MORE THAN ARCHITECTS

ADDENDUM

NO. 3

TO THE DRAWINGS AND THE PROJECT MANUAL

PROJECT NAME: Jarrell Middle School Addition and Renovations

CLIENT NAME: Jarrell Independent School District

LOCATION: Jarrell, Texas

PROJECT NUMBER: 01756-02-01

PROPOSAL DATE: 13, September 2018, 2:00 PM

ADDENDUM DATE: 7, September 2018

For additional information regarding this project, contact Mike Boyle at 512-387-3413

09/07/2018

THIS ADDENDUM INCLUDES:

Architectural Items	1 Page
Structural	10 Pages

AND ALL ATTACHED REVISED DRAWING REFERENCES IN THE ADDENDUM

Project Name: Jarrell Middle School Addition and Renovations
Client: Jarrell ISD
Jarrell, Texas
Project Number: 01756-02-01

09/07/2018

ARCHITECTURAL ITEMS FOR ADDENDUM NO. 3

NOTICE TO PROPOSERS:

A. This Addendum shall be considered part of the contract documents for the above-mentioned project as though it had been issued at the same time and incorporated integrally therewith. Where provisions of the following supplementary data differ from those of the original contract documents, this Addendum shall govern and take precedence.
B. Proposers are hereby notified that they shall make any necessary adjustments in their estimate on account of this Addendum. It will be construed that each Proposer's proposal is submitted with full knowledge of all modifications and supplemental data specified therein. Acknowledge receipt of this addendum in the space provided on the proposal form. Failure to do so may subject Proposer to disqualification.

REFERENCE IS MADE TO THE DRAWINGS AND THE PROJECT MANUAL AS NOTED:

PROJECT MANUAL:

AD No 1, Arch. Item 1: **01 6210 Schedule of Materials and Colors**

1. Change tile TP01 from 12x12 Dal-Tile Suede Grey Speckle to 12x12 Dal-Tile Porcealto Labradorite.

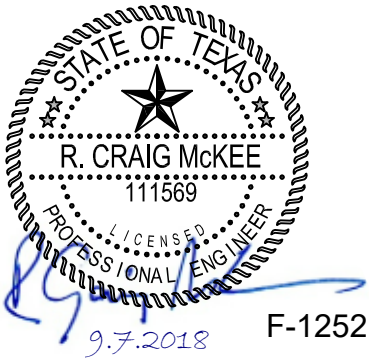
DRAWINGS:

AD No 1, Arch. Item 2: **Sheet A4.1, Room Finish Schedule**

1. At Corridors, change wall tile from TC03, 04 to TC02, 03.

END OF ARCHITECTURAL ADDENDUM

Project Name: Jarrell Middle School Addition and Renovations
Client: Jarrell ISD
Jarrell, Texas
Project Number: 01756-02-01



F-1252

STRUCTURAL ITEMS FOR ADDENDUM NO. 3

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REFERENCE IS MADE TO THE DRAWINGS AND THE PROJECT MANUAL AS NOTED:

DRAWINGS:

AD No 3, Struct Item 1: **To the Drawings, Sheet S0.0, "GENERAL NOTES"**

1) Replace this sheet in its entirety with Sheet S0.0, included herein as part of this Addendum.

AD No 3, Struct Item 2: **To the Drawings, Sheet S1.1, "MASTER PLAN – FOUNDATION & DFE"**

1) Replace this sheet in its entirety with Sheet S1.1, included herein as part of this Addendum.

AD No 3, Struct Item 3: **To the Drawings, Sheet S2.1, "DFE AND GRADE BEAM PLAN – SECTION 1"**

2) Replace this sheet in its entirety with Sheet S2.1, included herein as part of this Addendum.

AD No 3, Struct Item 4: **To the Drawings, Sheet S2.2, "FOUNDATION PLAN – SECTION 1"**

1) Replace this sheet in its entirety with Sheet S2.2, included herein as part of this Addendum.

AD No 3, Struct Item 5: **To the Drawings, Sheet S2.3, "ENLARGED RAMP PLAN"**

1) Replace this sheet in its entirety with Sheet S2.3, included herein as part of this Addendum.

AD No 3, Struct Item 6: **To the Drawings, Sheet S2.4, "VENEER LEDGE PLAN"**

1) Replace this sheet in its entirety with Sheet S2.4, included herein as part of this Addendum.

AD No 3, Struct Item 7: **To the Drawings, Sheet S3.1, "TYPICAL FOUNDATION DETAILS"**

1) Replace this sheet in its entirety with Sheet S3.1, included herein as part of this Addendum.

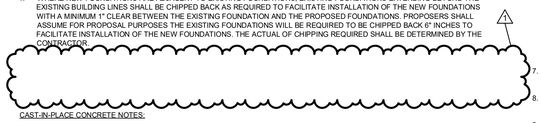
AD No 3, Struct Item 8: **To the Drawings, Sheet S3.2, "TYPICAL FOUNDATION DETAILS"**

1) Replace this sheet in its entirety with Sheet S3.2, included herein as part of this Addendum.

AD No 3, Struct Item 9: **To the Drawings, Sheet S3.3, "FOUNDATION DETAILS"**

1) Replace this sheet in its entirety with Sheet S3.3, included herein as part of this Addendum.

END OF STRUCTURAL ADDENDUM



GENERAL NOTES:

1. REFER TO THE PROJECT MANUAL FOR GOVERNING JOB REQUIREMENTS AND MATERIAL SPECIFICATIONS. THE FOLLOWING NOTES ARE SUPPLEMENTAL TO THE ABOVE REQUIREMENTS.

2. ALL DIMENSIONS TO, OF, AND IN EXISTING STRUCTURES SHALL BE VERIFIED IN FIELD BY CONTRACTOR WITH ALL DISCREPANCIES REPORTED TO THE ENGINEER.

3. DO NOT CHANGE THE SIZE NOR SPACING OF STRUCTURAL ELEMENTS WITHOUT THE APPROVAL OF THE ENGINEER.

4. DETAILS SHOWN AS TYPICAL APPLY TO SIMILAR CONDITIONS UNLESS NOTED OTHERWISE.

5. THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

6. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHODS OF CONSTRUCTION UNLESS SPECIFICALLY STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN AND OTHER PERSONS DURING CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL REQUIRED BRACING OF THE BUILDING AND OTHER STRUCTURES REQUIRED FOR CONSTRUCTION AND WIND LOADS UNTIL ALL STRUCTURAL ELEMENTS NEEDED FOR STABILITY ARE INSTALLED.

7. THE DESIGN IS BASED ON 2012 INTERNATIONAL BUILDING CODE.

8. CONTRACTOR SHALL DETERMINE EXACT LOCATION OF EXISTING UTILITIES BEFORE COMMENCING WORK. HE AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY HIS FAILURE TO EXACTLY LOCATE AND PRESERVE UNDERGROUND UTILITIES.

9. INCORRECTLY FABRICATED, DAMAGED, OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE OWNER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER'S APPROVAL.

10. EACH CONTRACTOR SHALL COOPERATE WITH THE OWNER'S REPRESENTATIVE AND COORDINATE HIS WORK WITH THE WORK OF OTHERS.

11. PRINCIPAL OPENINGS ARE INDICATED ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, BLOCKOUTS, INSERTS, CURBS, OPENINGS AND SLAB DEPRESSIONS THAT ARE REQUIRED BUT NOT SHOWN ON THE STRUCTURAL DRAWINGS. VERIFY SIZE AND LOCATION OF OPENINGS PRIOR TO BEGINNING WORK. FOR DIMENSIONS NOT SHOWN, SEE MECHANICAL, ELECTRICAL, CIVIL AND ARCHITECTURAL DRAWINGS.

12. VERIFY SIZE AND LOCATION OF EQUIPMENT PADS WITH MECHANICAL AND/OR ELECTRICAL CONTRACTOR AND EQUIPMENT MANUFACTURER.

13. THE STRUCTURAL DRAWINGS SHALL NOT BE SCALED FOR DETERMINATION OF QUANTITY, LENGTH OR FIT OF MATERIALS.

14. CONTRACTOR SHALL COMPARE STRUCTURAL AND ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCY TO THE ARCHITECT PRIOR TO FABRICATION OR INSTALLATION OF STRUCTURAL MEMBERS.

15. CONTRACTOR SHALL ENSURE THAT CONSTRUCTION MATERIALS WHOSE WEIGHT EXCEEDS THE DESIGN LIVE LOADS INDICATED ON THE STRUCTURAL DRAWINGS ARE NOT STORED ON STRUCTURALLY SUPPORTED FLOOR OR ROOF FRAMING.

16. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY CONDITION WHICH, IN HIS OR HER OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.

17. REFER TO SECTION 014533 OF THE PROJECT SPECIFICATIONS FOR QUALITY ASSURANCE PLAN AND SPECIAL INSPECTION REQUIREMENTS.

IMPORTANT NOTE REGARDING STABILITY DURING CONSTRUCTION:

1. TOP OF BEAMS ELEVATIONS ARE DELIBERATELY NOT PROVIDED ON THE FRAMING PLANS BECAUSE THE TOP OF BEAM ELEVATION SHALL BE ESTABLISHED BY THE STEEL DETAILER BASED ON THE GEOMETRIC ROOF REQUIREMENTS DEFINED BY THE CONSTRUCTION DRAWINGS AND BOTTOM OF DECK ELEVATIONS. THE TOP OF BEAM ELEVATIONS SHALL ACCOUNT FOR THE ACTUAL JOIST SEAT DEPTHS, AS ESTABLISHED ON THE STEEL JOIST SHOP DRAWINGS.

IMPORTANT REMODEL NOTES:

1. WHERE NEW FOUNDATIONS ARE ADJACENT TO EXISTING FOUNDATIONS, EXISTING FOUNDATIONS EXTENDING BEYOND THE EXISTING BUILDING LINES SHALL BE CHIPPED BACK AS REQUIRED TO FACILITATE INSTALLATION OF THE NEW FOUNDATIONS WITH A MINIMUM 1" CLEAR BETWEEN THE EXISTING FOUNDATION AND THE PROPOSED FOUNDATIONS. PROPOSERS SHALL ASSUME FOR PROPOSAL PURPOSES THE EXISTING FOUNDATIONS WILL BE REQUIRED TO BE CHIPPED BACK 6" INCHES TO FACILITATE INSTALLATION OF THE NEW FOUNDATIONS. THE ACTUAL OF CHIPPING REQUIRED SHALL BE DETERMINED BY THE CONTRACTOR.

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CAST-IN-PLACE CONCRETE NOTES:

1. REFER TO THE PROJECT SPECIFICATIONS FOR CONCRETE MIX DESIGN REQUIREMENTS.

2. IF THE AIR TEMPERATURE IS GREATER THAN 90 DEGREES WITHIN 24 HOURS AFTER PLACEMENT, HOT WEATHER CONCRETE PROCEDURES SHALL BE USED. THE CONTRACTOR SHALL SUBMIT A PROCEDURE TO THE ENGINEER FOR APPROVAL. THESE PROCEDURES MAY INCLUDE THE FOLLOWING:
   A. PLACING THE CONCRETE IN THE EARLY MORNING HOURS
   B. THE USE OF EVAPORATION REDUCER (SEE BELOW)
   C. THE USE OF MISTING AS A CURING METHOD
   D. THE USE OF WET BLANKETS AS A CURING METHOD
   E. THE USE OF A RETARDING ADMIXTURE (NOT PREFERABLE)

3. CONCRETE CYLINDERS SHALL BE MADE IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS FOR EACH DAYS POUR. THE CONCRETE SLUMP, TEMPERATURE, AND AIR CONTENT SHALL BE MEASURED EVERY TIME A SET OF FOUR CYLINDERS IS MADE.

4. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF THE AMERICAN CONCRETE INSTITUTE STANDARDS "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318) AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301). SPLICES IN REINFORCEMENT SHALL MEET CLASS B TENSION LAP REQUIREMENTS UNLESS NOTED OTHERWISE.

5. COVER FOR ALL REINFORCEMENT SHALL MEET THE COVERAGE REQUIREMENTS AS SHOWN IN THE LATEST ACI 318, AS NOTED BELOW OR AS SHOWN ON THE DETAILS. COVER DIMENSIONS SHOWN ON THE DETAILS CONTROL OVER THE ACI 318 OR THOSE NOTED BELOW:
   A. AGAINST FORMED SURFACES: 1 1/2"
   B. AGAINST EARTH: 3"
   C. BETWEEN REBAR: 1 1/2"
   D. TOP OF SLAB ON GRADE: 1 1/2"
   E. TOP OF SLAB OVER METAL DECK: 1"
   F. SLAB OVER VOID BOX: 1"

6. ANY CONCRETE TO BE PLACED FURTHER THAN 16 FEET FROM THE END OF A CONCRETE TRUCK SHALL BE PUMPED WITH A COMMERCIAL CONCRETE PUMPING TRUCK OR OTHER PLACEMENT METHOD APPROVED BY THE ENGINEER. THE CONCRETE TRUCK SHALL NOT BE ALLOWED TO DRIVE OVER THE SUBGRADE OR THE SLAB REINFORCEMENT.

7. REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 60. REINFORCEMENT SMALLER THAN #4 BARS SHALL BE COLD BENT WHENEVER BENDING IS REQUIRED IN THE FIELD. REINFORCEMENT GREATER THAN A #4 BAR MAY BE BENT IN THE FIELD WITH HEAT UNLESS NOTED ON THE PLANS OR DIRECTED BY THE ENGINEER OTHERWISE.

8. PROVIDE CORNER BARS IN FOOTINGS/GRADE BEAMS, THE SAME SIZE AND NUMBER AS CONTINUOUS REINFORCEMENT UNLESS NOTED ON THE PLANS OR TYPICAL DETAILS OTHERWISE.

9. DOWEL CONCRETE AND CMU WALLS, PILASTERS AND PIERS INTO GRADE BEAMS WITH DOWELS THE SAME SIZE AND SPACING AS VERTICAL REINFORCEMENT. EXTEND DOWELS TO WITHIN 3" OF BOTTOM OF FOOTING, TERMINATED WITH ACI STD. 90 DEGREE HOOK, UNLESS SPECIFICALLY NOTED OTHERWISE.

10. PROVIDE A ROUGH CONCRETE SURFACE (1/4" MINIMUM AMPLITUDE) AT THE INTERSECTION OF CONCRETE WALLS, STEM WALLS, AND PILASTERS WITH THE TOP OF GRADE BEAMS, DO NOT PROVIDE A KEYWAY UNLESS SHOWN OR NOTED ON THE DRAWINGS

11. PROVIDE 1" x 1" CHAMFER AT ALL EXPOSED CORNERS UNLESS NOTED OTHERWISE.

12. NO HOLES OR OPENINGS ARE PERMITTED THROUGH CONCRETE SLABS, BEAMS, OR WALLS EXCEPT AS FOLLOWS:
   A. WHERE SHOWN AND AS DETAILED ON DRAWINGS.
   B. MISCELLANEOUS HOLES THROUGH SLABS OR WALLS WHICH DO NOT DISPLACE MORE THAN ONE BAR. THESE DO NOT REQUIRE ADDITIONAL REINFORCEMENT.

13. LOCATE ADDITIONAL CONSTRUCTION JOINTS REQUIRED TO FACILITATE CONSTRUCTION AS ACCEPTABLE TO ENGINEER. LOCATE WALL CONSTRUCTION JOINTS AT MASONRY CONTROL JOINTS WHERE POSSIBLE. PLACE REINFORCEMENT CONTINUOUSLY THROUGH JOINT. DETAIL JOINT AND SHOW ON SHOP DRAWINGS.

14. WHERE MASONRY VENEER IS LOCATED ADJACENT TO WALLS, STEM WALLS, COLUMNS AND GRADE BEAMS GREATER THAN 24 INCHES HIGH, PROVIDE FULL HEIGHT DOVETAIL SLOT INSERTS AT 2'-0" O.C. FOR MASONRY ANCHORS. PLACE ANCHORS AT 16" O.C. VERTICALLY.

15. PLACE INTERMEDIATE HORIZONTAL BARS (#4 AT 12" MAXIMUM) ON EACH VERTICAL FACE OF ALL BEAMS GREATER THAN 36" IN DEPTH UNLESS NOTED OTHERWISE.

16. CAST CONCRETE ON SLOPED SURFACES BEGINNING AT LOWEST ELEVATION AND CONTINUING MONOLITHICALLY TOWARD HIGHER ELEVATIONS UNTIL INTENDED POUR IS COMPLETED.

17. REINFORCING BARS, BAR SUPPORTS, AND SPACERS SHALL BE DETAILED AND PROVIDED IN ACCORDANCE WITH THE LATEST ACI DETAILING MANUAL. USE WIRE-BAR SUPPORTS COMPLYING WITH CRSI SPECIFICATIONS. SUPPORTS SHALL NOT BE PLACED FURTHER THAN 4 FEET APART. DAYTON/RICHMOND PRODUCTS (800-745-3703) OR EQUAL UNLESS NOTED OTHERWISE IN THE SPECIFICATIONS:
   A. AT GRADE BEAMS OVER VOID BOXES: 2 1/2 INCHES HIGH, TYPE R21
   B. AT SLABS-ON-GRADE: (SLAB THICKNESS MINUS 1 1/2 INCHES) HIGH, TYPE R21, OR TYPE BBP USE SUPPORTS WITH SAND PLATES OR HORIZONTAL RUNNERS WHERE BASE MATERIAL WILL NOT SUPPORT CHAIR LEGS. CONCRETE BLOCK OR CLAY MASONRY MAY NOT BE USED.
   C. AT SLABS OVER VOID BOXES:
      1) BOTTOM BARS: TYPE BBP
      2) TOP BARS: TYPE HC WITH TYPE R28 BASE
   D. AT FOOTINGS: 3 IN. HIGH, TYPE R21
   E. FOR EXPOSED TO VIEW CONCRETE SURFACES WHERE LEGS OF SUPPORTS ARE IN CONTACT WITH THE FORMS, PROVIDE SUPPORTS WITH LEGS THAT ARE PLASTIC PROTECTED (CRSI, CLASS 1) OR STAINLESS STEEL PROTECTED (CRSI, CLASS 2).

18. ALL GRADE BEAM STEEL SHALL BE CONTINUOUS WHERE POSSIBLE. TOP BARS SHALL BE SPLICED AT THE CENTER OF THE SPAN, BOTTOM BARS OVER THE SUPPORTS, AND OTHER HORIZONTAL AND TEMPERATURE BARS AS REQUIRED. MINIMUM SPLICE LENGTH SHALL BE 30 BAR DIAMETERS UNLESS NOTED OTHERWISE ON THE PLANS OR STANDARD DETAILS. REFER TO THE DETAILS AND SCHEDULES FOR SPLICE LENGTHS .

19. SEE ARCHITECTURAL AND MECHANICAL/ELECTRICAL DRAWINGS FOR EXACT LOCATIONS AND DETAILS OF DEPRESSED SLABS, FLOOR DRAIN LOCATIONS, PLATFORMS, CURBS, AND PADS.

20. ALL GRADE BEAMS SHALL BE FORMED ON BOTH SIDES WITH NO TRENCHING ALLOWED. FORMS SHALL BE EITHER PLYWOOD BOX FORMS OR STEEL FORMS WITH FORM TIES, WHALERS AND ADJUSTABLE BRACES. NO DIMENSION LUMBER (2X12'S) SHALL BE USED FOR FORMING GRADE BEAMS. ANY PLYWOOD UNDER THE GRADE BEAM SHALL BE REMOVED.

21. SAW-CUTS IN CONCRETE SLABS SHALL BE MADE USING EARLY-ENTRY EQUIPMENT AS SOON AS THE CONCRETE IS OF SUFFICIENT STRENGTH TO SAW WITHOUT RAVELING THE AGGREGATE. ANY TIME LAPSE GREATER THAN 4 HOURS AFTER PLACING THE CONCRETE SHALL BE PERMITTED ONLY IF APPROVED BY THE ENGINEER. PROVIDE SAW CUT AT STRUCTURAL SLAB AT GRADE BEAM.

22. EPOXY OR ADHESIVE ANCHORS AND DOWELS INTO CONCRETE WITH REBAR OR THREADED RODS, SHALL BE AS NOTED BELOW. INSTALL ACCORDING TO THE MANUFACTURER'S RECOMMENDATIONS, WHICH INCLUDES CLEANING THE HOLE WITH AIR AND USING A MANUFACTURER APPROVED DISPENSING TOOL WITH MIXING NOZZLE.
   A. INTO CONCRETE: HILTI HIT-HY 200 OR EQUAL
   B. UNLESS NOTED MORE STRINGENTLY OTHERWISE, THE MINIMUM EMBEDMENT DEPTH OF EPOXY OR ADHESIVE ANCHORS AND DOWELS INTO CONCRETE SHALL BE AS FOLLOWS:
      1) 3/8" DIA. THREADED ROD OR #3 REBAR: 4 ½"
      2) 1/2" DIA. THREADED ROD OR #4 REBAR: 4 1/2"
      3) 5/8" DIA. THREADED ROD OR #5 REBAR: 5 5/8"
      4) 3/4" DIA. THREADED ROD OR #6 REBAR: 6 ¾"
      5) 7/8" DIA. THREADED ROD OR #7 REBAR: 7 7/8"
      6) 1" DIA. THREADED ROD OR #8 REBAR: 9"

23. NO PIPING OR CONDUITS SHALL BE INSTALLED IN ANY CONCRETE WITHOUT THE APPROVAL OF THE ENGINEER. IN GENERAL, IF APPROVED BY THE ENGINEER, ANY PIPING OR CONDUITS MUST BE LOCATED IN THE MIDDLE OF THE SLAB AND NOT BE OVER ONE INCH IN DIAMETER. REFER TO THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS FOR PENETRATIONS.

24. ALL DUMBBELL TYPE WATERSTOPS SHALL BE 6" PVC, CENTER BULB TYPE, SUCH AS GREENSTREAK STYLE 732. BENTONITE STRIP SUCH AS GREENSTREAK SWELLSTOP STYLE 594.

25. ALL DOWELS, ANCHOR BOLTS, EMBEDDED STEEL, ELECTRICAL CONDUITS, PIPE SLEEVES, PIPING, WATERSTOPS, INSERTS, GROUNDS, AND ALL OTHER EMBEDDED ITEMS AND FORMED DETAILS SHALL BE IN PLACE BEFORE START OF CONCRETE PLACEMENT. FOR EMBEDDED ITEMS AND REQUIRED DETAILS, SEE MECHANICAL, ELECTRICAL, AND ARCHITECTURAL DRAWINGS. VERIFY SIZE AND LOCATION OF ALL OPENINGS.

26. ALL PIPING AND DUCT PENETRATIONS THROUGH NEW STRUCTURAL SLABS ARE TO BE SLEEVED OR CHASED. NO CORING OF SLAB IS PERMITTED. ALL PIPING THROUGH EXISTING STRUCTURAL SLABS MAY BE CORED IF APPROVED BY ENGINEER.

27. ALL GRADE BEAM STEEL SHALL BE CONTINUOUS WHERE POSSIBLE. TOP BARS SHALL BE SPLICED AT THE CENTER BETWEEN THE INTERSECTION OF BEAMS, BOTTOM BARS OVER THE INTERSECTION OF BEAMS, AND OTHER HORIZONTAL AND TEMPERATURE BARS AS REQUIRED. MINIMUM SPLICE LENGTH SHALL BE 24 INCHES OR 30 BAR DIAMETERS WHICHEVER IS GREATER.

28. THE CONTRACTOR SHALL VERIFY VAPOR BARRIER IS PROVIDED UNDER ALL SLAB AREAS WITH TILE, CARPET OR OTHER MOISTURE SENSITIVE COVERINGS PER THE FLOOR FINISH SCHEDULE (AFTER ALL ADDENDA).

STEEL NOTES:

1. STRUCTURAL STEEL FABRICATION AND ERECTION SHALL CONFORM TO THE AISC MANUAL OF STEEL CONSTRUCTION.

2. WELDED CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING SOCIETY.

3. ANY CONNECTIONS WITHOUT WELD SYMBOLS SHALL BE AT A MINIMUM WELDED ALL AROUND WITH THE MINIMUM FILLET, FLARE BEVEL OR BUTT WELD SIZE.

4. STRUCTURAL STEEL C SHAPES, ANGLES, PLATES, ETC. SHALL CONFORM TO ASTM A36 REQUIREMENTS (36 KSI). STRUCTURAL STEEL W SHAPES SHALL CONFORM TO ASTM A992 (50 KSI). STRUCTURAL TUBING AND PIPES SHALL CONFORM TO THE ASTM A500 GRADE B REQUIREMENTS (46 KSI).

5. DO NOT PLACE HOLES THROUGH STRUCTURAL STEEL MEMBERS EXCEPT AS SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.

6. CONNECTIONS:
   A. ALL STEEL CONNECTIONS SHALL BE DESIGNED BY A CONNECTION ENGINEER TO BE HIRED BY THE CONTRACTOR. THE CONTRACTOR'S CONNECTION ENGINEER RESPONSIBLE FOR THE CONNECTION DESIGN SHALL BE A PROFESSIONAL ENGINEER FAMILIAR WITH THE DESIGN OF SUCH ELEMENTS AND SHALL BE LICENSED TO PRACTICE ENGINEERING IN THE STATE OF TEXAS. CONNECTION DESIGNS AND DETAILS SHOWN ON THE DRAWINGS SHALL BE CONSIDERED MINIMUMS. FINAL CONFIGURATION, PLATE AND ANGLE THICKNESSES, NUMBER OF BOLTS, BOLT DIAMETERS, AND WELD DESIGNS SHALL BE DESIGNED BY THE CONTRACTOR'S CONNECTION ENGINEER. CONNECTION DESIGN WILL BE SUBJECT TO REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER OF RECORD. SEE PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
   B. ALL BOLTED CONNECTIONS ARE TO BE 3/4" MINIMUM DIAMETER A325 TYPE N OR SC BOLTS IN STANDARD HOLES UNLESS NOTED OTHERWISE OR AS DETERMINED BY THE CONNECTION DESIGNER OR NOTED ON THE PLANS. DESIGN USING STANDARD HOLES UNLESS OTHERWISE NOTED OR REQUIRED FOR ERECTION.
   C. MINIMUM CAPACITY OF BEAM CONNECTIONS: FOR CONNECTIONS NOT DETAILED, PROVIDE CONNECTION CAPACITY OF AT LEAST THAT REQUIRED BY PART 3 OF THE AISC MANUAL IN THE SECTION "MAXIMUM TOTAL UNIFORM LOAD TABLES" FOR LRFD DESIGN OR "ALLOWABLE LOADS ON BEAMS" FOR ALLOWABLE STRESS DESIGN, FOR THE GIVEN MEMBER AND STEEL SPECIFICATIONS. CONCENTRATED LOADS NEAR SUPPORTS MUST BE ADDED.
   D. THE DESIGN SHEAR FOR EACH CONNECTION UNLESS NOTED ON THE DRAWINGS SHALL BE 110% OF THE REACTION FROM A UNIFORM LOAD OVER THE SPAN WHICH CREATES THE MAXIMUM DESIGN MOMENT FOR ROOF BEAM CONNECTIONS AND 180% FOR COMPOSITE FLOOR CONNECTIONS. THE MINIMUM REACTION SHALL BE 14 KIPS.
   E. INDICATOR BOLTS EQUAL TO TENSION FOR CONTROL BOLTS OF THE LeJEUNE COMPANY OF LAKEVILLE, MINNESOTA (800-872-2658) SHALL BE USED.
   F. USE MINIMUM OF TWO 3/4" DIAMETER A325 BOLTS PER CONNECTION.

7. FRAME ALL OPENINGS IN ROOF DECK 8" OR LARGER WITH A STRUCTURAL STEEL MEMBER ON ALL SIDES EXCEPT WHERE A SIDE MAY BE WITHIN 1'-0" OF ANOTHER FRAMING MEMBER. FRAMING NOT REQUIRED FOR ROOF OPENINGS SMALLER THAN 8".

8. THE CONTRACTOR SHALL VERIFY THAT THE FLOW LINE OF ALL SCUPPERS, INCLUDING OVERFLOW SCUPPERS, IS NO HIGHER THAN 4" ABOVE THE TOP OF THE ROOFING MATERIAL (DISCOUNTING ANY CANT STRIPS OR FLASHING).

9. ALL STEEL BEAMS TO RECEIVE SHEAR CONNECTORS SHALL HAVE NO PAINTING ON THE TOP FLANGES.

10. SHEAR CONNECTORS (SC) SHOWN ON THE PLAN SHALL BE 3/4" DIAMETER EQUAL TO NELSON FLUXED SHEAR CONNECTOR STUDS (UNO) WITH LENGTH EQUAL TO THE SLAB THICKNESS MINUS 1". STUDS SHALL BE AUTOMATICALLY END WELDED IN THE FIELD. EQUAL SPACE STUDS AND STAGGER ON BEAMS PARALLEL TO THE DECK. ON BEAMS PERPENDICULAR TO THE DECK, PLACE ONE STUD IN EACH RIB AND DOUBLE UP REMAINING REQUIRED ON EACH END. THE ENGINEER SHALL APPROVE ALL STUD PLACEMENT OPERATIONS.

11. PROVIDE 3/8" STIFFENER PLATES ON EACH SIDE OF THE WEB OF ALL BEAMS AT ALL SUPPORTS THAT ARE BELOW THE BEAM, AND AT ALL COLUMNS THAT ARE ABOVE THE BEAM, OMITTING WHERE BEAMS INTERSECT.

STEEL JOIST AND ROOF DECK NOTES:

1. METAL ROOF DECK SHALL BE PROVIDED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND AS INDICATED ON THE PLANS. METAL ROOF DECK SHALL BE ATTACHED TO SUPPORTING MEMBERS PER THE PROJECT SPECIFICATIONS.

2. PROVIDE L3X3X1/4 AROUND ALL ROOF PENETRATIONS NOT SHOWN OVER 8 INCHES FOR METAL DECK SUPPORT.

3. THE ROOF DECK IS STANDING SEAM AND WILL NOT BRACE THE TOP CHORD OF THE BAR JOISTS. EXTRA BRIDGING IS SHOWN ON THE PLAN, BUT THE JOIST MANUFACTURER IS RESPONSIBLE FOR VERIFYING BRIDGING SPACING AND DESIGNING THE TOP CHORD AS REQUIRED FOR THE BRACING SPACING.

4. LOCATE CONCENTRATED LOADS ON JOISTS AND JOIST GIRDERS AT PANEL POINTS. PROVIDE ANGLE WEB MEMBERS TO CREATE INTERMEDIATE PANEL POINTS AS REQUIRED. MANUFACTURER SHALL DIRECT INSTALLER AS TO METHOD OF INSTALLATION AND MATERIAL REQUIRED. JOISTS AND JOIST GIRDERS SHALL BE SHOP REINFORCED FOR ALL LOADS PROVIDED ON DRAWINGS. FIELD REINFORCING SHALL BE PROVIDED AS DETAILED ON THE DRAWINGS.

5. VERIFY SIZE, LOCATION, AND NUMBER OF ROOF OPENINGS WITH MECHANICAL AND ELECTRICAL PLANS AND CONTRACTORS.

6. PROVIDE BRIDGING ANCHORS FIRMLY ANCHORED TO MASONRY WALLS, BEAMS, AND COLUMNS AT EACH END OF EACH ROW OF BRIDGING, TOP AND BOTTOM.

7. METAL ROOF DECK IS DESIGNED FOR 3 SPAN CONDITIONS. IT IS THE RESPONSIBILITY OF THE DECK SUPPLIER TO ADJUST THE THICKNESS (GAUGE) OF THE ROOF DECK AT SINGLE OR DOUBLE SPAN CONDITIONS AS REQUIRED BY THE STEEL DECK INSTITUTE.

8. PROVIDE SUPPORT FOR EDGES OF DECK WHETHER SHOWN ON THE DRAWINGS OR NOT.

FLOOR DECK NOTES (COMPOSITE AND NON COMPOSITE):

1. PROVIDE SUPPORT FOR METAL DECK AT ALL INTERIOR COLUMN LOCATIONS.

2. ALL FLOOR DECK SHALL HAVE A MINIMUM OF 3" BEARING ON CONCRETE OR MASONRY AND SHALL LAP A MINIMUM OF 2 INCHES AT ENDS. CONTRACTOR MAY AT HIS OPTION BUTT ENDS OF DECK OVER STEEL BEAMS AND TAPE JOINTS TO PREVENT SLURRY PENETRATION.

3. METAL DECK SHALL BE CAPABLE OF SUPPORTING DEAD LOAD AND 20 PSF CONSTRUCTION LOAD AS A FORM AND SUPERIMPOSED LOADS INDICATED ON FULL COMPOSITE SECTION. MAXIMUM LIVE LOAD DEFLECTION OF COMPOSITE SECTION SHALL BE 1/360 OF CLEAR SPAN.

4. METAL FLOOR DECK SHALL BE PROVIDED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND AS INDICATED ON THE PLANS. METAL ROOF DECK SHALL BE ATTACHED TO SUPPORTING MEMBERS PER THE PROJECT SPECIFICATIONS.

5. FOR STEEL FRAMED FLOORS, PROVIDE ADDITIONAL CONCRETE AS NECESSARY TO FINISH THE FLOORS TO WITHIN THE SPECIFIED TOLERANCES BY ACCOUNTING FOR THE STEEL JOIST, BEAM AND DECK DEFLECTION UNDER THE WET WEIGHT OF THE CONCRETE. IT IS SUGGESTED TO ALLOW FOR AN ADDITIONAL ONE INCH OF CONCRETE PER FLOOR TO COMPENSATE FOR THE DEFLECTION. CONTRACTOR TO USE THE APPROPRIATE PLACEMENT MEASUREMENT METHOD TO ACCOUNT FOR THIS DEFLECTION.

6. FRAME ALL OPENINGS IN SLABS 2'-0" SQUARE OR LARGER WITH A STRUCTURAL STEEL MEMBER ON ALL SIDES EXCEPT WHERE A SIDE MAY BE WITHIN 1'-0" OF ANOTHER FRAMING MEMBER. FRAME OPENINGS WITH C8X11.5 UNLESS NOTED OTHERWISE. PROVIDE TWO #5 BARS, 6'-0" LONG, AT ALL CORNERS AND AT EDGES OF OPENINGS IN SLAB.

7. DO NOT APPLY PAINT TO TOP FLANGES ON BEAMS AND GIRDERS WHICH RECEIVE SHEAR CONNECTORS.

8. LOCATE MECHANICAL OPENINGS THROUGH SLABS NO CLOSER THAN 4'-0" TO GIRDER CENTERLINE AND 2'-0" TO BEAM CENTERLINE.

9. FOR SLAB OPENINGS GREATER THAN 1'-0" BUT LESS THAN 2'-0" SQUARE, PROVIDE A #4 BY 3'-0" LONG DIAGONAL BAR AT ALL CORNERS OF OPENINGS.

10. PROVIDE CONTINUOUS SCREED ANGLE FOR EDGE OF SLAB.

PRE-ENGINEERED METAL BUILDING NOTES:

SPECIAL REQUIREMENTS FOR THE METAL BUILDING DESIGN:

1. BEFORE FABRICATION, SHOP DRAWINGS OF THE METAL BUILDING SHALL BE SUBMITTED TO HUCKABEE, FOR REVIEW AND COMMENT.

2. THE METAL BUILDING SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM LOADS AND SHALL BE DESIGNED IN ACCORDANCE WITH THE 2012 BUILDING CODE:
   A. MAIN FRAMES: 20 PSF (THIS SHALL NOT BE REDUCED)
   B. COMPONENTS: MINIMUM 20PSF
   C. COLLATERAL: 6 PSF
   D. WIND LOAD PER ASCE 7-10:
      1) 3 SECOND GUST DESIGN WIND SPEED= 120MPH
      2) EXPOSURE CATEGORY: C
      3) ASSUME ALL WINDOWS AND OH DOORS ARE OPENINGS FOR THE ENCLOSURE CLASSIFICATION, BUT ASSUME THEY ARE CLADDING FOR THE CALCULATION OF LOADS ON SECONDARY FRAMING.
   E. SNOW LOAD: Pg= 5PSF
   F. EARTHQUAKE DESIGN DATA:
      SEISMIC IMPORTANCE FACTOR, I: 1.25
      MAPPED SPECTRAL RESPONSE ACCELERATIONS: SS=0.064 S1=0.036
      SITE CLASS: D
      SEISMIC DESIGN CATEGORY: A

3. THE LATERAL DEFLECTION OF THE LATERAL FORCE RESISTING SYSTEMS SHALL NOT EXCEED H/180 FOR ANY BUILDING AND H/300 FOR ANY BUILDING WITH MASONRY VENEER HIGHER THAN 8 FEET ABOVE FINISH FLOOR, THE LATERAL DEFLECTION OF THE GIRTS AND METAL SIDING SHALL BE L/120 FOR ANY WALL, AND L/240 FOR ANY TRIBUTARY AREA WITH MASONRY VENEER.

4. ALL COLD-FORMED STEEL SHALL BE DESIGNED BY THE METAL BUILDING ENGINEER PER AISI-NASPEC 2001. THE FOLLOWING ARE ADDITIONAL MINIMUM REQUIREMENTS FOR THE PURLINS AND GIRTS ON THIS PROJECT:
   A. BOTH FLANGES SHALL BE BRACED AT 8FT O.C. MAXIMUM.
   B. BRACING LINES SHALL CONSIST OF 16 GAGE STRAPS X 1.5" WITH A #12 TEK SCREW AT EACH FLANGE.
   C. BRACING LINES SHALL BE ANCHORED WITH 2 HORIZONTAL AND 2 DIAGONAL L1.5X1.5X16 GAUGE ANGLES AT EACH END BAY, AT BOTH SIDES OF RIDGES, AND AT 50 FT OC MAXIMUM. HORIZONTAL ANGLES SHALL BE COPED AND ATTACHED TO EACH FLANGE WITH 2#12 TEK SCREWS. ATTACH DIAGONALS TO HORIZONTALS WITH 2#12 TEK SCREWS.
   D. BRACING STRAPS MAY BE OMITTED ON FLANGES THAT ARE BRACED WITH THROUGH-FASTENED ROOF PANELS OR METAL SIDING.

DESIGN LOADS:

THE FOLLOWING DESIGN LOADS WERE USED FOR THIS BUILDING BASED ON THE 2012 INTERNATIONAL BUILDING CODE:

FLOOR LIVE LOADS: 100 PSF
   CLASSROOMS ABOVE THE GROUND FLOOR: 40 PSF
   CORRIDORS ABOVE THE GROUND FLOOR: 80 PSF
   CORRIDORS AND STAIRWAYS: 100 PSF
   STAGES AND PLATFORMS: 125 PSF
   LIGHT STORAGE AREAS: 125 PSF
   LIBRARY: 150 PSF
   MECHANICAL ACCESS SPACE: 50 PSF
   MECHANICAL ROOMS: 150 PSF
   ALL OTHER AREAS: 100 PSF

ROOF LIVE LOAD: 20 PSF

ROOF SNOW LOAD
   GROUND SNOW LOAD = 5 PSF
   FLAT-ROOF SNOW LOAD = 4 PSF
   SNOW EXPOSURE FACTOR = 0.9
   SNOW LOAD IMPORTANCE FACTOR, = 1.1
   THERMAL FACTOR. = 1.0

WIND DESIGN DATA:
   BASIC WIND SPEED (3 SECOND GUST): 120 MPH
   RISK CATEGORY=III
   WIND EXPOSURE CATEGORY: C
   INTERNAL PRESSURE COEFFICIENTS: +/- 0.18

   ALL NEW COMPONENTS AND CLADDING NOT DESIGNED BY THE ENGINEER SHALL BE DESIGNED IN ACCORDANCE WITH WIND LOADS INDICATED BELOW

   COMPONENT AND CLADDING WIND PRESSURES (ULTIMATE PRESSURES):

   STRUCTURES LESS THAN 60 FEET IN HEIGHT
   WALLS:
      EFFECTIVE AREA = 10 FT^2
         ZONE 4 (MORE THAN 12 FT FROM A BUILDING CORNER): +/- 39 PSF
         ZONE 5 (LESS THAN 12 FT FROM A BUILDING CORNER): +/- 48 PSF
      EFFECTIVE AREA = 100 FT^2
         ZONE 4 (MORE THAN 12 FT FROM A BUILDING CORNER): +/- 30 PSF
         ZONE 5 (LESS THAN 12 FT FROM A BUILDING CORNER): +/- 30 PSF

   PARAPETS:
      EFFECTIVE AREA = 10 FT^2
         ZONE 4 (MORE THAN 12 FT FROM A BUILDING CORNER): +/- 86 PSF
         ZONE 5 (LESS THAN 12 FT FROM A BUILDING CORNER): +/- 120 PSF
      EFFECTIVE AREA = 100 FT^2
         ZONE 4 (MORE THAN 12 FT FROM A BUILDING CORNER): +/- 58 PSF
         ZONE 5 (LESS THAN 12 FT FROM A BUILDING CORNER): +/- 83 PSF

   ROOFS:
      EFFECTIVE AREA = 10 FT^2
         ZONE 1 (MORE THAN 12 FT EDGE OF ROOF): +/- 39 PSF
         ZONE 2 (LESS THAN 12 FT FROM A BUILDING CORNER): +/- 60 PSF
         ZONE 3 (CORNER ZONE): +/- 94 PSF
      EFFECTIVE AREA = 100 FT^2
         ZONE 1 (MORE THAN 12 FT EDGE OF ROOF): +/- 39 PSF
         ZONE 2 (LESS THAN 12 FT FROM A BUILDING CORNER): +/- 42 PSF
         ZONE 3 (CORNER ZONE): +/- 67 PSF
   OVERHANGS:
      EFFECTIVE AREA = 10 FT^2
         ZONE 2 (LESS THAN 12 FT FROM A BUILDING CORNER): +/- 85 PSF
         ZONE 3 (CORNER ZONE): +/- 143 PSF
      EFFECTIVE AREA = 100 FT^2
         ZONE 2 (LESS THAN 12 FT FROM A BUILDING CORNER): +/- 73 PSF
         ZONE 3 (CORNER ZONE): +/- 101 PSF

   SOFFITS AT OVERHANGS:
      EFFECTIVE AREA = 10 FT^2
         ZONE 4 (MORE THAN 12 FT FROM A BUILDING CORNER): +/- 39 PSF
         ZONE 5 (LESS THAN 12 FT FROM A BUILDING CORNER): +/- 48 PSF
      EFFECTIVE AREA = 100 FT^2
         ZONE 4 (MORE THAN 12 FT FROM A BUILDING CORNER): +/- 30 PSF
         ZONE 5 (LESS THAN 12 FT FROM A BUILDING CORNER): +/- 30 PSF

EARTHQUAKE DESIGN DATA:
   SEISMIC IMPORTANCE FACTOR, I: 1.25
   MAPPED SPECTRAL RESPONSE ACCELERATIONS: SS=0.064 S1=0.036
   SITE CLASS: D
   SEISMIC DESIGN CATEGORY: A
   DESIGN BASE SHEAR: 0.01 X DL
   ANALYSIS PROCEDURE USED: SEISMIC DESIGN REQUIREMENTS FOR SEISMIC DESIGN CATEGORY A

FLOOD HAZARD INFORMATION:
   THIS BUILDING IS NOT DESIGNED FOR FLOOD LOADS.

IMPORTANT NOTE TO PROPOSERS:
1. THESE DRAWINGS AND SPECIFICATIONS DO NOT NECESSARILY INDICATE ALL OF THE WORK REQUIRED FOR THE COMPLETION OF THE PROJECT. THESE DRAWINGS AND SPECIFICATION ARE ONLY INTENDED TO SHOW THE STRUCTURAL REQUIREMENTS TO COMPLETE THE SUPERSTRUCTURE FOR THE BUILDING, WHICH INCLUDES THE PRIMARY FOUNDATION AND FRAMING ELEMENTS TO SUPPORT THE FLOORS AND ROOFS. THESE DRAWINGS DO NOT NECESSARILY INDICATE ALL SECONDARY FRAMING WHICH MAY BE REQUIRED BASED ON THE FORTHCOMING ARCHITECTURAL DRAWINGS.
2. WHERE MISCELLANEOUS METAL ITEMS ARE IMPLIED OR INDICATED ON THE STRUCTURAL OR ARCHITECTURAL DRAWINGS, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE FABRICATION AND INSTALLATION OF THESE ITEMS. THIS INCLUDES ANY MISCELLANEOUS METAL ITEMS INDICATED ON THE ARCHITECTURAL DRAWINGS AND NOT SHOWN ON THE STRUCTURAL DRAWINGS AND ANY ITEMS SHOWN ON THE STRUCTURAL DRAWINGS NOTED. PROPOSERS SHALL ASSUME FOR PROPOSAL PURPOSES ONLY THE SIZE OF THESE ITEMS ARE FOLLOWS. THE CONTRACTOR SHALL SUBMIT AN RFI AND THE ENGINEER SHALL APPROVE THE SIZE OF THE BEFORE CONSTRUCTION OR FABRICATION:
a. ANGLES: L6X6X3/8
b. CHANNELS: C12X20.7
c. PLATES: 3/4" THICK
d. ANCHOR RODS: 1" DIAMETER X 18"
e. EPOXY ANCHORS: ¾" DIAMETER, SPACED AT 48" OC MAX SPACING
f. WIDE FLANGE BEAMS: W24X55
g. WF COLUMNS: W12X79
h. HSS COLUMNS AND BEAMS: 8"X8"X3/8"
3. PROVIDE SUPPORT FOR EDGES OF ROOF AND FLOOR DECK WHETHER SHOWN ON THE DRAWINGS OR NOT. PROVIDE CONTINUOUS SCREED ANGLE OR BENT PLATE FOR SLAB EDGES.
4. PROVIDE SHEAR STUDS AT ALL FLOOR BEAMS. BEAMS WITHOUT STUDS NOTED SHALL HAVE ¾" DIAMETER STUDS SPACED AT 12 INCHES ON CENTER.

1

IMPORTANT NOTE:
ALL PRE-ENGINEERED METAL BUILDING (PEMB) COMPONENTS SHOWN ON THIS SHEET ARE ONLY SHOWN HERE FOR CONCEPTUAL PURPOSES, AND ARE ONLY INTENDED TO INDICATE THE DESIGN INTENT OF MAJOR BUILDING COMPONENTS SUCH AS RIGID FRAME LOCATIONS, LOCATIONS OF PORTAL FRAMES, AND APPLICABLE DIMENSIONAL LIMITATIONS OF THESE COMPONENTS. ALL OF THESE COMPONENTS, INCLUDING BUT NOT LIMITED TO ALL MISCELLANEOUS FRAMING ASSOCIATED WITH THESE ITEMS, SHALL BE DESIGNED BY THE CONTRACTOR'S PEMB ENGINEER IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

1

FOUNDATION AND SOIL PREPARATION NOTES:

1. THE FOUNDATION DESIGN IS BASED ON AN ALLOWABLE END BEARING PRESSURE OF 3,000 POUNDS PER SQUARE FOOT AS RECOMMENDED IN THE GEOTECHNICAL REPORT NO. A182085 BY ALPHA TESTING, INC., AND DATED SEPTEMBER 4, 2018. BEARING STRATUM FOR FOOTINGS SHALL BE VERIFIED IN FIELD BY THE SITA BEFORE PLACING CONCRETE FOOTINGS.

2. FOUNDATION INSTALLATION WILL LIKELY REQUIRED EXCAVATION OF "MARL" MATERIAL, WHICH MAY BEHAVE LIKE ROCK WHEN EXCAVATING, THEREBY REQUIRING CONSTRUCTION EQUIPMENT AND PROCEDURES TYPICALLY USED FOR ROCK. THE CONTRACTOR SHALL HAVE EXPERIENCE WITH EXCAVATION IN THIS MARL/ROCK.

3. THE CONTRACTOR SHALL REVIEW THE REPORT AND BORING LOGS DURING THE BIDDING PHASE OF THE PROJECT. IF THE CONTRACTOR BELIEVES DISCREPANCIES EXIST BETWEEN THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT AND THESE DOCUMENTS, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF SUCH DISCREPANCIES PRIOR TO BIDDING. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO INCLUDE THE DIFFERENCE BETWEEN FINAL AND EXISTING GRADES IN THEIR BID. ADDITIONAL COST WILL NOT BE ALLOWED FOR FAILURE TO ACCOUNT FOR THE DIFFERENCE BETWEEN FINAL AND EXISTING GRADES. REFER TO THE EXISTING SITE GRADING PLAN FOR FINAL AND EXISTING GRADES.

4. PROVIDE POSITIVE DRAINAGE FOR ALL TRENCHES DURING CONSTRUCTION. DO NOT ALLOW ANY PONDING OF WATER DURING CONSTRUCTION.

5. THE BEARING STRATUM FOR FOOTINGS SHALL BE AS INDICATED ON THE DRAWINGS. BEARING STRATUM FOR ALL FOOTINGS SHALL BE INSPECTED AND APPROVED BY THE SITA IMMEDIATELY PRIOR TO PLACING CONCRETE. BEARING SURFACES, PREVIOUSLY ACCEPTED BY THE SITA THAT ARE ALLOWED TO BECOME SATURATED OR DISTURBED SHALL BE REWORKED TO SATISFACTION OF THE SITA. EXCAVATIONS FOR FOOTINGS SHALL NOT BE LET OPEN FOR MORE THAN 48 HOURS.

6. THE SOIL BENEATH THE BUILDING AND OUTSIDE THE PERIMETER OF THE BUILDING TO THE EXTENDS INDICATED ON S1.1 SHALL BE TREATED AS FOLLOWS:

   A. STRIP THE AREA OF ALL VEGETATION AND OTHERWISE DELETERIOUS MATERIAL.
   B. REMOVE ALL EXISTING DARK BROWN EXPANSIVE SOILS AND EXISTING FILL MATERIAL TO EXPOSE THE EXISTING NATIVE CLAYEY SAND OR MARL. THE ACTUAL DEPTH OF THE EXISTING MATERIAL THAT WILL REQUIRE REMOVAL WILL NOT BE KNOWN UNTIL CONSTRUCTION BEGINS. HOWEVER, PROPOSERS SHALL ASSUME FOR PROPOSAL PREPARATION PURPOSES 6 FEET OF EXISTING DARK BROWN EXPANSIVE SOILS WILL REQUIRE REMOVAL AND REPLACEMENT WITH SELECT FILL MATERIAL. THIS VALUE SHALL BE USED FOR PROPOSAL PURPOSES ONLY AND SHALL NOT BE USED FOR CONSTRUCTION. (NOTE: PERIMETER FOOTINGS ARE REQUIRED TO BEAR ON A MARL LAYER. THEREFORE, EXCAVATION OF NATIVE CLAYEY SAND MATERIAL MAY ALSO BE REQUIRED UNDER EXTERIOR FOOTINGS.)
   C. ALL FOUNDATION EXCAVATIONS SHALL BE MONITORED BY THE SITA TO VERIFY FOUNDATIONS BEAR ON SUITABLE MATERIAL.
   D. THE EXPOSED SUBGRADE SHALL BE PROOF-ROLLED WITH A HEAVY PNEUMATIC TIRED ROLLER, SUCH AS A LOADED DUMP TRUCK OR SIMILAR EQUIPMENT WEIGHING APPROXIMATELY 20 TONS. ANY UNDESIRABLE MATERIAL, SUCH AS ORGANIC MATERIAL, WET, SOFT, OR LOOSE SOIL, EXPOSED DURING PROOF-ROLLING SHALL BE REMOVED AND REPLACED WITH PROPERLY COMPACTED SELECT FILL MATERIAL. ALL PROOF-ROLLING OPERATIONS SHALL BE CONTINUOUSLY OBSERVED BY THE SITA. ANY SOIL REMOVED UNDER THE EXTERIOR FOOTINGS REQUIRED TO BEAR ON MARL SHALL BE REPLACED WITH A LEAN CONCRETE FILL WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI.
   E. AFTER PROOF-ROLLING, ANY EXPOSED CLAYEY SAND MATERIAL SHALL BE SCARIFIED TO A DEPTH OF AT LEAST 6 INCHES AND RECOMPACTED TO A DENSITY IN THE RANGE OF 98% TO 100% OF MODIFIED PROCTOR AT A MOISTURE CONTENT AT WITHIN 2 PERCENTAGE POINTS OF OPTIMUM. THE FIRST LIFT OF FILL SHALL BE PLACED ON THE COMPACTED SUBGRADE WITHIN EIGHT HOURS OF COMPLETING THE COMPACTION. EXPOSED MARL MATERIAL SHALL NOT REQUIRE SCARIFICATION.
   F. ALL FILL REQUIRED TO RAISE THE BUILDING TO BENEATH THE FLOOR SLAB, (EXCEPT FOR THE GRANULAR BACKFILL REQUIRED BEHIND THE FOUNDATION WALLS), SHALL BE ONE OF THE FOLLOWING SELECT FILL MATERIALS (NOTE: ALL FILL GREATER THAN 10 FEET BELOW THE FINAL BUILDING PAD GRADE (BOTTOM OF THE FLOOR SLAB) SHALL BE "FLEXIBLE BASE SELECT FILL"):
   1. "NON-EXPANSIVE" GRANULAR SELECT FILL COMPACTED TO A DRY DENSITY IN THE RANGE OF AT LEAST 98 PERCENT OF MODIFIED PROCTOR (ASTM D 1557) AT A MOISTURE CONTENT WITHIN 2 PERCENTAGE POINTS OF OPTIMUM, AND SHALL HAVE THE FOLLOWING PROPERTIES:
      1. LIQUID LIMIT LESS THAN 35 PERCENT
      2. PLASTICITY INDEX BETWEEN 5 AND 15 PERCENT
      3. NO MORE THAN 0.5 PERCENT FIBROUS ORGANIC MATERIALS BY WEIGHT
      4. NO MORE THAN 30 PERCENT BY WEIGHT PASSING A NO. 200 SIEVE
      5. CONTAIN NO DELETERIOUS MATERIAL
   2. FLEXIBLE BASE SELECT FILL MEETING THE REQUIREMENTS OF TxDOT STANDARD SPECIFICATION ITEM 247, TYPE A, B, C, OR D, GRADE 1-2, AND COMPACTED TO A DRY DENSITY IN THE RANGE OF AT LEAST 98 PERCENT OF MODIFIED PROCTOR (ASTM D 1557) AT A MOISTURE CONTENT WITHIN 2 PERCENTAGE POINTS OF OPTIMUM.
      A. ANY EXISTING SLOPES (NATURAL OR CONSTRUCTED) STEEPER THAN SIX HORIZONTAL UNITS TO ONE VERTICAL UNIT (6:1) THAT ARE TO RECEIVE FILL MATERIAL ON TOP OF THE SLOPES SHALL BE BENCHED AS REQUIRED TO PROVIDE A MINIMUM HORIZONTAL BENCH WIDTH OF 5 FEET.
      B. THE PLASTICITY INDEX AND LIQUID LIMIT OF MATERIAL USED AS SELECT FILL MATERIAL SHALL BE ROUTINELY VERIFIED, DURING PLACEMENT USING LABORATORY TEST. THE ACTUAL FREQUENCY OF VERIFICATION OF THESE PROPERTIES SHALL BE DETERMINED BY THE SITA AS THE FILL IS PLACED.
      C. EACH LIFT SHALL BE TESTED FOR MOISTURE CONTENT AND IN PLACE DENSITY AT A RATE OF ONE TEST PER 3,000 SQUARE FEET (MIN OF THREE PER LIFT).
      D. REFER TO THE SPECIFICATIONS FOR ADDITIONAL SOIL PREPARATION NOTES.

7. UNLESS PAVEMENT OR FLATWORK IS IMMEDIATELY ADJACENT TO THE BUILDING, THERE SHALL BE A MINIMUM 2-FOOT-THICK LAYER OF CLAY BACKFILL AGAINST THE EXTERIOR OF THE FOUNDATION EXTENDING A DISTANCE FOR 5 FEET AWAY FROM THE EXTERIOR OF THE BUILDING. THIS CLAY BACKFILL SHALL BE COMPACTED TO A DRY DENSITY IN THE RANGE OF AT LEAST 95 PERCENT OF STANDARD PROCTOR AT A MOISTURE CONTENT OF 0 TO 4 PERCENTAGE POINTS ABOVE OPTIMUM, AND SHALL HAVE THE FOLLOWING PROPERTIES:
   A. PLASTICITY INDEX BETWEEN 20 AND 35 PERCENT
   B. NO MORE THAN 30 PERCENT BY WEIGHT PASSING A NO. 200 SIEVE
   C. NO MORE THAN 15 PERCENT BY WEIGHT RETAINED ON A NO. 4 SIEVE

= POUNDS, NUMBER
& = AND
A/E = ARCHITECT AND ENGINEER
AB = ANCHOR BOLT
ADDL = ADDITIONAL
ADH = ADHESIVE
ADJ = ADJACENT
AESS = ARCHITECTURAL EXPOSED STRUCTURAL STEEL
ALT = ALTERNATE
APPROX = APPROXIMATE
ARCH(L) = ARCHITECT (ARCHITECTURAL)
B/ = BOTTOM OF
BLDG = BUILDING
BLKG = BLOCKING
BM = BEAM
BO = BOTTOM OF
BOB = BOTTOM OF BEAM
BOD = BOTTOM OF DECK
BOL = BOTTOM OF LINTEL
BOS = BOTTOM OF STEEL
BOTT = BOTTOM
BRG = BEARING
BTWN = BETWEEN
CANT = CANTILEVER
CFMF = COLD FORM METAL FRAMING
CJ = CONSTRUCTION JOINT OR CONTROL JOINT
CJP = COMPLETE JOINT PENETRATION
CL = CENTER LINE
CLR = CLEAR, CLEARANCE
CMU = CONCRETE MASONRY UNIT
COL = COLUMN
CONC = CONCRETE
CONN = CONNECTION
CONT = CONTINUOUS
COORD = COORDINATE
COV = COVER
CTR = CENTER
CTU = CONCRETE TILT-UP
CWI = CERTIFIED WELDING INSPECTOR
db = BAR DIAMETER
DBA = DEFORMED BAR ANCHOR
DBL = DOUBLE
DL = DEAD LOAD
DEG = DEGREE
DET = DETAIL
DFE = DEEP FOUNDATION ELEMENT
DIA = DIAMETER
DIAG = DIAGONAL
DIM = DIMENSION
DN = DOWN
DWG = DRAWING
DWL = DOWEL
EA = EACH
EF = EACH FACE
EJ = EXPANSION JOINT
EL = ELEVATION
EMBED = EMBEDMENT
ENGR = ENGINEER
EOS = EDGE OF SLAB
EQ = EQUAL
EQUIP = EQUIPMENT
EW = EACH WAY
EXIST = EXISTING
EXT = EXTERIOR
F.F.E. = FINISHED FLOOR ELEVATION
f'c = CONCRETE 28 DAY MINIMUM COMPRESSIVE STRENGTH
FD = FLOOR DRAIN
FIN = FINISH(ED)
FLR = FLOOR
f'm = MASONRY MINIMUM COMPRESSIVE STRENGTH
FND = FOUNDATION
FOA = FACE OF ANGLE
FOC = FACE OF CONCRETE
FOM = FACE OF MASONRY
FS = FAR SIDE
FT = FEET
FTG = FOOTING
FV = FIELD VERIFY
Fy = YIELD STRESS
GA = GAGE
GALV = GALVANIZED
GB = GRADE BEAM
GEN = GENERAL
GLULAM = GLUED-LAMINATED BEAM
GR = GRADE
HI = HIGH
HK = HOOK
HORIZ = HORIZONTAL
HSA = HEADED STUD ANCHOR
HSS = HOLLOW STRUCTURAL SECTION
ICF = INSULATED CONCRETE FORM
INFO = INFORMATION
INT = INTERIOR
ISF = INSIDE FACE
JST = JOIST
JT = JOINT
K = KIPS (1000 LBS)
L = ANGLE, ANGLE SHAPE
LBS = POUNDS
LF = LINEAR FOOT
LG = LENGTH
LL = LIVE LOAD
LLH = LONG LEG HORIZONTAL
LLV = LONG LEG VERTICAL
LO = LOW LONG = LONGITUDINAL
LSH = LONG SIDE HORIZONTAL
LSV = LONG SIDE VERTICAL
LT WT = LIGHTWEIGHT
MAS = MASONRY
MAX = MAXIMUM
MECH = MECHANICAL
MEZZ = MEZZANINE
MFR = MANUFACTURER
MID = MIDDLE
MIN = MINIMUM
MISC = MISCELLANEOUS
MO = MASONRY OPENING
MSL = MEAN SEA LEVEL
MTL = METAL
NS = NEAR SIDE
NTS = NOT TO SCALE
OC = ON CENTER
OH = OPPOSITE HAND or OVERHANG
OPH = OPPOSITE HAND
OPNG = OPENING
OSF = OUTSIDE FACE
OVH = OVERHANG, OVERHEAD
PAF = POWER ACTUATED FASTENER
PCC = PRECAST CONCRETE
PEMB = PRE-ENGINEERED METAL BUILDING
PERP = PERPENDICULAR
PJP = PARTIAL JOINT PENETRATION
PL = PLATE
PLF = POUNDS PER LINEAR FOOT
PLYWD = PLYWOOD
PRELIM = PRELIMINARY
PSF = POUNDS PER SQUARE FOOT
PSI = POUNDS PER SQUARE INCH
PT = POST TENSIONED
QTY = QUANTITY
R = RADIUS, REACTION
REF = REFERENCE
REINF = REINFORCEMENT(ING)
REM = REMAINDER
REQD = REQUIRED
REV = REVISION
RTU = ROOF TOP UNIT
SCHED = SCHEDULE
SDS = SELF-DRILLING SCREW
SECT = SECTION
SEOR = STRUCTURAL ENGINEER OF RECORD
SF = SQUARE FEET
SHT = SHEET
SIM = SIMILAR
SOG = SLAB ON GRADE
SPA = SPACING
SPEC = SPECIFICATIONS
SQ = SQUARE
STD = STANDARD
STIFF = STIFFENER
STIRR = STIRRUP(S)
STL = STEEL
STRUCT = STRUCTURAL, STRUCTURE
SYM = SYMMETRICAL
TO = TOP OF
TOVL = TOP OF VENEER LEDGE
TEMP = TEMPORARY, TEMPERATURE
THD = THREADED
TOC = TOP OF CONCRETE
TOM = TOP OF MASONRY
TOS = TOP OF STEEL, TOP OF SLAB
TOW = TOP OF WALL (CMU, ICF & CFMF)
TWP = TILT-WALL PANEL
TYP = TYPICAL
UNO = UNLESS NOTED OTHERWISE
VERT = VERTICAL
VL = VENEER LEDGE
W/ = WITH
W/O = WITHOUT
WP = WORKING POINT

ABBREVIATIONS

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Revision / 1
Date 08/30/18
ADDENDUM 1

Project:
JARRELL MIDDLE SCHOOL ADDITIONS
FOR
JARRELL I.S.D.
JARRELL, TEXAS

R. CRAIG McKEE
111569
9.7.2018
F-1252

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GENERAL NOTES

Job No.
1756-02-01
Drawn By:
MK,AM
Date:
09/07/2018
Sheet No.
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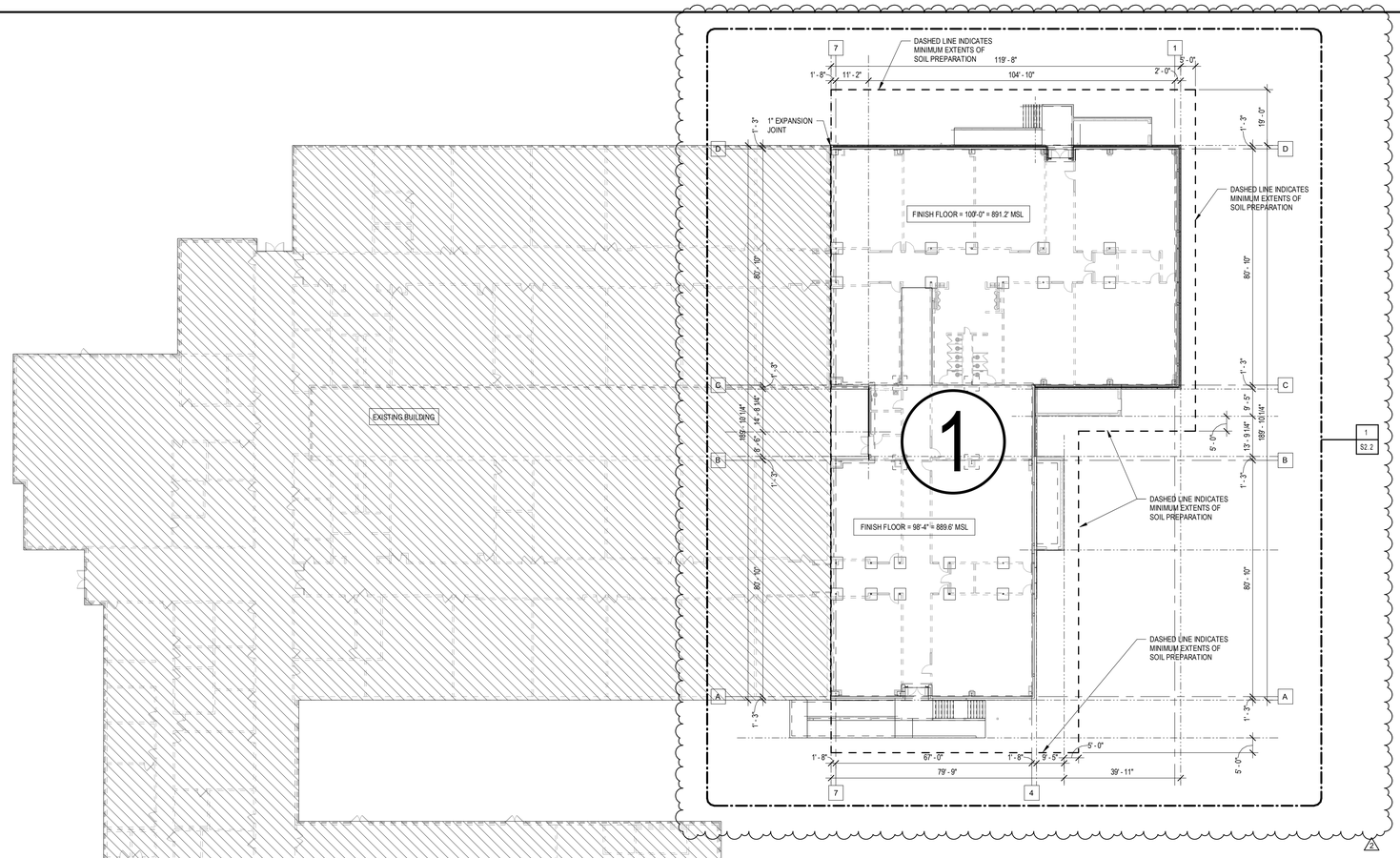

THE MASTER FLOOR PLAN SHOWS THE OVERALL BUILDING CONFIGURATION AND THE RELATIONSHIP OF THE SECTION PLANS WHICH FOLLOW. REFER TO LARGER SCALE PLANS AND PLAN DETAILS FOR ADDITIONAL INFORMATION AND CONSTRUCTION REQUIREMENTS.

MASTER FLOOR PLAN AND PLAN SECTIONS

IMPORTANT NOTE:
ALL PRE-ENGINEERED METAL BUILDING (PEMB) COMPONENTS SHOWN ON THIS SHEET ARE ONLY SHOWN HERE FOR CONCEPTUAL PURPOSES, AND ARE ONLY INTENDED TO INDICATE THE DESIGN INTENT OF MAJOR BUILDING COMPONENTS SUCH AS RIGID FRAME LOCATIONS, LOCATIONS OF PORTAL FRAMES, AND APPLICABLE DIMENSIONAL LIMITATIONS OF THESE COMPONENTS. ALL OF THESE COMPONENTS, INCLUDING BUT NOT LIMITED TO ALL MISCELLANEOUS FRAMING ASSOCIATED WITH THESE ITEMS, SHALL BE DESIGNED BY THE CONTRACTOR'S PEMB ENGINEER IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

Revision /	Date
2	09/07/18

ADDENDUM 3

Project:

JARRELL MIDDLE SCHOOL ADDITIONS
FOR
JARRELL I.S.D.
JARRELL, TEXAS

STATE OF TEXAS
R. CRAIG McKEE
117300
LICENSED PROFESSIONAL ENGINEER
9.7.2018

F-1252

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EXISTING

KEY PLAN
N.T.S.

N PLAN TRUE

MASTER PLAN - FOUNDATION & WALL FOOTING PLAN

Job No.	Sheet No.
1756-02-01	S1.1
Drawn By: MK AM	
Date: 09/07/2018	

1 / S1.1 MASTER PLAN - WALL FOOTING AND FOUNDATION
1" = 20'-0"

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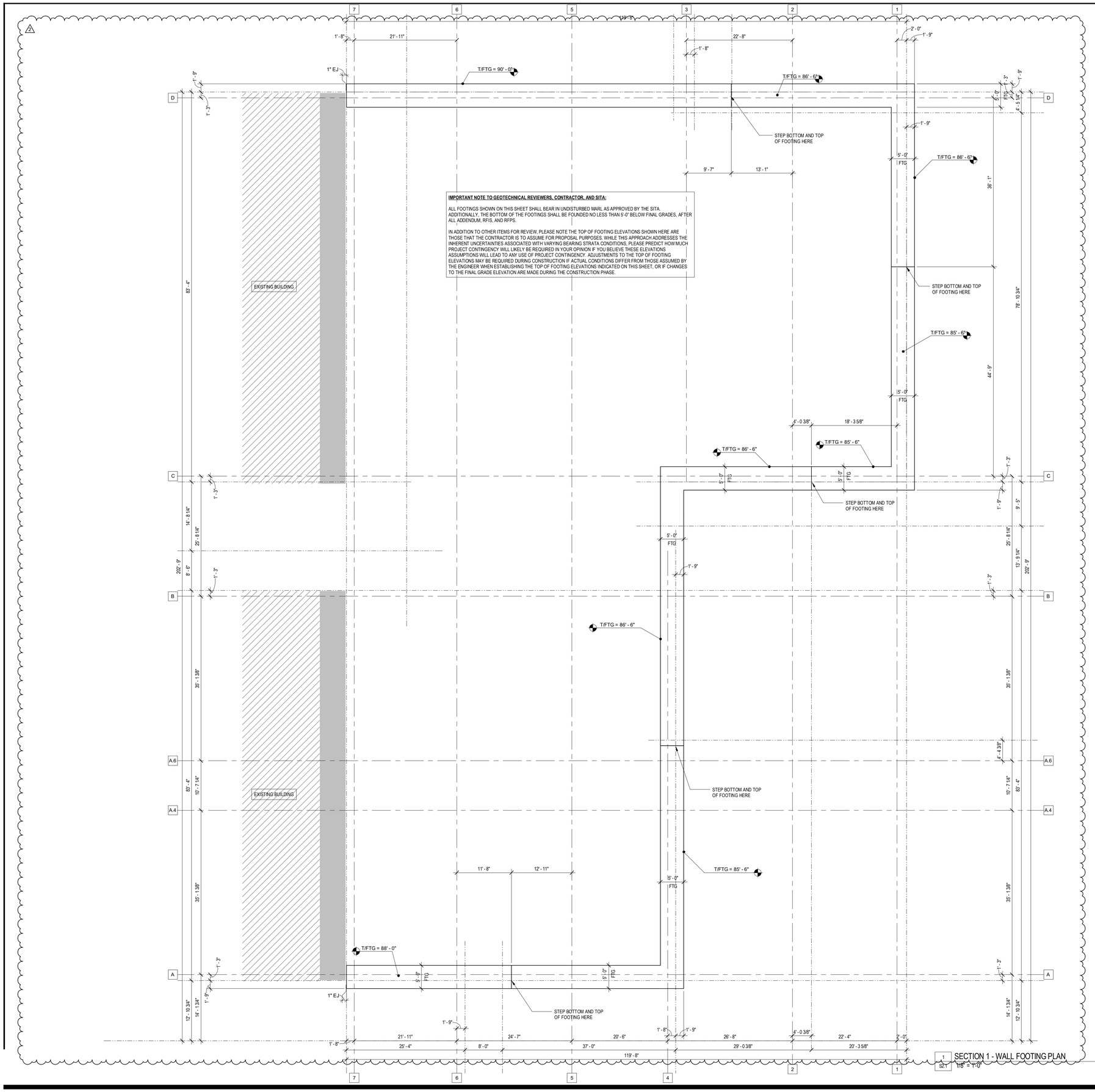

IMPORTANT NOTE TO GEOTECHNICAL REVIEWERS, CONTRACTOR, AND SITA:

ALL FOOTINGS SHOWN ON THIS SHEET SHALL BEAR IN UNDISTURBED MARL AS APPROVED BY THE SITA. ADDITIONALLY, THE BOTTOM OF THE FOOTINGS SHALL BE FOUNDED NO LESS THAN 5'-0" BELOW FINAL GRADES, AFTER ALL ADDENDUM, RFIS, AND RFPS.

IN ADDITION TO OTHER ITEMS FOR REVIEW, PLEASE NOTE THE TOP OF FOOTING ELEVATIONS SHOWN HERE ARE THOSE THAT THE CONTRACTOR IS TO ASSUME FOR PROPOSAL PURPOSES. WHILE THIS APPROACH ADDRESSES THE INHERENT UNCERTAINTIES ASSOCIATED WITH VARYING BEARING STRATA CONDITIONS, PLEASE PREDICT HOW MUCH PROJECT CONTINGENCY WILL LIKELY BE REQUIRED IN YOUR OPINION IF YOU BELIEVE THESE ELEVATIONS ASSUMPTIONS WILL LEAD TO ANY USE OF PROJECT CONTINGENCY. ADJUSTMENTS TO THE TOP OF FOOTING ELEVATIONS MAY BE REQUIRED DURING CONSTRUCTION IF ACTUAL CONDITIONS DIFFER FROM THOSE ASSUMED BY THE ENGINEER WHEN ESTABLISHING THE TOP OF FOOTING ELEVATIONS INDICATED ON THIS SHEET, OR IF CHANGES TO THE FINAL GRADE ELEVATION ARE MADE DURING THE CONSTRUCTION PHASE.

1 S2.1 SECTION 1 - WALL FOOTING PLAN
1/8" = 1'-0"

GRADE BEAM

BUILDING LINE

DFE PLAN LEGEND

Revision /	Date	
2	09/07/18	ADDENDUM 3

Project:
JARRELL MIDDLE SCHOOL ADDITIONS
FOR
JARRELL I.S.D.
JARRELL, TEXAS

STATE OF TEXAS
R. CRAIG McKEE
117909
LICENSED PROFESSIONAL ENGINEER
9.7.2018
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WALL FOOTING PLAN - SECTION 1

Job No.	Sheet No.
1756-02-01	S2.1
Drawn By: MK AM	
Date: 09/07/2018	

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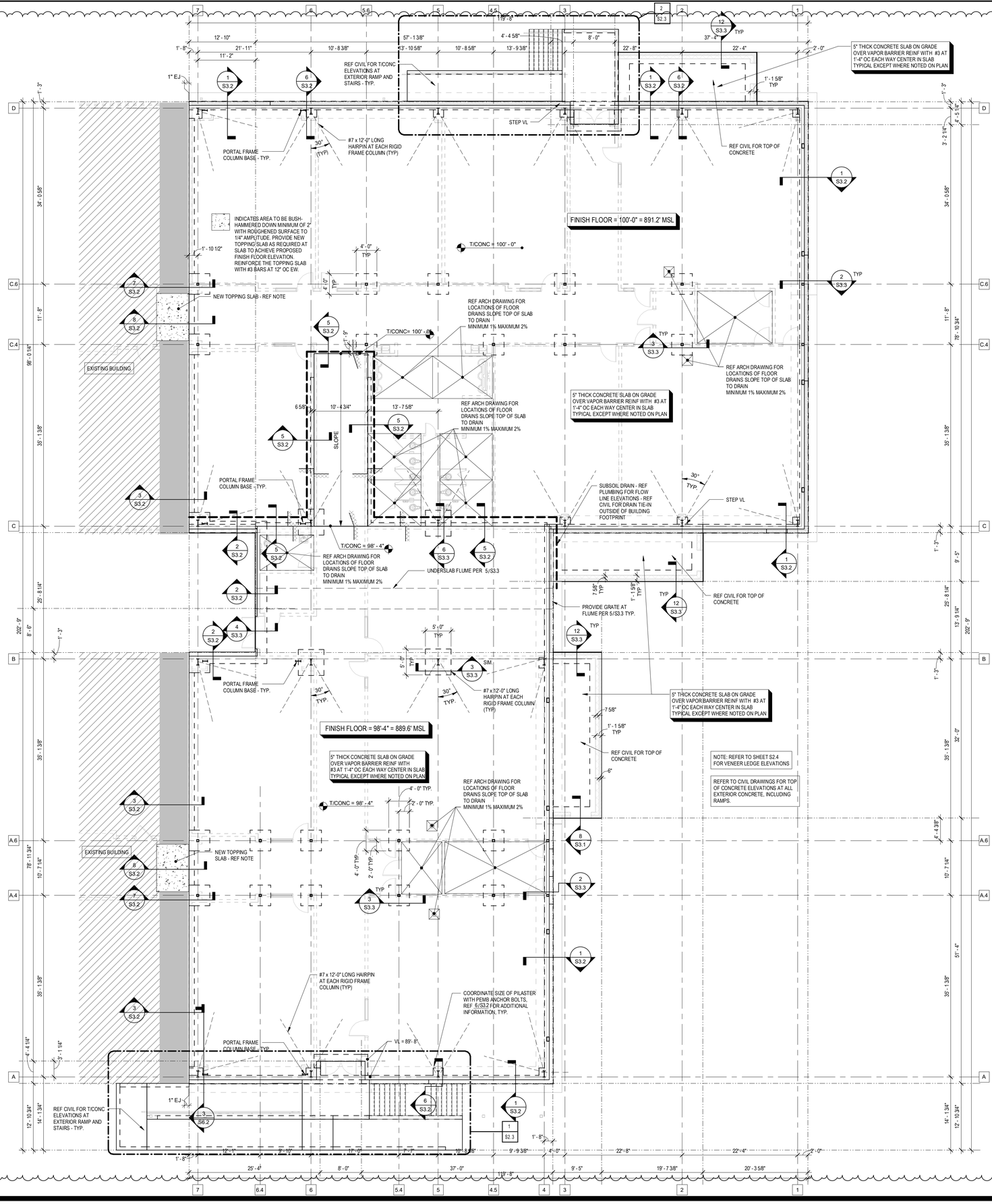


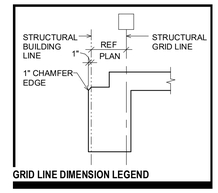
1 SECTION 1 - FOUNDATION PLAN
S2.2 1/8" = 1'-0"

INT NON LOAD-BEARING STUD WALL
INT NON LOAD-BEARING CMU OR STUD WALL
LOAD BEARING OR EXTERIOR NON-LOAD BEARING CMU
GRADE BEAM
FOOTING
SLAB STEP
FD FLOOR DRAIN
FS FLOOR SINK
T/CONC = XX'-XX" T/SLAB ELEVATION
STRUCTURAL STEEL COLUMN
BUILDING LINE

FOUNDATION LEGEND

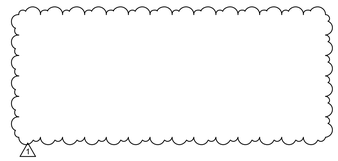
1. FOR GENERAL NOTES, SEE DWG. S0.0.
2. FOR FOUNDATION SECTIONS & DETAILS, SEE S3 SERIES DRAWINGS.
3. SEE ARCH DRAWINGS FOR EXACT LOCATION OF SLAB DEPRESSIONS, OPENINGS AND ENTRY AREAS.
4. SLOPE TO AND BOTTOM OF STRUCTRUAL SLAB AT ALL FLOOR DRAINS, MAINTAINING THE MINIMUM OVERALL CONCRETE SLAB THICKNESS AS SPECIFIED AND THE MINIMUM VOID DEPTH AS SPECIFIED.

FOUNDATION PLAN NOTES



GRID LINE DIMENSION LEGEND

IMPORTANT NOTE:
ALL PRE-ENGINEERED METAL BUILDING (PEMB) COMPONENTS SHOWN ON THIS SHEET ARE ONLY SHOWN HERE FOR CONCEPTUAL PURPOSES, AND ARE ONLY INTENDED TO INDICATE THE DESIGN INTENT OF MAJOR BUILDING COMPONENTS SUCH AS RIGID FRAME LOCATIONS, LOCATIONS OF PORTAL FRAMES, AND APPLICABLE DIMENSIONAL LIMITATIONS OF THESE COMPONENTS. ALL OF THESE COMPONENTS, INCLUDING BUT NOT LIMITED TO ALL MISCELLANEOUS FRAMING ASSOCIATED WITH THESE ITEMS, SHALL BE DESIGNED BY THE CONTRACTOR'S PEMB ENGINEER IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.



1 ADDENDUM 1
Revision / Date
08/30/18

JARRELL MIDDLE SCHOOL ADDITIONS
FOR
JARRELL I.S.D.
JARRELL, TEXAS
Project:

STATE OF TEXAS
R. CRAIG McKEE
111569
REGISTERED
ENGINEER
9.7.2018
F-1252

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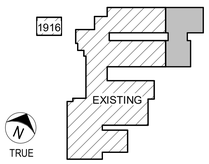

KEY PLAN
N.T.S.

FOUNDATION PLAN - SECTION 1

Job No.	Sheet No.
1756-02-01	S2.2
Drawn By: MK,AM	
Date: 09/07/2018	

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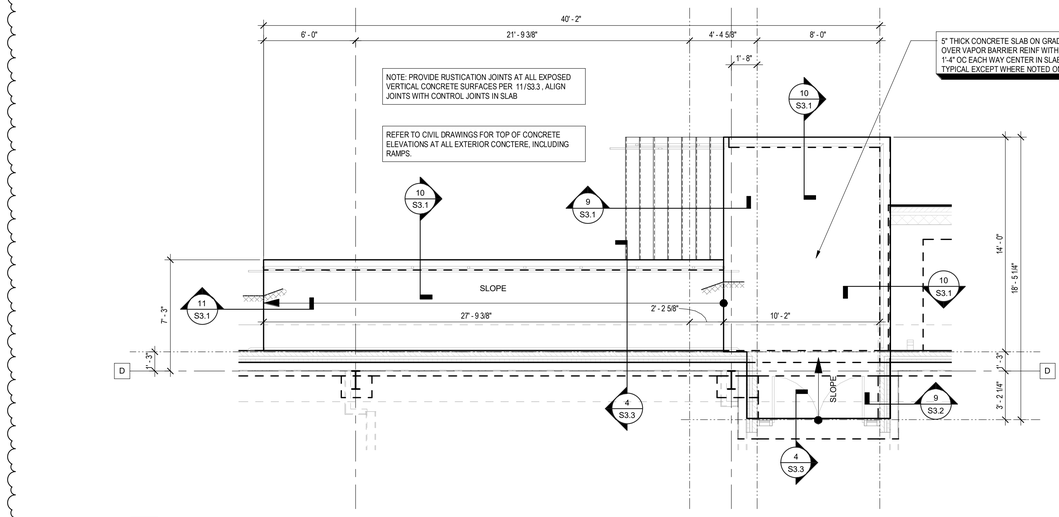
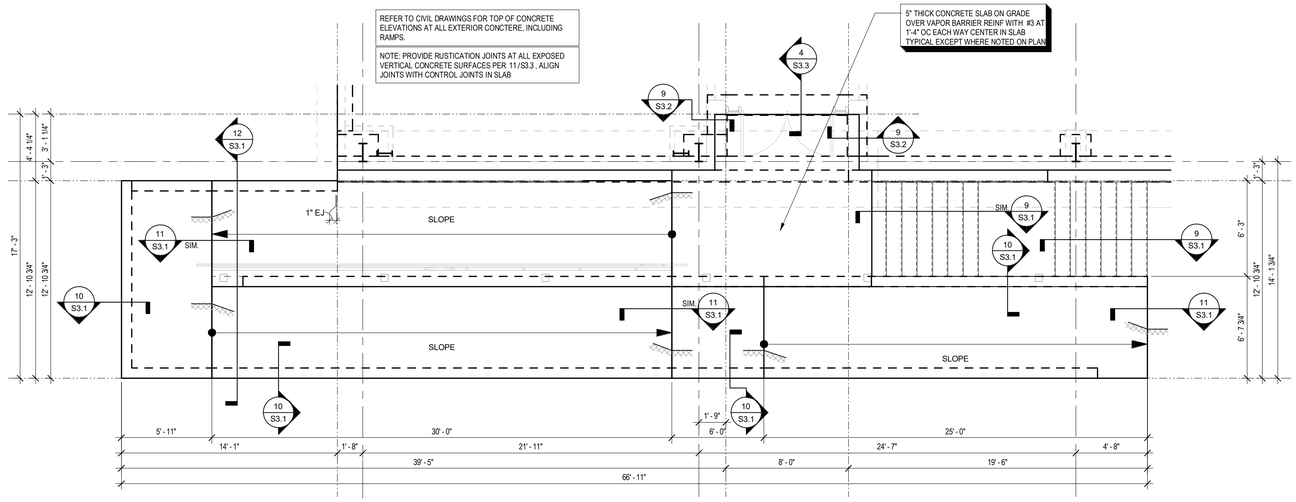
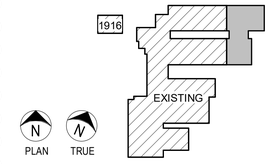


2 S2.3 SECTION 1 - FOUNDATION PLAN - RAMP 2
1/4" = 1'-0"

REF CIVIL FOR ALL TOC ELEVATIONS OUTSIDE OF BUILDING FOOTPRINT



1 S2.3 SECTION 1 - FOUNDATION PLAN - RAMP
1/4" = 1'-0"



KEY PLAN
N.T.S.

N PLAN
N TRUE

Revision /	Date	
2	09/07/18	ADDENDUM 3

Project:
JARRELL MIDDLE SCHOOL ADDITIONS
FOR
JARRELL I.S.D.
JARRELL, TEXAS

STATE OF TEXAS
R. CRAIG McKEE
111669
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ENLARGED RAMP PLANS

Job No.	Sheet No.
1756-02-01	S2.3
Drawn By: MK,KM	
Date: 09/07/2018	

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IMPORTANT NOTE TO CONTRACTOR REGARDING COORDINATION OF VENEER LEDGES:

IN NO CASE SHALL ANY VENEER EXTEND BELOW FINAL GRADE.

IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE THE VENEER LEDGE ELEVATIONS SHOWN ON THIS SHEET WITH ALL OTHER ASSOCIATED DISCIPLINES, SUCH AS ARCHITECTURAL AND CIVIL.

ADJUSTMENTS TO VENEER LEDGE ELEVATIONS MAY BE REQUIRED DURING CONSTRUCTION TO ENSURE NO MASONRY VENEER EXTENDS BELOW FINAL GRADES. IF THE CONTRACTOR BELIEVES ADJUSTMENTS TO THE VENEER LEDGE ELEVATIONS SHOWN HERE ARE REQUIRED, THE CONTRACTOR SHALL SUBMIT AN RFI TO THE ARCHITECT INDICATING AS SUCH.

SECTION 1 - VENEER LEDGE PLAN
S2.4 1/8" = 1'-0"

KEY PLAN
N.T.S.

Revision / 2
Date 09/07/18
ADDENDUM 3

Project:
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FOR
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VENEER LEDGE PLAN

Job No. 1756-02-01
Drawn By: MK,AM
Date: 09/07/2018
Sheet No. S2.4

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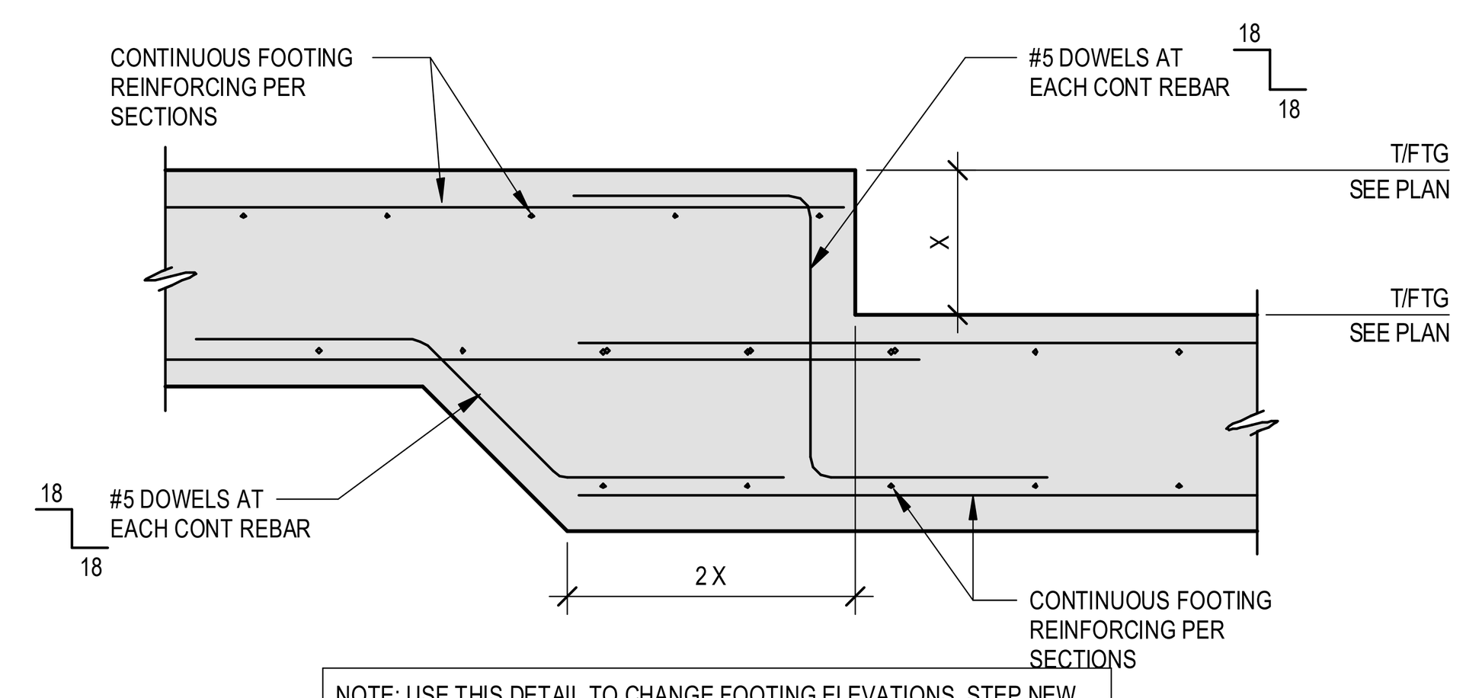
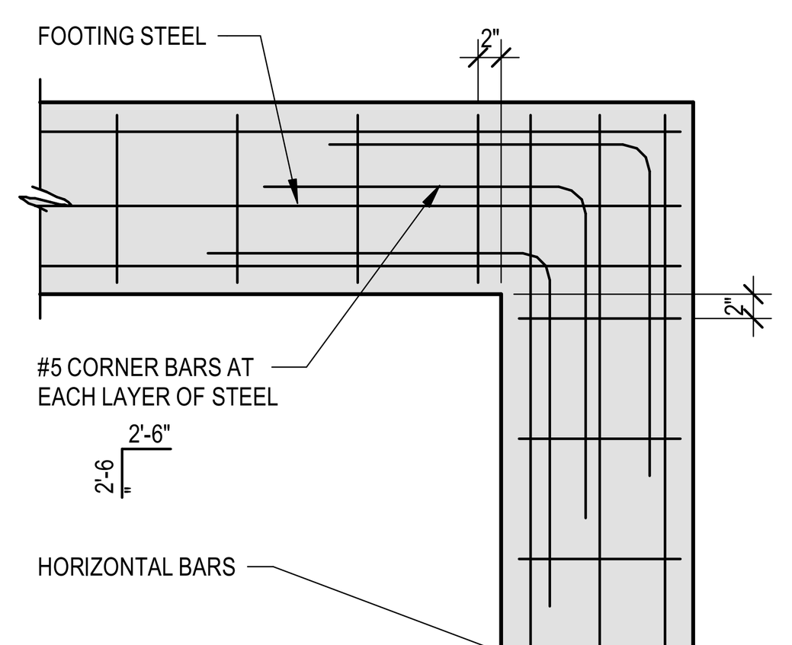
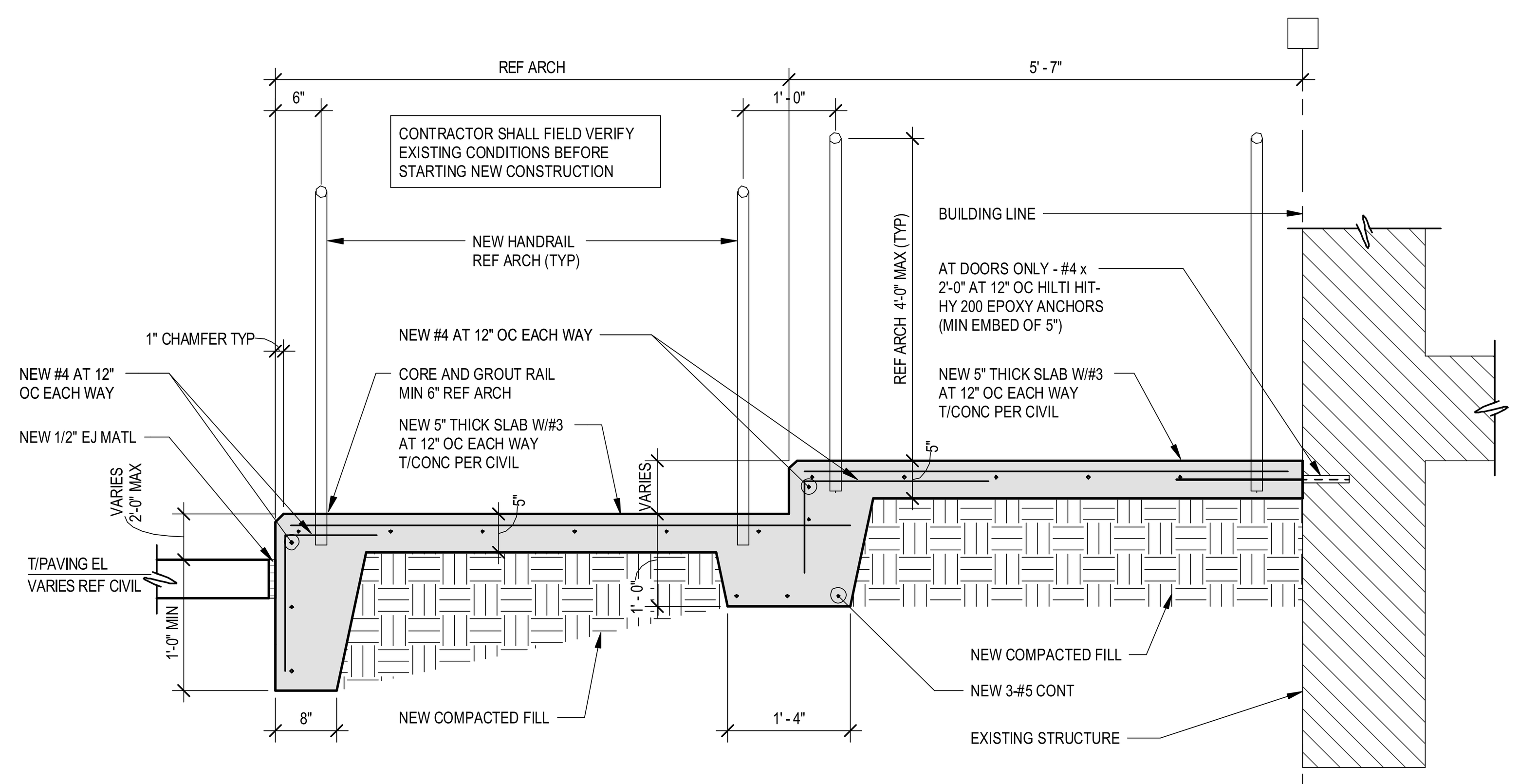


NOTE: USE THIS DETAIL TO CHANGE FOOTING ELEVATIONS, STEP NEW FOOTINGS UP OR DOWN IN RATIO OF TWO HORIZONTALS TO ONE VERTICAL TO JOIN EXISTING FOOTINGS, CAST STEPPED FOOTINGS MONOLITHICALLY, REF FOUNDATION SECTIONS FOR ADDITIONAL INFORMATION

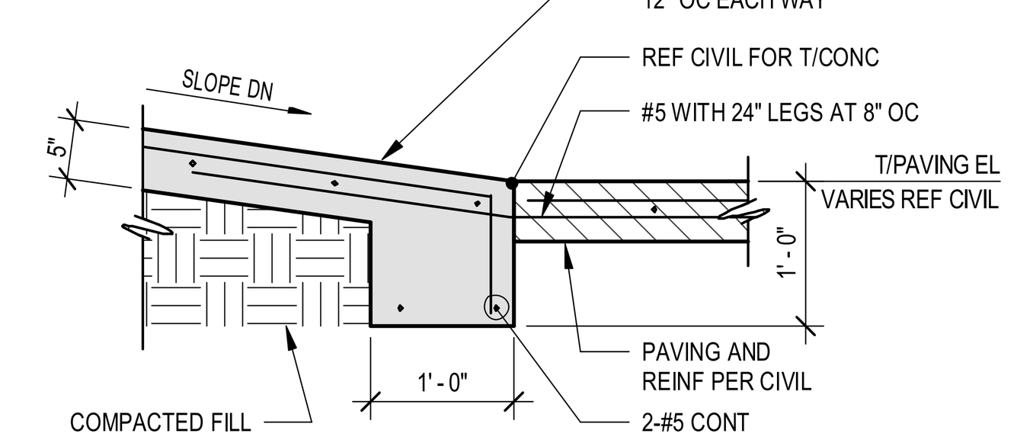
14 TYPICAL FOOTING STEP DETAIL
S3.1 3/4" = 1'-0"



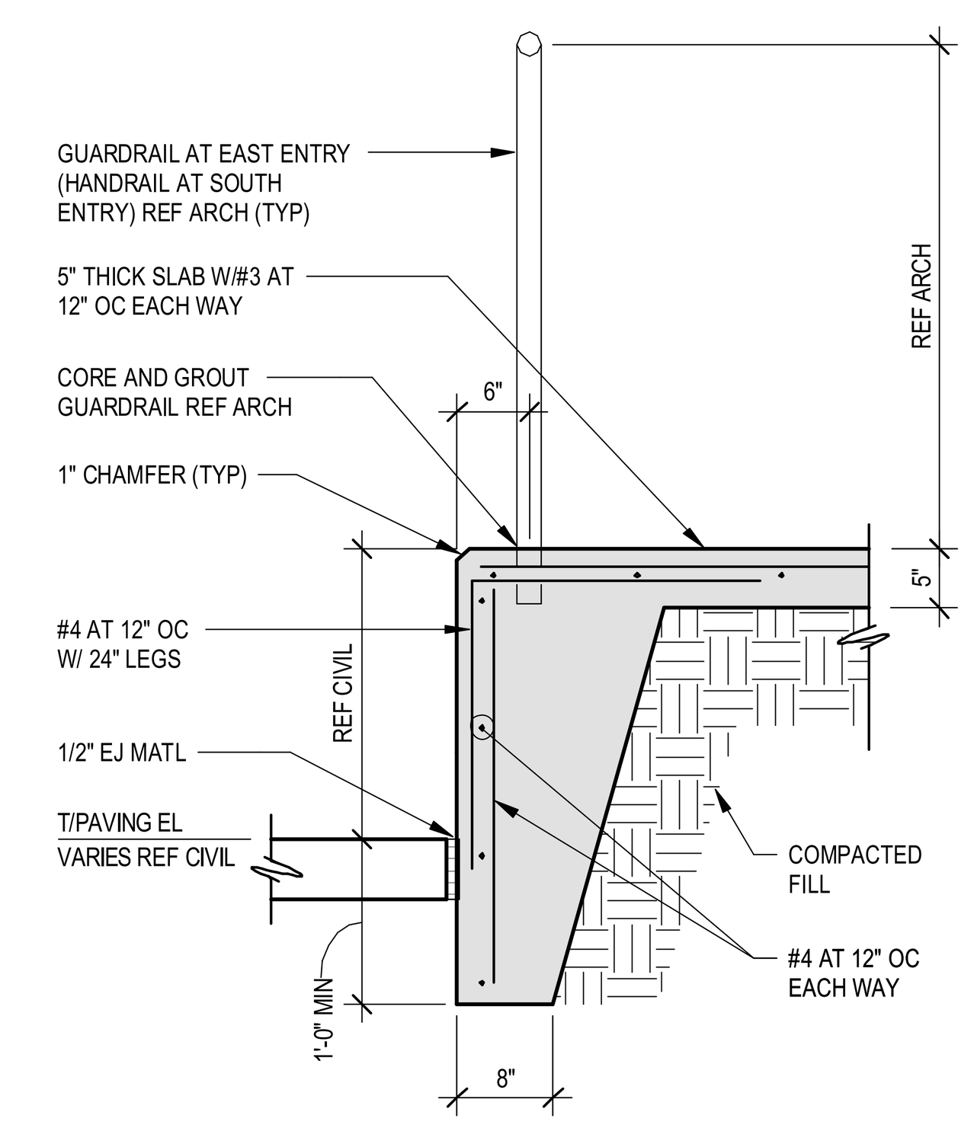
13 TYPICAL ADDITIONAL CORNER BARS AT FOOTING
S3.1 3/4" = 1'-0"



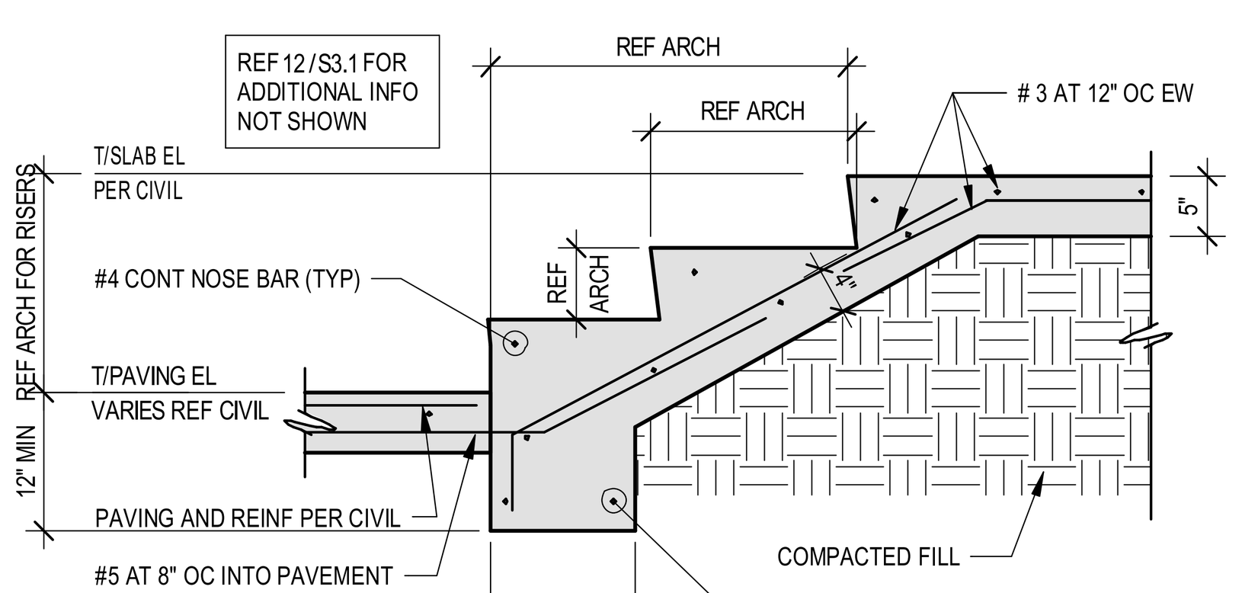
12 SECTION
S3.1 3/4" = 1'-0"



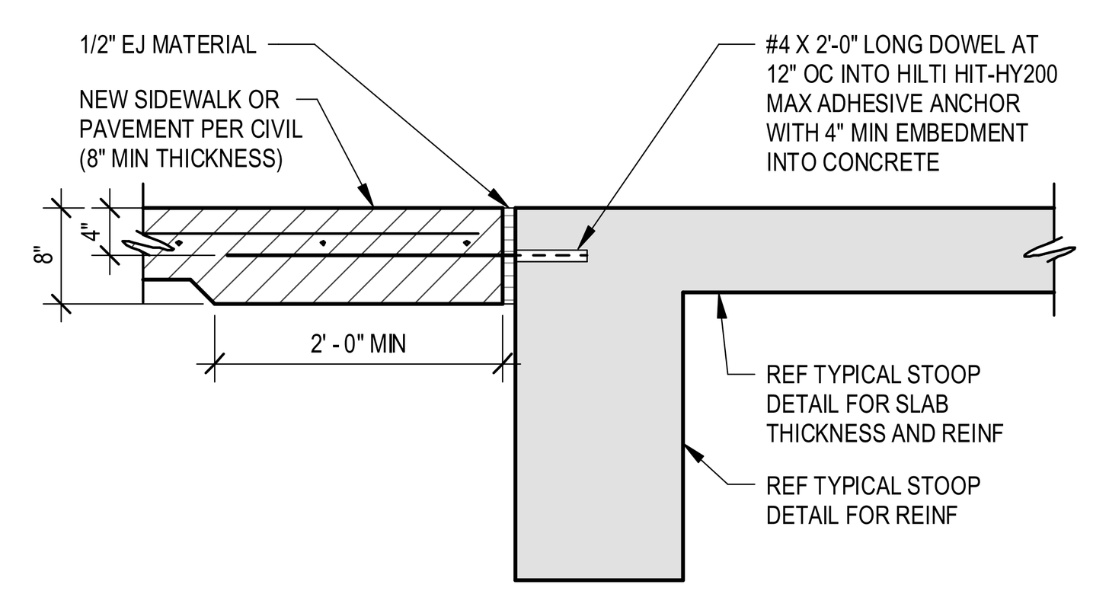
11 SECTION
S3.1 3/4" = 1'-0"



10 SECTION
S3.1 3/4" = 1'-0"

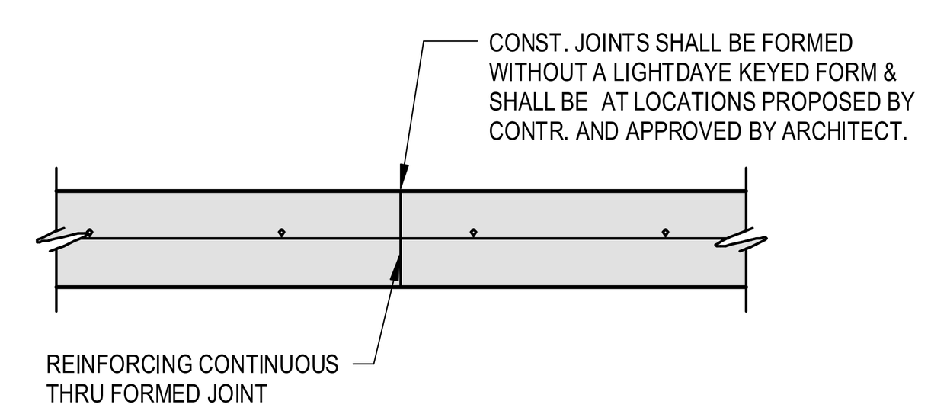


9 SECTION
S3.1 3/4" = 1'-0"



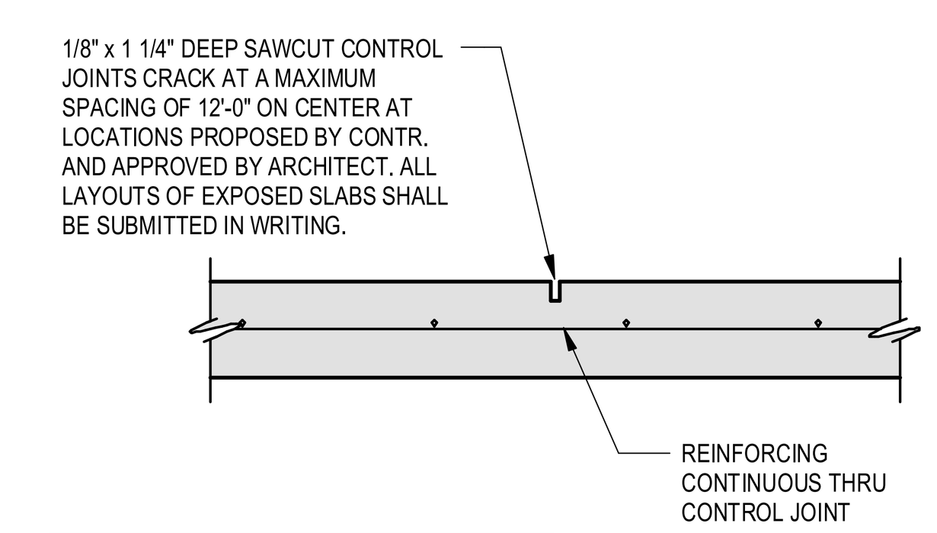
NOTE:
THIS DETAIL SHALL APPLY AT LOCATIONS WHERE PEDESTRIAN TRAFFIC CROSSES THE JOINT AND WHERE NEW CONCRETE SIDEWALK OR PAVEMENT ON GRADE IS ADJACENT TO A NEW OR EXISTING FOUNDATION

8 TYPICAL SLAB TO PAVEMENT DETAIL
S3.1 3/4" = 1'-0"



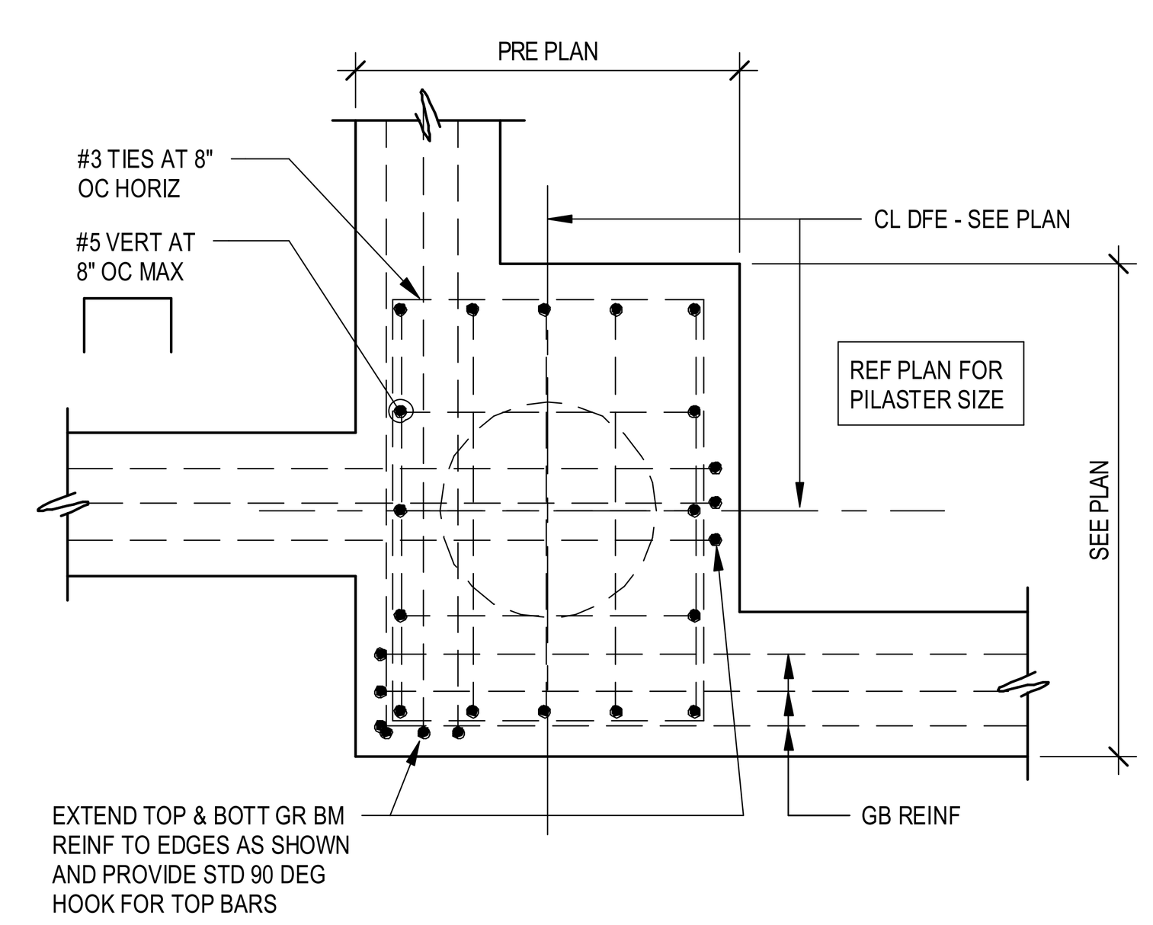
CONSTRUCTION JOINT SHALL BE SMOOTH AND LEVEL WITH NO APPARENT DIFFERENCE IN ELEVATION BETWEEN TWO ADJACENT CONCRETE PLACEMENTS. HIGH AREAS SHALL BE GROUND AND LOW SPOTS SHALL BE FILLED WITH LATEX FLOOR LEVELING COMPOUND.

7 TYPICAL SLAB CONSTRUCTION JOINT DETAIL
S3.1 3/4" = 1'-0"

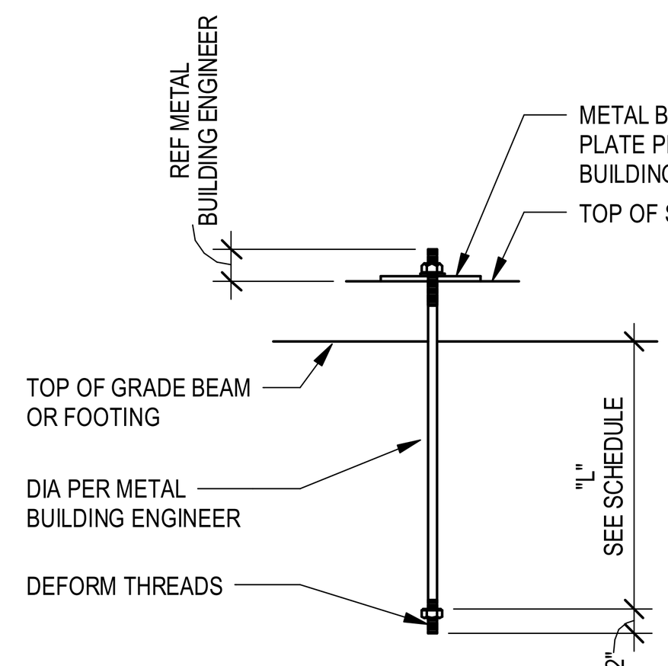


NOTE: SAWCUTS MUST BE INSTALLED WITHIN 4 HRS OF COMPLETING FLOOR FINISHING.

6 TYPICAL SLAB CONTROL JOINT DETAIL
S3.1 3/4" = 1'-0"

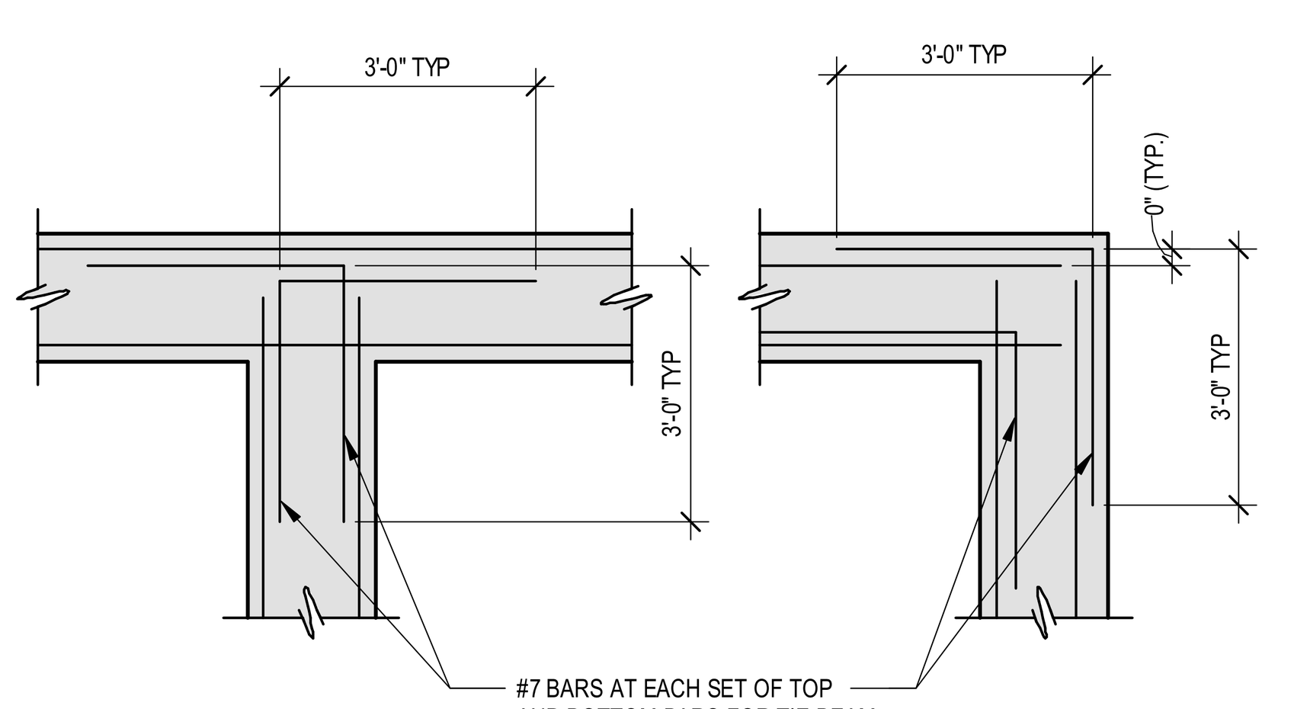


5 TYPICAL REBAR LAYOUT AT PILASTER
S3.1 3/4" = 1'-0"

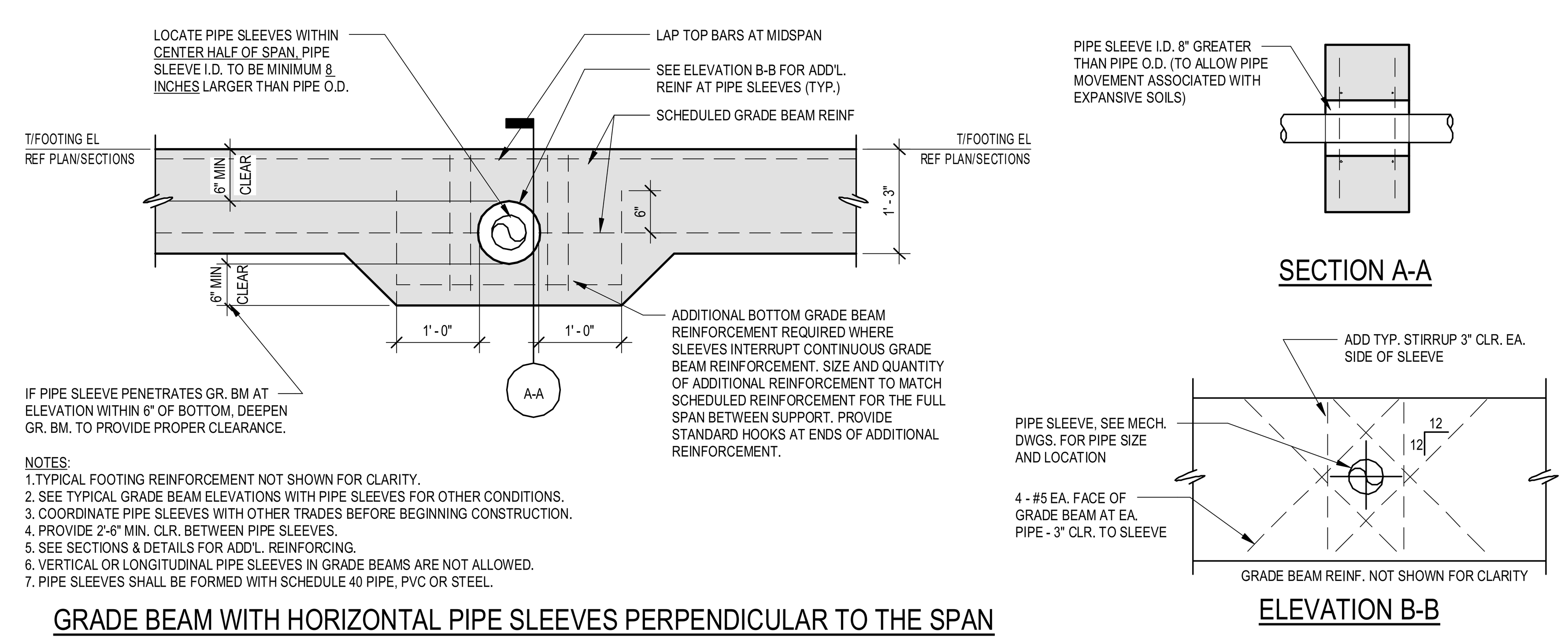


ANCHOR BOLT SCHEDULE	
BOLT DIA	"L" DIM
3/4", 7/8"	12"
1", 1 1/8"	15"
1 1/4"	18"

4 TYPICAL METAL BUILDING ANCHOR ROD DETAIL
S3.1 3/4" = 1'-0"



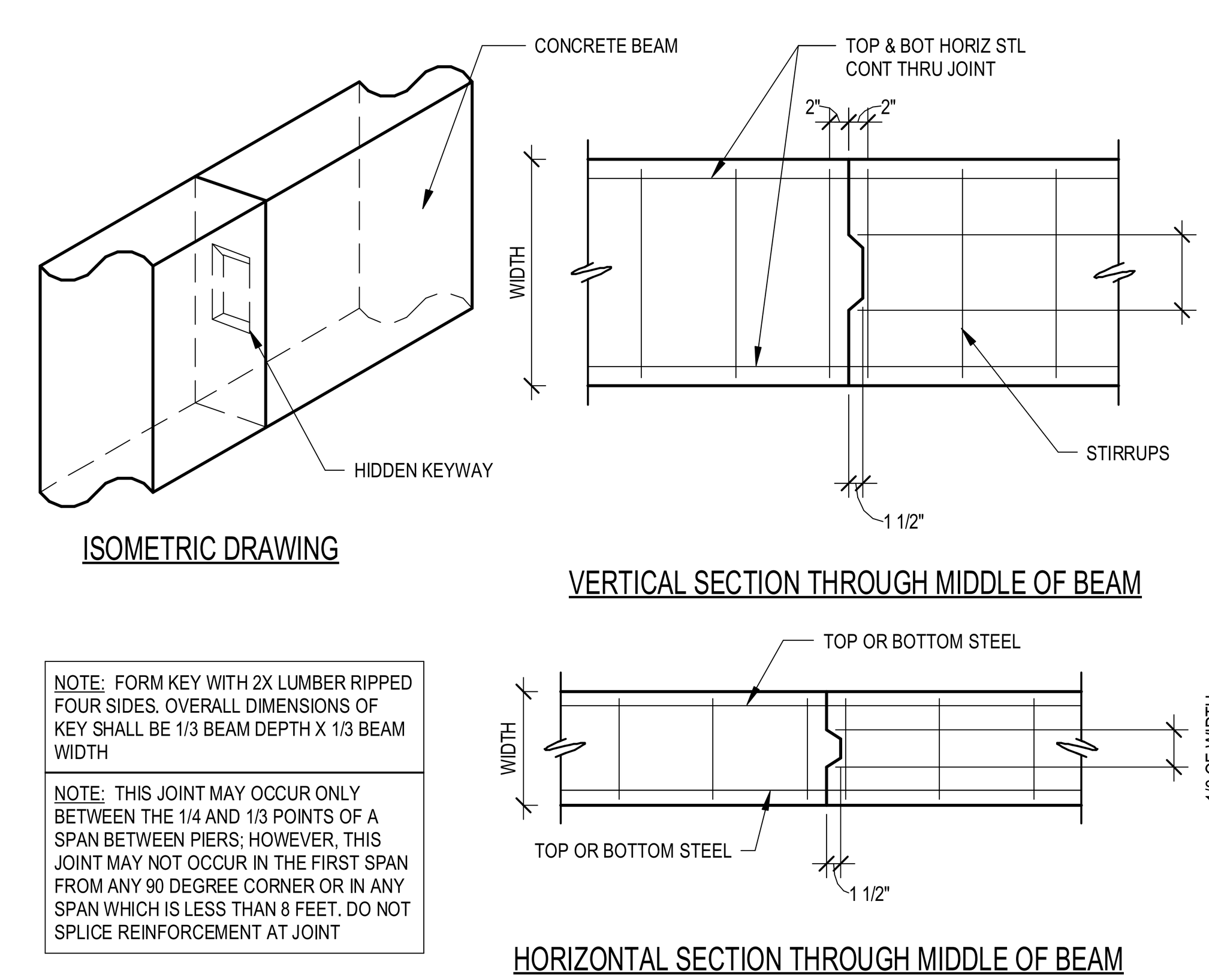
3 TYPICAL ADDITIONAL CORNER BARS AT FOUNDATION WALL INTERSECTIONS
S3.1 3/4" = 1'-0"



NOTES:
1. TYPICAL FOOTING REINFORCEMENT NOT SHOWN FOR CLARITY.
2. SEE TYPICAL GRADE BEAM ELEVATIONS WITH PIPE SLEEVES FOR OTHER CONDITIONS.
3. COORDINATE PIPE SLEEVES WITH OTHER TRADES BEFORE BEGINNING CONSTRUCTION.
4. PROVIDE 2'-6" MIN. CLR. BETWEEN PIPE SLEEVES.
5. SEE SECTIONS & DETAILS FOR ADD'L. REINFORCING.
6. VERTICAL OR LONGITUDINAL PIPE SLEEVES IN GRADE BEAMS ARE NOT ALLOWED.
7. PIPE SLEEVES SHALL BE FORMED WITH SCHEDULE 40 PIPE, PVC OR STEEL.

GRADE BEAM WITH HORIZONTAL PIPE SLEEVES PERPENDICULAR TO THE SPAN

2 TYPICAL GRADE BEAM WITH MID-DEPTH PIPE SLEEVES DETAIL
S3.1 3/4" = 1'-0"



NOTE: FORM KEY WITH 2X LUMBER RIPPED FOUR SIDES. OVERALL DIMENSIONS OF KEY SHALL BE 1/3 BEAM DEPTH X 1/3 BEAM WIDTH

NOTE: THIS JOINT MAY OCCUR ONLY BETWEEN THE 1/4 AND 1/3 POINTS OF A SPAN BETWEEN PIERS; HOWEVER, THIS JOINT MAY NOT OCCUR IN THE FIRST SPAN FROM ANY 90 DEGREE CORNER OR IN ANY SPAN WHICH IS LESS THAN 8 FEET. DO NOT SPLICE REINFORCEMENT AT JOINT

1 TYPICAL FOOTING CONSTRUCTION JOINT DETAIL
S3.1 3/4" = 1'-0"

Revision /	Date	
2	09/07/18	ADDENDUM 3

Project: JARRELL MIDDLE SCHOOL ADDITIONS FOR JARRELL I.S.D. JARRELL, TEXAS

STATE OF TEXAS — R. CRAIG McKEE — 111569 — 9.7.2018 — F-1252

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TYPICAL FOUNDATION DETAILS

Job No. 1756-02-01
Drawn By: MK AM
Date: 09/07/2018
Sheet No. S3.1

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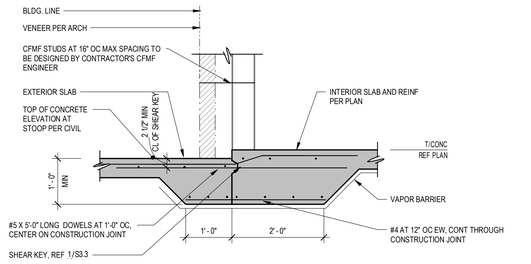
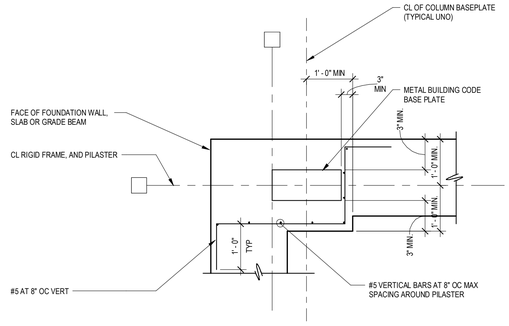
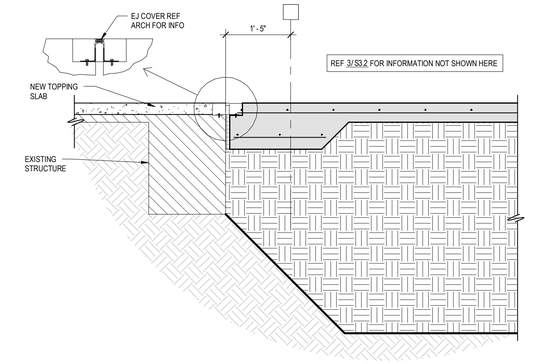


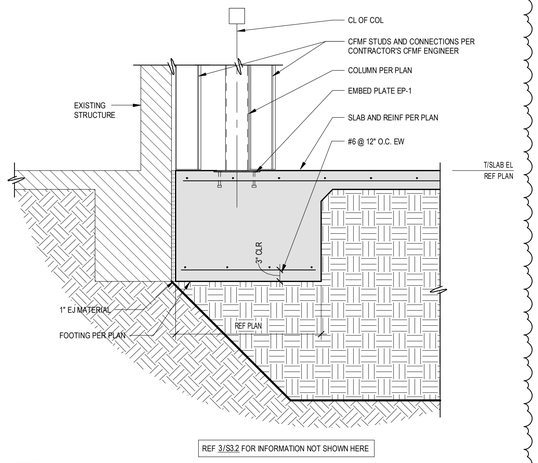
9 S3.2 SECTION AT DOOR
3/4" = 1'-0"



PLAN VIEW AT PILASTER FOR METAL BUILDING COLUMN AT CORNER

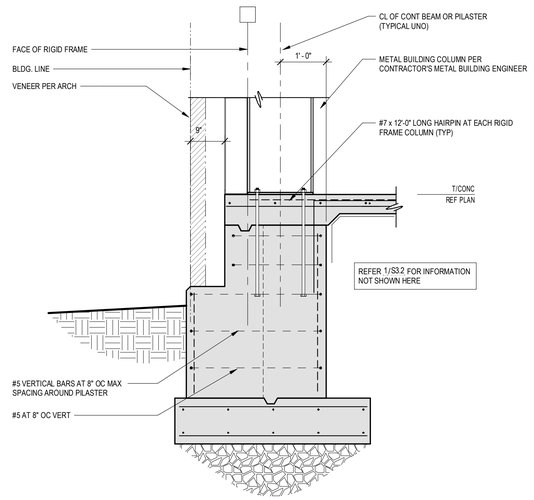


8 S3.2 SECTION
3/4" = 1'-0"



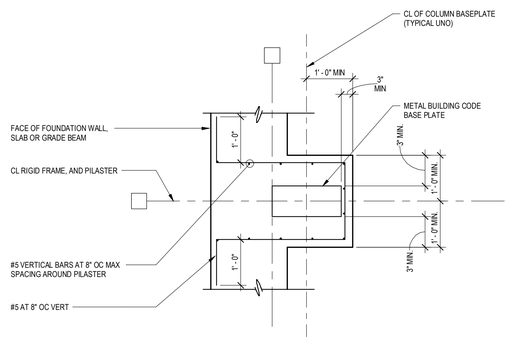
REF 3/S3.2 FOR INFORMATION NOT SHOWN HERE

7 S3.2 SECTION
3/4" = 1'-0"

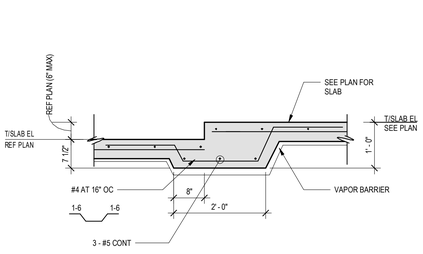


AT PILASTER FOR METAL BUILDING COLUMN

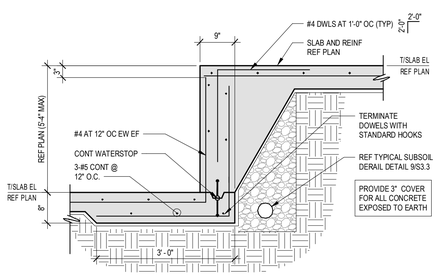
6 S3.2 SECTION
3/4" = 1'-0"



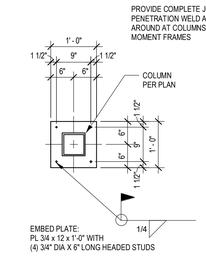
PLAN VIEW AT PILASTER FOR METAL BUILDING COLUMN



CHANGE IN SLAB ELEVATION 6" OR LESS



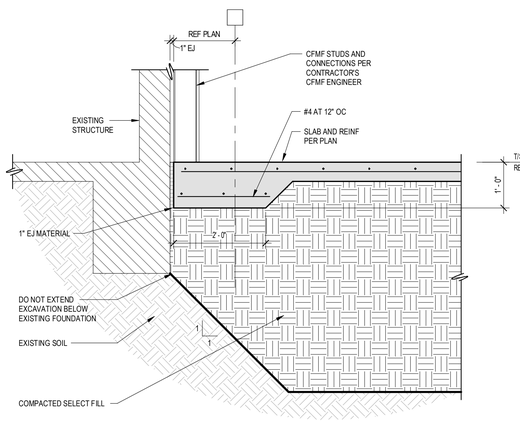
AT CHANGE IN SLAB ELEVATION 6" TO 5'-4" WITH OUT LOAD BEARING ICF OR CMU WALL



EMBED PLATE EP-1

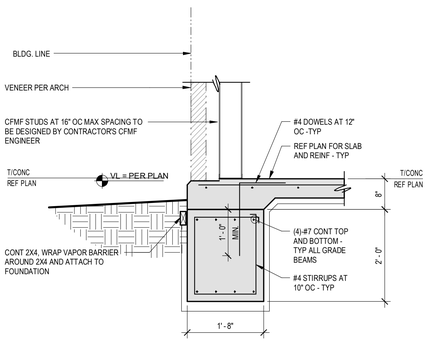
5 S3.2 TYPICAL DETAILS AT CHANGE IN INTERIOR SLAB ELEVATION
3/4" = 1'-0"

4 S3.2 TYPICAL EMBED PLATE
3/4" = 1'-0"



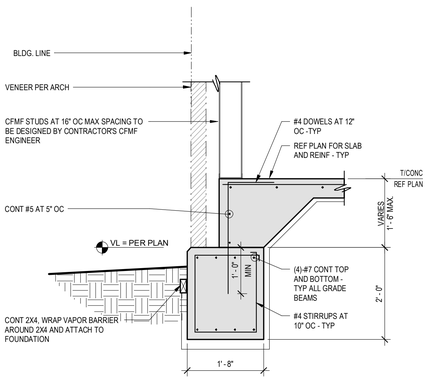
AT VENEER LEDGE ABOVE TOP OF GRADE BEAM

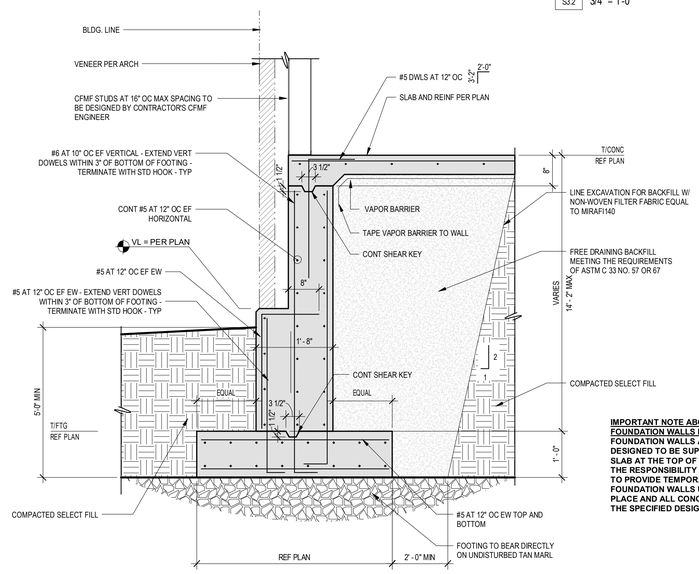
3 S3.2 SECTION
3/4" = 1'-0"



AT VENEER LEDGE BETWEEN 8" AND 1'-6" BELOW FINISH FLOOR

2 S3.2 SECTION
3/4" = 1'-0"





IMPORTANT NOTE ABOUT STABILITY OF FOUNDATION WALLS DURING CONSTRUCTION: FOUNDATION WALLS AND GRADE BEAMS ARE DESIGNED TO BE SUPPORTED BY THE FLOOR SLAB AT THE TOP OF THE WALL. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE TEMPORARY SHORING FOR THE FOUNDATION WALLS UNTIL THE SLAB IS IN PLACE AND ALL CONCRETE HAS REACHED THE SPECIFIED DESIGN STRENGTH.

1 S3.2 TYPICAL PERIMETER FOOTING
3/4" = 1'-0"

Revision / 2 — Date 09/07/18 — ADDENDUM 3

Project: JARRELL MIDDLE SCHOOL ADDITIONS FOR JARRELL I.S.D. JARRELL, TEXAS

STATE OF TEXAS — R. CRAIG McKEE — 111569 — REGISTERED — 9.7.2018 — F-1252

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TYPICAL FOUNDATION DETAILS

Job No. 1756-02-01 | Sheet No. S3.2
Drawn By: MK,AM
Date: 09/07/2018

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M:\2017 Locals\1756-02-01 Jarrell MS Additions_STR\CT_cmckeevt

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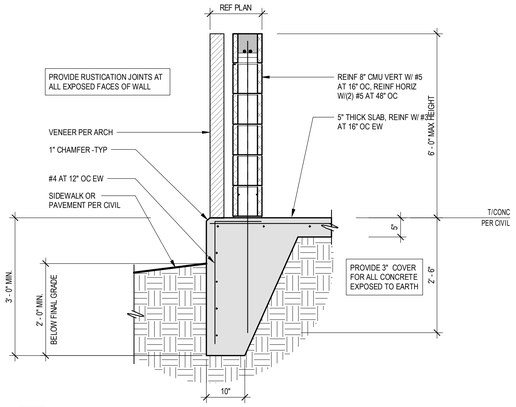
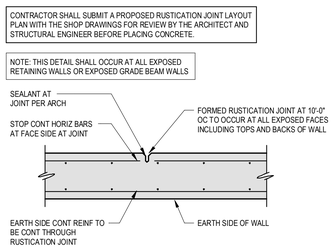
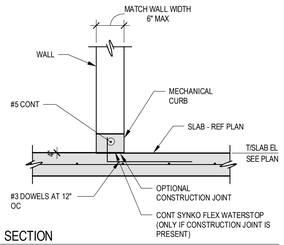


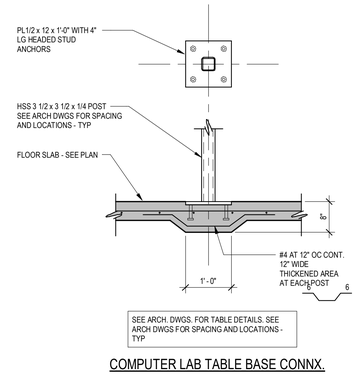
12 S3.3 TYPICAL SECTION AT CMU SCREEN WALL
3/4" = 1'-0"



11 S3.3 TYPICAL RUSTICATION JOINT
3/4" = 1'-0"

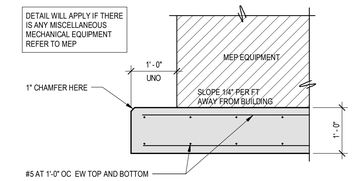


10 S3.3 SECTION
3/4" = 1'-0"

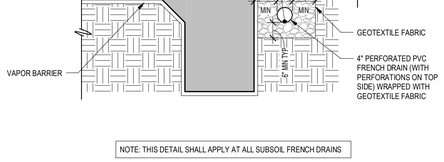


COMPUTER LAB TABLE BASE CONNX.

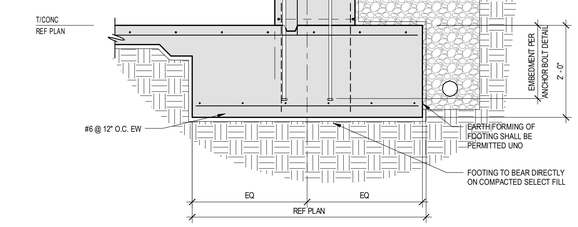
9 S3.3 TYPICAL DETAIL
3/4" = 1'-0"



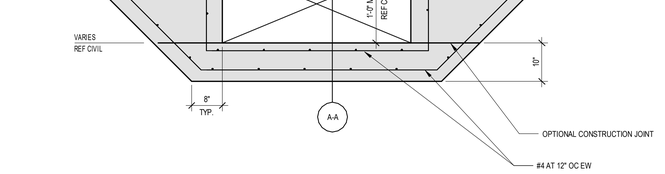
8 S3.3 TYPICAL EQUIPMENT PAD SECTION
3/4" = 1'-0"



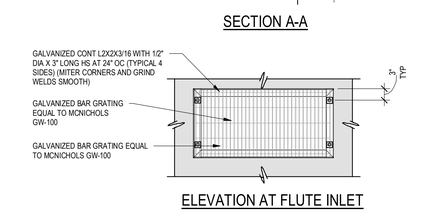
7 S3.3 TYPICAL SUBSOIL DRAIN DETAIL
3/4" = 1'-0"



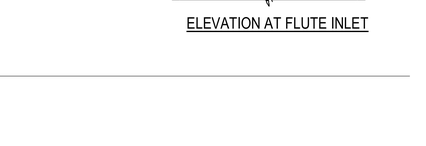
6 S3.3 METAL BUILDING COLUMN AT SLAB STEP
3/4" = 1'-0"



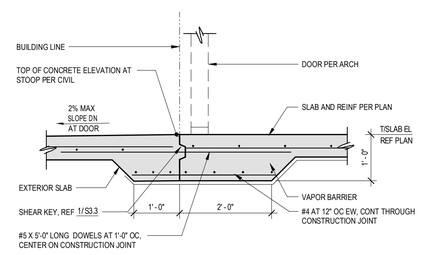
5 S3.3 TYPICAL UNDER SLAB FLUME
3/4" = 1'-0"



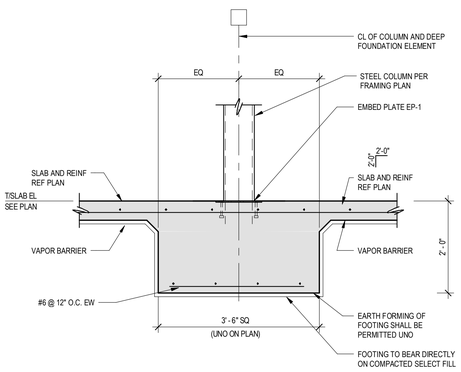
SECTION A-A



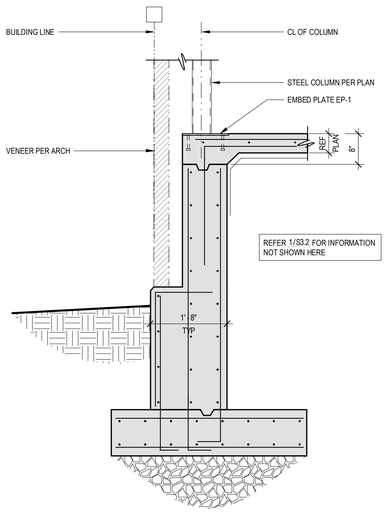
ELEVATION AT FLUTE INLET



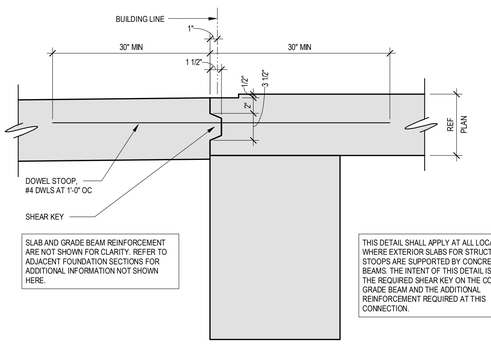
4 S3.3 SECTION
3/4" = 1'-0"



3 S3.3 SECTION
3/4" = 1'-0"



2 S3.3 SECTION
3/4" = 1'-0"



1 S3.3 TYPICAL STOOP SLAB CONNECTION
1 1/2" = 1'-0"

Revision / Date
2 09/07/18 ADDENDUM 3

Project:
JARRELL MIDDLE SCHOOL ADDITIONS
FOR
JARRELL I.S.D.
JARRELL, TEXAS

STATE OF TEXAS
R. CRAIG NICKEL
117589
LICENSED PROFESSIONAL ENGINEER
9.7.2018
F-1252

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FOUNDATION DETAILS

Job No. 1756-02-01
Drawn By: MK,AM
Date: 09/07/2018
Sheet No. S3.3

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