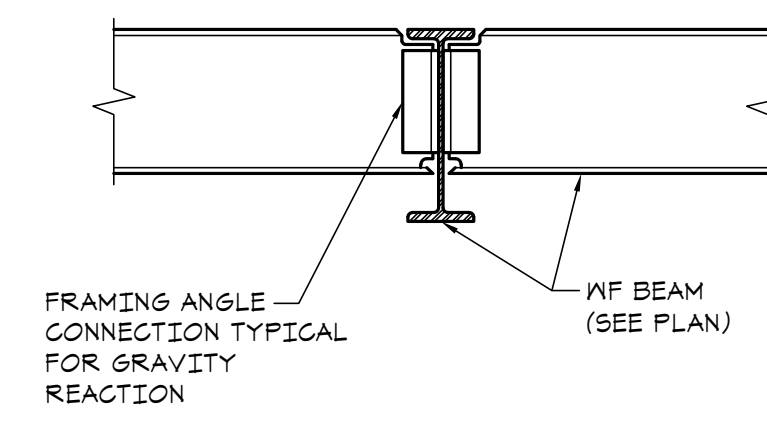
Typical Wide Flange Bearing On Masonry Detail



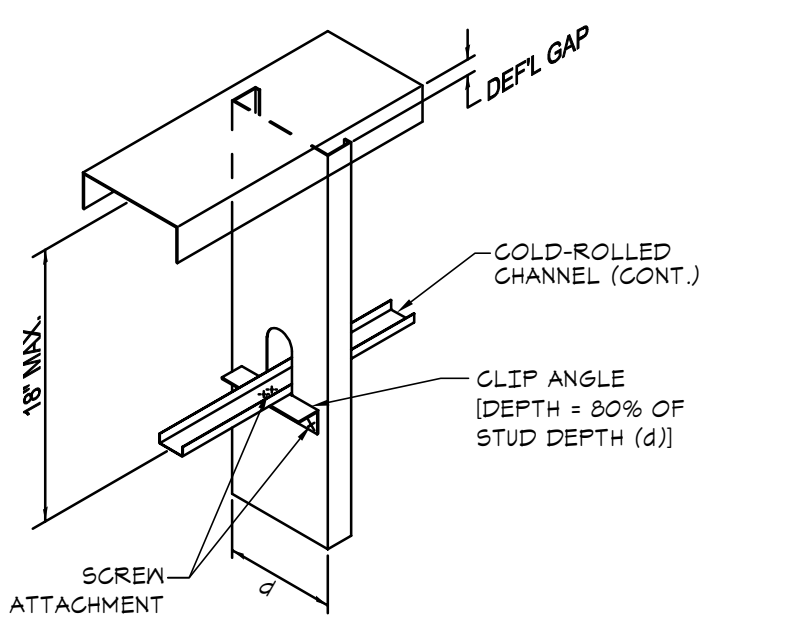
Typical Detail For Openings In Web Of Steel Beams



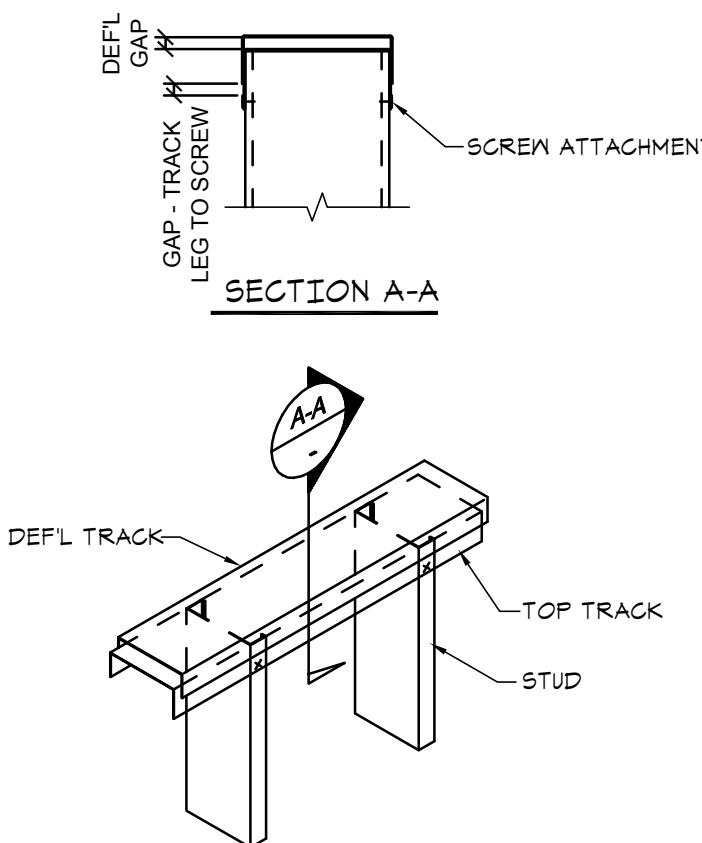
Typical Beam To HSS Column Connection



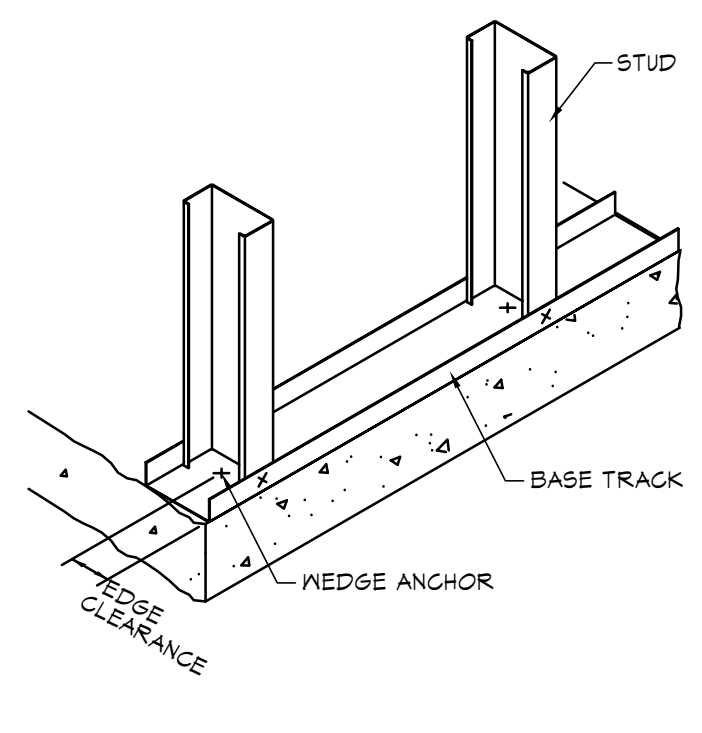
Typical Moment Connection Detail



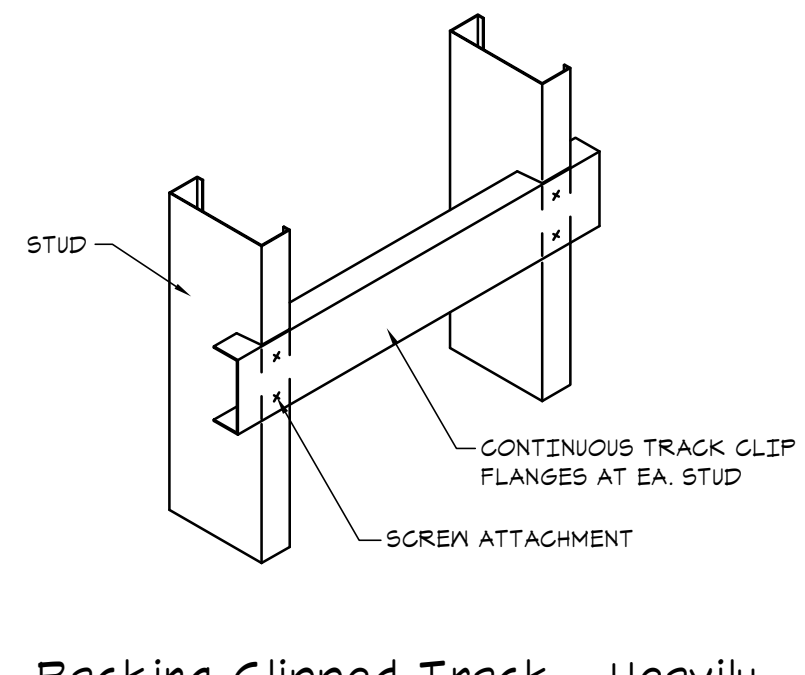
Deflection Track Assembly Single Track with Cold-rolled Channel



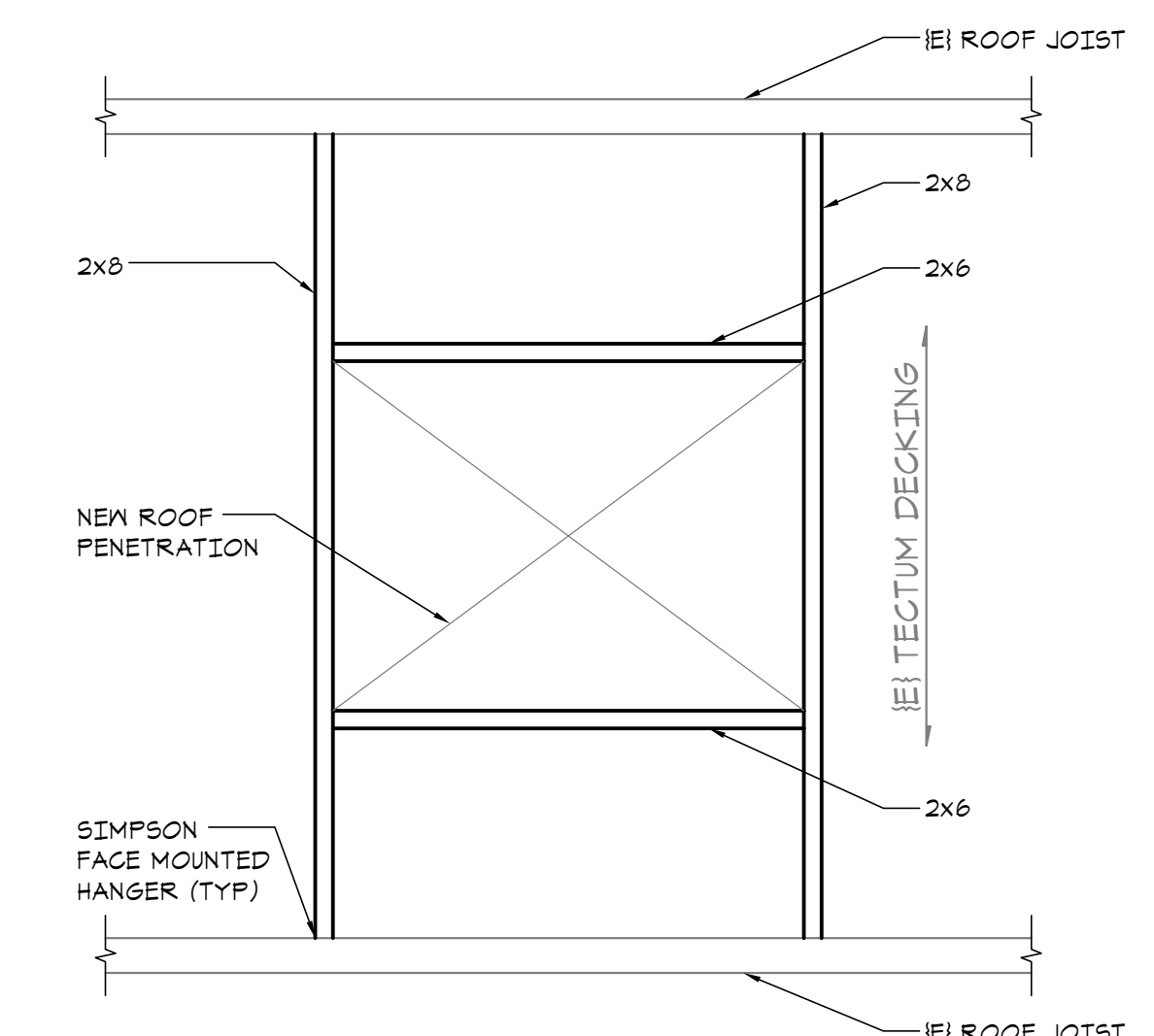
Deflection Track Assembly Double Deflection Track



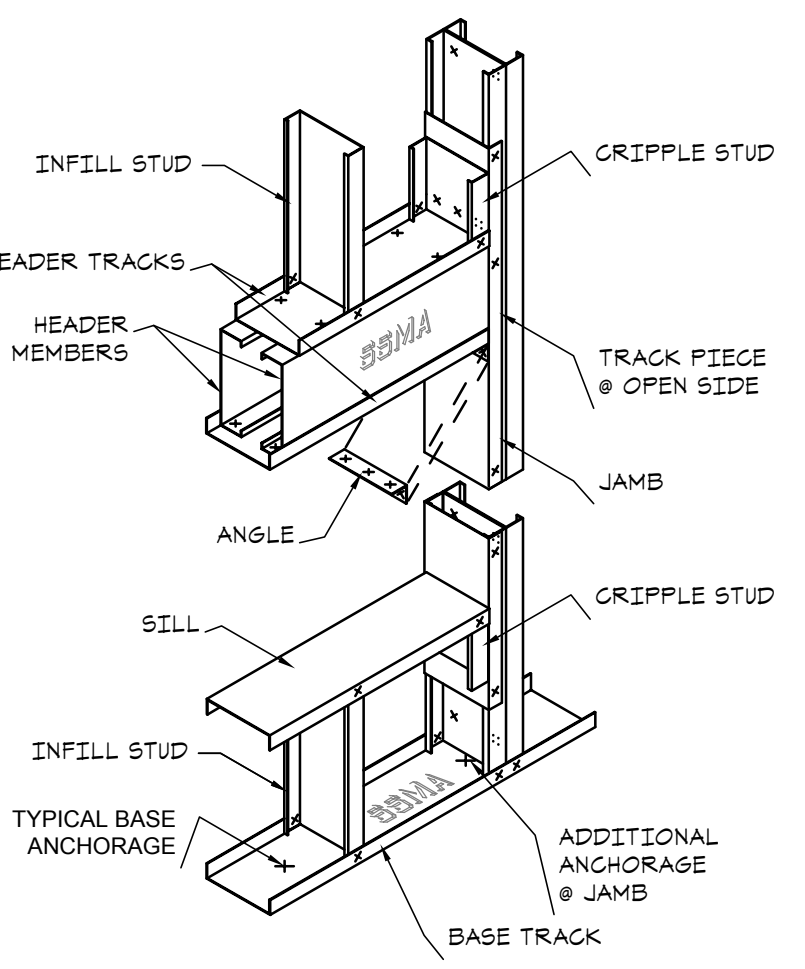
Bottom Track Anchorage Wedge Anchor



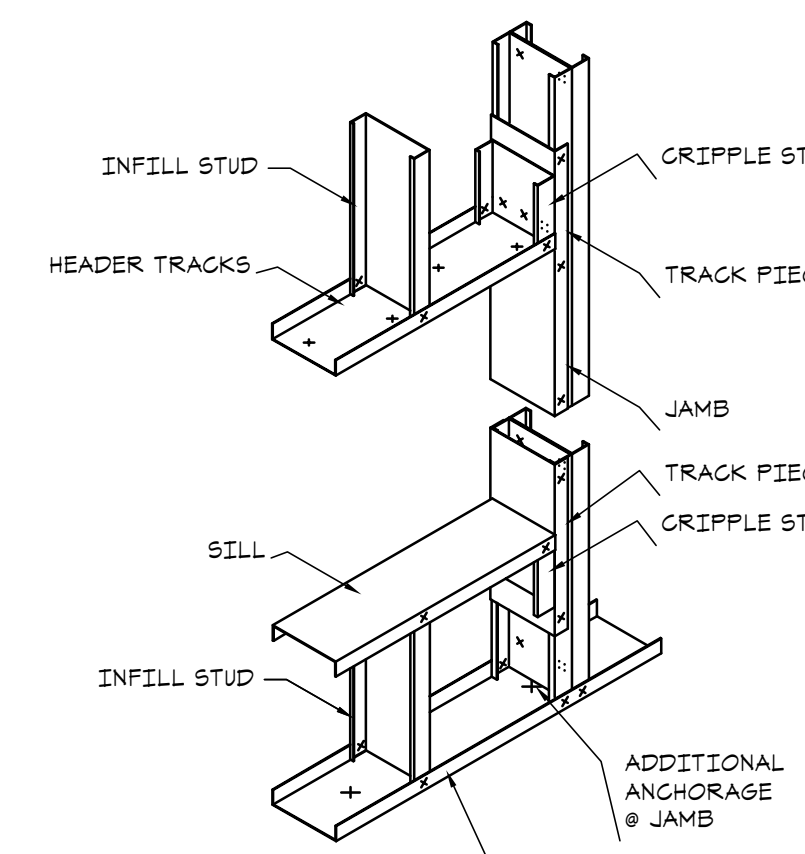
Backing Clipped Track - Heavily Loaded (Grab Bars, Handrails, Wall Hung Cabinets)



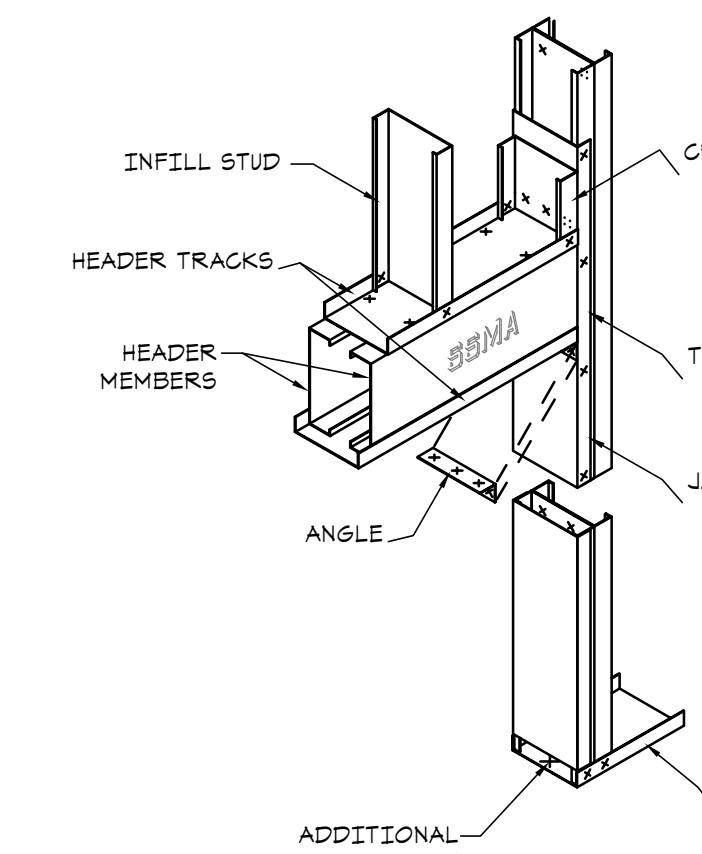
Typical Penetration Through Existing Roof Detail



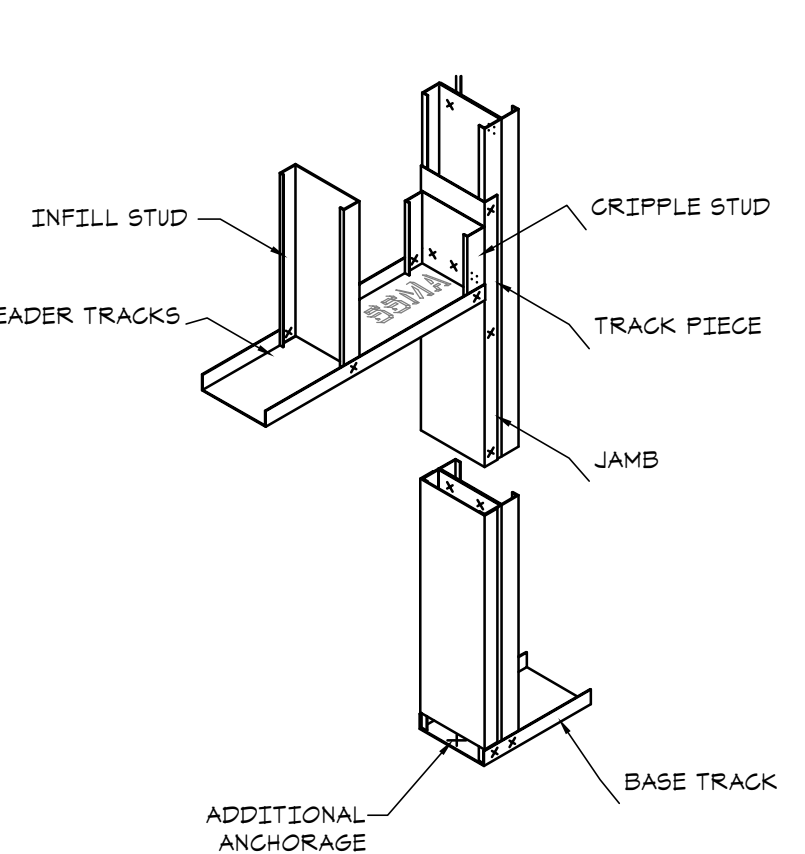
Window - Load Bearing Boxed Header Back to Back Jamb



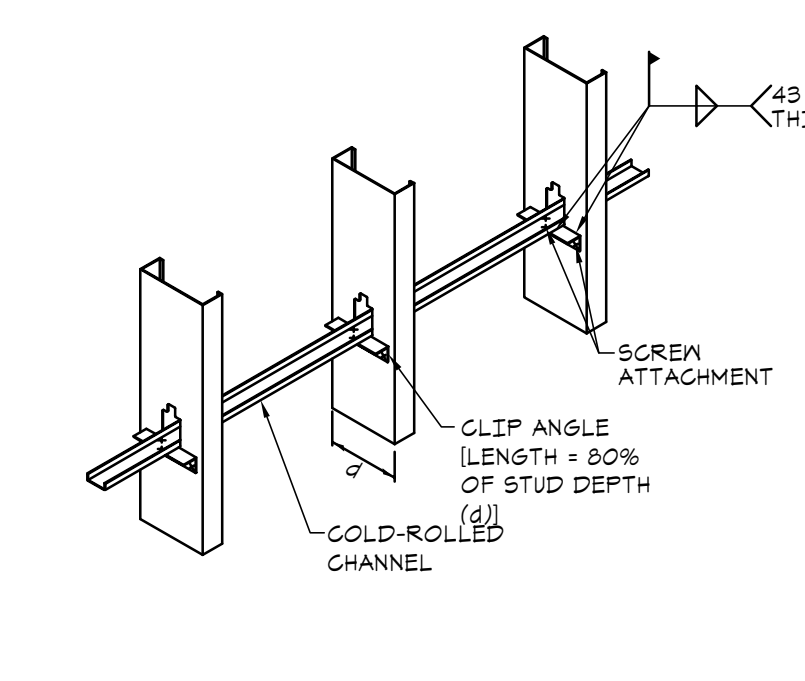
Window - Non-Load Bearing Single Track Header Back to Back Jamb



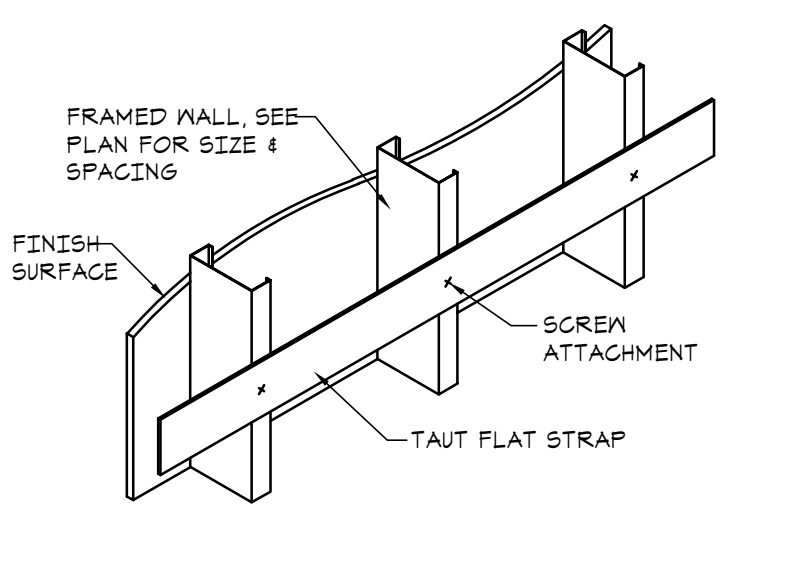
Door - Load Bearing Boxed Header Back to Back Jamb



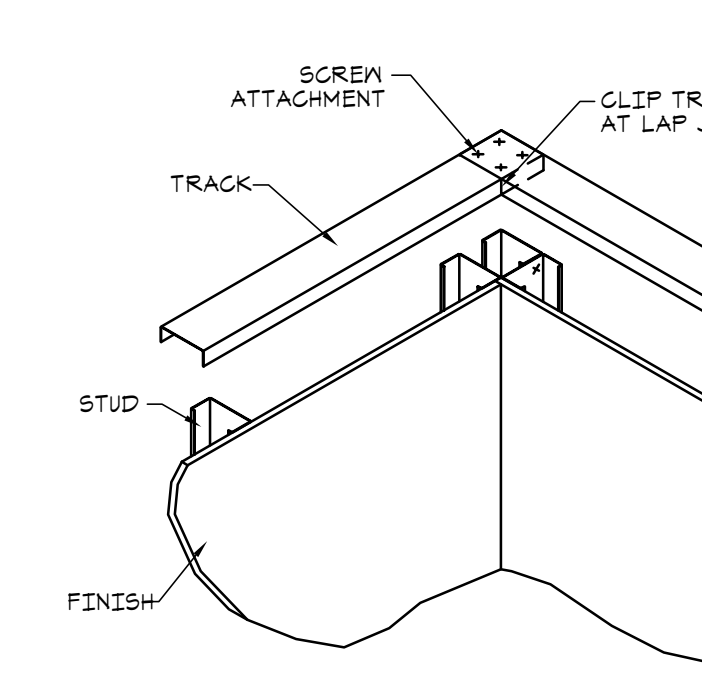
Door - Non-Load Bearing Single Track Header Back to Back Jamb



Bridging Cold-rolled Channel With Clip Angle



Backing Flat Strap - Lightly Loaded (Paper towel Dispensers, towel bars, toilet paper holders)



Wall Framing at Corner Track Lap Connection

METAL STUD WALL ASSEMBLY SPECS
BRICK TIES:
PROVIDE HOHMANN & BARNARD X-SEAL BRICK TIES W/ X-SEAL TAPE & VWT VEE WALL TIE (H.D. GALV.) @ 16" O/C E.M.
SCREWS:
ALL SCREWS TO BE POLYMER COATED SCREWS (2 PER TIE)

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FRAMING SECTIONS

SCALE	AS NOTED
DRAWN	BAS
CHECKED	BMS
APPROVED	RTM
DATE	SEPTEMBER 28, 2018

SYMBOL	DATE	DESCRIPTION
	11/2/2018	50% CD SUBMITTAL
	3/8/2019	FOR PERMITS
	6/21/2019	FOR BID AND PERMITS ONLY

