## CONCRETE NOTES:

- I. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO THE ACI-318 BUILDING CODE AND ACI-301 SPECIFICATIONS,
- 2. ALL FOOTING SHALL BEAR ON UNDISTURBED SOIL WITH A FOOTING DEPTH A MINIMUM OF 36" BELOW GRADE.
- 3. SEE CODE NOTES FOR SOIL BEARING PRESSURE,
- 4. FILL MATERIAL SHALL BE FREE OF DEBRIS, VEGETATION AND OTHER FOREIGN SUBSTANCES. 5, CONCRETE DESIGN BASED ON ACI 318, CONCRETE SHALL ATTAIN THE FOLLOWING MIN, COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.:  $f'c = 4,000 psi: \dots FOUNDATION WALLS$
- f'c = 3,500 psi: ..... FOOTINGS & INTERIOR SLABS ON GRADE f'c = 3,500 psi: ..... Garage & EXTERIOR SLABS ON GRADE
- fy = 60,000 psi
- 6. BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY ADEQUATE TEMPORARY BRACING OR INSTALL 1st FLOOR DECK,
- BASEMENT WALL DESIGN IS BASED ON 30 OR 45 PCF BACKFILL SOIL TYPE CLASSIFICATIONS: 30 PCF TYPE (GW, GP, SW, SP) • 45 PCF TYPE (GM, GC, SM, SM-SC, ML)
- IMPORTANT IF 60 PCF SOIL TYPE (SC, ML-CL OR CL) IS UTILIZED FOR BACKFILL, CONTACT FLUENT DESIGN GROUP FOR FURTHER EVALUATION OF FOUNDATION DESIGN, 8, ALL REINFORCEMENT SHALL BE ASTM AG15, GRADE 60, ALL WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM
- A185, 9, TYPICAL REINFORCEMENT DETAILS: PROVIDE 3" MIN, CLEAR COVER WHERE CAST AGAINST EARTH, 1/2" MIN, CLEAR COVER AGAINST FORMS, LAP ALL REBAR 48 BAR DIAMETERS MIN, (24" FOR #4 BARS) & BEND BARS AND LAP AT
- CORNERS, PROVIDE 6' HOOK INTO SUPPORTING FOOTINGS WHEN FOOTINGS INTERSECT. 10. ALL REINFORCING BARS SHALL BE SECURELY SUPPORTED AND WIRED IN PLACE PRIOR TO CONCRETE PLACEMENT. 11, PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BASEMENT FOUNDATION WALL WITH 2" CLEAR, REINFORCEMENT SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS,
- FOR OPENINGS UP TO 36", PROVIDE MINIMUM 10" CONCRETE DEPTH OVER OPENINGS OR (3)2x10 W/ (2)2x6 JACK STUDS, U.N.O. LARGER OPENINGS SHALL BE PER PLAN, 12. ADHESIVE ANCHORS SHALL BE HILTI HITHY 150 INJECTION ADHESIVE ANCHORS AND SHALL BE INSTALLED IN STRICT
- COMPLIANCE WITH THE MANUFACTURER'S RECOMMENDED INSTALLATION PROCEDURES, INSERTS OR RODS SHALL BE ASTM A-316 STAINLESS STEEL. 13, NON-SHRINK GROUT OR DRY PACK: A PREMIXED NONMETALLIC FORMULA HAVING THE FOLLOWING PROPERTIES:
- ON-SHRINK, NON-EXPANSIVE (ASTM C-827), 24 HOUR COMPRESSIVE STRENGTH OF 3,000 PSI, AND MINIMUM OF 5,000 PSI AT 30 DAYS, 14, FOUNDATION ELEVATIONS SHOWN ARE MINIMUM, EXTERIOR FOOTINGS SHALL REST AT LEAST 3'-O BELOW FINISHED
- GRADE AND ALL FOOTINGS SHALL REST AT LEAST 1'-6 BELOW EXISTING GRADE.

### MASONRY NOTES

1. MASONRY CONSTRUCTION SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTE BUILDING CODE FOR MASONRY, ACI-530 2. CONCRETE MASONRY UNITS SHALL BE ASTM C90, GRADE N, NORMAL WEIGHT, NON-MOISTURE CONTROLLED UNITS,

- UNLESS NOTED OTHERWISE, 3. MORTAR SHALL BE TYPE M OR 5 AND MORTAR JOINTS SHALL BE CONCAVE.
- 4. ALL MASONRY WALLS TO BE BUILT STAGGERED BOND WITH A FULL MORTAR BED.
- GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C467 AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH
- OF 2000 PSI. WHERE COARGE GROUT IS REQUIRED, 89 STONE AGGREGATE SHALL BE USED. 6, THE COMPRESSIVE STRENGTH OF THE MASONRY ASSEMBLAGE (F'm) SHALL BE 1500 PSI,
- 7. PROVIDE 9 GA TRUSS JOINT REINFORCEMENT EVERY OTHER COURSE, U.N.O.
- 8, PRECAST LINTELS SHALL BE 8" NOMINAL DEPTH WITH 1-#5 BOTTOM AND 1-#4 TOP FOR EACH 4" OF WALL THICKNESS,

# GENERAL NOTES

- THESE NOTES ARE PROVIDED FOR TYPICAL CONDITIONS, SEE PLANS AND DETAILS FOR SPECIFIC REQUIREMENTS IN OTHER AREAS,
- VERIFY ALL DIMENSIONS, ELEVATIONS, AND DETAIL OF EXISTING STRUCTURE WHERE THEY AFFECT THIS CONSTRUCTION.
- 3. NOTIFY ENGINEER IF THERE ARE ANY DEVIATIONS FROM THE CONTRACT DOCUMENTS.

4. OBTAIN PROPER APPROVAL FROM STRUCTURAL ENGINEER BEFORE CUTTING OPENINGS OR RECESS OR MAKING OTHER MODIFICATIONS TO EXISTING STRUC

5. VERIFY ALL DEPRESSIONS, DIMENSIONS, ELEVATIONS, OPENINGS, EQUIPMENT SUPPORTS, AND DETAILS AND COORDINATE BY REFERENCE TO ARCHITECTUR,

6. VERIFY SIZE AND LOCATION OF ALL OPENINGS WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS, STRUCTURAL DRAWINGS DO NOT NECESSA PLACE OPENINGS IN FLOOR AND ROOF NOT SHOWN ON STRUCTURAL DRAWINGS BETWEEN STRUCTURAL MEMBERS, NOTIFY STRUCTURAL ENGINEER BEFORE

8. OBTAIN PRIOR APPROVAL FROM STRUCTURAL ENGINEER BEFORE MAKING ANY OPENINGS THROUGH STRUCTURAL MEMBERS IF THE OPENINGS ARE NOT SI

9. THIS STRUCTURE IS A NON-SELF-SUPPORTING WOOD FRAME THAT REQUIRES INTERACTION WITH SLABS, FLOOR SHEATHING, AND WOOD SHEAR WALLS TO F BRACING UNTIL PERMANENT BRACING, FLOORS, AND WALL ARE IN PLACE.

10. PROVIDE TEMPORARY BRACING FOR ALL WALLS (CONCRETE, MASONRY, COLD FORMED STEEL, OR WOOD) UNTIL THEY ARE OF ADEQUATE DESIGN STRENG

THE DESIGN OF STUD WALL FRAMING IS FOR THE COMPLETED, SHEATHED CONDITIONS, INCLUDING PROPER FASTENING, THE CONTRACTOR IS RESPONSIBLE ELECTS TO PLACE WALL SHEATHING OR FLOOR TOPPING ON ANY LEVELS ABOVE STUD WALLS THAT DO NOT YET HAVE SHEATHING, ONE OF THE FOLLOWING PROVIDE BLOCKING MATCHING THE SIZE AND SPECIES OF STUDS AT MID-HEIGHT OF ALL LOAD BEARING STUD WALLS AT THE BOTTOM THREE LEVEL 11.1. LEVEL WOOD STRICTURES, EXCEPT THAT WALLS WITH DOUBLE OR TRIPLED SUDS FASTENED WITH 10d NAILS AT 8" O.C. STAGGERED DO NOT REQU

ELIMINATE BY THE ABOVE NAILING PATTERN 11.2. RETAIN A SPECIALITY STRUCTURAL ENGINEER TO EVALUATE THE ADEQUACY OF TEMPORARY UNBRACED STUD WALL CALCULATIONS, DESIGN RELATED AND DRAWINGS FOR REVIEW BY THE STRUCTURAL ENGINEER OF RECORD AND ARCHITECT.

- THE FOLLOWING MINIMUM GRADES:
- STRUCTURAL STEEL NOTES: STRUCTURAL STEEL WIDE FLANGE SECTIONS SHALL CONFORM TO ASTM-A392, UNLESS NOTED OTHERWISE, ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM-A36, UNLESS NOTED OTHERWISE, SPECIAL CARE SHOULD BE TAKEN TO PROCURE PLATE AND ROLLED SECTIONS FREE FROM INTERNAL LAMINATIONS, ALL 4X12 OR SMALLER FRAMING MEMBERS - #2 2. ALL PIPE SHALL CONFORM TO ASTM A-53, TYPE "S" GRADE B. ALL FRAMING MEMBERS LARGER THAN 4X12 - #1 ALL POST LARGER THAN 4X4 - #1 ALL 2X STUDS WITH UNBRACED LENGTH LESS THAN 10' - STUD OR STANDARD U.N.O. 3, ALL HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A-500 GR, B, ALL 2X STUDS WITH UNBRACED LENGTH GREATER THAN 10' - \*2ALL TOP AND SILL PLATES - STANDARD 4, WELD FILLER WIRE SHALL BE E70XX, LOW HYDROGEN FOR ALL CONNECTIONS, ALTERNATE WIRE MUST BE APPROVED ALL TRUGG MEMBERG - PER MANUFACTURERS SPECIFICATIONS BY ENGINEER, 5. ALL BOLTS SHALL CONFORM TO ASTM A-325, UNLESS NOTED OTHERWISE. BOLTS MUST HAVE ADEQUATE LENGTH FOR 8d NAILS:  $2^{1}_{2}$  LONG, O.131<sup>"</sup> SHANK, FULL HEAD WASHERS, NUT, AND MATERIAL BEING CONNECTED. 10d NAILS: 3" LONG, 0.131"Ø SHANK, FULL HEAD 6. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED PER THE REQUIREMENTS OF THE AISC 12d NAILS:  $3_4^{\perp}$  LONG, 0.131"  $\phi$  SHANK, FULL HEAD SPECIFICATIONS AND CODE OF STANDARD PRACTICE AS AMENDED TO DATE. 16d NAILS:  $3^{|||}_2$  LONG, 0.131<sup>||</sup>  $\phi$  SHANK, FULL HEAD 7. PRIOR TO FABRICATION AND ERECTION, STEEL SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL, 8, ALL MEMBERS WHEN FINISHED SHALL BE FREE OF TWISTS, BENDS, AND OPEN JOINTS BETWEEN COMPONENT PARTS, MEMBERS SHALL BE STRAIGHTENED IN THE SHOP IN A MANNER THAT WILL NOT INJURE THEM, BEFORE BEING WORKED. 9, MEMBER SPLICES ARE PERMITTED ONLY WHERE INDICATED ON THE DRAWINGS, IO, ALL MEMBER CONNECTIONS ARE DETAILED ON THE DRAWINGS, WHERE NO DETAIL IS PROVIDED THE CONTRACTOR MUST DETAIL THE CONNECTION, 11. ALL BOLT HOLES SHALL BE DRILLED OR REAMED, FLAME CUT HOLES ARE UNACCEPTABLE. LVL; Fb=2,600 PSI, Fv=220 PSI, E=1,800,000 PSI 12. MANUAL OXYGEN CUTTING SHALL BE DONE ONLY WITH A MECHANICALLY GUIDED TORCH, ALTERNATIVELY AN UNGUIDED PSL: Fb=2,900 PSI, Fv=290 PSI, E=2,000,000 PSI TORCH MAY BE USED PROVIDED THE CUT IS NOT WITHIN 1/2" OF THE FINAL DIMENSION AND FINAL REMOVAL IS THROUGH CHIPPING OR GRINDING, TO PROVIDE A SURFACE EQUAL TO A CUT EDGE. CLEAN ALL CUT EDGES BY GRINDING TO REMOVE ALL BURRS, GOUGES, CUTS, AND JOGS. 13. FOR STRUCTURAL-STEEL CONNECTIONS INDICATED TO COMPLY WITH DESIGN LOADS, INCLUDE STRUCTURAL ANALYSIS DATA PREPARED BY THE QUALIFIED PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION, PREFABRICATED TIMBER TRUSSES SUBMIT SHOP DRAWINGS, INCLUDING TRUSS LAYOUT, AND CALCULATIONS OR STRESS DIAGRAMS FOR EACH TYPE OF TRUSS TO THE ARCHITECT FOR REVIEW, ALLOW TEN WORKING DAYS FOR THE REVIEW, 2. SHOP DRAWINGS AND CALCULATIONS TO BE REVIEWED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O. STATE OF PENNSYLVANIA.
- ALL DIMENSIONAL LUMBER SHALL BE SPF, HEM FIR, OR BETTER WITH MOISTURE CONTENT <19 U.N.O. AND SHALL HAVE EQUAL, ALL WALL FRAMING TO THE FOUNDATION, ADDITIONALLY VERTICAL GRAIN BLOCKING IS TO BE PROVIDED THROUGH THE FLOOR FRAMING, PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FOUNDATION/BEARING, BLOCKING TO MATCH POST ABOVE , ALL LAMINATED VENEER LUMBER (LVL) AND PARALLEL STRAND LUMBER (PSL) SPECIFIED SHALL HAVE THE FOLLOWING MINIMUM DESIGN STRENGTHS: ALL MULTIPLE LVL MEMBERS ARE TO BE ATTACHED WITH (3) ROWS OF 16d COMMON NAILS @ 12" O.C. FOR A (3) PLY MEMBER NAILING IS TO BE FROM EACH SIDE, ALL MEMBERS WITHIN 6" OF THE GROUND OR IN DIRECT CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESERVATIVE TREATED, ), ALL MULTILAYER JOIST OR RAFTERS SHALL BE ATTACHED WITH (2) ROWS OF 16d COMMON NAILS @ 12" O.C., WITH AN  $\,$  1. EDGE DISTANCE OF 2". APPLY NAILING FROM BOTH FACES @ 3-PLY OR GREATER. ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STUD, MINIMUM:
- WOOD FRAMING NOTES: 2. ALL SPECIFIED NAILS SHALL HAVE THE FOLLOWING MINIMUM DIMENSIONS, U.NO.: 3, ALL PLYWOOD SHALL CONFORM TO APA PS 1, ALL SHEAR PLYWOOD SHALL BE C-D, C-C, 303 (T1-11), OR APPROVED 4. WHERE MULTIPLE TRIMMERS, STUDS, OR POSTS ARE SPECIFIED, THOSE TRIMMERS OR STUDS ARE TO BE STACKED IN 9, MEMBERS OF LARGER DEPTH OR GREATER STRENGTH MAY BE SUBSTITUTED FOR THOSE SPECIFIED IN HERE WITHIN,

## STRUCTURAL NOTES

- 3. ALL DESIGN IS TO CONFORM TO THE REQUIREMENTS OF THE LOCAL BUILDING CODE, NFPA, AND TRUSS PLATE INSTITUTE (TPI) SPECIFICATIONS,
- 4. PROVIDE CONTINUOUS BRIDGING AS REQUIRED BY TPI RECOMMENDATIONS. 5. TEMPORARY BRACING AND/OR SHORING OF THE LIGHT GAUGE METAL TRUSSES IS THE CONTRACTOR'S RESPONSIBILITY,
- MECHANICAL NOTES:

WALL BRACING & SHEAR WALL SHEATHING SPECIFICATIONS

- 1. ALL EQUIPMENT AND MAJOR PIPE RUNS SHALL BE SUPPORTED FROM AT LEAST TWO ADJACENT ROOF JOIGTS, IDEALLY HANG ALL EQUIPMENT, PIPING AND PANELS FROM A TOP CHORD PANEL POINT, PLACE ONE SUPPORT OR CURB AT A CHORD POINT AS SHOWN.
- 2. REINFORCE JOIST TOP CHORD WHERE HANGER IS CENTERED MORE THAN 6" FROM THE CENTERLINE OF THE JOIST PANEL POINT (D>6") AND THE HANGER LOAD EXCEEDS 250 POUNDS,
- , REINFORCE JOIST BOTTOM CHORD WHERE HANGER IS CENTERED MORE THAN 6" FROM THE CENTERLINE OF THE JOIST PANEL POINT (D>6") AND THE HANGER LOAD EXCEEDS 150 POUNDS,
- 4. UNLESS OTHERWISE NOTED, HANGER LOADS LESS THAN 250 POUNDS ATTACHED TO THE TOP CHORD DO NOT REQUIRE JOIST REINFORCEMENT.
- 5. UNLESS OTHERWISE NOTED, HANGER LOADS LESS THAN 150 POUNDS ATTACHED TO THE BOTTOM CHORD DO NOT REQUIRE JOIST REINFORCEMENT.
- 6. L2"x2" REINFORCEMENT WHERE REQUIRED SHALL EXTEND FROM HANGER LOCATION ON ONE CHORD TO THE NEAREST PANEL POINT ON THE OPPOSITE CHORD.
- 7. REINFORCEMENT REQUIRED FOR HUNG EQUIPMENT SHALL BE PROVIDED BY THE MECHANICAL CONTRACTOR. ALL OTHER REINFORCEMENT SHALL BE PROVIDED BY THE STRUCTURAL STEEL CONTRACTOR,
- 1. ALL EXTERIOR WALL SHEATHING SHALL BE  $\frac{7}{16}$  OSB OR  $\frac{15}{32}$  PLYWOOD: FASTEN SHEATHING W/ 10d NAILS @ 6" O.C. AT EDGES & @ 12" O.C. IN THE PANEL FIELD, TYP, U.N.O.

	CODE N	IOTES
	CODE REFERENCE AND DESIGN CRITERIA:	
	THE FOLLOWING CODES AND REFERENCES WER	E USED FOR THIS PROJECT:
RICTURE NOT SHOWN ON THE STRUCTURAL DRAWINGS,	<ol> <li>2015 INTERNATIONAL BUILDING CODE</li> <li>AMERICAN CONCRETE INSTITUTE (ACI) 318- STRUCTURAL CONCRETE</li> <li>AMERICAN INSTITUTE OF STEEL CONSTRUCT FOURTEENTH EDITION</li> <li>ASCE STANDARD ASCE/SEI 7-10</li> </ol>	
RAL, MECHANICAL, AND ELECTRICAL DRAWINGS.	DESIGN CRITERIA	
ARILY SHOW ALL OPENINGS,	PUBLIC AREA LIVE LOAD PUBLIC AREA DEAD LOAD	100 PSF 20 PSF
E OPENINGS LARGER THAN 12" IN ANY DIMENSION ARE ADDED.	STAIRS LIVE LOAD MECHANICAL ATTIC SPACE LIVE LOAD	100 PSF 100 PSF
SHOWN ON THE STRUCTURAL DRAWINGS,	ELEVATED PATIO LIVE LOAD ELEVATED PATIO DEAD LOAD	100 PSF 25 PSF
PROVIDE THE REQUIRED LATERAL STABILITY, PROVIDE REQUIRED TEMPORARY	ROOF LIVE LOAD STEEL BEAM DESIGN METHOD: STEEL COLUMN DESIGN METHOD:	20 PSF ALLOWABLE STRENGTH DESIGN (ASD) ALLOWABLE STRENGTH DESIGN (ASD)
IGTH AND ARE PROPERLY ANCHORED IN FINAL	FOUNDATION DESIGN METHOD:	LOAD RESISTANCE FACTOR DESIGN (LRFD)
E FOR ALL MEANS AND METHODS OF TEMPORARY BRACING, IF THE CONTRACT NG ARE REQUIRED PRIOR TO SUCH PLACEMENT:	ALLOWABLE SOIL BEARING PRESSURE	GEOTECHNICAL REPORTED 3,000 PSI
_S IN FIVE LEVEL WOOD STRICTURES AND AL THE BOTTOM TWO LEVELS IN FOUR	WIND LOADS PER IBC 2015 [ASCE 7-10]	
UIRE BLOCKING, BLOCKING REQUIRED BY THE SHEAR WALL SCHEDULE IS NOT	115 MPH, EXPOSURE (3 SEC GUST) IMPORTANCE FACTOR INTERNAL PRESSURE COEFFICIENT	B 1.00 ±0.18
D BRACING FOR THE CONDITIONS, AND SUBMIT SIGNED/VERIFIED CALCULATIONS	SNOW LOADS PER IBC-2015 [ASCE 7-10]	
	GROUND SNOW LOAD PG FLAT ROOF SNOW LOAD PF IMPORTANCE FACTOR	25 PSF 20 PSF 1.00
	LIMIT DEFLECTIONS TO THE FOLLOWING:	
	ROOF L/180 TOTAL LOAD L/240 LIVE LOADS	
	FLOORS L/360 TOTAL LOAD L/480 LIVE LOADS	
	STEEL BEAMS (FLOOR) L/360 TOTAL LOAD L/480 LIVE LOADS	
	STEEL BEAMS (ELEVATED PATIO) L/480 TOTAL LOAD L/600 LIVE LOADS	
	SEISMIC LOADS PER IBC-2015 [ASCE 7-10] SEISMIC IMPORTANCE FACTOR OCCUPANCY CATEGORY S <sub>5</sub> S <sub>1</sub> SITE CLASS S <sub>D5</sub> S <sub>D1</sub>	1.00 II 0.207 0.061 D 0.221 0.098
	SEISMIC DESIGN CATEGORY SEISMIC FORCE RESISTING SYSTEM RESPONSE COEFFICIENT Co RESPONSE MODIFICATION FACTOR ANALYSIS PROCEDURE	B LIGHT-FRAME WALLS RATED FOR SHEAR RESISTANCE 0,04 6 <sup>1</sup> / <sub>2</sub> EQ, LATERAL FORCE DESIGN PROC,

6	<b>S</b>		7		8
IE OUNTY,	PA				<b>FLUENT</b> DESIGN G R O U P, L L C Engineering Design 2588 W. MAPLE AVE. LANGHORNE, PA 19053 215.372.0333 FLUENTDESIGNGROUP.NET
		STRU	JCTURAL SYMBC	DLS	
	DIMENSIONAL LUMBER	BPI	BEARING PLATE MARK - SEE SCHEDULE		- MATCH LINE
	WOOD BLOCKING	CI	COLUMN MARK - SEE SCHEDULE	~~~~~	- BREAK LINE
	FINISH WOOD	EPI	EMBED PLATE MARK - SEE SCHEDULE	-	- DIRECTION OF GLOPE
	STEEL OR METAL	FI	FOOTING MARK - SEE SCHEDULE		- STEEL BEAM SPLICE
	CONCRETE BLOCK - HOLLOW OR GOLID	$\langle 1 \rangle$	KEY NOTES - SPECIFIC NOTES KEYED TO AN ELEMENT OR		STEEL BEAM CONTINUOUS OVER SUPPORT
	FACE OR COMMON BRICK		AREA, SEE SCHEDULE LINTEL MARK - SEE SCHEDULE		MOMENT CONNECTION FOR STEEL BEAMS
	RIGID INSULATION	IG	PIER MARK - SEE SCHEDULE		SPAN OF STRUCTURAL ELEMENT
	BATT OR BLANKET INSULATION		REVISION MARK - INDICATES A CHANGE AFTER CONTRACT		REINFORCING STEEL SPAN
	PLYWOOD	SUI	DOCUMENT ISSUE SHEAR WALL MARK - SEE SCHEDULE	+	H DIMENSION LINES
	EARTH, FILL OR BACKFILL	_ wi	WALL REINFORCING MARK -		MATERIAL LIMIT LINES
	SAND OR GROUT		SEE SCHEDULE		MATERIAL NOT INCLUDED IN LIMIT LINES
	CONCRETE		ELEVATION CHANGE	G	FOOTING STEP

GRID BUBBLE

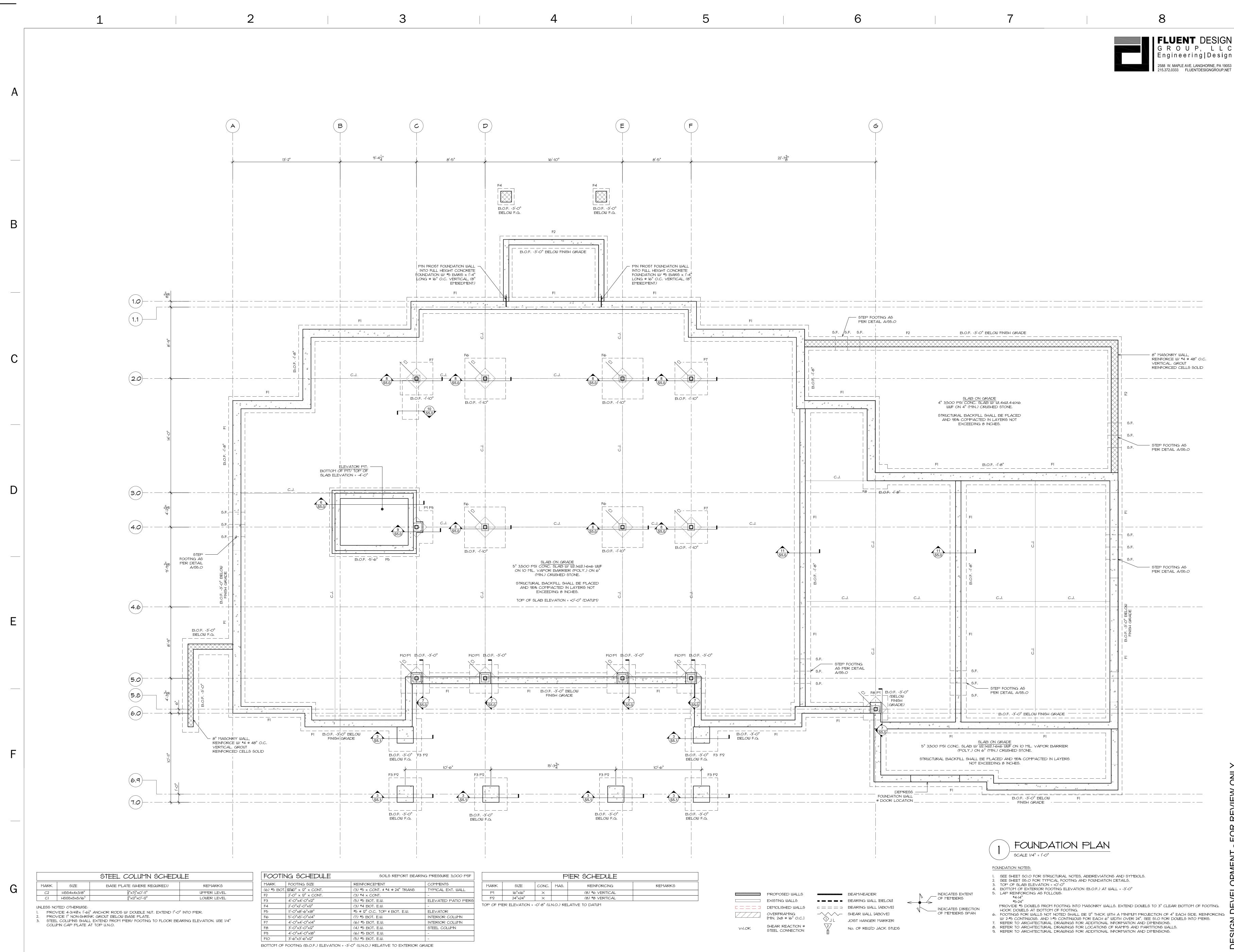
		STRUCT	JRAL ABBREVIAT	TIONS	
ADD'L	Additional	FF	Far Face	NTG	Not To Scale
a	At	FV	Field Verify	NO or #	Number
	And	FLG	Flange	<i>oc</i>	On Center
ANCH	Anchor	FLR	Floor	OPNG or OPG	Opening
AB	Anchor Bolt	FT FT	Foot	OPP	Opposite
	Angle	FTG	Footing		Outside Diameter
	Alternate	FDN	Foundation		Outside Face
ARCH	Architect or Architectural	GALV	Galvanized		Per
BSMT	Basement	GALY		PCF	Pounds per Cubic Foot
BM	Beam	GC	Gauge General Contractor	PLK	Plank
BRG		GLOR	Glue Laminated Wood	PL	Plate
BNT	Bearing	GLU-LAM		PT	
	Bent		H Shaped Column		Post Tension(ing)
BTWN or BET	Between		High Point		Pound Down Common Food
BLK	Block	HS	Headed Stud(s)	PSF	Pound per Square Foot
BB	Bond Beam / Lintel Beam	HSS	Hollow Structural Shape	PSI	Pound per Square Inch
BOT or BTM	Bottom	HK	Hook	PSL	Parallel Strand Lumber
Brkt	Bracket	HORIZ	Horizontal	PC	Precast
Bldg	Building	IN	Inch	RFG	Reinforcing
Cant	Cantilever	INFO	Information	REINF	Reinforcing or Reinforce
CLG	Ceiling	ID	Inside Diameter	REBAR	Reinforcing Bar
CTR	Center	IF	Inside Face	REQ'D	Required
CTR'D	Centered	INSUL	Insulation	REV	Reverse
CL	Center Line	INT	Interior	RD	Roof Drain
С	Channel	JT	Joint	SCHED or SCH	Schedule
CLR	Clear	JST(s)	Joist(s)	SEC	Section
COL	Column	K	Kips		Shear
COMP	Composite	KIP	1 Kip = 1,000 lbs	SHT	Sheet
CONC	Concrete	K/FT	Kips per Foot	SIM	Similar
CMU	Concrete Masonry Unit	KSF	Kips per Square Foot	SOG	Slab on Grade
COND	Condition	KSI	Kips per Square Inch	S	South
CONN	Connection	LΨ	Light Weight	SPS	Spaces
CONST	Construction	LTL	Líntel	SPEC	Specification
CJ	Control Joint		Live Load	SQ	Square
CONT	Continuous		Long Leg Horizontal	STAGG	Stagger
CONTR	Contractor		Long Leg Vertical	STD	Standard
DBA	Deformed Bar Anchor		Low Point	STL	Steel
DL	Dead Load	LSH	Long Side Horizontal	STRUCT or STR	Structure
DEFL	Deflection	LSL	Laminated Strand Lumber	SUPP	Support
DTL or DET	Detail	LSV	Long Side Vertical		Temporary or Temperature
	Diameter		Laminated Veneer Lumber		Through
DIAG	Diagonal	MFG	Manufacturer / Manufacturing	T#B	Top and Bottom
DIM	Dimension	MAS	Masonry	TOB	Top of Beam
DWL(s)	Dowel(s)	MO		TOF	Top of Footing
DWG(s) / DRWG		MAT	Masonry Opening Material	TOS	
	Drawing(s)	MAX		TYP	Top of Slab
EA EF	Each Each		Maximum Maalaatiaal		Typical
	Each Face	MECH	Mechanical Manula an	UN	Unless Noted
EW	Each Way	MBR	Member		Unless Noted Otherwise
E-W	East - West	MEZZ	Mezzanine	VER	Verify
E	East	MIN	Minimum Minis allen a suu		Vertical
Fe	Effective Force (P.T.)	MISC	Miscellaneous	VIF	Verify in Field
ELEC	Electrical	MC	Miscellaneous Channel		Welded Wire Fabric
EL	Elevation	M	Moment	UN INC	West or Wide Flange
ELEV	Elevator	MPH	Miles per Hour	WND	Window
EQ	Equal	NF	Near Face	W/	With
EXP	Expansion	N	North	W/O	Without
EJ or EXP JT	Expansion Joint	N-S	North - South	WD	Wood
EXT	Extend or Exterior	NOM	Nominal		

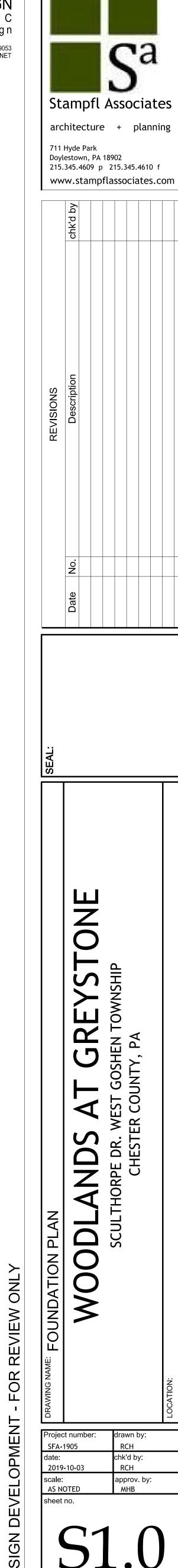
# SHEET LIST

SO.1	COVER SHEET
S1.0	FOUNDATION PLAN
SI.1	FIRST FLOOR FRAMING PLAN
S1.2	CEILING FRAMING PLAN
SI.3	ROOF FRAMING PLAN
S5.0	STRUCTURAL FOUNDATION DETAILS
S5.1	STRUCTURAL FRAMING DETAILS
S5.2	TRUSS FRAMING DETAILS & PROFILES

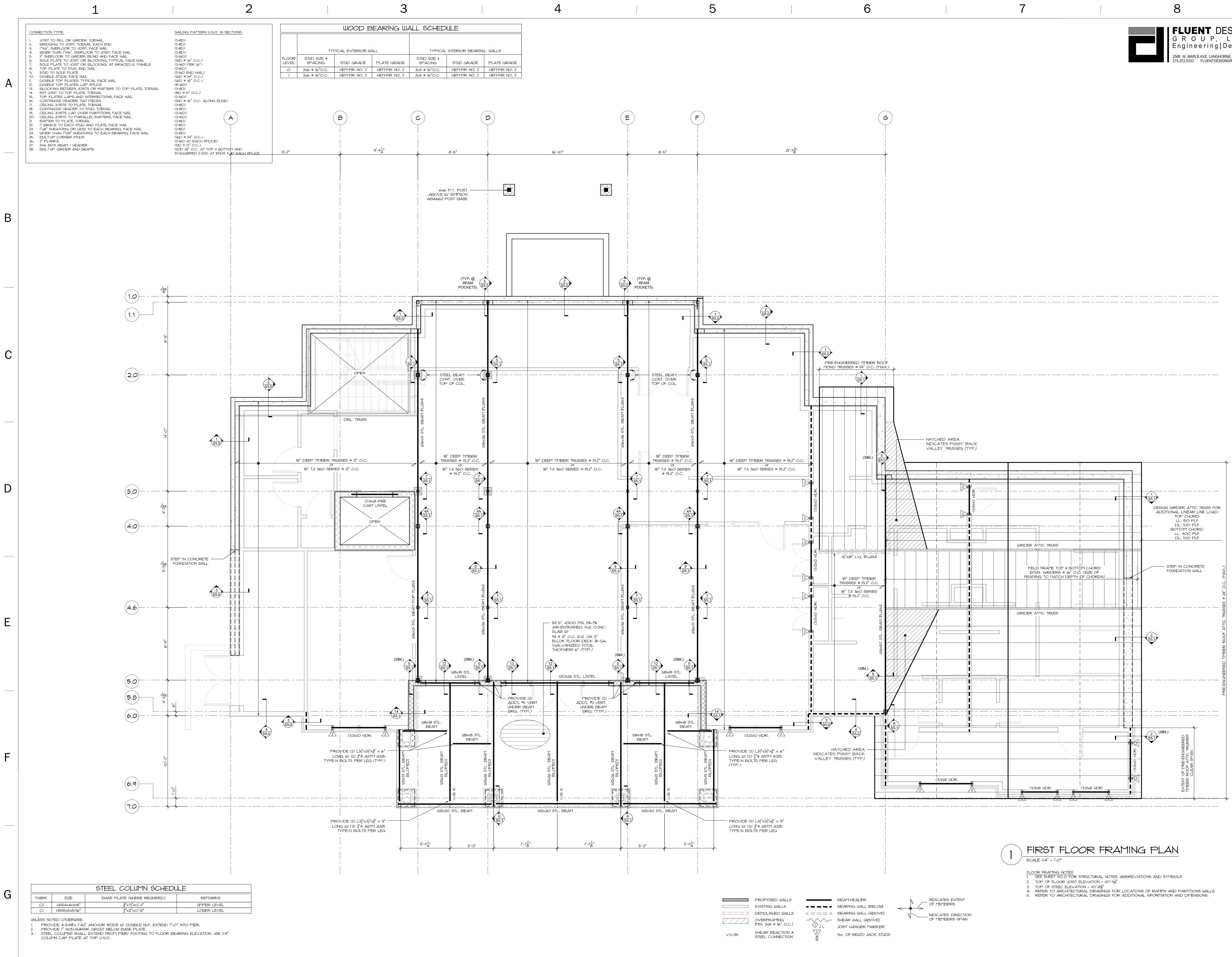
STONE

DESIGN DEVELOPMENT	IT - FOR REVIEW ONLY			Đ	JS		GN LC ign A 19053 UP.NET
SFA-7 date:	DRAWING NAME: COVER SHEET	SEAL:		REVISIONS		arc 711 Doyl 215.	
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drawn RCH chk'd RCH appro MH	SCULTHORPE DR. WEST GOSHEN TOWNSHIP					e + 18902 215.34	C
H by: H ov. by:	CHESTER COUNTY, PA					planniı 5.4610 f	a
	LOCATION:					ng	
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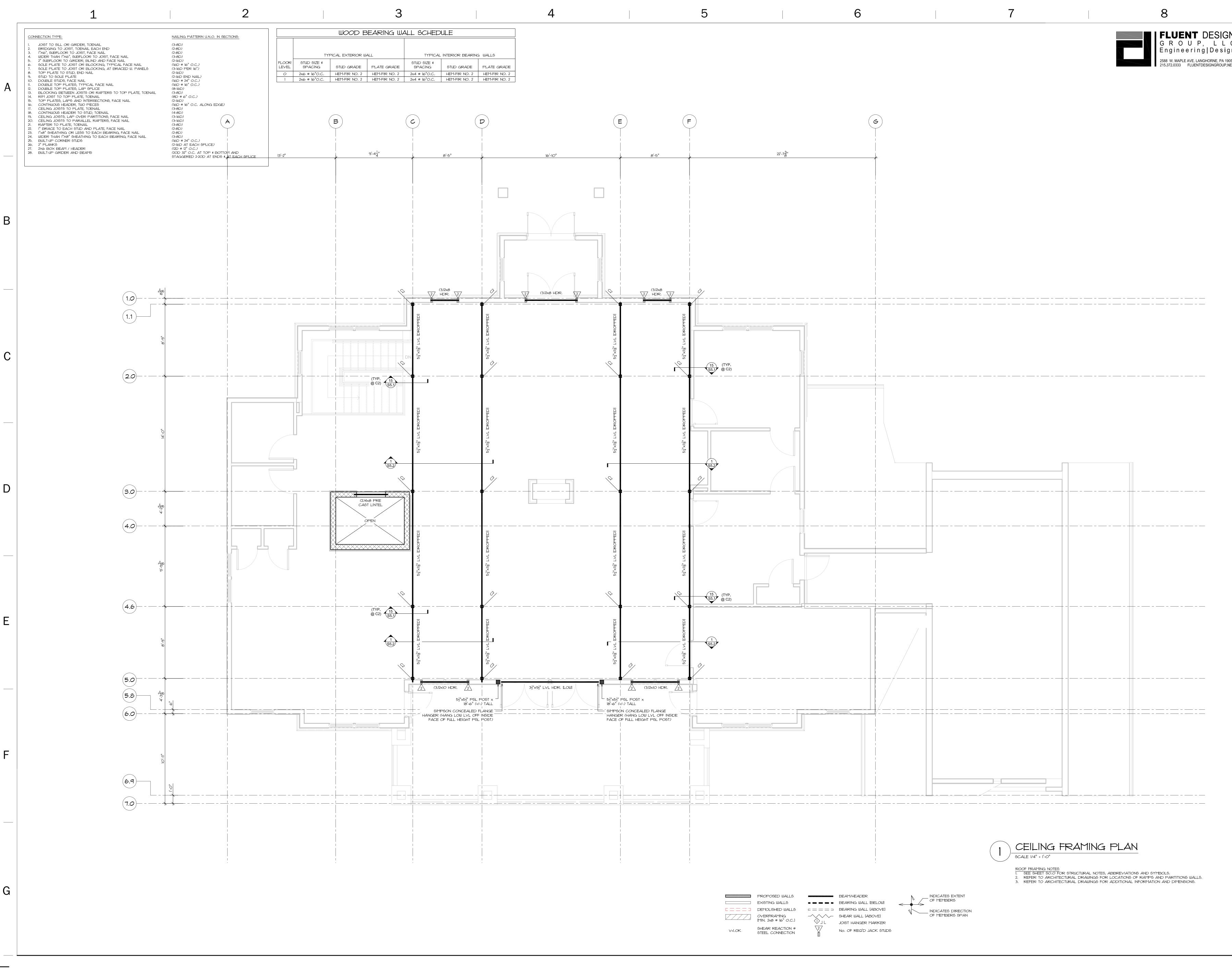


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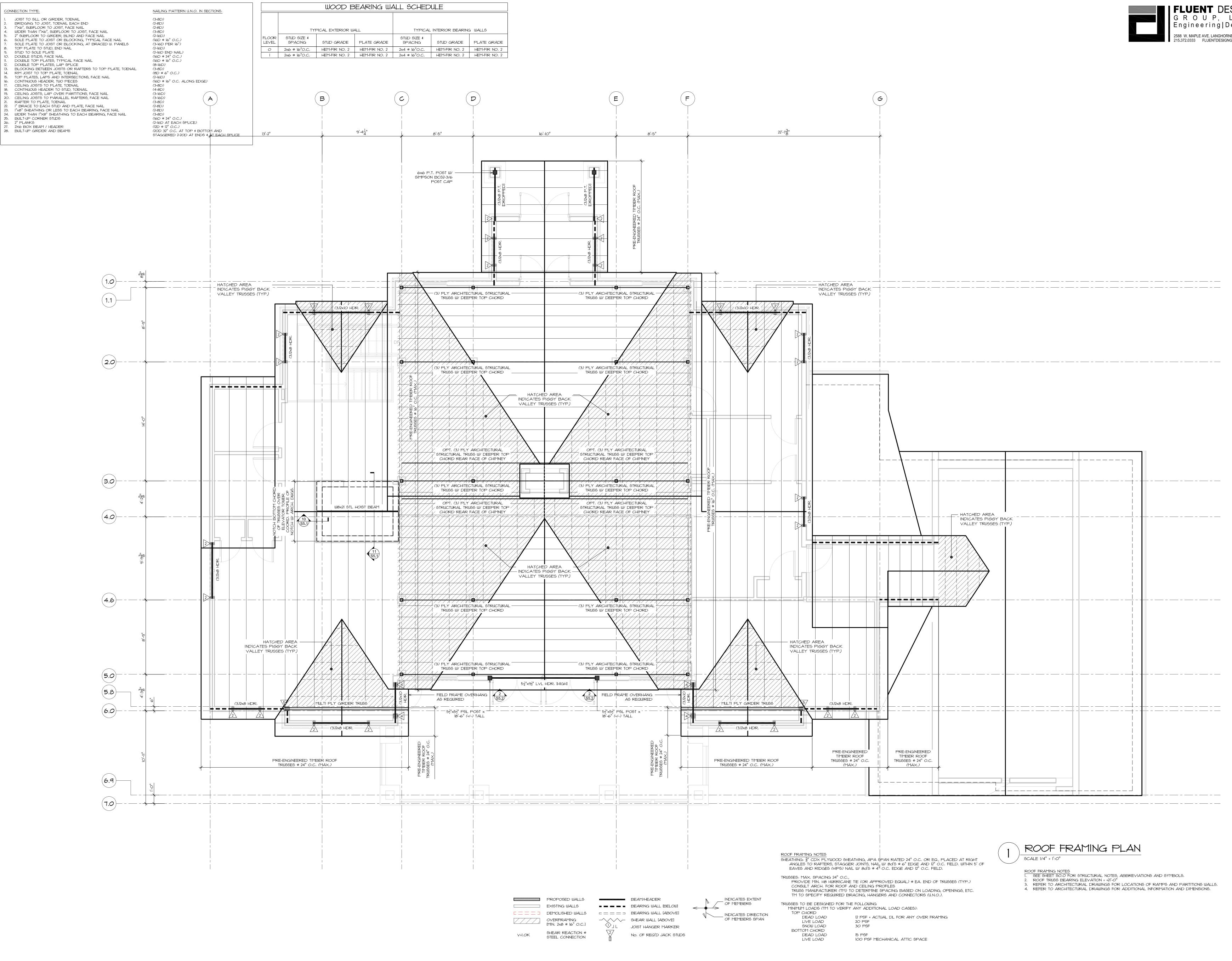
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-10-03 DTED			Date No.	Description	chk'd by	hitect Hyde Pa estown, 345.460 w.sta	amp	
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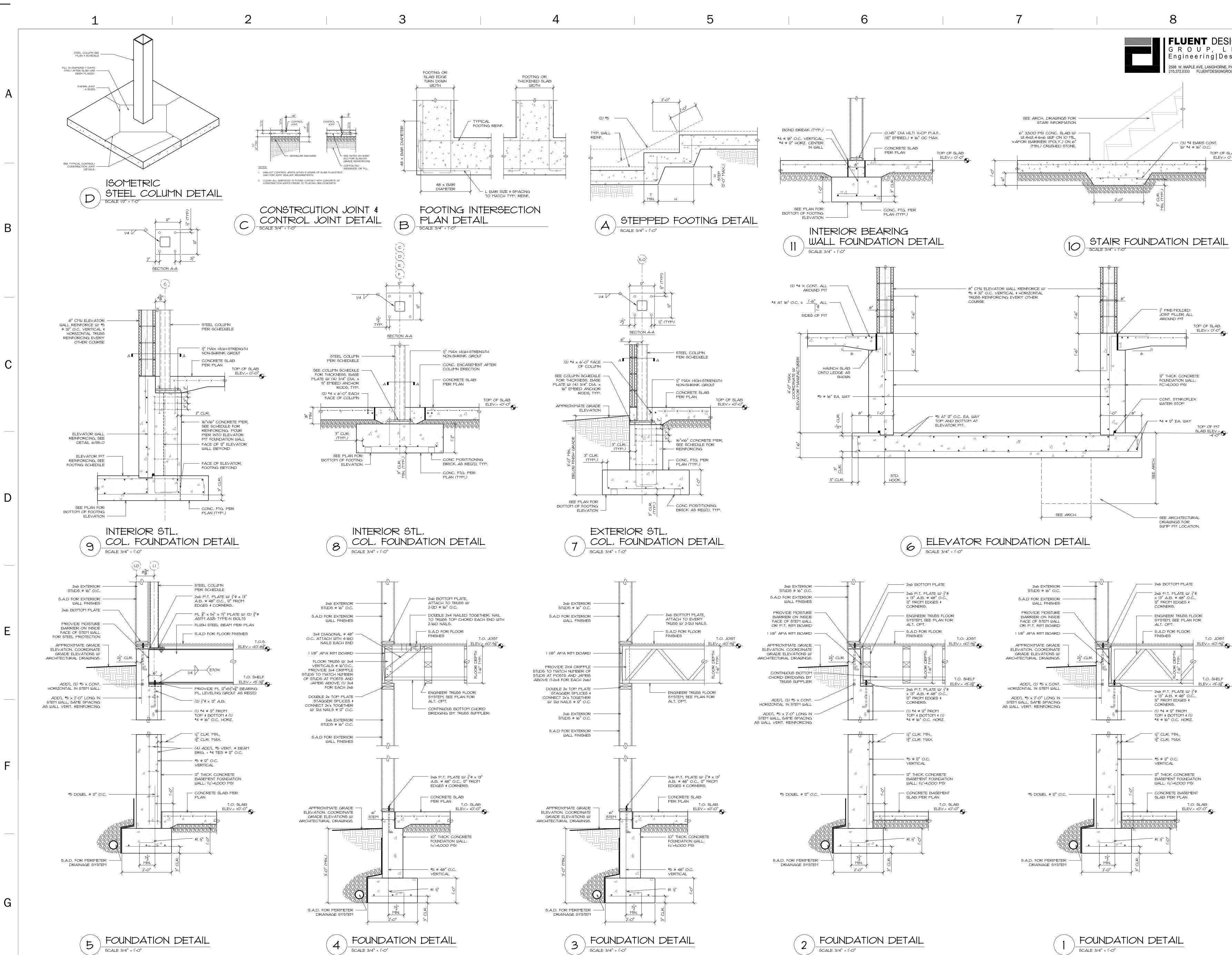
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DRAWING NAME: STRUCTURAL FOUNDATION DETAILS	SEAL: REVISIONS	Doy 215
WOODLANDS AT GREYSTONE SCULTHORPE DR. WEST GOSHEN TOWNSHIP	Date No.	hitecture + Hyde Park estown, PA 18902 345.4609 p 215.345 w.stampflassoci

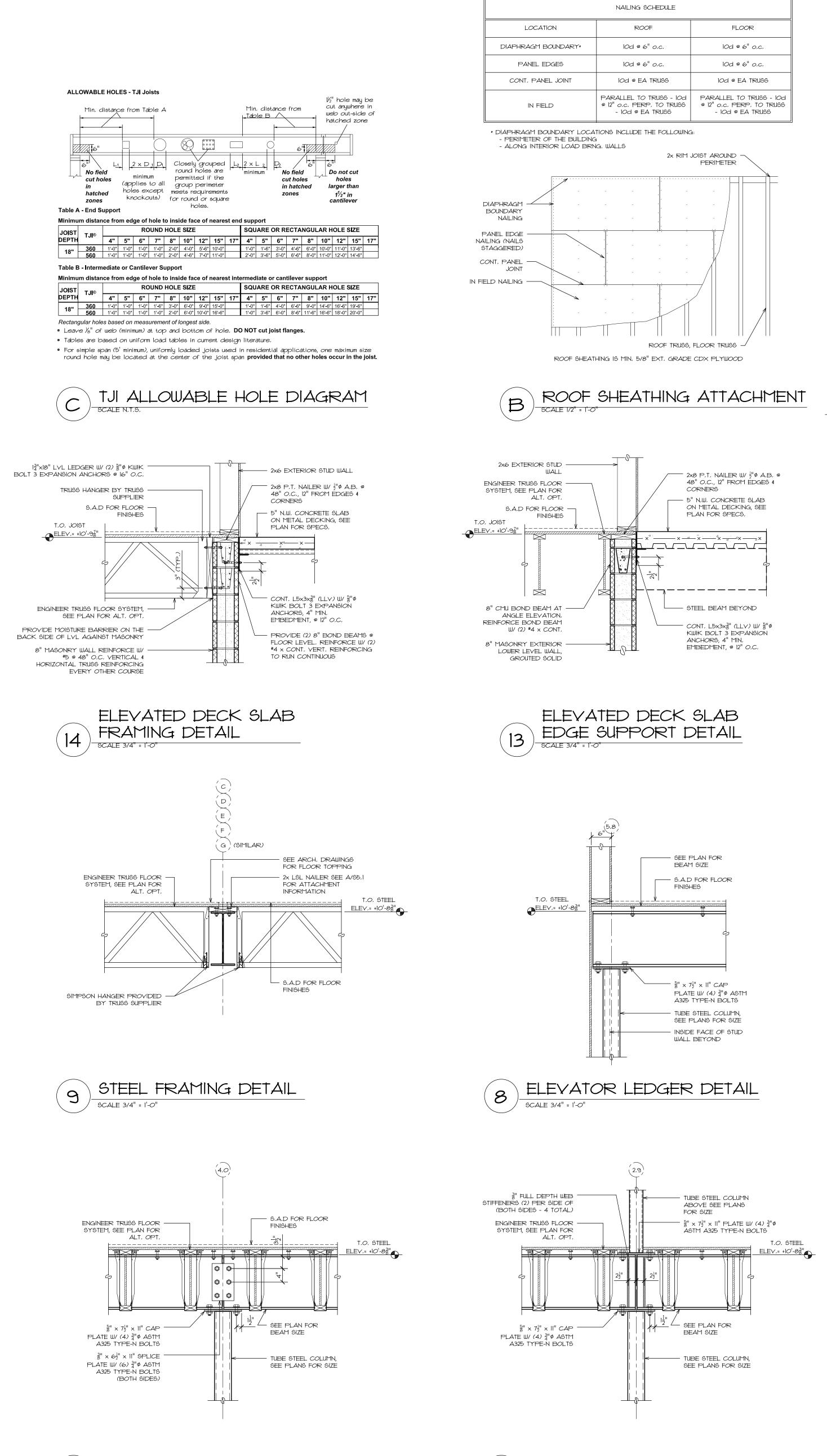




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(4A)

SCALE 3/4" = 1'-0'

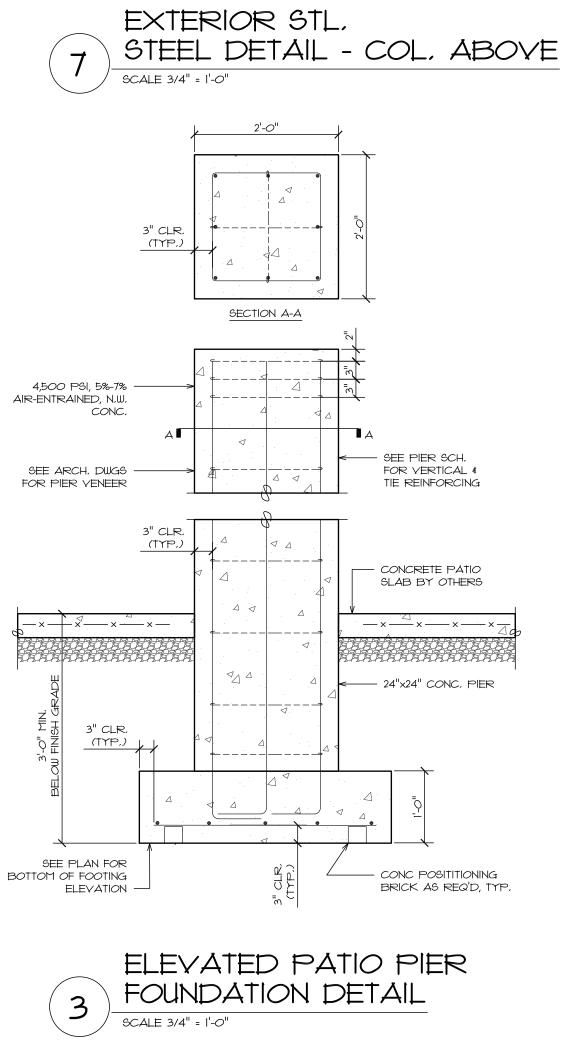


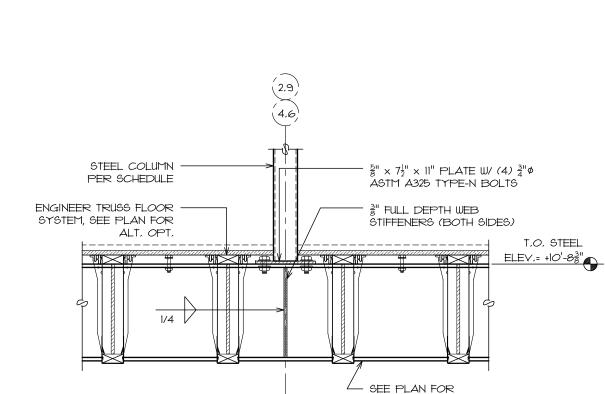
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# STEEL DETAIL - COLUMN ABOVE

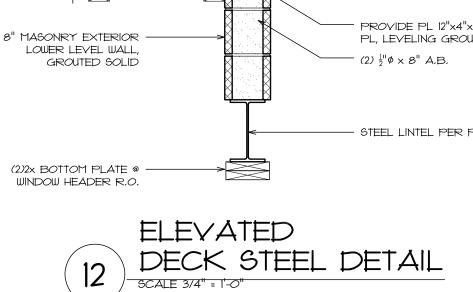
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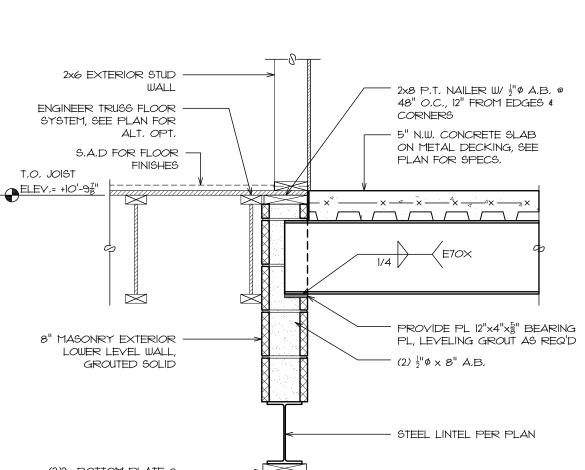
SCALE 3/4" = 1'-0"

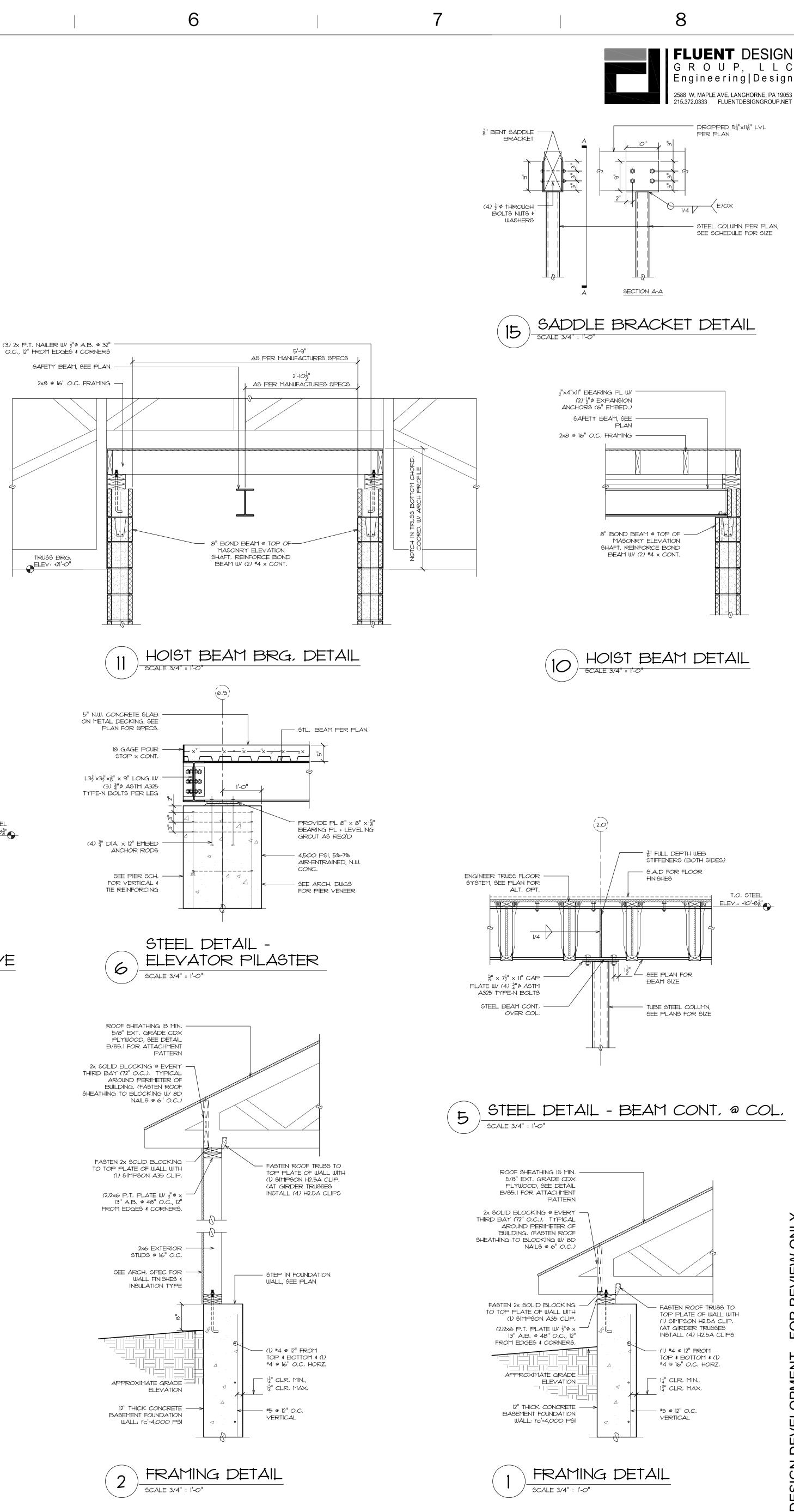


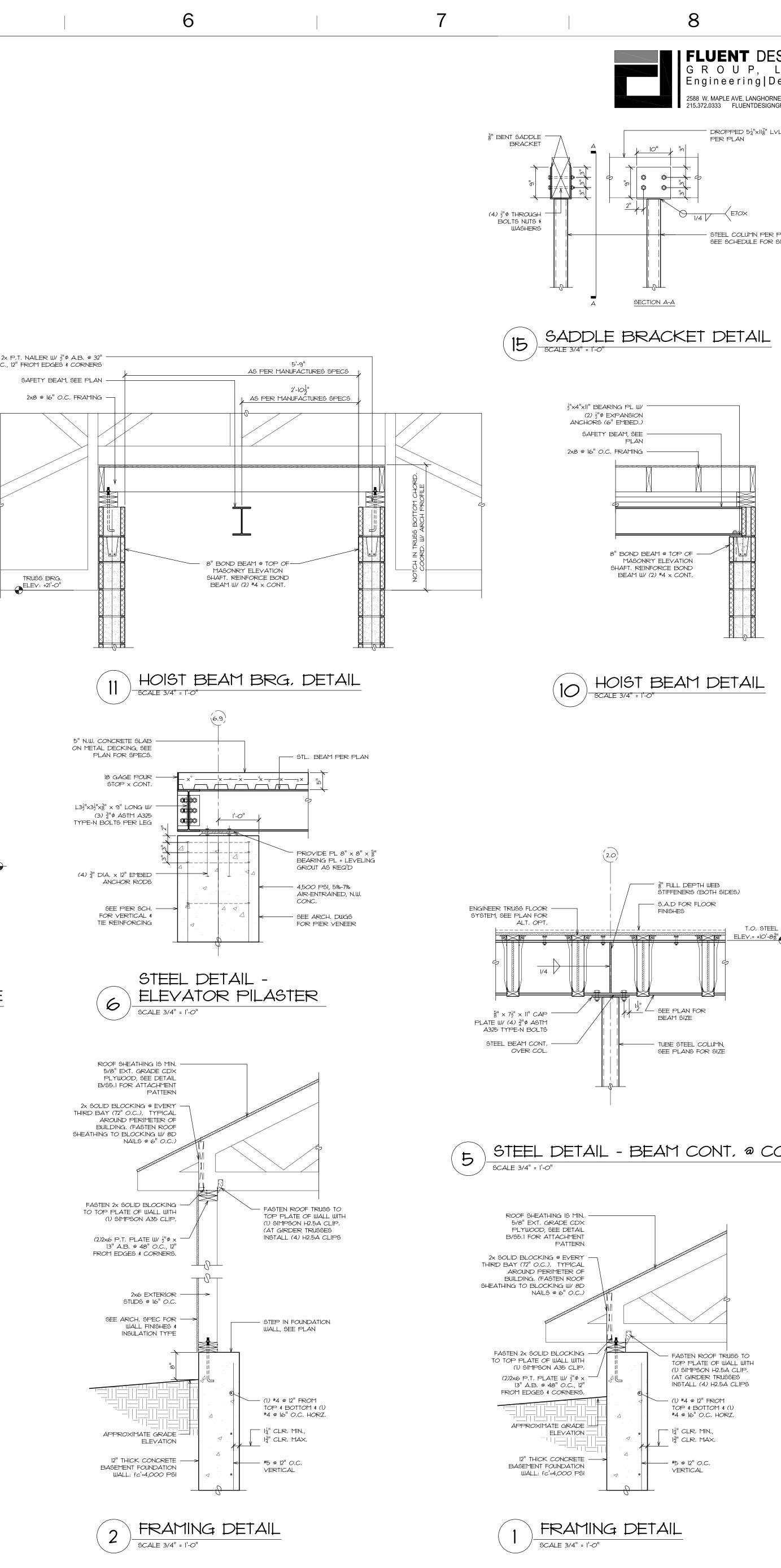


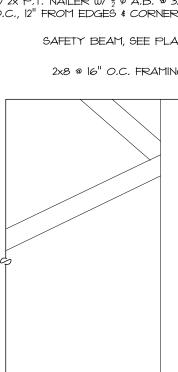
BEAM SIZE

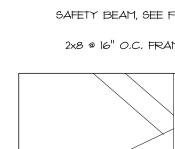


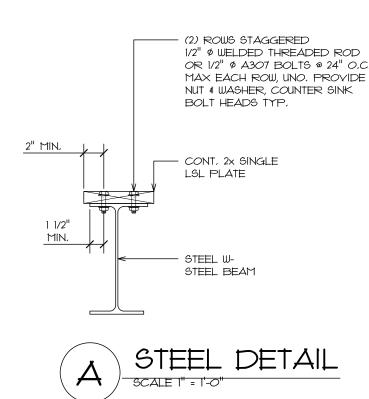










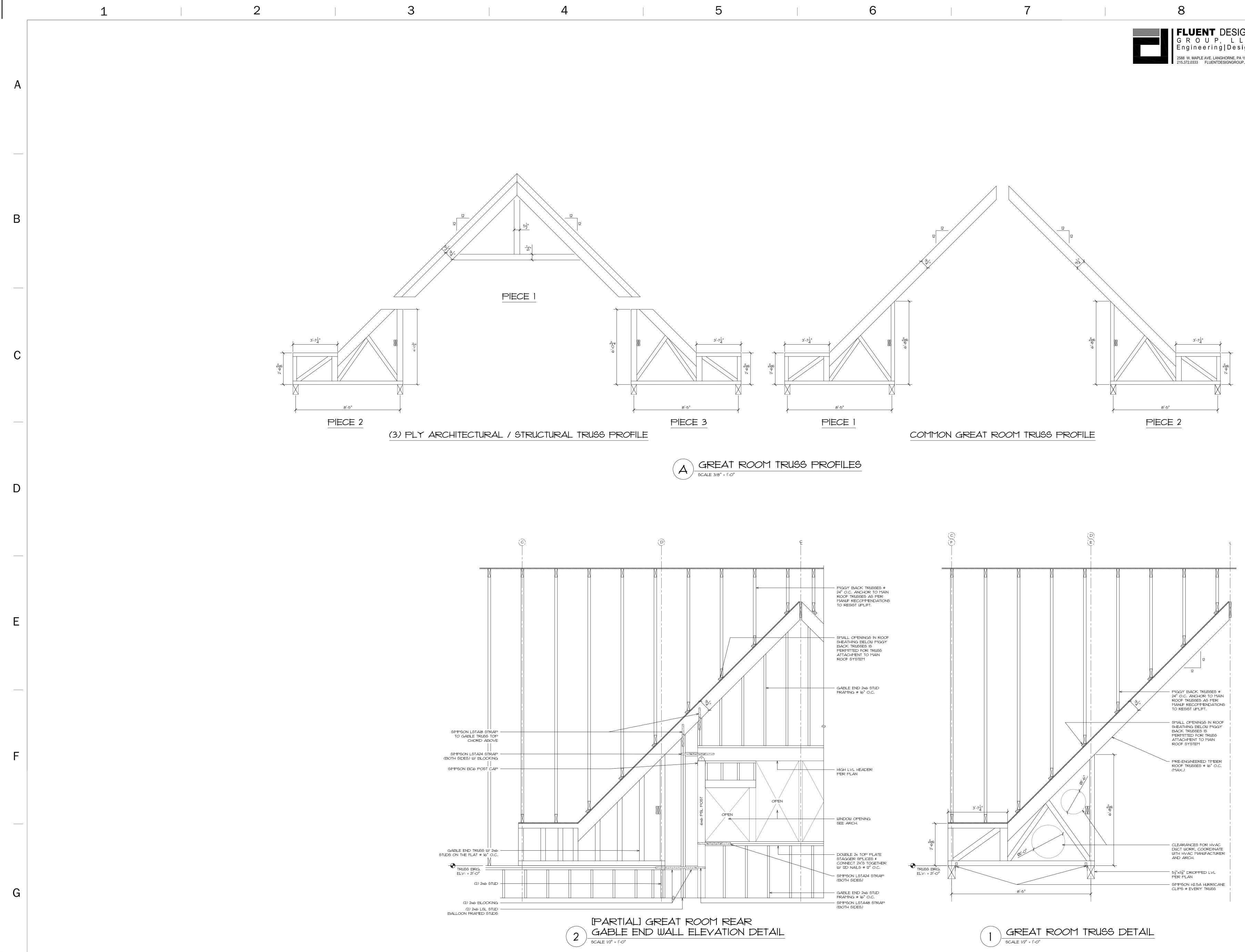


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chk'd by: -10-03 RCH approv. by: DTED MHB	CHESTER COUNTY, PA	Date No.	Description chk'd by	Ampfl Associat hitecture + planr Hyde Park estown, PA 18902 345.4609 p 215.345.4610 w.stampflassociates.
	LOCATION:			ning f



Stampfl Associa architecture + pla 711 Hyde Park Doylestown, PA 18902 215.345.4609 p 215.345.46 www.stampflassociate	architecture + p 711 Hyde Park Doylestown, PA 18902 215.345.4609 p 215.345 www.stampflassoci
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scale: AS NOTED

sheet no.

approv. by: MHB