COLORS, MATERIALS & FINISHES LEGEND

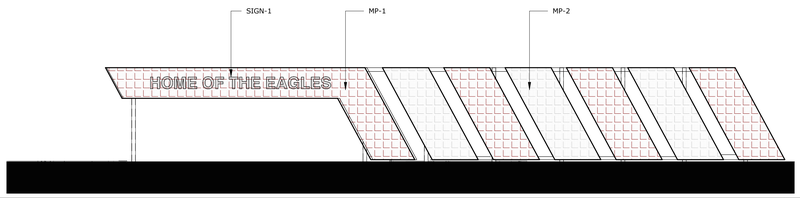
	ITEM	MATERIAL	LOCATION
05 75 10 - PERFORATED METAL			
05 75 10 - PERFORATED METAL	**MP-1**	ASTYL, CUSTOM PERFORTATION FLIGHT A 17% OPEN AREA, 'REDWOOD' SATIN #RST1	ENTRY GATE/LUNCH SHELTER CANOPY @ EXTERIOR PLAZA
05 75 10 - PERFORATED METAL	**MP-2**	ASTYL, CUSTOM PERFORTATION FLIGHT A 17% OPEN AREA, 'SEASHELL WHITE' SATIN #RST1	ENTRY GATE ACCENT @ EXTERIOR PLAZA
05 75 10 - PERFORATED METAL	**MP-3**	MCNICHOLS, PERFORATED METAL ROUND 'GALVANIZED', 5/32" ROUND ON 3/16" STAGGERED CENTERS, 63% OPEN ADD.	ACCENT @ EXTERIOR QUEUING ADD.
09 96 00 - HIGH-PERFORMANCE COATINGS			
09 96 00 - HIGH-PERFORMANCE COATINGS	**P-1**	TNEMEC, 'BLACK' #35GR	EXTERIOR STRUCTURAL STEEL @ ENTRY GATE/LUNCH SHELTER & MARKERBOARDS
10 14 00 - SIGNAGE			
10 14 00 - SIGNAGE	**SIGN-1**	CAST ALUMINUM DIMENSIONAL LETTERING	ENTRY SIGNAGE TEXT
13 31 33 - PRE-ENGINEERED FABRIC SHADE STRUCTURES			
13 31 33 - PRE-ENGINEERED FABRIC SHADE STRUCTURES	**FSS-1**	USA SHADE, FR701 'RED', POLE AND STRUCTURE TO BE BLACK	TYPICAL FABRIC SHADE STRUCTURES
32 13 13 - CONCRETE PAVING			
32 13 13 - CONCRETE PAVING	**CONC-1**	WHITE PORTLAND CEMENT CONCRETE WITH WHITE SILICA SAND MIX	COLLABORATIVE SPACE @ EXTERIOR PLAZA ADD.
32 13 13 - CONCRETE PAVING	**CONC-2**	SIKACOLOR, 140G POWDER INTEGRAL COLOR SG07-860 HEMATITE GRAY SEALED WITH CURSEAL-W	PATH OF TRAVEL @ EXTERIOR PLAZA
32 13 13 - CONCRETE PAVING	**CONC-3**	ORDINARY NATURAL CONCRETE	TYPICAL CONCRETE @ EXTERIOR PLAZA
32 31 19 - DECORATIVE METAL FENCES AND GATES			
32 31 19 - DECORATIVE METAL FENCES AND GATES	**OSF-1**	AMERISTAR, MONTAGE COMMERCIAL, MAJESTIC 'BLACK'	EXTERIOR
32 93 00 - LANDSCAPING			
32 93 00 - LANDSCAPING	**DG-1**	SOUTHWEST BOULDER, DECOMPOSED GRANITE 'GOLDEN WHITE FINES'	TYPICAL DG @ EXTERIOR PLAZA

AGENCY APPROVAL
File No: A.P

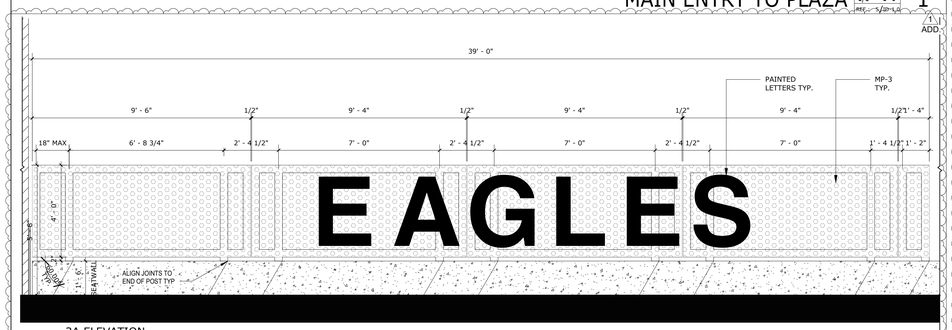
STAMPS

RUHNAU CLARKE ARCHITECTS

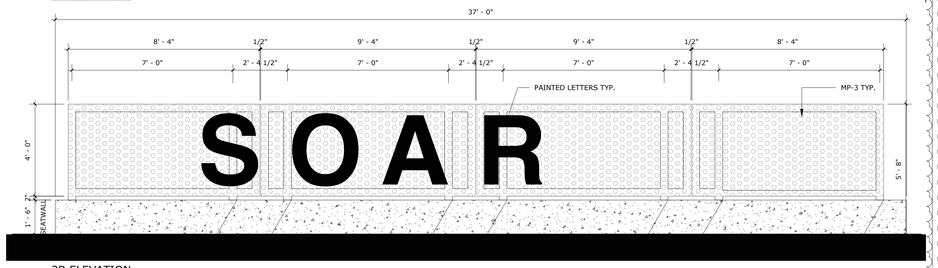
CONSULTANT BRANDING



MAIN ENTRY TO PLAZA — SCALE: 1/8" = 1'-0" — REF: 1/ID-1.0 — 1

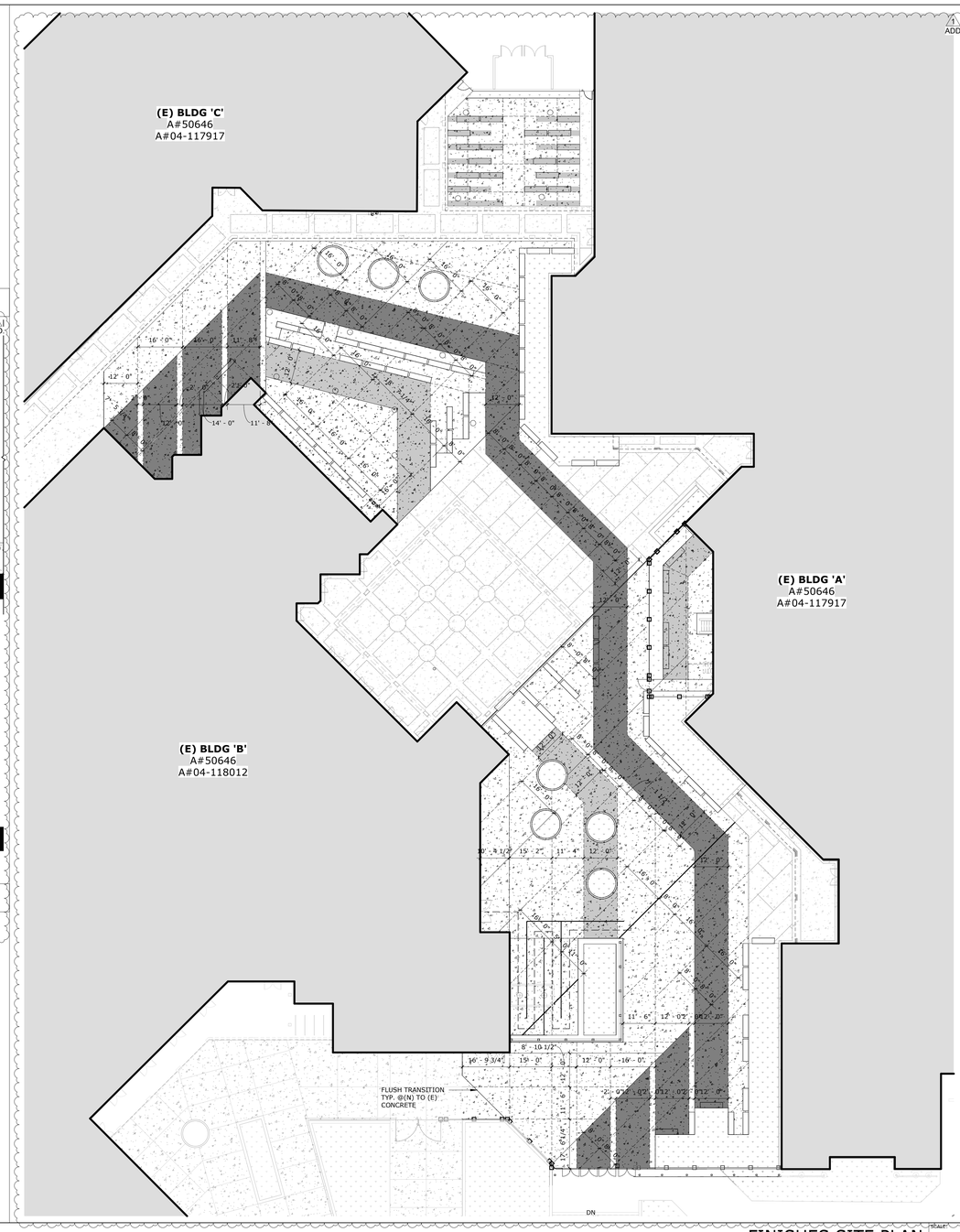


2A ELEVATION



2B ELEVATION

ELEVATION PERFORATED PANEL SIGNAGE @ QUEUEING — SCALE: 3/8" = 1'-0" — REF: 1/AS-1.1 — 2



FINISHES SITE PLAN — SCALE: 1" = 20'-0" — 5

FINISH NOTES

1. ALL FINISHES SHALL COMPLY WITH CBC CHAPTERS 3, 8, & 10, CFC AND CCR TITLE 19 AND STATE OF CALIFORNIA TECHNICAL BULLETIN 117-2013.
2. WALL AND CEILING MATERIALS SHALL NOT EXCEED THE FLAME SPREAD CLASSIFICATIONS IN CBC TABLE 803.13.
3. ALL ADJACENT DISSIMILAR FINISH MATERIALS TO RECEIVE SEALANT PER MFR INSTRUCTIONS, U.N.O.
4. SEE INTERIOR ELEVATION SHEETS FOR ACCENT PAINT LOCATIONS.
5. ALL EXPOSED STEEL, LOUVERS, ACCESS PANELS AND WALL GRILLES TO BE PAINTED TO MATCH ADJACENT SURFACE, U.N.O.
6. TOILET ROOM, TOILET ROOM VESTIBULES, AND FOOD SERVICE AREA WALLS AND CEILINGS NOT COVERED BY EITHER TILE OR F.R.P. TO HAVE MOISTURE RESISTANT GYPSUM BOARD, U.N.O.
7. COORDINATE FINISHES WITH MECHANICAL FOR PLUMBING FIXTURES.
8. FOR EXTENT AND PATTERN OF CEILING FINISH, SEE REFLECTED CEILING PLAN. REFER TO RCP GENERAL NOTE #4 FOR FINISH AT U.O.S.
9. AT COVE BASE, PROVIDE CONTINUOUS 3/8" RADIUS
10. FOR TYPICAL CERAMIC TILE DETAILS, REFER TO DETAIL
11. ALL WALL BASE SHOULD BE INSTALLED USING WHOLE UNITS. NO PIECE SHALL BE SMALLER THAN 1'-0". PREFORMED CORNERS STANDARDIZED.
12. FOR MORE INFORMATION ON COLORS, MATERIALS, & FINISHES, REFER TO SHEET ID-1.0
13. ALL FLOOR FINISHES SHALL BE STABLE, FIRM, & SLIP RESISTANT PER CBC SECTION 11B-302.1.
14. ALL FLOOR FINISHES SHALL EXTEND BELOW ACCESSIBLE WORKSTATIONS, SINKS, ETC. FOR THE REQUIRED TOE/KNEE CLEARANCE.
15. EPOXY FLOORING AT FOOD SERVICE AREAS IS GREASE/ ACID RESISTANT AND USDA APPROVED.
16. REFER TO CORRESPONDING SPECIFICATIONS FOR BURNING CHARACTERISTICS AND TESTING REQ.

FINISH ABBREVIATIONS

ACP	ACOUSTICAL CEILING PANEL (SUSPENDED)
ACT	ACOUSTICAL CEILING TILE
AWP	ACOUSTIC WALL PANEL
CG	CORNER GUARD
CMU	CONCRETE MASONRY UNIT
CONC	CONCRETE
CPL	CEMENT PLASTER
CPT	CARPET
ECO	ECORESIN PANEL
EPOXY	EPOXY FLOOR
EPT	EPOXY TERRAZZO
EXP	EXPOSED
FF	FACTORY FINISH
FRP	FIBERGLASS REINFORCED PANEL
G	GROUT
GPL	GYPSUM PLASTER
GWB	GYPSUM WALLBOARD
GWWB	WATER RESISTANT GWB
LVT	LUXURY VINYL TILE
MESH	METAL MESH SYSTEM
MPS	METAL PANEL SYSTEM
OP	OPERABLE PARTITION
PCPL	PORTLAND CEMENT PLASTER OVER METAL LATH, SAND FINISH
PES	PAINT - EGGSHELL
PL	PLASTIC LAMINATE
PLAS	PLASTER
PF	PAINT, FLAT
PSG	PAINT, SEMI-GLOSS
RB	RUBBER BASE
RES	RESINOUS/EPOXY FLOORING
RF	RUBBER FLOORING
RS	ROLLER SHADE SYSTEM
S	SEAL
SAF	SYNTHETIC ATHLETIC FLOORING
SCW	SOLID CORE WOOD
SEAL	SEALED
SSM	SOLID SURFACE MATERIAL
ST	STAIN WOOD COLOR
STV	STONE TILE VENEER
SV	SHEET VINYL
T	TILE
TWP	TACKABLE WALL PANEL
U.O.S.	UNDERSIDE OF STRUCTURE
VCT	VINYL COMPOSITION TILE
VF	VINYL FABRIC
VWC	VINYL WALL COVERING
WACP	WASHABLE ACOUSTICAL CEILING PANEL (SUSPENDED)
WAF	WOOD ATHLETE FLOORING
WD	WOOD
WDM	WOOD MASIVE FLOORING
WP	WALL PADDING
WS	WALL SYSTEM

FINISHES LEGEND

32 13 13 CONCRETE PAVING

- CONC-1
- CONC-2
- CONC-3

05 75 00 PERFORATED METAL (1 ADD.)

- MP-1
- MP-2
- MP-3

CONTROL JOINTS

- (E) CONTROL JOINTS TO REMAIN
- (N) CONTROL JOINTS
- (N) EXPANSION JOINTS

VALLEY VIEW HIGH SCHOOL PLAZA RENOVATION:

PROJECT No. :14812
6/17/2026 3:12:50 PM

DRAWN BY:		CHECKED BY:			
DELTA # 1	DATE 06/05/2026	☒ ADD	☐ AFO	☐ CCD	☐ REV
DELTA #	DATE	☐ ADD	☐ AFO	☐ CCD	☐ REV
DELTA #	DATE	☐ ADD	☐ AFO	☐ CCD	☐ REV
DELTA #	DATE	☐ ADD	☐ AFO	☐ CCD	☐ REV

RUHNAUCLARKE.COM
3775 TENTH STREET, RIVERSIDE CALIFORNIA 92501 (951) 684 4664 / 5751 PALMER WAY, SUITE C, CARLSBAD CALIFORNIA 92010 (760) 438 5899

VALLEY VIEW HIGH SCHOOL PLAZA RENOVATION
13135 NASON ST. MORENO VALLEY, CA 92555
MORENO VALLEY UNIFIED SCHOOL DISTRICT

COLORS, MATERIALS COLOR MATERIAL AND FINISHES SITE PLAN

ID-1.0

14812

PLAN SYMBOLS

SYMBOL	DESCRIPTION
SLV	APPROXIMATE LOCATION OF SLEEVE FOR PIPES/ CONDUIT ACROSS FOOTING PER PLUMBING, ELECTRICAL OR MECHANICAL
BOF -1'-0"	BOTTOM OF FOOTING ELEVATION AT NOTED LOCATION
S S	STEP IN FOOTING
F1	FOOTING PER FOOTING SCHEDULE
PC1 CC1	PILE CAP OR COLUMN CAP PER APPLICABLE SCHEDULE
DP1	DRIVEN PILE OR DRILLED PIER PER APPLICABLE SCHEDULE
GB1	GRADE BEAM PER GRADE BEAM SCHEDULE
6"	STEP IN SLAB ELEVATION, DIMENSION AS INDICATED ON PLAN OR DETAIL
CJ	SLAB CONSTRUCTION/ CONTROL JOINT
C1	STEEL COLUMN STARTING AT THIS LEVEL PER COLUMN SCHEDULE. WHERE NO COLUMN TYPE IS INDICATED, COLUMN CONTINUES FROM LEVEL BELOW.
W8x45 HSS6x6x1/8	STEEL COLUMN STARTING AT THIS LEVEL. WHERE NO COLUMN TYPE IS INDICATED, COLUMN CONTINUES FROM LEVEL BELOW.
P1 6x	WOOD POST STARTING AT THIS LEVEL PER POST SCHEDULE
6x6 6x	WOOD POST STARTING AT THIS LEVEL PER POST SCHEDULE
HD1 HD9	HOLDOWN ANCHOR TYPE AS NOTED
A 10	SHEAR WALL PANEL, HOLDOWN TYPE, AND MINIMUM LENGTH AT THIS LEVEL PER TYPICAL SHEAR WALL ELEVATION
BF1	LOCATION OF BRACE FRAME AT THIS LEVEL, SEE ELEVATION OR DETAILS FOR SPECIFIC INFORMATION
	LOCATION OF BRACE FRAME BELOW
TOS 21'-0"	TOP OF STEEL
TOS 21'-0"	TOP OF PLYWOOD SHEATHING
CJ	CMU WALL CONTROL JOINT WHERE OCCURS
1	CMU WALL BOUNDARY REINFORCING PER APPLICABLE SCHEDULE
S1	RAISED CONCRETE SLAB PER SLAB SCHEDULE
W12x25	BEAM SIZE AND SPAN
B1	BEAM SPAN, SIZE PER SCHEDULE
(C=1 1/2")	DIMENSION OF MID-SPAN CAMBER
[-3"]	ELEVATION OF T/O BEAM ABOVE OR BELOW TYPICAL T/O FRAMING ELEVATION
[18]	NUMBER OF COMPOSITE BEAM SHEAR STUDS EQUALLY SPACED ALONG BEAM SPAN WHEN OTHER THAN REQUIRED PER PLAN NOTES
	SPLICE IN FRAMING MEMBER
	WOOD HEADER, SIZE PER SCHEDULE
	STEEL BEAM MOMENT CONNECTION
	STEEL BEAM-TO-COLUMN MOMENT FRAME CONNECTION
	(1)-BOLT ROW STEEL DRAG BEAM CONNECTION
	(2)-BOLT ROWS STEEL DRAG BEAM CONNECTION
	WELDED STEEL DRAG BEAM CONNECTION
	MOMENT CONNECTION WITH (1)-BOLT ROW STEEL DRAG CONNECTION
	MOMENT CONNECTION WITH (2)-BOLT ROWS STEEL DRAG CONNECTION
	MOMENT CONNECTION WITH WELDED STEEL DRAG CONNECTION
	STEEL COLLECTOR BEAM CONNECTION
(S)	FRAMING MEMBER PART OF THE SEISMIC LATERAL FORCE RESISTING SYSTEM
DT	WOOD DRAG TRUSS
	SEISMIC/ WIND DRAG STRAP
	DIAPHRAGM TIE PER PLAN NOTES
2x12 FJ1 RJ1	WOOD JOIST FRAMING SPAN, SIZE PER PLAN OR APPLICABLE SCHEDULE
D1	DIRECTION AND/OR TYPE OF METAL DECK PER PLAN, DETAIL, DECK SCHEDULE OR PLAN NOTES
	OPENINGS IN FLOOR OR ROOF PER ARCHITECTURAL OR MECHANICAL DRAWINGS
	MECHANICAL UNIT ABOVE FRAMING (SOLID) OR BELOW FRAMING (DASHED) PER MECHANICAL DRAWINGS
	HORIZONTAL DIAGONAL BRACING AT THIS LEVEL
	DIAGONAL BRACING AT THIS LEVEL
	DIAGONAL BRACING BELOW THIS LEVEL

ABBREVIATIONS:

CODES/INSTITUTIONS/ASSOCIATIONS

ACI	AMERICAN CONCRETE INSTITUTE
AF&PA	(NDS) AMERICAN FOREST & PAPER ASSOCIATION (NDS)
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISI	AMERICAN IRON AND STEEL INSTITUTE
APA-EWA	APA-ENGINEERED WOOD ASSOCIATION
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AWS	AMERICAN WELDING SOCIETY
CBC	CALIFORNIA BUILDING CODE
CRSI	CONCRETE REINFORCING STEEL INSTITUTE
DSA	DIVISION OF THE STATE ARCHITECT
HCAI	DEPARTMENT OF HEALTH CARE ACCESS AND INFORMATION
IBC	INTERNATIONAL BUILDING CODE
NDS	NATIONAL DESIGN SPECIFICATIONS
SDI	STEEL DECK INSTITUTE
SJI	STEEL JOIST INSTITUTE
SSMA	STEEL STUD MANUFACTURERS ASSOCIATION

SYMBOLS

#	POUND, NUMBER, QUANTITY	LBS	POUNDS
@	AT	ld	DEVELOPMENT LENGTH
<	LESS THAN	LDGR	LEDGER
>	GREATER THAN	ldh	HOOK DEVELOPMENT LENGTH
±	PLUS OR MINUS	LG	LONG
°	DEGREE	LL	LIVE LOAD
Ø	DIAMETER	LLH	LONG LEG HORIZONTAL
		LLV	LONG LEG VERTICAL

ABBREVIATIONS

		LOCS	LOCATIONS
AB	ANCHOR BOLT	LONG	LONGITUDINAL
ABV	ABOVE	ls	LAP SPLICE LENGTH
ADDL	ADDITIONAL	LSH	LONG SIDE HORIZONTAL
ADH	ADHESIVE	LSV	LONG SIDE VERTICAL
ADJ	ADJACENT	LTWT	LIGHT-WEIGHT
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL	LVL	LEVEL
		LVL	LAMINATED VENEER LUMBER
ALT	ALTERNATE	LWT	LIGHT-WEIGHT
ALUM	ALUMINUM		
ANCH	ANCHOR, ANCHORAGE	MANUF	MANUFACTURER
APPROX	APPROXIMATE, APPROXIMATELY	MATL	MATERIAL
ARCH	ARCHITECT, ARCHITECTURAL	MAX	MAXIMUM
ATR	ALL-THREAD ROD	MECH	MECHANICAL
		MEZZ	MEZZANINE
B/O	BOTTOM OF	MFR	MANUFACTURER
BEL	BELOW	MHP	MHP STRUCTURAL ENGINEERS
BLDG	BUILDING	MID	MIDDLE
BLKG	BLOCKING, BLOCK	MIN	MINIMUM
BM	BEAM	MISC	MISCELLANEOUS
BN	BOUNDARY NAIL	MULT	MULTIPLE
BOT	BOTTOM		
BRBF	BUCKLING RESTRAINED BRACE FRAME	(N)	NEW
BRG	BEARING	N/A	NOT APPLICABLE
BS	BOUNDARY SCREW	NAT	NATURAL
BSMT	BASEMENT	NIC	NOT IN CONTRACT
BTWN	BETWEEN	NTS	NOT TO SCALE
		NWT	NORMAL-WEIGHT
CALCS	CALCULATIONS		
CANT	CANTILEVER	O/F	OUTSIDE FACE
CFRP	CARBON FIBER REINFORCED POLYMER	OC	ON CENTER
CG	CENTER OF GRAVITY	OD	OUTSIDE DIAMETER
CIP	CAST IN PLACE	OH	OPPOSITE HAND
CJ	CONTROL JOINT	OP	OPERATING
CL	CENTERLINE	OPER	OPERATING
CLG	CEILING	OPNG	OPENING
CLR	CLEAR, CLEARANCE	OPP	OPPOSITE
CMU	CONCRETE MASONRY UNIT	OSB	ORIENTED STRAND BOARD
COL	COLUMN		
CONC	CONCRETE	PARA	PARALLEL
CONN	CONNECT, CONNECTION	PERP	PERPENDICULAR
CONT	CONTINUOUS	PL	PLATE, PROPERTY LINE
CJP	COMPLETE JOINT PENETRATION	PLF	POUNDS PER LINEAR FOOT
COORD	COORDINATE	PLMB	PLUMBING
CSK	COUNTERSINK, COUNTERSUNK	PLY	PLYWOOD
CTR	CENTER	PJP	PARTIAL JOINT PENETRATION
		PROJ	PROJECT, PROJECTION
d	PENNY (NAIL SIZE)	PSF	POUNDS PER SQUARE FOOT
db	BAR DIAMETER (REBAR)	PSI	POUNDS PER SQUARE INCH
DBL	DOUBLE	PSL	PARALLAM
DCW	DEMAND CRITICAL WELD	PT	POST-TENSIONED (CONCRETE)
DET	DETAIL	PT	PRESSURE-TREATED (WOOD)
DFL	DOUGLAS FIR LARCH		
DIA	DIAMETER	QTY	QUANTITY
DIAG	DIAGONAL		
DIM	DIMENSION	R	RADIUS
DIR	DIRECTION	RAD	RADIUS
DIST	DISTANCE	REF	REFER TO, REFERENCE
DN	DOWN	REINF	REINFORCING
DL	DEAD LOAD	REQD	REQUIRE, REQUIRED
DO	DITTO	RET	RETAINING
DWG	DRAWING	REV	REVISION
		RF	ROOF
(E)	EXISTING	RM	ROOM
EA	EACH	RR	ROOF RAFTER
EBF	ECCENTRIC BRACED FRAME		
EF	EACH FACE	SCHED	SCHEDULE
ELEC	ELECTRICAL	SECT	SECTION
ELEV	ELEVATION, ELEVATOR	SEL	SELECT
EMB	EMBED, EMBEDDED, EMBEDMENT	SEOR	STRUCTURAL ENGINEER OF RECORD
EMBED	EMBED, EMBEDDED, EMBEDMENT	SHT	SHEET
EN	EDGE NAIL	SHTG	SHEATHING
ENGR	ENGINEER	SIM	SIMILAR
EOR	ENGINEER OF RECORD	SIMP	SIMPSON STRONG-TIE COMPANY
EQ	EQUAL, EQUALLY	SMRF	SPECIAL MOMENT-RESISTING FRAME
EQUIP	EQUIPMENT	SMS	SHEET METAL SCREW
ES	EDGE SCREW, EACH SIDE	SOG	SLAB-ON-GRADE
EW	EACH WAY	SPCL	SPECIAL
EXC	EXCAVATE, EXCAVATION	SPA	SPACED, SPACING
EXP	EXPANSION	SPCS	SPACES
EXT	EXTERIOR	SPEC	SPECIFICATION
		SQ	SQUARE
FDN	FOUNDATION	SS	STAINLESS STEEL
FIN	FINISH	STAGG	STAGGER
FLNG	FLANGE	STD	STANDARD
FLR	FLOOR	STIFF	STIFFEN, STIFFENER
FN	FIELD NAIL	STIRR	STIRRUP
F/O	FACE OF	STL	STEEL
FRMG	FRAMING	STRUC	STRUCTURAL
FS	FIELD SCREW	SW	SHEAR WALL
FTG	FOOTING	SYM	SYMMETRICAL
FTR	FULLY-THREADED ROD		
		T&B	TOP & BOTTOM
GA	GAUGE	T&G	TONGUE & GROOVE
GALV	GALVANIZE	TBD	TO BE DETERMINED
GC	GENERAL CONTRACTOR	TEMP	TEMPERATURE, TEMPORARY
GLB	GLUED-LAMINATED BEAM	THK	THICK, THICKNESS
GR	GRADE	THRU	THROUGH
		TN	TOE NAIL
HD	HOLDOWN	T/O	TOP OF
HDR	HEADER	TRANS	TRANSVERSE
HEX	HEXAGONAL	TS	TUBE STEEL
HGR	HANGER	TSG	TAPERED STEEL GIRDER
HT	HEIGHT	TYP	TYPICAL
HORIZ	HORIZONTAL		
HR	HOUR (FIRE RATING)	UNO	UNLESS NOTED OTHERWISE
I/F	INSIDE FACE	VERT	VERTICAL
ID	INSIDE DIAMETER	V/F	VERIFY IN FIELD
INFO	INFORMATION		
INSP	INSPECTION	W/	WITH
INT	INTERIOR, INTERSECTION	W/O	WITHOUT
INV	INVERT, INVERTED	WP	WORK POINT
IOR	INSPECTOR OF RECORD	WT	WEIGHT
		WWF	WELDED WIRE FABRIC
JST	JOIST		
JT	JOINT	STRUCTURAL STEEL SHAPES	
		Cx	STANDARD CHANNEL
K	KIPS (1000#)	HSS	HOLLOW STRUCTURAL SECTIONS
KB-TZ2	HILTI KWIK BOLT TZ2 (ANCHOR)	Lx	ANGLE
KP	KING POST	MCx	MISCELLANEOUS CHANNEL
KSF	KIPS PER SQUARE FOOT	Mx	M SHAPES
KSI	KIPS PER SQUARE INCH	STD PIPE	STANDARD PIPE
		Sx	S SHAPES
		WTx, STx	STRUCTURAL TEES
		Wx	W SHAPES
		X-STRG	EXTRA STRONG PIPE
		XX-STRG	DBL EXTRA STRONG PIPE

GENERAL STRUCTURAL NOTES:

1. DESIGN CRITERIA: THE FOLLOWING DESIGN PARAMETERS HAVE BEEN USED IN THE DESIGN OF THE STRUCTURAL, AND WHERE APPLICABLE, NON-STRUCTURAL SYSTEMS REPRESENTED IN THESE STRUCTURAL DRAWINGS.

   A. CODE OF RECORD: 2022 CALIFORNIA BUILDING

   B. AUTHORITY HAVING JURISDICTION: THE DIVISION OF THE STATE ARCHITECT (DSA) (REFERENCE TO THE "AUTHORITY HAVING JURISDICTION" OR "GOVERNING AGENCY" THROUGHOUT THESE STRUCTURAL DRAWINGS SHALL BE CONSIDERED TO BE THE AUTHORITY LISTED ABOVE.)

   C. DESIGN LOADS (VERTICAL/GRAVITY):
   - FREE-STANDING FENCE:_____ DL: 5 PSF

   THESE LOAD VALUES REPRESENT THE ANTICIPATED DEAD LOADS AND ALLOWABLE SUPERIMPOSED LIVE LOADS FOR WHICH THE STRUCTURE HAS BEEN DESIGNED, ASSUMING THE NOTED MATERIALS STRENGTHS ARE ACHIEVED AND THE STRUCTURE HAS BEEN FULLY ERECTED AND SECURED. MINOR ADJUSTMENTS MAY HAVE BEEN USED FOR SPECIAL DESIGN CASES WHERE MORE REFINED VALUES ARE REQUIRED.

   D. BUILDING RISK CATEGORY:
   - FREE-STANDING FENCE:_____ II

   E. WIND DESIGN PARAMETERS:
   - ULTIMATE (LFRD) DESIGN WIND SPEED (V 3 SEC. GUST): 96 MPH
   - WIND DIRECTIONALITY FACTOR, (KD):_____ 0.85
   - EXPOSURE CATEGORY:_____ C
   - TOPOGRAPHIC FACTOR, (KZT):_____ 1.0
   - GUST EFFECT FACTOR:_____ 0.85
   - ENCLOSURE CLASSIFICATION:_____ OPEN
   - INTERNAL PRESSURE COEFFICIENT, (GCPI):_____ ±0.18
   - ULTIMATE DESIGN WIND PRESSURE:
   - MWFRS:_____ 30 PSF

   F. SEISMIC DESIGN PARAMETERS:
   - SEISMIC SITE CLASS:_____ D (DEFAULT)
   - SITE COEFFICIENTS, SPECTRAL ACCELERATION PARAMETERS:
     - S_S: 1.965
     - S_1: 0.776
   - DESIGN SPECTRAL ACCELERATION PARAMETERS:
     - S_{DS}: 1.572
     - S_{D1}: 0.880
   - SEISMIC IMPORTANCE FACTOR:_____ 1.0
   - SEISMIC DESIGN CATEGORY:_____ E

   FREESTANDING FENCE

   G. SEISMIC DESIGN PARAMETERS:
   - BASIC SEISMIC FORCE-RESISTING SYSTEM: GROUND-SUPPORTED CANTILEVER WALLS OR FENCES
   - RESPONSE MODIFICATION COEFFICIENT, (R):_____ 1.25
   - OVER-STRENGTH FACTOR, (Ω0):_____ 2.0
   - DEFLECTION AMPLIFICATION FACTOR, (CD):_____ 2.5
   - ANALYSIS PROCEDURE: SEISMIC DESIGN REQUIREMENTS FOR NON-BUILDING STRUCTURES (ASCE 7-16, CHAPTER 15)
   - SEISMIC BASE SHEAR COEFFICIENT, (CS, I.E. %G):_____ 1.258
   - REDUNDANCY (ρ): N-S=1.0, E-W=1.0
   - SEISMIC BASE ELEVATION:_____ 00.00'
   - HORIZONTAL IRREGULARITY:_____ NONE
   - VERTICAL IRREGULARITY:_____ NONE

2. ASTM DESIGNATIONS, ICC REPORT REFERENCES, AND ALL STANDARDS AND REFERENCED DOCUMENTS SHALL REFER TO THE LATEST EDITIONS AND/OR AMENDMENTS AS ADOPTED BY THE CODE OF RECORD.

3. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE PROJECT. WHERE SPECIFIC DETAILS ARE NOT PROVIDED, THE GENERAL NOTES AND TYPICAL DETAILS SHALL GOVERN. HOWEVER, WHEN A SPECIFIC DETAIL OR NOTE HAS BEEN PROVIDED, REFERENCED OR CALLED OUT ON PLAN, THAT DETAIL OR NOTE SHALL GOVERN FOR THE SPECIFIC CASE. IN ADDITION, WHEN A DETAIL IS REFERENCED AS SIMILAR (SIM) TO A SPECIFIC DETAIL, OR WHEN IT CAN BE REASONABLY INFERRED THAT A SITUATION IS SIMILAR TO A SPECIFIC DETAIL, THAT SPECIFIC DETAIL SHALL BE ASSUMED TO APPLY UNTIL OTHERWISE DIRECTED BY THE ENGINEER OF RECORD.

4. THESE STRUCTURAL DRAWINGS ARE AN INTEGRAL PART OF, AND RELIANT ON, INFORMATION CONTAINED WITHIN THE ENTIRE SET OF CONSTRUCTION DOCUMENTS PREPARED FOR THE PROJECT. AS SUCH, IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND SUBCONTRACTORS TO REVIEW AND COORDINATE INFORMATION CONTAINED IN THE CIVIL/ARCH/MEP AND SPECIALTY CONSULTANT DRAWINGS AND SPECIFICATIONS TO ENSURE A FULL UNDERSTANDING OF THE PROJECT REQUIREMENTS. ANY OMISSIONS AND/OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER AND THE CONTRACTOR SHALL NOT PROCEED WITH THE AFFECTED WORK UNTIL CLARIFICATION AND/OR DIRECTION HAS BEEN GIVEN.

5. PRIOR TO STARTING WORK ON THE PROJECT AND/OR ON A SPECIFIC AREA OF THE PROJECT, THE CONTRACTOR SHALL CHECK ALL DIMENSIONS. DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE COORDINATED FROM THE ARCHITECTURAL AND OTHER CONSULTANT DRAWINGS. WHERE DIMENSIONS DIFFER BETWEEN PLANS, THE ARCHITECT AND STRUCTURAL ENGINEER SHALL BE NOTIFIED AND THE CONTRACTOR SHALL NOT PROCEED WITH THE AFFECTED WORK UNTIL DIRECTION HAS BEEN PROVIDED. WRITTEN DIMENSIONS SHALL GOVERN OVER SCALES SHOWN ON THE PLANS.

6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES. THE ARCHITECTURAL AND MEP DRAWINGS SHALL BE USED FOR THE SIZE AND LOCATION OF ALL OPENINGS EXCEPT AS SPECIFICALLY CONTROLLED BY THE STRUCTURAL DRAWINGS), INTERIOR NON-BEARING PARTITIONS, CONCRETE CURBS, SLOPES, DEPRESSIONS, CHANGES IN LEVEL, FINISHES, CHAMFERS OR GROOVES, FLOOR AND ROOF DRAINS, INSERTS, SLEEVES, PIPES, CONDUITS AND OTHER NON-STRUCTURAL ITEMS.

7. UNLESS SPECIFICALLY NOTED ON THE STRUCTURAL PLANS OR REFERENCED DETAILS, DO NOT PLACE MECHANICAL, ELECTRICAL (INCLUDING LOW-VOLTAGE AV), AND PLUMBING (MEP) SLEEVES, PIPES, INSERTS OR CONDUIT IN OR THROUGH STRUCTURAL MEMBERS, REGARDLESS OF MATERIAL, WITHOUT PRIOR APPROVAL AND DIRECTION BY THE STRUCTURAL ENGINEER. EXCEPT AS SPECIFICALLY SHOWN ON PLAN, NO STRUCTURAL MEMBER (BEAM, COLUMN, SHEAR WALL, STRUCTURAL SLAB, GRADE BEAM, BRACE, ETC.) SHALL BE CUT, DRILLED, OR NOTCHED WITHOUT PRIOR AUTHORIZATION AND DIRECTION FROM THE STRUCTURAL ENGINEER AND AUTHORITY HAVING JURISDICTION.

8. ALL INFORMATION SHOWN ON THE DRAWINGS RELATIVE TO EXISTING CONDITIONS IS GIVEN AS THE BEST CURRENT KNOWLEDGE BUT WITHOUT GUARANTEE OF ACCURACY. WHERE THE ACTUAL CONDITIONS VARY FROM THAT SHOWN IN THESE DRAWINGS, THEY SHALL BE REPORTED TO THE ARCHITECT AND STRUCTURAL ENGINEER SO THAT PROPER CLARIFICATION OR REVISION MAY BE MADE. THE CONTRACTOR SHALL NOT PROCEED WITH THE AFFECTED WORK UNTIL CLARIFICATION AND/OR DIRECTION HAS BEEN GIVEN.

9. THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OR SEQUENCE OF CONSTRUCTION, UNLESS SPECIFICALLY NOTED OTHERWISE. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING BUT NOT LIMITED TO BRACING, SHORING, AND LAY DOWN OF MATERIALS.

10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DETERMINATION OF LOADS AND EVALUATION OF THE LOADING EFFECTS ON THE STRUCTURE FROM TEMPORARY CONSTRUCTION CONDITIONS, INCLUDING FROM EQUIPMENT SUCH AS SKIP LOADERS, SCISSOR LIFTS, ETC., FROM HORIZONTAL WIND AND SEISMIC FORCES IMPOSED DURING THE CONSTRUCTION SCHEDULE, ALONG THE PATH-OF-TRAVEL FOR MOVING PERMANENT EQUIPMENT TO ITS FINAL LOCATION, AND FROM TEMPORARY LOADING THAT MAY OCCUR DURING EQUIPMENT INSTALLATION. THE CONTRACTOR MAY USE THE "DESIGN LOADS" INFORMATION PROVIDED IN NOTE #1 ABOVE WHEN CONSIDERING TEMPORARY CONSTRUCTION LOADING CONDITIONS ON A COMPLETED RAISED FLOOR OR ROOF SYSTEM.

11. OBSERVATION VISITS TO THE SITE BY REPRESENTATIVES OF THE STRUCTURAL ENGINEER SHALL NOT BE DEEMED TO INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR OF THE CONSTRUCTION PROCEDURES. OBSERVATION VISITS AND CONSTRUCTION SUPPORT SERVICES ARE PROVIDED BY THE STRUCTURAL ENGINEER OR HIS/HER REPRESENTATIVE SOLELY FOR THE PURPOSE OF ASSISTING THE OWNER IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH THE CONTRACT DOCUMENTS. THESE SUPPORT SERVICES SHALL NOT BE DEEMED TO PROVIDE A GUARANTEE OF THE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSTRUCTED AS ACCEPTANCE OR SUPERVISION OF CONSTRUCTION.

12. SUBMITTALS:

   A. SHOP DRAWINGS AND SUBMITTALS, INCLUDING CONCRETE MIX DESIGNS, REQUIRED BY THE SPECIFICATIONS AND/OR THESE DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION OR USE. A SCHEDULE FOR THE RELEASE OF SHOP DRAWINGS AND SUBMITTALS SHALL BE PREPARED BY THE CONTRACTOR AND SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO THE FIRST SUBMITTAL. THIS SUBMITTAL SCHEDULE SHALL PROPORTION THE NUMBER OF SHOP DRAWINGS TO BE REVIEWED IN EACH SUBMITTAL TO ALLOW SUFFICIENT TIME AS DEEMED REASONABLE IN THE PROFESSIONAL JUDGMENT OF THE ARCHITECT AND STRUCTURAL ENGINEER TO PERMIT ADEQUATE REVIEW. MHP WILL COORDINATE WITH THE ARCHITECT AND/OR CONTRACTOR TO ENSURE THAT THE SUBMITTAL SCHEDULE PRIORITIZES THE REVIEW IN A MANNER CONSISTENT WITH THE ANTICIPATED CONSTRUCTION SEQUENCE. SHOP DRAWINGS AND SUBMITTALS SHALL REFERENCE THE LATEST REVISION OF EACH STRUCTURAL DESIGN DRAWING USED FROM WHICH TO DETAIL. SHOP DRAWINGS AND/OR SUBMITTALS THAT DO NOT IDENTIFY THE LATEST REVISION OF THE STRUCTURAL DRAWINGS AND/OR DO NOT REFERENCE THE AGENCY APPROVED CONSTRUCTION DOCUMENTS SHALL BE RETURNED FOR THE DETAILER TO UPDATE AND RESUBMIT. PARTIALLY COMPLETE SHOP DRAWINGS AND SHOP DRAWINGS COMPLETED WITHOUT INCORPORATING THE INFORMATION PROVIDED IN RESPONSE TO REQUESTS FOR INFORMATION (RFI/RFC) MAY BE RETURNED (AT MHP'S SOLE DISCRETION) FOR UPDATING IF IT IS DETERMINED THAT SUCH INFORMATION WAS PROVIDED IN A TIMELY MANNER PRIOR TO THE RELEASE OF THE SHOP DRAWING SUBMITTAL. IF A SUBMITTAL MUST BE REVISED, IT SHALL IDENTIFY EACH REVISION AND/OR ADDITION TO EACH SHOP DRAWING BY CLOUDING OR OTHER MEANS.

   B. SUBMITTALS, INCLUDING SHOP DRAWINGS, AND RFI/RFCS, WILL NOT BE ACCEPTED BY MHP DIRECTLY FROM THE PROJECT SUB-CONTRACTORS. SUBMITTALS WILL BE ACCEPTED FROM THE OWNER'S AUTHORIZED CONTRACT ADMINISTRATOR (I.E. GENERAL CONTRACTOR, CONSTRUCTION MANAGER, OR ARCHITECT) ONLY AFTER THEY HAVE BEEN CONFIRMED TO COMPLY WITH CONTRACT SUBMITTAL REQUIREMENTS AND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE OWNER'S AUTHORIZED CONTRACT ADMINISTRATOR SHALL COORDINATE AND VERIFY COMPLIANCE WITH THE REQUIREMENTS OF THE PROJECT SPECIFICATIONS REGARDING DISTRIBUTION OF THE SUBMITTALS TO AND THROUGH THE ARCHITECT'S OFFICE.

   C. WHEN THE ALLOWANCE FOR SUBSTITUTION OF A SPECIFIED MATERIAL OR PRODUCT DESIGNATION IS IMPLIED WITHIN THE CONSTRUCTION DOCUMENTS BY THE USE OF THE WORDS "ACCEPTED ALTERNATES", "ALTERNATIVE PRODUCTS/SUPPLIERS", ETC. OR "OR APPROVED EQUAL", A FORMAL SUBSTITUTION REQUEST SHALL BE SUBMITTED BY THE GENERAL CONTRACTOR. THE FORMAL SUBSTITUTION REQUEST SHALL CLEARLY IDENTIFY THE ORIGINAL PRODUCT AND THE REQUESTED SUBSTITUTE PRODUCT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THAT THE REQUESTED SUBSTITUTION IS "EQUAL" TO, OR BETTER THAN, THE ORIGINAL PRODUCT FOR ALL CONSIDERATIONS SUCH AS, BUT NOT NECESSARILY LIMITED TO, STRENGTH, PERFORMANCE, AND INSTALLATION VARIABLES, INCLUDING SCHEDULE. SUCH VERIFICATION SHALL BE CLEARLY OUTLINED IN THE FORMAL SUBMITTAL AS A TABULAR COMPARISON BETWEEN THE ORIGINAL PRODUCT CRITERIA AND THE REQUESTED SUBSTITUTE'S PRODUCT CRITERIA. APPROVAL OF THE SUBSTITUTION REQUEST SHALL BE OBTAINED FROM THE ARCHITECT, STRUCTURAL ENGINEER OF RECORD AND, IF NECESSARY, THE GOVERNING AGENCY, PRIOR TO FABRICATION OR INSTALLATION OF THE SUBSTITUTED MATERIAL OR PRODUCT.

REINFORCING STEEL:

1. DETAILING, FABRICATION AND PLACING OF REINFORCING STEEL SHALL CONFORM TO STANDARDS AND RECOMMENDATIONS CONTAINED WITHIN THE CRSI "MANUAL OF STANDARD PRACTICE". DETAILING FABRICATION AND PLACING OF WELDED WIRE REINFORCING SHALL CONFORM TO THE STANDARDS AND RECOMMENDATIONS CONTAINED WITHIN THE WRI "MANUAL OF STANDARD PRACTICE - STRUCTURAL WELDED WIRE REINFORCEMENT".

2. REINFORCING BARS (REBAR), STEEL WELDED WIRE REINFORCING (WWR), AND TIE WIRE USED TO SECURE REBAR AND WWR SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS, UNO:
   A. REINFORCING, (ALL BAR SIZES, UNO):_____ASTM A-615, GR 60
   B. REINFORCING BARS TO BE WELDED (ALL BAR SIZES UNO):_ASTM A-706, GR 60
   C. WELD WIRE REINFORCING:_____ASTM A-1064
   D. TIE WIRE:_____ASTM A-82

3. ALL REINFORCING STEEL SHALL BE BENT COLD. GRADE 60 BARS MAY ONLY BE BENT ONCE, STRAIGHTENING AND/OR RE-BENDING IS NOT ALLOWED. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO SEQUENCE PLACEMENT OF REINFORCING SUCH THAT INCIDENTAL BENDING DOES NOT OCCUR.

4. PRIOR TO PLACING CONCRETE: REINFORCING STEEL, INCLUDING WWR, AND OTHER EMBEDDED ITEMS SHALL BE WELL-SECURED IN POSITION AND SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. WHERE TWO LAYERS OF REINFORCING STEEL ARE REQUIRED (I.E. FOOTING PADS OR SLABS) PROVIDE APPROPRIATE CHAIRS TIED TO AND SUPPORTED BY LOWER MAT OF REINFORCING TO SUPPORT THE UPPER MAT OF REINFORCING. "HOOK AND PULL" METHODS SHALL NOT BE ALLOWED.

5. CONCRETE PROTECTION FOR REINFORCING BARS SHALL BE AT LEAST EQUAL TO THE DIAMETER OF THE BAR. MINIMUM COVER FOR CAST IN PLACE CONCRETE SHALL BE AS FOLLOWS:
   A. CAST AGAINST AND PERMANENTLY IN CONTACT WITH GROUND:
      - ALL MEMBERS, ALL REINFORCEMENT:_____ 3"
   B. EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
      - ALL MEMBERS, #6 THROUGH #18 BARS:_____ 2"
      - ALL MEMBERS, #5, W31 OR D31 WIRE AND SMALLER_____ 1 1/2"
   C. NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
      - SLABS, JOISTS, WALLS; #14 AND #18 BARS:_____ 1 1/2"
      - SLABS, JOISTS, WALLS; #11 BARS AND SMALLER:_____ 3/4"
      - BEAMS, COLUMNS: PRIMARY REINF, STIRRUPS, TIES, SPIRALS AND HOOPS:_____ 1 1/2"

6. MINIMUM CLEAR SPACING BETWEEN PARALLEL BARS IN A SINGLE LAYER SHALL NOT BE LESS THAN 1 1/2", 4/3 TIMES LARGEST AGGREGATE, 1 1/2 TIMES DIAMETER OF THE LARGER BAR, WHICHEVER IS GREATER. WHERE PARALLEL REINFORCING IS PLACED IN TWO OR MORE LAYERS, BARS IN THE UPPER LAYERS SHALL BE PLACED DIRECTLY ABOVE THE BARS IN LOWER LAYERS WITH NOT LESS THAN 1" CLEAR SPACE BETWEEN LAYERS.

7. DEVELOPMENT AND LAP SPLICE LENGTHS FOR REINFORCING STEEL AND WELDED WIRE REINFORCING SHALL BE AS NOTED ON PLANS AND DETAILS CONTAINED THEREIN. WHERE SPLICE LOCATIONS ARE NOT SPECIFICALLY INDICATED, SPLICES SHALL BE STAGGERED A MINIMUM OF ONE (1) LAP LENGTH. WHERE SPECIFIC LAP LENGTH REQUIREMENTS ARE NOT SPECIFICALLY SHOW ON PLANS, THE FOLLOWING MINIMUM LENGTHS SHALL BE USED:
   A. REBAR IN CONCRETE:_____SEE TYPICAL DETAILS

8. COMPLETE REINFORCING PLACEMENT DRAWINGS (SHOP DRAWINGS) SHALL BE PREPARED IN ACCORDANCE WITH ACI 315 AND SHALL BE SUBMITTED TO THE SEOR FOR REVIEW PRIOR TO FABRICATION. APPROVED SHOP DRAWINGS SHALL BE MADE AVAILABLE ON THE JOB SITE PRIOR TO PLACING OF CONCRETE. SEE SUBMITTALS SECTION OF THE STRUCTURAL NOTES FOR ADDITIONAL INFORMATION AND REQUIREMENTS.

9. CONTRACTOR SHALL SCHEDULE SPECIAL INSPECTIONS SO THAT BAR SIZE, SPACING, LAP SPLICE AND EM

STRUCTURAL AND MISCELLANEOUS STEEL:

1. ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS (AISC 360), LATEST EDITION, AND SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS (AISC 341) LATEST EDITION.

2. ALL WELDING SHALL COMPLY WITH AWS SPECIFICATION D1.1/D1.1M. WELDING ON PARTS OF THE SEISMIC FORCE RESISTING SYSTEM (SFRS) SHALL ALSO CONFORM TO AWS D1.8/D1.8M. WELDING SHALL BE PERFORMED BY WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED.

3. WELDING PROCEDURE SPECIFICATIONS (WPS) SHALL BE SUBMITTED FOR REVIEW FOR EACH WELD TYPE, WELDING PROCESS, WELDING ELECTRODE CLASSIFICATION AND/OR BASE MATERIAL (WHERE ASTM GRADE OR ALLOY VARIES) SHOWN ON THE DRAWINGS IN CONFORMANCE WITH AWS D1.1/D1.1M, D1.4/D1.4M AND D1.8/D1.8M AS APPLICABLE. EACH WPS SHALL BE RECORDED ON FORMS RECOMMENDED BY THE APPLICABLE AWS STANDARD AND SHALL BE REVIEWED BY AN AWS SCWI CERTIFIED INSPECTOR AND SUBMITTED TO THE SEOR AND, IF THE PROJECT IS GOVERNED BY DSA OR OSHPD, THE INSPECTOR OF RECORD (IOR) FOR REVIEW. IF A WELD ASSEMBLY NOTED ON PLANS DOES NOT FALL WITHIN THE PREQUALIFIED CRITERIA, CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING A WELDING PROCEDURE QUALIFICATION RECORD FOR THE SPECIFIED WELD AS PART OF THE WPS SUBMITTAL.

4. ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO THE FOLLOWING SPECIFICATION (UNLESS NOTED OTHERWISE):
   A. HSS TUBE (SQ.RECTANG)(_____ASTM A-500, GR C_____FY=50KSI
      SULFUR CONTENT SHALL BE LESS THAN OR EQUAL TO 0.05%
   B. HSS TUBE (ROUND)_____ASTM A-500, GR C_____FY=46KSI
      SULFUR CONTENT SHALL BE LESS THAN OR EQUAL TO 0.05%

5. STRUCTURAL STEEL SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND SEOR FOR REVIEW PRIOR TO FABRICATION AND ERECTION, IN ACCORDANCE WITH NOTE 12 OF GENERAL STRUCTURAL NOTES.

6. WHEN FABRICATING BEAMS PLACE NATURAL CAMBER UP. A TOLERANCE OF + 0, -1/4 INCH MAY BE APPLIED TO ALL SPECIFIED CAMBERS.

7. AFTER FABRICATION, ALL STEEL SHALL BE CLEANED FREE OF RUST, LOOSE MILL SCALE, AND OIL.

8. ALL STEEL EXPOSED TO MOISTURE OR WEATHER SHALL BE HOT-DIPPED GALVANIZED, UNLESS NOTED OTHERWISE. GALVANIZING SHALL BE IN CONFORMANCE WITH ASTM A123 AND A153. REPAIR GALVANIZING AFTER WELDING SHALL BE IN ACCORDANCE WITH ASTM A780.

9. HOT-DIP GALVANIZE OR PROVIDE THREE-INCH (3") MINIMUM CONCRETE COVER AROUND ALL STRUCTURAL STEEL BELOW GRADE.

10. SEE ARCHITECTURAL DRAWINGS FOR PAINTING OF STRUCTURAL STEEL. STRUCTURAL STEEL EMBEDDED IN CONCRETE OR MASONRY SHALL BE UNPAINTED.

11. WHERE CARBON STEEL IS IN CONTACT WITH STAINLESS STEEL OR WHERE EITHER CARBON OR STAINLESS STEEL IS IN CONTACT WITH ALUMINUM, PHENOLIC SHIM OR BREAK SHALL BE INSTALLED TO SEPARATE DISSIMILAR METALS.

12. BOLTS, THREADED RODS, AND WASHERS SHALL CONFORM TO THE FOLLOWING, UNO:
    A. MACHINE BOLTS_____ASTM A-307
    B. THREADED RODS, UNO_____ASTM A-36
    C. WELDED HEADED STUDS_____ASTM A-108, GRADE 1010-1020

13. BOLTED CONNECTIONS SHALL HAVE A MINIMUM OF TWO 3/4" DIAMETER BOLTS UNLESS SHOWN OTHERWISE.

14. UNLESS NOTED OTHERWISE, BOLTS SHALL BE INSTALLED "SNUG TIGHT". THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS THAT EXISTS WHEN ALL PLIES OF THE JOINT ARE IN FIRM CONTACT. BOLTS OF SLOTTED CONNECTIONS SHALL BE INSTALLED "HAND TIGHT" ONLY, WITH THREADS SPOILED. THE HAND TIGHT CONDITION IS DEFINED AS THE TIGHTNESS THAT EXISTS WHEN THE WASHER AND BOLT HEAD ARE BROUGHT INTO FULL CONTACT WITH WEB OR SHEAR TAB WITH A SPUD WRENCH AND THE NUT IS THEN BACKED OFF ONE QUARTER OF A TURN. WHERE SLOTTED CONNECTIONS ARE INDICATED, THE WASHER SHALL BE PLACED OVER THE SLOTTED HOLE. BOLTS INDICATED AS SLIP-CRITICAL SHALL BE PRE-TENSIONED PER SECTION 8.2 OF SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS.

15. HOLES FOR BOLTED CONNECTIONS AND ANCHOR BOLTS SHALL BE AISC "STANDARD" HOLES LIMITED TO 1/16-INCH LARGER IN DIAMETER THAN NOMINAL BOLT DIAMETER FOR 7/8-INCH DIAMETER BOLTS AND SMALLER, AND 1/8-INCH LARGER IN DIAMETER THAN NOMINAL BOLT DIAMETER FOR 1-INCH AND LARGER BOLTS, UNLESS NOTED OTHERWISE. OVERSIZED AND SLOTTED HOLES REQUIRE THE APPROVAL OF THE SEOR, UNLESS SPECIFICALLY DETAILED ON THE APPROVED DRAWINGS.

16. PROVIDE BEVELED WASHERS WHERE JOINT FACE SLOPE IS GREATER THAN 1:20.

17. ALL STRUCTURAL STEEL WELDING SHALL BE PERFORMED WITH E70XX ELECTRODES.

18. WHERE FILLET WELD SYMBOL IS SHOWN WITHOUT INDICATION OF THROAT SIZE, USE MINIMUM SIZE WELDS AS SPECIFIED BELOW:

TABLE J2.4
MINIMUM SIZE OF FILLET WELDS

MATERIAL THICKNESS OF THINNER PART JOINED	MINIMUM SIZE OF FILLET WELD
TO 1/4" (6MM) INCLUSIVE	1/8" (3MM)
OVER 1/4" (6MM) TO 1/2" (13MM)	3/16" (5MM)
OVER 1/2" (13MM) TO 3/4" (19MM)	1/4" (6MM)
OVER 3/4" (19MM)	5/16" (8MM)

19. WHERE LENGTH OF WELD IS NOT INDICATED, WELD SHALL BE FULL LENGTH OF JOINT.

20. ALL COMPLETE- AND PARTIAL-JOINT-PENETRATION GROOVE WELDS SHALL BE PERFORMED USING "INNERSHIELD" AND "ML-2" SEMI-AUTOMATIC EQUIPMENT.

21. SEE SPECIAL INSPECTION AND TESTING NOTES FOR INSPECTION AND TESTING REQUIREMENTS.

SPECIAL INSPECTIONS:

1. THE OWNER, OR THE OWNER'S AUTHORIZED AGENT (OTHER THAN THE CONTRACTOR AS APPLICABLE) SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS, INCLUDING AS APPLICABLE AN INSPECTOR OF RECORD (IOR), WHO SHALL PROVIDE SPECIAL INSPECTIONS DURING CONSTRUCTION FOR CERTAIN TYPES OF WORK WHEN SO SPECIFIED IN THE CONTRACT DOCUMENTS AND PROJECT SPECIFICATIONS. WHERE AN IOR IS REQUIRED BY THE GOVERNING AGENCY, THE IOR MAY PERFORM SPECIAL INSPECTIONS IF THAT PERSON IS QUALIFIED PER THE GOVERNING AGENCY'S STANDARDS FOR THE SPECIAL INSPECTION REQUIRED. WHERE AN IOR IS NOT REQUIRED, THESE SPECIAL INSPECTIONS SHALL BE IN ADDITION TO AND COMPLEMENTARY WITH THE INSPECTIONS PROVIDED BY THE GOVERNING AGENCY.

2. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON FROM AN APPROVED AGENCY CONFORMING TO ASTM C1077 WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE ARCHITECT, STRUCTURAL ENGINEER OF RECORD AND THE GOVERNING AGENCY, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.

3. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE DESIGN DRAWINGS, SPECIFICATIONS AND APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE AND OTHER APPLICABLE REGULATIONS IDENTIFIED WITHIN THE CONSTRUCTION DOCUMENTS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE GENERAL CONTRACTOR FOR CORRECTION AND THEN, IF UNCORRECTED, TO THE ATTENTION OF THE ARCHITECT, STRUCTURAL ENGINEER OF RECORD AND THE GOVERNING AGENCY. IT SHALL BE THE GENERAL CONTRACTOR'S RESPONSIBILITY TO COORDINATE WITH THE SPECIAL INSPECTOR AND SCHEDULE THE SPECIAL INSPECTIONS WITH ADEQUATE TIME TO ADDRESS ANY AND ALL POTENTIAL DISCREPANCIES PRIOR TO PROCEEDING WITH SUBSEQUENT WORK THAT COVERS OR OTHERWISE MAKES INACCESSIBLE ANY WORK IDENTIFIED AS DEVIATING FROM THE PROJECT REQUIREMENTS.

4. THE SPECIAL INSPECTOR SHALL FURNISH REGULAR INSPECTION REPORTS TO THE ARCHITECT, STRUCTURAL ENGINEER OF RECORD AND THE GOVERNING AGENCY IDENTIFYING THE WORK INSPECTED AND ANY UNCORRECTED DISCREPANCIES FROM THE CONSTRUCTION DOCUMENTS. AT THE CONCLUSION OF THE PROJECT OR THE SPECIAL INSPECTORS ASSIGNED SCOPE OF WORK, THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF HIS OR HER KNOWLEDGE, COMPLETED IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS AND SPECIFICATIONS (INCLUDING APPROVED RFI'S, ADDENDUMS, ETC.) AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE AND OTHER APPLICABLE REGULATIONS IDENTIFIED WITHIN THE CONSTRUCTION DOCUMENTS.

5. SPECIAL INSPECTIONS INDICATED BELOW SHALL BE PROVIDED IN EITHER A CONTINUOUS OR PERIODIC CAPACITY, AS DEFINED BELOW, AS REQUIRED BY THE INDIVIDUAL CODE OR REFERENCED STANDARD.

6. CONTINUOUS - SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED. FOR STRUCTURAL STEEL, CONTINUOUS INSPECTION IS FURTHER DEFINED SUCH THAT INSPECTION SHALL TAKE PLACE ON EACH ELEMENT TO BE INSPECTED (I.E. EACH BOLT OR WELD).

7. PERIODIC - SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS INTERMITTENTLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING PERFORMED. FOR STRUCTURAL STEEL, PERIODIC INSPECTION IS FURTHER DEFINED SUCH THAT ITEMS ARE OBSERVED ON A RANDOM BASIS.

8. SEE APPROVED DSA FORM-103, STATEMENT OF STRUCTURAL TEST & SPECIAL INSPECTIONS FOR INSPECTION AND TESTING REQUIREMENTS. NOTES BELOW COMPLEMENT OR ARE IN ADDITION TO THE DSA FORM-103 REQUIREMENTS:

CONCRETE SPECIAL INSPECTION AND TESTING NOTES:

1. SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION ARE INCLUDED IN THE RESEARCH REPORT FOR EACH POST-INSTALLED ANCHOR ISSUED BY AN APPROVED SOURCE. THESE SPECIAL INSPECTION REQUIREMENTS SHOULD BE FOLLOWED. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, CONTACT STRUCTURAL ENGINEER FOR SPECIAL INSPECTION REQUIREMENTS PRIOR TO PROCEEDING WITH THE WORK. PROJECT SPECIFIC SPECIAL INSPECTION MEASURES SHALL BE APPROVED BY THE GOVERNING AGENCY PRIOR TO THE COMMENCEMENT OF THE WORK. THE INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL AND UPWARDLY INCLINED POSITIONS SHALL BE PERFORMED BY AN ACI/CRSI CERTIFIED ADHESIVE ANCHOR INSTALLER. SEE POST INSTALLED CONCRETE AND MASONRY ANCHOR SPECIAL INSPECTION NOTES BELOW FOR MORE INFORMATION.

2. A STRENGTH TEST SHALL BE THE AVERAGE OF, AT A MINIMUM, TWO 6X12 CYLINDERS OR THREE 4X8 CYLINDERS MADE FROM THE SAME SAMPLE OF CONCRETE. A TESTING LABORATORY SHALL MAKE AND TEST ONE SAMPLE SET FOR EACH 50 CUBIC YARDS OF CONCRETE BUT NOT LESS THAN ONE SAMPLE SET FOR EACH 2,000 SQFT OF SURFACE AREA FOR SLABS OR WALLS. IF TOTAL VOLUME OF CONCRETE IS SUCH THAT FREQUENCY OF TESTING WOULD PRODUCE FEWER THAN 5 STRENGTH TESTS FOR A GIVEN CONCRETE MIXTURE, THEN STRENGTH TEST SPECIMENS SHALL BE MADE FROM AT LEAST 5 RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN 5 BATCHES ARE USED. AN ADDITIONAL SAMPLE FOR SEVEN-DAY COMPRESSIVE STRENGTH TESTS SHALL BE TAKEN FOR EACH CLASS OF CONCRETE AT THE BEGINNING OF CONCRETE WORK OR WHENEVER THE MIX OR AGGREGATE IS CHANGED.

3. CONTINUOUS BATCH PLANT INSPECTION IS REQUIRED AT THE BATCH PLANT. THE QUALITY AND QUANTITY OF MATERIALS USED IN TRANSIT-MIXED CONCRETE AND IN BATCHED AGGREGATES SHALL BE CONTINUOUSLY INSPECTED BY AN APPROVED AGENCY AT THE LOCATION WHERE THE MATERIALS ARE MEASURED.

STEEL SPECIAL INSPECTION AND TESTING NOTES:

1. THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.

2. WHERE CJP GROOVE WELDS OCCUR IN ELEMENTS AND CONNECTIONS THAT ARE NOT PART OF THE SEISMIC FORCE-RESISTING SYSTEM (SFRS) , IN RISK CATEGORY III AND IV (SEE GENERAL NOTES), ULTRASONIC TESTING (UT) SHALL BE PERFORMED ON ALL CJP GROOVE WELDS IN MATERIALS 5/16 INCH AND THICKER. IN RISK CATEGORY II, UT SHALL BE PERFORMED ON 10% OF CJP GROOVE WELDS IN MATERIAL 5/16 INCH AND THICKER.

STRUCTURAL OBSERVATION:

1. THE STRUCTURAL ENGINEER OF RECORD (SEOR), OR HIS/HER DESIGNATED ENGINEER, SHALL PROVIDE VISUAL STRUCTURAL OBSERVATION OF THE STRUCTURAL SYSTEM FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS AND SPECIFICATIONS AT SIGNIFICANT CONSTRUCTION STAGES AND AT COMPLETION OF THE STRUCTURAL SYSTEM, AS REQUIRED BY CBC SECTION 1704A.6 AND AS DEFINED IN CBC SECTION 1704A. WRITTEN REPORTS SHALL BE SUBMITTED TO THE OWNER'S REPRESENTATIVE OR HIS DESIGNEE FOR DISTRIBUTION TO THE SPECIAL INSPECTOR, CONTRACTOR AND BUILDING OFFICIAL.

2. THE STRUCTURAL OBSERVER SHALL SUBMIT A WRITTEN STATEMENT TO THE GOVERNING AGENCY THAT THE SITE VISITS HAVE BEEN MADE. SUCH REPORTS SHALL IDENTIFY ANY OBSERVED DEFICIENCIES, WHICH TO THE BEST OF THE STRUCTURAL OBSERVER'S KNOWLEDGE HAVE NOT BEEN RESOLVED. AT THE COMPLETION OF THE STRUCTURAL SYSTEM THE STRUCTURAL OBSERVER SHALL PROVIDE A FINAL OBSERVATION REPORT INDICATING THAT TO THE BEST OF HIS/HER KNOWLEDGE ALL OBSERVED DEFICIENCIES HAVE BEEN RESOLVED AND THE STRUCTURAL SYSTEM GENERALLY CONFORMS TO THE APPROVED PLANS AND SPECIFICATIONS.

3. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY OF THE INSPECTIONS BY THE BUILDING OFFICIAL AS REQUIRED BY THE CALIFORNIA ADMINISTRATIVE CODE, SPECIAL INSPECTIONS REQUIRED BY CBC SECTION 1705A, OR ANY OTHER INSPECTION REQUIRED BY OTHER SECTIONS OF THE CODE OR AS NOTED ELSEWHERE IN THE CONTRACT DOCUMENTS. THE STRUCTURAL OBSERVER DOES NOT HAVE THE AUTHORITY TO APPROVE COVERING OF CONSTRUCTION AND HIS/HER POSITIVE DISPOSITION OF THE OBSERVATION REPORT DOES NOT WARRANT THAT THE CONSTRUCTION WILL PASS THE BUILDING OFFICIAL'S INSPECTION.

4. STRUCTURAL OBSERVATION FOR THIS PROJECT SHALL BE PROVIDED BY MHP STRUCTURAL ENGINEERS, INC.; 3900 COVER STREET, LONG BEACH, CALIFORNIA, 90808; TELEPHONE (562) 985-3200; FAX (562) 985-1011.

5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION SCHEDULE AND SHALL NOTIFY THE STRUCTURAL OBSERVER NO LESS THAN THREE (3) BUSINESS DAYS IN ADVANCE OF REQUIRED OBSERVATIONS. FAILURE OF THE CONTRACTOR TO PROVIDE ADEQUATE NOTIFICATION MAY RESULT IN DELAYS DUE TO CORRECTIVE WORK OR REMOVAL OF SUBSEQUENT WORK TO ALLOW ADEQUATE OBSERVATION. REMOVAL AND REPLACEMENT OF ANY FINISHED WORK AND/OR FRAMING DAMAGED BY THE REMOVAL PROCESS OR AS REQUIRED FOR CORRECTIVE ACTION RESULTING FROM INADEQUATE NOTIFICATION SHALL BE AT THE CONTRACTOR'S EXPENSE.

6. THE STRUCTURAL OBSERVER SHALL AS A MINIMUM PERFORM STRUCTURAL OBSERVATION AT THE FOLLOWING STAGES OF CONSTRUCTION (CONSTRUCTION STAGES AND ELEMENTS/CONNECTIONS TO BE OBSERVED):

   A. CONCRETE & REINFORCING STEEL
      AFTER EXCAVATION OR FORMING AND PLACEMENT OF REINFORCING STEEL (INCLUDING POST-TENSIONING HARDWARE AND TENDONS), PRIOR TO CLOSING FORMS AND PLACEMENT OF CONCRETE, FOR FIRST SIGNIFICANT POUR OF STRUCTURAL WORK.

   B. STEEL FRAMING
      FOLLOWING ERECTION AND COMPLETION OF REPRESENTATIVE SAMPLING OF CONNECTIONS (INCLUDING GUSSET PLATES AND BRACES), PRIOR TO COVERING CONNECTIONS.

   C. COMPLETION OF STRUCTURAL SYSTEM

REGISTERED PROFESSIONAL ENGINEER DANIEL C. TOY S 5809 STRUCTURAL STATE OF CALIFORNIA 06/05/25

LICENSED ARCHITECT RUDDER C. WILLIAMS Exp. 10-31-27 STATE OF CALIFORNIA

STAMPS

AGENCY APPROVAL
File No: A.P

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DIV. OF THE STATE ARCHITECT
APP: 04-123574 INC:
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SS ☑ FLS ☑ ACS ☑
DATE: 01/15/2026

MHP
STRUCTURAL ENGINEERS
LONG BEACH • SAN DIEGO
LONG BEACH OFFICE
3900 Cover Street, Long Beach
CA 90808 • 562.985.3200
Project: 23-0590-00

CONSULTANT BRANDING

RUHNAU CLARKE
ARCHITECTS

VALLEY VIEW HIGH SCHOOL MODERNIZATION:

PROJECT No. :14812
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3775 TENTH STREET, RIVERSIDE CALIFORNIA 92501 (951) 684 4664 / 5751 PALMER WAY, SUITE C, CARLSBAD CALIFORNIA 92010 (760) 438 5899

VALLEY VIEW HIGH SCHOOL MODERNIZATION
24100 Delphinium Ave, Moreno Valley, CA 92553
MORENO VALLEY UNIFIED SCHOOL DISTRICT

STRUCTURAL NOTES

S-1.1

14812

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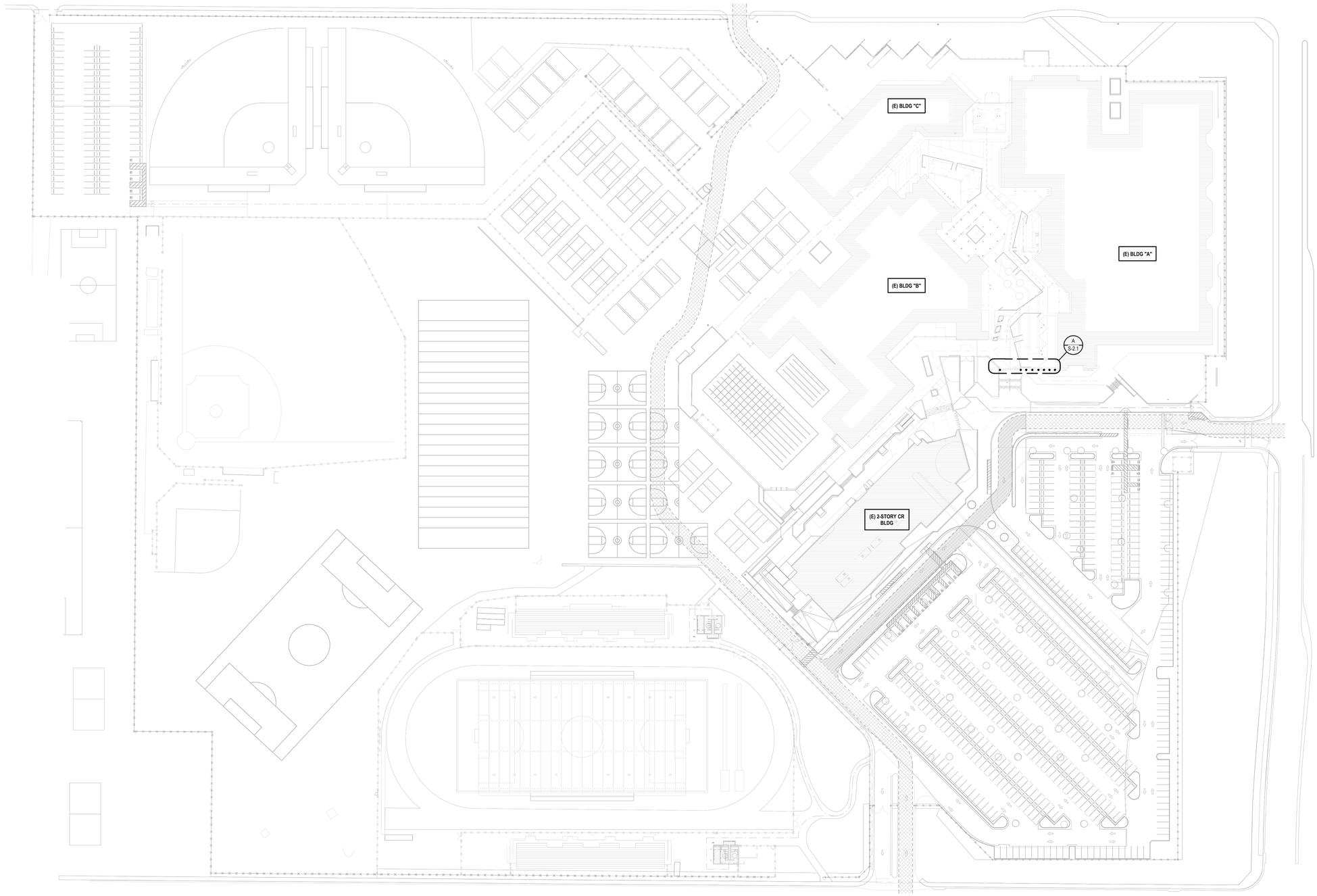

OVERALL SITE PLAN

SCALE: 1" = 60'-0"

A
S-2.0

STAMPS

REGISTERED PROFESSIONAL ENGINEER
DANIEL C. TOY
S-5809
STRUCTURAL
STATE OF CALIFORNIA
Exp. 12-31-25

LICENSED ARCHITECT
ROGER CLARKE
Exp. 10-31-27
STATE OF CALIFORNIA

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LONG BEACH OFFICE
3900 Cover Street, Long Beach
CA 90808 • 562.985.3200
Project: 23-0590-00

RUHNAU
CLARKE
ARCHITECTS

FRAMING PLAN NOTES

1. SEE GENERAL NOTES SHEET(S) FOR APPLICABLE NOTES AND ABBREVIATIONS.
2. EXISTING CONSTRUCTION IS SHOWN FADED AND IS LABELED AS EXISTING OR (E). NEW CONSTRUCTION IS SHOWN DARK AND IS SPECIFICALLY REFERENCED BY DETAILS AND NOTES.
3. EXISTING CONDITIONS ARE SHOWN TO THE BEST OF OUR KNOWLEDGE. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ACTUAL CONDITIONS. DISCREPANCIES BETWEEN ACTUAL FIELD CONDITIONS AND THOSE SHOWN ON THESE DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO PROCEEDING.

VALLEY VIEW HIGH SCHOOL MODERNIZATION:

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VALLEY VIEW HIGH SCHOOL MODERNIZATION

24100 Delphinium Ave, Moreno Valley, CA 92553
MORENO VALLEY UNIFIED SCHOOL DISTRICT

OVERALL SITE PLAN

S-2.0

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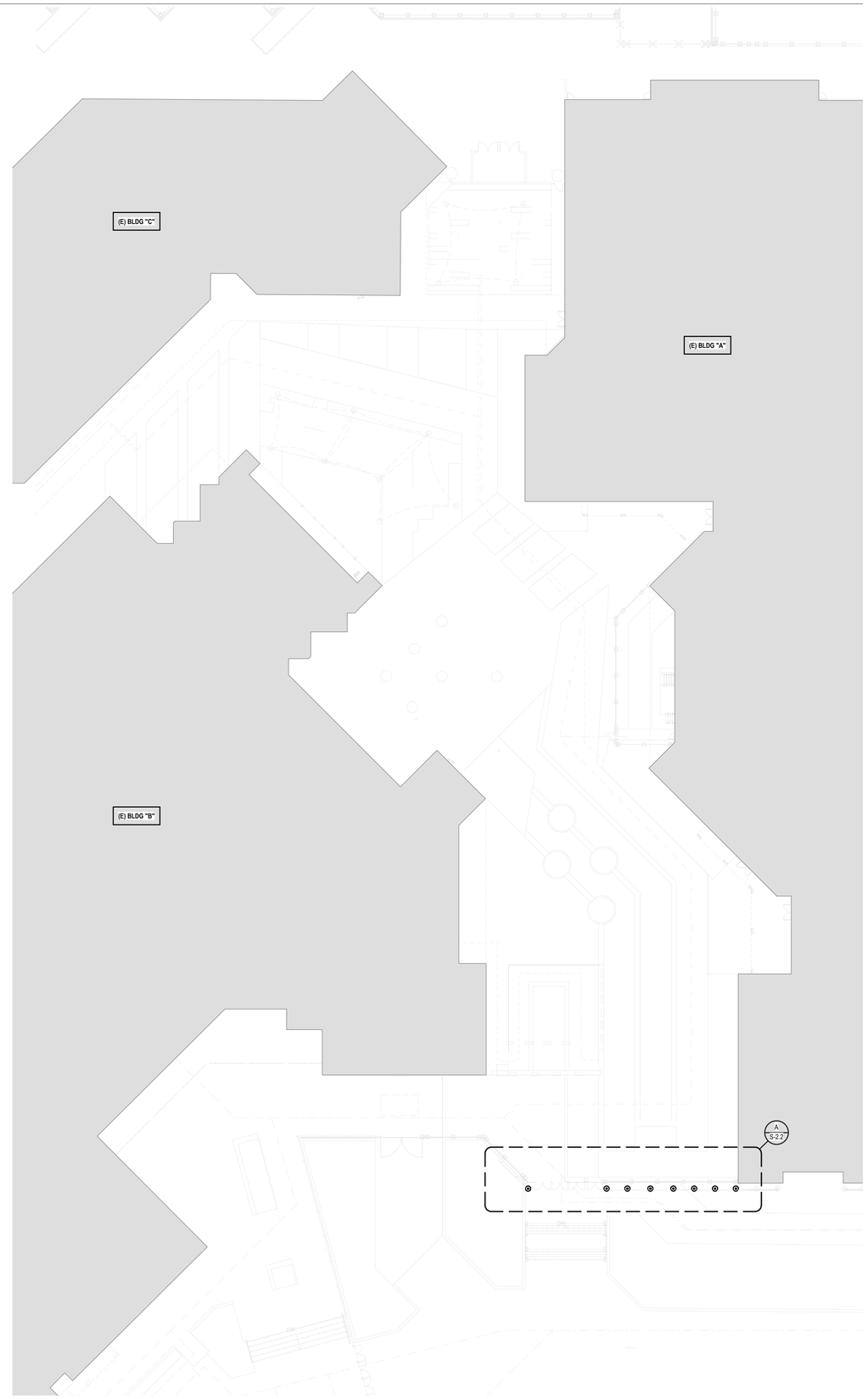

ENLARGED OVERALL SITE PLAN

SCALE: 1" = 20'-0"

A
S-2.1

STAMPS

REGISTERED PROFESSIONAL ENGINEER
DANIEL C. TOY
S-5809
STRUCTURAL
STATE OF CALIFORNIA
Exp. 12/31/2026

LICENSED ARCHITECT
ROGER CLARKE
Exp. 10-31-27
STATE OF CALIFORNIA

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LONG BEACH OFFICE
3900 Cover Street, Long Beach
CA 90808 • 562.985.3200
Project: 23-0590-00

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FRAMING PLAN NOTES

1. SEE GENERAL NOTES SHEET(S) FOR APPLICABLE NOTES AND ABBREVIATIONS.
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3. EXISTING CONDITIONS ARE SHOWN TO THE BEST OF OUR KNOWLEDGE. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ACTUAL CONDITIONS. DISCREPANCIES BETWEEN ACTUAL FIELD CONDITIONS AND THOSE SHOWN ON THESE DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO PROCEEDING.

VALLEY VIEW HIGH SCHOOL MODERNIZATION:

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VALLEY VIEW HIGH SCHOOL MODERNIZATION
24100 Delphinium Ave, Moreno Valley, CA 92553
MORENO VALLEY UNIFIED SCHOOL DISTRICT

ENLARGED OVERALL SITE PLAN

S-2.1

14812

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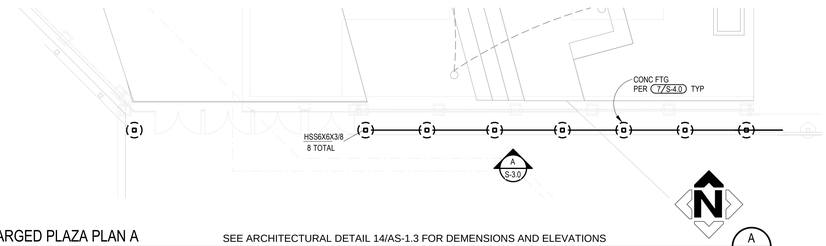

ENLARGED PLAZA PLAN A

SCALE: 1/8" = 1'-0"

SEE ARCHITECTURAL DETAIL 14/AS-1.3 FOR DEMENSIONS AND ELEVATIONS

A
S-2.2

STAMPS

REGISTERED PROFESSIONAL ENGINEER
DANIEL C. TOY
S-5809
STRUCTURAL
STATE OF CALIFORNIA
EXPIRES

LICENSED ARCHITECT
ROGER CLARKE
C-18736
Exp. 10-31-27
STATE OF CALIFORNIA

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LONG BEACH OFFICE
3900 Cover Street, Long Beach
CA 90808 • 562.985.3200
Project: 23-0590-00

CONSULTANT BRANDING

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PLAN NOTES

1. SEE GENERAL NOTES SHEET(S) FOR APPLICABLE NOTES, ABBREVIATIONS AND PLAN SYMBOLS LEGEND. SEE PLAN SHEET FOR LIST OF APPLICABLE TYPICAL DETAILS NOT CALLED OUT ON PLANS. **SEE PROJECT SPECIFICATIONS WHERE APPLICABLE FOR ADDITIONAL INFORMATION**.
2. EXISTING CONSTRUCTION IS SHOWN FADED AND IS LABELED AS EXISTING OR (E). NEW CONSTRUCTION IS SHOWN DARK AND IS SPECIFICALLY REFERENCED BY DETAILS AND NOTES.
3. EXISTING CONDITIONS ARE SHOWN TO THE BEST OF OUR KNOWLEDGE. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ACTUAL CONDITIONS. DISCREPANCIES BETWEEN ACTUAL FIELD CONDITIONS AND THOSE SHOWN ON THESE DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO PROCEEDING.
4. UNLESS NOTED OTHERWISE ON PLAN OR DETAIL, CENTER LINE OF FOUNDATION ELEMENT SHALL MATCH WALL AND/OR COLUMN CENTER LINE.
5. UNLESS NOTED OTHERWISE, DIMENSIONS SHOWN ARE TO CENTERLINE OF STEEL COLUMN OR FACE OF STUD, CONCRETE OR CMU.
6. SEE ARCHITECTURAL PLANS FOR CURBS, SLAB DEPRESSIONS AND SLOPES NOT SHOWN ON PLAN.
7. DO NOT CUT ANY NEW OPENINGS OR ENLARGE ANY EXISTING OPENINGS IN THE ROOF (FOR DUCTS, PIPES, ETC.) UNLESS SPECIFICALLY SHOWN ON THESE STRUCTURAL DRAWINGS.

VALLEY VIEW HIGH SCHOOL MODERNIZATION:

PROJECT No. :14812
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VALLEY VIEW HIGH SCHOOL MODERNIZATION
24100 Delphinium Ave, Moreno Valley, CA 92553
MORENO VALLEY UNIFIED SCHOOL DISTRICT

ENLARGED PLAZA PLANS

S-2.2

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3900 Cover Street, Long Beach
CA 90808 • 562.985.3200
Project: 23-0590-00

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FRAMING PLAN NOTES

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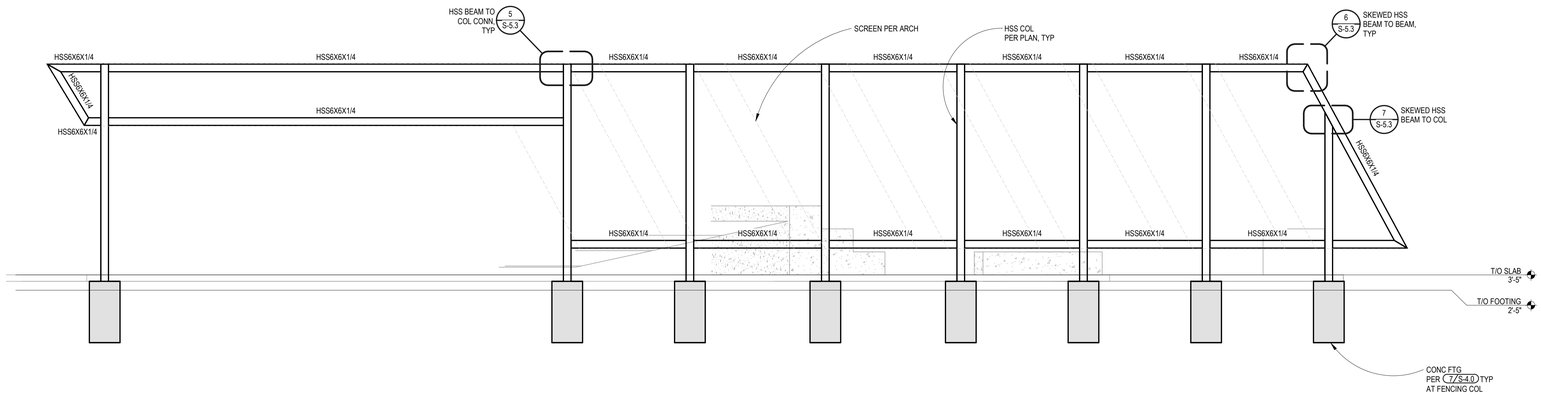
3. EXISTING CONDITIONS ARE SHOWN TO THE BEST OF OUR KNOWLEDGE. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ACTUAL CONDITIONS. DISCREPANCIES BETWEEN ACTUAL FIELD CONDITIONS AND THOSE SHOWN ON THESE DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO PROCEEDING.
4. DO NOT CUT ANY NEW OPENINGS OR ENLARGE ANY EXISTING OPENINGS IN THE ROOF OR WALLS (FOR DUCTS, PIPES, DRAINS, ETC.) UNLESS SPECIFICALLY SHOWN ON THESE STRUCTURAL DRAWINGS.
5. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND INFORMATION NOT SHOWN ON THESE DRAWINGS.



MAIN ENTRY TO PLAZA ELEVATION
SCALE: 1/4" = 1'-0"

SEE ARCHITECTURAL DETAIL 14/AS-1.3 FOR DEMENSIONS AND ELEVATIONS

A
S-3.0

VALLEY VIEW HIGH SCHOOL MODERNIZATION:

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VALLEY VIEW HIGH SCHOOL MODERNIZATION
24100 Delphinium Ave, Moreno Valley, CA 92553
MORENO VALLEY UNIFIED SCHOOL DISTRICT

PLAZA ELEVATION AND SHADE STRUCTURE

S-3.0

14812

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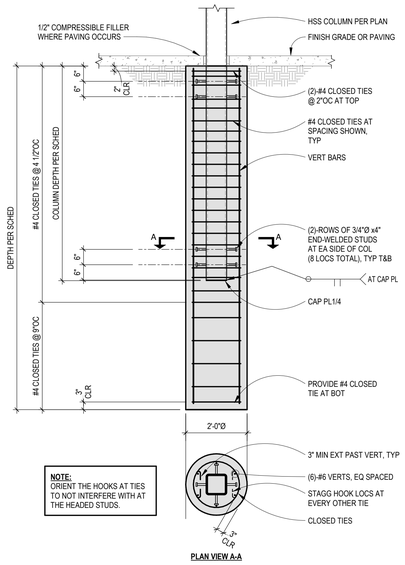
FOOTING SCHEDULE		
SITE ELEMENT	COLUMN EMBED	FOOTING DEPTH
FENCING	7'-0"	11'-0"

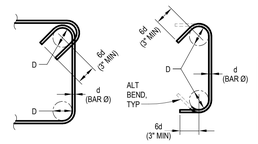


PLAN VIEW A-A

SHADE STRUCTURE FOUNDATION
SCALE: 1/2" = 1'-0"
7 / S-4.0

NOT USED
SCALE: NONE
4 / S-4.0

NOT USED
SCALE: NONE
1 / S-4.0

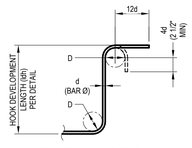


135° STIRRUPS & TIES

90° & 135° CROSS-TIE HOOKS

TIES, STIRRUPS & CROSSTIES	
BAR SIZE	D
#3	1 1/2"
#4	2"
#5	2 1/2"

STIRRUPS, TIES AND CROSS-TIES



90° & 180° STANDARD HOOKS

TYPICAL REINFORCING			
BAR SIZE	D	BAR SIZE	D
#3	2 1/4"	#9	9"
#4	3"	#10	10 1/4"
#5	3 3/4"	#11	11 1/4"
#6	4 1/2"	#14	17"
#7	5 1/4"	#18	22 1/4"
#8	6"		

TYPICAL REINFORCING

NOTE: UNLESS OTHERWISE DIMENSIONED OR NOTED ON PLAN OR DETAIL, EVERY BAR SHOWN OR NOTED AS HOOKED (INCLUDING STIRRUPS, TIES OR CROSSTIES) SHALL HAVE HOOKS PER THIS DETAIL.

ALL GRADES OF STEEL
D = INSIDE BEND DIAMETER
d = NOMINAL BAR DIAMETER

TYPICAL REINFORCING HOOKS
SCALE: NONE
2 / S-4.0

NOT USED
SCALE: NONE
8 / S-4.0

NOT USED
SCALE: NONE
5 / S-4.0

DEVELOPMENT LENGTH (ld) FOR TYPICAL BARS (INCREASE PER NOTES 4 AND/OR 5 AS OCCURS)					
BAR SIZE	CONCRETE STRENGTH f'c (PSI)				
(GR 60)	3000	4000	4500	5000	6000
#3	17"	15"	14"	14"	12"
#4	22"	20"	19"	18"	16"
#5	28"	25"	23"	22"	20"
#6	33"	29"	28"	25"	24"
#7	48"	43"	40"	38"	34"
#8	55"	48"	46"	43"	39"
#9	62"	55"	51"	49"	44"
#10	70"	61"	56"	55"	50"
#11	78"	67"	64"	60"	55"

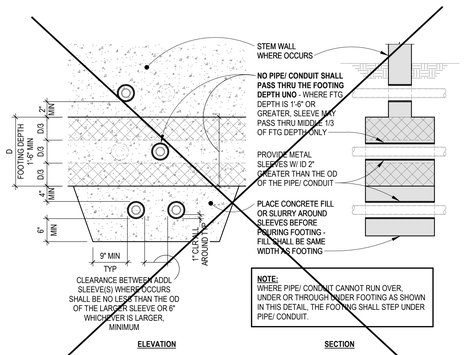
LAP SPLICE LENGTH (ls) FOR TYPICAL BARS (INCREASE PER NOTES 4 AND/OR 5 AS OCCURS)					
BAR SIZE	CONCRETE STRENGTH f'c (PSI)				
(GR 60)	3000	4000	4500	5000	6000
#3	22"	19"	18"	18"	16"
#4	29"	26"	24"	23"	21"
#5	36"	32"	30"	29"	26"
#6	43"	38"	36"	34"	31"
#7	63"	55"	52"	49"	45"
#8	72"	63"	59"	56"	51"
#9	81"	71"	67"	63"	57"
#10	91"	79"	75"	71"	64"
#11	101"	87"	82"	78"	71"

HOOK DEVELOPMENT LENGTH (ldh) FOR TYPICAL STANDARD HOOKS (INCREASE PER NOTES 4 AND/OR 5 AS OCCURS)					
BAR SIZE	CONCRETE STRENGTH f'c (PSI)				
(GR 60)	3000	4000	4500	5000	6000
#3	9"	8"	7"	7"	6"
#4	11"	10"	9"	9"	8"
#5	14"	12"			10"
#6	17"	15"	14"	13"	12"
#7	20"	17"	16"	15"	14"
#8	22"	19"	18"	17"	16"

NOTES:
1. PROVIDE THE MINIMUM DEVELOPMENT LENGTH (ld or ldh) OR LAP SPLICE (ls) FOR REINFORCING BARS IN NORMAL WEIGHT CONCRETE PER THE ADJACENT TABLES (12" MINIMUM) FOR TYPICAL BARS, UNLESS NOTED OTHERWISE ON PLANS AND DETAILS. ld AND ls SHALL BE NO LESS THAN 12" AND ldh SHALL BE NO LESS THAN THE LARGER OF 8" OR 8 TIMES THE BAR DIAMETER. CONTRACTOR SHALL NOTIFY SEOR WHEN MINIMUM LENGTHS CANNOT BE ACHIEVED.
2. LAP SPLICE LENGTHS SHOWN REPRESENT "CLASS B" TENSION SPLICE VALUES PER ACI 318 SECTION 25.4.2.2, 25.5.1.4, AND 25.5.2.1.
3. ld AND ls LENGTHS ARE BASED ON THE CLEAR SPACING OF BARS TO BE NOT LESS THAN TWICE THE BAR DIAMETER, AND CLEAR COVER TO BE NOT LESS THAN THE BAR DIAMETER. CONTRACTOR SHALL NOTIFY THE SEOR WHEN MINIMUM SPACING OR CLEARANCE CANNOT BE ACHIEVED.
4. WHERE HORIZONTAL BARS HAVE MORE THAN 12" OF FRESH CONCRETE CAST BELOW THE BAR, I.E. HORIZONTAL WALL OR STEMWALL BARS, INCREASE TYPICAL BAR LENGTHS BY 30% (ld, ls, ldh x 1.3).
5. FOR LIGHT WEIGHT CONCRETE, INCREASE LENGTH BY THIRTY PERCENT (ld OR ls OR ldh x 1.3).

TYPICAL REINFORCING BAR IN CONCRETE DEVELOPMENT AND LAP SPLICE SCHEDULE
SCALE: NONE
3 / S-4.0

NOT USED
SCALE: NONE
9 / S-4.0



TYPICAL PIPE/ CONDUIT SLEEVES ACROSS FOOTINGS
SCALE: NONE
6 / S-4.0

STAMPS

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Project: 23-0590-00

RUHNAU CLARKE ARCHITECTS

VALLEY VIEW HIGH SCHOOL MODERNIZATION:

PROJECT No. :14812
12/3/2025 7:19:39 PM

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RUHNAUCLARKE.COM
3775 TENTH STREET, RIVERSIDE CALIFORNIA 92501 (951) 684 4664 / 5751 PALMER WAY, SUITE C, CARLSBAD CALIFORNIA 92010 (760) 438 5899

VALLEY VIEW HIGH SCHOOL MODERNIZATION
24100 Delphinium Ave, Moreno Valley, CA 92553
MORENO VALLEY UNIFIED SCHOOL DISTRICT

FOUNDATION DETAILS

S-4.0

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14812

STAMPS

AGENCY APPROVAL
File No: A.P

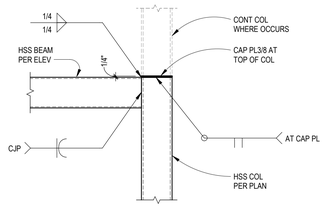
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 04-123574 INC:
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 01/15/2026

STRUCTURAL ENGINEERS
LONG BEACH • SAN DIEGO
LONG BEACH OFFICE
3900 Cover Street, Long Beach
CA 90808 • 562.985.3200
Project: 23-0590-00

CONSULTANT BRANDING

RUHNAU CLARKE ARCHITECTS

NOT USED
SCALE: 1" = 1'-0"
4 / S-5.3



HSS BEAM TO COLUMN CONNECTION
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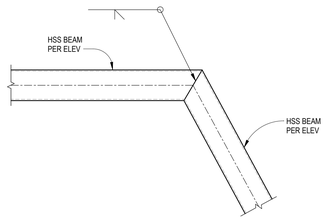

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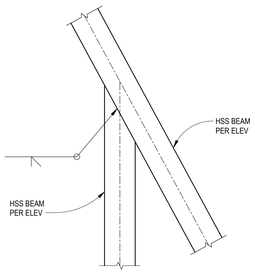

SKEWED HSS BEAM TO COLUMN CONNECTION
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VALLEY VIEW HIGH SCHOOL MODERNIZATION:

PROJECT No. :14812
12/3/2025 7:19:40 PM

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GENERAL NOTES

1. THESE DRAWINGS AND SPECIFICATIONS ARE INTENDED TO COVER A COMPLETE INSTALLATION OF SYSTEMS. THE OMISSION OR EXPRESSED REFERENCE TO ANY ITEM OF LABOR OR MATERIALS REQUIRED FOR THE PROPER EXECUTION OF THE WORK IN ACCORDANCE WITH PRESENT PRACTICE OF THE TRADE SHALL NOT RELIEVE THE CONTRACTOR FROM PROVIDING SUCH ADDITIONAL LABOR AND MATERIALS.

2. THIS PROJECT IS A COMPLETE REMODEL OF AN EXISTING BUILDING AND NEW BUILDING CONSTRUCTION. REFER TO THE ARCHITECTURAL DRAWINGS FOR NOTES AND OTHER ELECTRICAL REQUIREMENT NOT SHOWN ON THE ELECTRICAL DRAWINGS AND TO DETERMINE EXISTING CONSTRUCTION TO REMAIN AS WELL AS NEW CONSTRUCTION. IF THERE ARE OMISSIONS OR CONFLICTS BETWEEN THE ELECTRICAL DOCUMENTS AND THE DOCUMENTS OF OTHER TRADES, CLARIFY THESE POINTS WITH THE ARCHITECT BEFORE SUBMITTING A BID. NO EXTRA PAYMENT WILL BE ALLOWED FOR FAILURE TO OBTAIN THIS INFORMATION.

3. THESE PLANS, SPECIFICATIONS, AND ALL MATERIALS SHALL BE IN FULL ACCORDANCE WITH ALL LEGAL AND INDUSTRY REQUIREMENTS, AND STANDARDS INCLUDING WITHOUT LIMITATION TO THE FOLLOWING:

   APPLICABLE CODES
   2025 CALIFORNIA ADMINISTRATION CODE (CAC), PART 1, TITLE 24 CCR
   2022 CALIFORNIA BUILDING CODE (CBC), PART 2, TITLE 24 CCR
   2022 CALIFORNIA ELECTRICAL CODE (CEC), PART 3, TITLE CCR
   2022 CALIFORNIA MECHANICAL CODE (CMC), PART 4, TITLE 24 CCR
   2022 CALIFORNIA PLUMBING CODE (CPC), PART 5, TITLE 24 CCR
   2022 CALIFORNIA ENERGY CODE, PART 6, TITLE 24 CCR
   2022 CALIFORNIA FIRE CODE (CFC), PART 8, TITLE 24 CCR
   2022 CALIFORNIA EXISTING BUILDING CODE (CEBC), PART 10, TITLE 24 CCT
   2022 CALIFORNIA GREEN BUILDING STANDARDS CODE (CALGreen), PART 11, TITLE 24 CCR
   2022 CALIFORNIA REFERENCED STANDARDS CODE, PART 12, TITLE 24
   TITLE 19 CCR, PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS

   A. OTHER REGULATING AGENCIES WHICH MAY HAVE AUTHORITY OVER ANY PORTION OF THE WORK, INCLUDING THE STATE OF CALIFORNIA DIVISION OF INDUSTRIAL SAFETY, AND THOSE CODES AND STANDARDS LISTED IN THESE NOTES AND SPECIFICATIONS.

   B. THE ELECTRICAL SYSTEMS FUNCTIONALITY STANDARDS SET FORTH IN TITLE 7 OF THE CALIFORNIA CIVIL CODE (THE "RIGHT TO REPAIR.

   C. THE MANUFACTURER'S REQUIREMENTS OR RECOMMENDATIONS FOR ANY INCORPORATED PRODUCTS.

   D. THE MOST CURRENT APPROVED ISSUES OF ANY NOTED SPECIFICATIONS, CODES AND STANDARDS, INCLUDING SUPPLEMENTS, UNLESS NOTED OTHERWISE.

4. IN USING THE PLANS FOR BIDDING OR CONSTRUCTION PURPOSES, THE CONTRACTOR IS REQUIRED TO REVIEW ALL OF THE PROJECT'S CONSTRUCTION DOCUMENTS AS A WHOLE IN ORDER TO IDENTIFY ALL REQUIREMENTS THAT DIRECTLY OR INDIRECTLY AFFECT ITS PORTION OF THE ELECTRICAL WORK, EVEN REQUIREMENTS LOCATED IN SECTIONS DESIGNATED AS APPLICABLE TO OTHER TRADES. IN CASE OF CONFLICTS, THE CONTRACTOR SHALL EITHER OBTAIN DIRECTION FROM AN APPROPRIATE OWNER REPRESENTATIVE OR OTHERWISE APPLY THE MORE STRINGENT REQUIREMENT.

5. THE PLANS REPRESENT ONLY THE FINISHED ELECTRICAL, FIRE ALARM, AND LOW VOLTAGE SYSTEMS, AND THEY ARE NOT INTENDED TO INDICATE OR REQUIRE ANY CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCURES.

6. WHEN INTERPRETING THE PLANS, THE FOLLOWING GENERAL RULES APPLY:

   A. WRITTEN DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALED DRAWINGS.

   B. SCALED DIMENSIONS AND GRAPHICALLY SHOWN LOCATIONS ARE TO BE CONSIDERED ONLY APPROXIMATE. FIELD VERIFY DIMENSIONS PRIOR TO BID.

7. IN IMPLEMENTING THE PLANS, THE FOLLOWING GENERAL RULES APPLY:

   A. BECAUSE THE PLANS ARE INTENDED TO SET FORTH THE REQUIREMENTS FOR CONSTRUCTION IN ONLY AN INDUSTRY-STANDARD LEVEL OF QUALITY AND DETAIL, AND THEREFORE ARE INTENDED TO BE SUPPLEMENTED BY APPROPRIATE REQUESTS FOR CLARIFICATION AND INFORMATION, ERRORS AND OMISSIONS ARE TO BE EXPECTED AND ANTICIPATED; AND THE CONTRACTOR IS REQUIRED TO CAREFULLY REVIEW THE PLANS FOR ERRORS AND OMISSIONS AND TO BRING THESE ERRORS AND OMISSIONS TO THE ATTENTION OF AN APPROPRIATE OWNER REPRESENTATIVE IN A TIMELY MANNER AN ASSUMES THE RISK OF THE CONSEQUENCES OF FAILING TO DO SO BEFORE BIDDING OR OTHERWISE PROCEEDING.

   B. THE CONTRACTOR SHALL REVIEW AND VERIFY ALL DIMENSIONS PRIOR TO STARTING CONSTRUCTION, AND NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES OR INCONSISTENCIES.

8. SUBMITTALS WILL BE REVIEWED BY THE ELECTRICAL ENGINEER, IF AT ALL, ONLY PURSUANT TO THE INDUSTRY STANDARD PROTOCOL SET FORTH IN A1A DOCUMENT A201, AND IN NO EVENT WILL THE SUBMITTAL REVIEW PROCESS RELIEVE OR LESSEN THE SUBMITTING CONTRACTOR'S RESPONSIBILITY FOR AN OTHERWISE APPROPRIATE SUBMITTAL.

9. IN NO EVENT WILL ANY SITE VISITS BY THE ELECTRICAL ENGINEER CONCERN CONSTRUCTION MEANS AND METHODS OR CONSTRUCTION SAFETY, AND ALL SUCH MATTERS SHALL REMAIN THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

10. COPIES OF THE PLANS PROVIDED IN ANY ELECTRONIC FORM ARE SUBJECT TO THE SAME PROVISIONS AS THE OTHER INSTRUMENTS OF SERVICE PREPARED BY OR ON BEHALF OF ELECTRICAL ENGINEER FOR THE PROJECT, INCLUDING WITHOUT LIMITATION THE ENGINEER'S COMMON LAW, STATUTORY OR OTHER RESERVED RIGHTS, INCLUDING COPYRIGHTS. A RECIPIENT IS GRANTED AT MOST A TRANSFERABLE NONEXCLUSIVE LICENSE TO RE-USE THE PLANS SOLELY FOR PROJECT PURPOSES, AND NO RECIPIENT IS AUTHORIZED TO USE OR TO ALLOW THE USE OF ALL OR ANY PORTION OF THESE PLANS FOR NAY OTHER PURPOSE, AND ANY USE FOR ANY OTHER PURPOSE WOULD CONSTITUTE ACTIONABLE PLAGIARISM. ELECTRICAL ENGINEER PROVIDES DOCUMENTS IN AN ELECTRONIC FORM ONLY IN ITS STANDARD FORMATS AND CONVENTIONS AND WITH NO GUARANTEE OF COMPATIBILITY WITH ANY RECIPIENT'S SOFTWARE OR HARDWARE, AND ANY USE WITH OR CONVERSION TO OTHER FORMATS OR CONVENTIONS, OR THE USE WITH ANY PARTICULAR SOFTWARE OR HARDWARE, IS AT THE RECIPIENT'S SOLE RISK.

11. REFER TO THE DRAWINGS AND SHOP DRAWINGS OF OTHER TRADES FOR ADDITIONAL DETAILS WHICH AFFECT THE PROPER INSTALLATION OF THIS

12. BEFORE SUBMITTING A BID, THE CONTRACTOR SHALL FAMILIARIZE HIMSELF WITH ALL FEATURES OF THE EXISTING BUILDINGS AND SITE, AND ALL DRAWINGS WHICH MAY AFFECT THE EXECUTION OF THE WORK. NO EXTRA PAYMENT WILL BE ALLOWED FOR FAILURE TO OBTAIN THIS INFORMATION.

13. PROTECT ALL WORK, MATERIALS AND EQUIPMENT FROM DAMAGE FROM ANY CAUSE WHATEVER AND PROVIDE ADEQUATE AND PROPER STORAGE FACILITIEQUIRING THE PROGRESS OF THE WORK. PROVIDE FOR THE SAFETY AND GOOD CONDITION OF ALL THE WORK UNTIL FINAL ACCEPTANCE OF THE WORK BY THE OWNER AND REPLACE ALL DAMAGED OR DEFECTIVE WORK, MATERIALS AND EQUIPMENT BEFORE REQUESTING FINAL ACCEPTANCE.

14. THE DRAWINGS INDICATE IN A DIAGRAMMATIC MANNER, THE DESIRED LOCATIONS OF ARRANGEMENT OF THE COMPONENTS OF ELECTRICAL WORK. DETERMINE EXACT CONDUIT ROUTING, CONDUIT BENDS, AUXILIARY JUNCTION BOXES, SUPPORTS, AND UNDEFINED CONSTRUCTION DETAILS AS A JOB CONDITION TO BE INSTALLED IN ACCORDANCE WITH THE APPLICABLE CODE REQUIREMENTS. PROPER JUDGEMENT MUST BE EXERCISED IN EXECUTING THE WORK SO AS TO SECURE THE BEST POSSIBLE INSTALLATION IN THE AVAILABLE SPACE, AND TO OVERCOME LOCAL DIFFICULTIES DUE TO SPACE LIMITATIONS OR INTERFERENCE OF CONDITIONS ENCOUNTERED.

15. IN THE EVENT CHANGES IN THE INDICATED LOCATIONS OR ARRANGEMENTS ARE NECESSARY, DUE TO DEVELOPED CONDITIONS IN THE BUILDING CONSTRUCTION OR REARRANGEMENT OF EQUIPMENT, SUCH CHANGES SHALL BE MADE WITHOUT COST PROVIDING THE CHANGE IS ORDERED BEFORE THE CONDUIT RUNS, ETC., AND WORK DIRECTLY CONNECTED TO SAME IS INSTALLED AND NO EXTRA MATERIALS ARE REQUIRED.

16. THE DRAWINGS INDICATE APPROXIMATE LOCATIONS OF EXISTING CONDUITS. THE EXACT ROUTING SHALL BE VERIFIED IN FIELD AND LENGTH OF CONDUCTORS SHALL BE ADJUSTED TO THE LENGTH REQUIRED.

17. PERFORM CUTTING AND PATCHING ON THE CONSTRUCTION WORK WHICH MAY BE REQUIRED FOR THE PROPER INSTALLATION OF THE ELECTRICAL WORK. PATCHING SHALL BE OF THE SAME MATERIAL, WORKMANSHIP AND FINISH AS SPECIFIED AND ACCURATELY MATCH SURROUNDING WORK TO SATISFACTION OF THE ARCHITECT.

18. PROVIDE ALL EQUIPMENT WITH ENCLOSURES LISTED OR LABELED FOR USE AND LOCATION WHERE SUCH EQUIPMENT IS INSTALLED.

19. PROVIDE UL LISTED FIRE STOP FOR ALL PENETRATIONS THROUGH FIRE RATED WALLS AND CEILINGS TO MAINTAIN ALL FIRE RATINGS. THE FIRE STOP MATERIALS SHALL BE RE-ENTERABLE AND REUSABLE.

20. PROVIDE COORDINATED SHOP DRAWINGS. INDICATING DIMENSIONED LOCATIONS AND SIZES OF ALL CORE DRILLS FOR REVIEW AND APPROVAL. ALL CORE DRILL LOCATIONS SHALL BE VERIFIED AND APPROVED WITH OWNERS REPRESENTATIVE, STRUCTURAL AND ARCHITECT PRIOR TO CORE DRILL. UTILIZE X-RAY EQUIPMENT TO LOCATE AND VERIFY EXISTING STRUCTURAL ELEMENTS WITHIN SLAB.

21. WHERE EXISTING CIRCUITS ARE SHOWN ON PLANS, THE INFORMATION WAS OBTAINED FROM RECORD DRAWINGS. WHERE NEW CIRCUITS ARE SHOWN IN EXISTING PANELBOARD(S), THE CONTRACTOR SHALL VERIFY THAT THE INDICATED CIRCUITS ARE AVAILABLE IN THE EXISTING PANELBOARD(S), THE CONTRACTOR SHALL RELOCATE SUCH CIRCUITS TO AN AVAILABLE SPACE IN THE EXISTING PANELBOARD(S), AND "AS BUILT" PLANS. IF NO CIRCUIT SPACE IS AVAILABLE IN THE EXISTING PANELBOARD(S), THE CONTRACTOR SHALL REPORT THE DISCREPANCY TO THE ARCHITECT.

22. GROUNDING SHALL BE EXECUTED IN ACCORDANCE WITH ALL APPLICABLE CODES AND REGULATIONS, BOTH OF THE STATE OF CALIFORNIA AND LOCAL AUTHORITIES HAVING JURISDICTION.

23. PROVIDE GROUND WIRE IN EACH CONDUIT CONTAINING CIRCUITS FEEDING RECEPTACLES. THE CONDUIT SHALL NOT BE PERMITTED TO SERVE AS THE ONLY ELECTRICAL GROUND RETURN PATH.

24. WHERE CIRCUIT CHANGES OR ADDITIONS OCCUR IN EXISTING PANELBOARDS, UPDATE PANEL DIRECTORY CARDS WITH NEW TYPEWRITTEN CARDS INDICATING DESCRIPTION OF ALL CIRCUITS.

25. PROVIDE HANDLE TIES AT CIRCUIT BREAKERS TO SIMULTANEOUSLY DISCONNECT AL UNGROUNDED CONDUCTORS OF MULTI-WIRE BRANCH CIRCUITS WITH A SHARED NEUTRAL.

26. UNLESS NOTED OTHERWISE ALL 120 VOLT HOMERUNS OVER 100 FEET SHALL BE #10 AWG MINIMUM. ADJUST CONDUIT SIZE ACCORDINGLY.

27. UNLESS NOTED OTHERWISE ALL 277 VOLT HOMERUNS OVER 200 FEET SHALL BE #10 AWG MINIMUM, OVER 300 FEET SHALL BE #8 AWG MINIMUM. ADJUST CONDUIT SIZE ACCORDINGLY.

28. CONDUIT FOR DATA/ VOICE CABLING SHALL COMPLY WITH THE FOLLOWING ADDITIONAL REQUIREMENTS:

   A. INSIDE BEND RADIUS SHALL BE AT LEAST 10 TIMES ITS INTERNAL DIAMETER.

   B. PROVIDE PULL BOXES WHENEVER CONDUIT LENGTH EXCEEDS 150 FEET AND WHEN COMBINED BENDS ARE GREATER THAN 180 DEGREES.

   C. ALL CONDUIT SHALL BE PROVIDED WITH INSULATED BUSHINGS.

   D. MAINTAIN A MINIMUM CLEARANCE OF 4 FEET FROM MOTORS AND TRANSFORMERS.

   E. MAINTAIN A MINIMUM CLEARANCE OF 12 INCHES FROM POWER CIRCUITS.

   F. MAINTAIN A MINIMUM CLEARANCE OF 12 INCHES FROM LIGHT FIXTURES.

29. COORDINATE MOUNTING HEIGHTS OF RECEPTACLES, SWITCHES, A/V DEVICES, SECURITY DEVICES, ETC. MOUNTED ON COMMON WALLS SO THAT ALL OUTLETS ARE MOUNTED TO ALIGN HORIZONTALLY.

30. NOTIFY THE ARCHITECT IN WRITING WHEN INSTALLATION IS COMPLETE AND THAT A FINAL INSPECTION OF THIS WORK CAN BE PERFORMED. IN THE EVENT DEFECTS OR DEFICIENCIES ARE FOUND DURING THIS FINAL INSPECTION, THEY SHALL BE CORRECTED TO THE SATISFACTION OF THE ARCHITECT BEFORE FINAL ACCEPTANCE CAN BE ISSUED.

31. SHOULD, AT ANY GIVEN TIME DURING CONSTRUCTION, THE FIRE ALARM SYSTEM IS INOPERABLE, THE CONTRACTOR SHALL PROVIDE A FIRE WATCH DURING THE ENTIRE DOWNTIME. INCLUDE ALL COSTS IN BID TO COMPLY WITH THIS PROVISION.

SCHOOL EQUIPMENT ANCHORAGE NOTES

MEP COMPONENT ANCHORAGE NOTE

ALL MECHANICAL, PLUMBING, AND ELECTRICAL COMPONENTS SHALL BE ANCHORED AND INSTALLED PER THE DETAILS ON THE DSA APPROVED CONSTRUCTION DOCUMENTS. THE FOLLOWING COMPONENTS SHALL BE ANCHORED OR BRACED TO MEET THE FORCE AND DISPLACEMENT REQUIREMENTS PRESCRIBED IN THE 2019 CBC, SECTIONS 1617A.1.18 THROUGH 1617A.1.26 AND ASCE 7-16 CHAPTER 13, 26 AND 30.

1. ALL PERMANENT EQUIPMENT AND COMPONENTS.

2. TEMPORARY OR MOVABLE OR MOBILE EQUIPMENT THAT IS PERMANENTLY ATTACHED (E.G HARD WIRED) TO THE BUILDING UTILITY SERVICES SUCH AS ELECTRICITY, GAS OR WATER. "PERMANENTLY ATTACHED" SHALL INCLUDE ALL ELECTRICAL CONNECTIONS EXCEPT PLUGS FOR 110/220 VOLT RECECPTACLES HAVING A FLEXIBLE CABLE.

3. TEMPORARY, MOVABLE OR MOBILE EQUIPMENT WHICH IS HEAVIER 400 POUNDS OR HAS A CENTER OF MASS LOCATED 4 FEET OR MORE ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORT THE COMPONENT IS REQUIRED TO BE RESTRAINED IN MANNER APPROVED BY DSA.

THE FOLLOWING MECHANICAL AND ELECTRICAL COMPONENTS SHALL BE POSITIVELY ATTACHED TO THE STRUCTURE BUT NEED NOT DEMOSTRATED DESIGN COMPLIENCE WITH REFERENCES NOTED ABOVER. THESE COMPONENTS SHALL HAVE FLEXIBLE CONNECTIONS PROVIDED BETWEEN THE COMPONENT AND ASSOCIATED DUCTWORK, PIPING, AND CONDUIT. FLEXIBLE CONNECTIONS MUST ALLOW MOVEMENT IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTIONS:

A. COMPONENTS WEIGHING LESS THAN 400 POUNDS AND HAVING A CENTER OF MASS LOCATED 4 FEET OR LESS ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORT THE COMPONENT.

B. COMPONENTS WEIGHING LESS THAN 20 POUNDS, OR IN THE CASE OF DISTRIBUTED SYSTEMS, LESS THAN 5 POUNDS PER FOOT, WHICH ARE SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL.

THE ANCHORAGE OF ALL MECHANICAL, ELECTRICAL AND PLUMBING COMPONENTS SHALL BE SUBJECT TO THE DESING PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE OR STRUCTURAL ENGINEER DELEGATED RESPONSIBILITY AND ACCEPTANCE BY DSA. THE PROJECT INSPECTOR WILL VERIFY THAT ALL COMPONENTS AND EQUIPMENT HAVE BEEN ANCHORED IN ACCORDANCE WITH ABOVE REQUIREMENTS.

MEP DISTRIBUTION SYSTEM BRACING NOTE FOR PIPING,DUCTWORK,AND ELECTRICAL CONDUIT

PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEMS SHALL BE BRACED TO COMPLY WITH THE FORCES AND DISPLACEMENTS PRESCRIBED IN ASCE 7 SECTION 13.3 AS DEFINED IN ASCE 7 SECTIONS 13.6.5, 13.6.6, AND 13.68; AND 2022 CBC SECTIONS 1617A.1.25, AND IR 16-13.

THE METHOD OF SHOWING BRACING AND ATTACHMENTS TO THE STRUCTURE FOR THE IDENTIFIED DISTRIBUTION SYSTEMS ARE AS NOTED BELOW. THE MEP DESIGN PROFESSIONAL ENGINEER RESPONSIBLE FOR CONTENT ON THESE SHEETS HAS VERIFIED THAT THE DESIGN METHODS IDENTIFIED BELOW ARE IN ACCORDANCE WITH DSR IR 16-13.

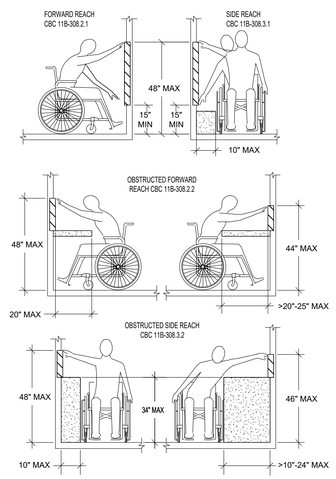
MECHANICAL PIPING (MEP), MECHANICAL DUCTS (MD), PLUMBING PIPING (PP), ELECTRICAL DISTRIBUTION SYSTEMS (E):

MP☐ MD☐ PP☐ E☒ OPTION 1: PROJECT-SPECIFIC DESIGN.

MP☐ MD☐ PP☐ E☐ OPTION 2: DESIGNED BASED ON OSPD OPM, WITHIN PROJECT SUBMITTAL.

MP☐ MD☐ PP☐ E☐ OPTION 3: DESIGN BASED ON OSPD OPM, DEFERRED SUBMITTAL.

ELECTRICAL MOUNTING REACH RANGES



SYMBOL LIST

(ALL SYMBOLS NOT NECESSARILY USED ON THESE DRAWINGS) ALL SYMBOL DESCRIPTIONS ARE SUBJECT TO MODIFICATION AS NOTED ON THE DRAWINGS. VERIFY EXACT LOCATIONS AND HEIGHTS OF OUTLETS WITH ARCHITECTURAL INTERIOR ELEVATIONS PRIOR TO ROUGH-IN.

2	LIGHTING FIXTURE RECESSED MOUNTED WITH OUTLET BOX AND REMOTE MOUNTED JUNCTION BOX CONCEALED ABOVE ACCESSIBLE CEILING. PROVIDE FLEXIBLE CONDUIT CONNECTION 6 FT. MAXIMUM LENGTH, 1/2" DIAMETER MINIMUM, FROM JUNCTION BOX TO FIXTURE OUTLET. PROVIDE CONDUCTORS IN CONDUIT, QUANTITY AS REQUIRED FOR INDICATED CIRCUITS AND SWITCHING CONTROLS, #12 (AWG) MINIMUM.
	OUTLET ON EMERGENCY OR NIGHT LIGHT LIGHTING CIRCUIT.
	POST TOP LIGHTING STANDARD, POLE MOUNTED LUMINAIRE AND POLE SUPPORT BASE.
	LIGHTING STANDARD WITH SINGLE ARM MOUNTED LUMINAIRE AND POLE SUPPORT BASE.
	LIGHTING STANDARD WITH TWIN ARM MOUNTED LUMINAIRES AND POLE SUPPORT BASE.
	UPLIGHT, MOUNTED FLUSH WITH FINISH GRADE.
	FLOODLIGHTING FIXTURE WITH WEATHERPROOF OUTLET BOX.
	EXIT SIGN SINGLE FACE, ON FLUSH CEILING MOUNTED OUTLET BOX.
OS	LIGHTING CONTROL OCCUPANCY MOTION SENSOR ON FLUSH CEILING MOUNTED OUTLET BOX. MOUNT CENTERED IN CEILING TILE.
LC	LIGHTING LEVEL CONTROLLER (PHOTO SENSOR) ON FLUSH CEILING MOUNTED OUTLET BOX. MOUNT CENTERED IN CEILING TILE.
2P a,b	SINGLE POLE TOGGLE SWITCH, ON FLUSH WALL MOUNTED OUTLET BOX, +45". INSTALL MULTIPLE SWITCHES UNDER COMMON COVER PLATE. SUBSCRIPT OR SUPERSCRIPT AT SWITCH SYMBOL INDICATES THE FOLLOWING: 2 - DOUBLE POLE 3 - THREE WAY 4 - FOUR WAY P - PILOT LIGHT M - MANUAL MOTOR STARTERS K - KEY OPERATED R - SPDT MOMENTARY CONTACT RELAY SWITCH V - VAPOR PROOF a,b,c,d, ETC. - MULTIPLE SWITCHES WITH IDENTIFICATION OF OUTLET CONTROLLED
2 / 100	FIXTURE SCHEDULE DESIGNATION: "2" INDICATES FIXTURE TYPE, "100" INDICATES FIXTURE TOTAL WATTAGE.
OS	LIGHTING CONTROL OCCUPANCY SENSOR ON FLUSH WALL MOUNTED OUTLET BOX, +45".
	SWITCH FOR CONTROL OF LOW VOLTAGE LIGHTING RELAY(S), ON FLUSH WALL MOUNTED OUTLET BOX, +45". INSTALL MULTIPLE SWITCHES UNDER COMMON COVER PLATE.
	SINGLE GANG WALL TOGGLE SWITCH ON WALL MOUNTED OUTLET BOX, +45"
	DUPLEX CONVENIENCE RECEPTACLE, ON FLUSH CEILING MOUNTED OUTLET BOX.
	DUPLEX CONVENIENCE RECEPTACLE, IN FLUSH FLOOR OUTLET BOX, UNLESS NOTED OTHERWISE. "R" DESIGNATION INDICATES RECESSED FLOOR BOX WITH MULTI SERVICE FITTINGS.
	CONDUIT, INSTALLED CONCEALED IN WALL OR IN CEILING SPACE. 1/2"C - 2#12, 1#12 (GRD) — 3/4"C - 6#12, 1#12 (GRD) 1/2"C - 3#12, 1#12 (GRD) — 1"C - 7#12, 1#12 (GRD) 3/4"C - 4#12, 1#12 (GRD) — 1"C - 8#12, 1#12 (GRD) 3/4"C - 5#12, 1#12 (GRD) — 1 1/4"C - 8#12, 9#12 (GRD)
	CONDUIT, INSTALLED CONCEALED IN OR UNDER FLOOR OR BELOW GRADE, 3/4" CONDUIT MINIMUM.
	CONDUIT, INSTALLED EXPOSED.
B-5,7,9	HOMERUN TO PANEL "B" FOR CIRCUITS 5, 7, 9 WITH SEPARATED NEUTRAL.
	UNDERGROUND CONDUIT STUBOUT, STUB 5'-0" FROM BUILDING OR WALKWAY, CAP, MARK AND RECORD.
EP-1	PANELBOARD, ADJACENT LINE INDICATES PANEL FRONT. ADJACENT BALLOON INDICATES PANEL DESIGNATION "A", SEE DRAWING E-1 FOR PANEL SCHEDULE.
DBA E-1	FLOOR STANDING SWITCHGEAR ADJACENT BALLOON INDICATES EQUIPMENT DESIGNATION "DBA", SEE SINGLE LINE DIAGRAM AND/OR SCHEDULE.
	TERMINAL CABINET OR EQUIPMENT CABINET. ADJACENT LINE INDICATES CABINET FRONT.
F	FUSED SAFETY SWITCH (DISCONNECT), HORSE POWER RATED. MOUNT ON WALL +45", OR ON EQUIPMENT +36". PROVIDE SWITCH AND FUSES SIZED PER EQUIPMENT MANUFACTURER REQUIREMENTS.
J	JUNCTION BOX CONCEALED ABOVE ACCESSIBLE CEILING OR ON EXPOSED CEILING. U.N.O.
	DUPLEX CONVENIENCE RECEPTACLE VERTICAL ON FLUSH WALL MOUNTED OUTLET BOX, +18". STEM INDICATES WALL MOUNTED OUTLET BOX. TYPICAL.
	DOUBLE DUPLEX (FOUR-PLEX) CONVENIENCE RECEPTACLE ON ONE FLUSH WALL MOUNTED OUTLET BOX +18".
	DUPLEX CONVENIENCE RECEPTACLE WITH INTERNAL GROUND FAULT INTERRUPTER, VERTICAL ON FLUSH WALL MOUNTED OUTLET BOX +18". U.N.O.
	DUPLEX CONVENIENCE RECEPTACLE HORIZONTAL ON FLUSH WALL MOUNTED OUTLET BOX, +6" ABOVE COUNTER SPLASH.
	COMBINATION DUPLEX CONVENIENCE RECEPTACLE WITH ONE (1) USB TYPE-A PORT AND ONE (1) USB TYPE-C PORT ON FLUSH WALL MOUNTED OUTLET BOX, +18". ADJACENT "P" INDICATES SERVICE PEDESTAL FRAME HOUSING WITH CHASE NIPPLE ON FLUSH MOUNT OUTLET BOX.
S	CEILING MOUNTED SPEAKER AND GRILLE ON FLUSH IN CEILING BACK BOX. PROVIDE ONE SET OF CAT6A CABLE ON J-HOOKS AND CONDUIT SLEEVES TO BUILDING DATA CABINET/RACK AND CONNECT AS REQUIRED.
ES	STANDARD SIZE COMBINATION IP/SPEAKER UNIT ON FLUSH IN WALL BACKBOX,+90" AFF WITH 1" CONDUIT TO ACCESSIBLE CEILING SPACE U.N.O. PROVIDE TWO (2) SETS OF CAT 6A CABLES THROUGH CONDUIT, J-HOOKS AND CONDUIT SLEEVES TO BUILDING MDF/IDF CABINET FOR INFORMATION.
C	COMPUTER/DATA OUTLET, WITH A SINGLE COMPUTER CONNECTOR, ON FLUSH WALL MOUNTED OUTLET BOX . PROVIDE 1" CONDUIT TO ACCESSIBLE CEILING SPACE UNLESS NOTED OTHERWISE, "C" INDICATES HORIZONTAL IN FLUSH WALL MOUNTED OUTLET BOX +6" ABOVE COUNTER SPLASH.
M	COMPUTER OUTLET WITH A SINGLE COMPUTER CONNECTOR, AND ONE HDMI CONNECTOR ON FLUSH MOUNTED OUTLET BOX, INSTALLED BEHIND FLAT SCREEN TV AT +44" UNLESS NOTED OTHERWISE. VERIFY EXACT LOCATION OF OUTLET PRIOR TO ROUGH-IN. PROVIDE 1" CONDUIT FROM OUTLET INTO ACCESSIBLE CEILING SPACE FOR EXTENSION OF DATA AND HDMI CABLES.
VCS	VIDEO CONTROL OUTLET WITH TOUCH SCREEN CONTROLLER ON 3-GANG WALL PLASTER RING AT +45", UNLESS NOTED OTHERWISE. PROVIDE 1 1/4" CONDUIT FROM THE BOX TO ACCESSIBLE CEILING.

REGISTERED PROFESSIONAL ENGINEER STEPHEN R. ZAUNER No. E 10372 Exp. 09-30-26 ELECTRICAL STATE OF CALIFORNIA
01/05/2026

REGISTERED ARCHITECT STEPHEN CLARKE No. 18346 Exp. 10-31-27 STATE OF CALIFORNIA

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DIV. OF THE STATE ARCHITECT
APP: 04-123574 INC:
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 01/15/2026

FBA Engineering
Consulting Electrical Engineers
150 Paularino Avenue Suite A120
Costa Mesa, CA 92626
949.852.9995 • 949.852.1657 (fax)
fbaeng.com
FBA Job Number: 874.991.15

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ABBREVIATIONS

A.F.F.	ABOVE FINISH FLOOR
A.F.G.	ABOVE FINISH GRADE
A.I.C.	AMPERES INTERRUPTING CAPACY (SYMMETRICAL)
AF / AT	AMP FRAME, AMP TRIP
AMP, A	AMPERE
AS / AF	AMP SWITCH, AMP FUSE
AWG	AMERICAN WIRE GAUGE
C	CONDUIT
C.O.	CONDUIT ONLY
CB	CIRCUIT BREAKER
CIRC., CKT.	CIRCUIT
CLCB	CURRENT LIMITING CIRCUIT BREAKER
CONN	CONNECTED
DIA	DIAMETER
E	EXISTING EQUIPMENT AND/OR DEVICE TO REMAIN AND BE PROTECTED IN PLACE.
E-O-L	END-OF-LINE CIRCUIT TERMINATOR
EF	EXHAUST FAN
EMCS	ENERGY MANAGERMENT CONTROL SYSTEM
EMT	ELECTRICAL METALLIC TUBING
ER	EXISTING EQUIPMENT AND/OR DEVICE TO BE DISCONNECTED, RELOCATED AND REINSTALLED AS INDICATED.
EWC	ELECTRICAL WATER COOLER
FA	FIRE ALARM
FLA	FULL LOAD AMPS
FT OR"	FEET
GFI	GROUND FAULT INTERRUPTER
GRD	GROUND
H.W.D.L.	HEIGHT, WIDTH, DEPTH, LENGTH
HID	HIGH INTENSITY DISCHARGE
HOA	HAND-OFF-AUTO
HP	HORSEPOWER
HVAC	HEATING, VENTILATING AND AIR CONDITIONING
IG	ISOLATED GROUND
IN. OR"	INCHES
J-BOX	JUNCTION BOX
KVA	KILOVOLT AMPERES
KW	KILOWATT
L. F.	LINEAR FEET
LCL	LONG CONTINUOUS LOAD
LTG, LTS	LIGTING
MCB	MAIN CIRCUIT BREAKER
MCC	MOTOR CONTROL CENTER
MCM	THOUSAND CIRCULAR MILS
MCP	MOTOR CIRCUIT PROTECTOR
MLO	MAIN LUGS ONLY
MTD	MOUNTED
MW	MICROWAVE
NC	NORMALLY CLOSED
NEC	NATIONAL ELECTRIC CODE
NF	NON-FUSED
NIC	NOT IN CONTRACT
NO	NORMALLY OPEN
NO. OR#	NUMBER
OFCI	OWNER FURNISHED, CONTRACTOR INSTALLED
PA	PUBLIC ADDRESS
PH. OR ϕ	PHASE
PRIMARY	OVER 600 VOLTS
PROVIDE	FURNISH, INSTALL AND CONNECT
R	EXISTING EQUIPMENT AND/OR DEVICE TO BE DISCONNECTED AND REMOVED. INCLUDING RELATED CONDUIT AND CONDUCTORS.
REC, RECPT	RECEPTACLE
U.N.O.	UNLESS NOTED OTHERWISE

SHEET INDEX

E-0.1 SYMBOL LIST AND GENERAL NOTES
E-0.2 LIGHTING FIXTURE SCHEDULE AND DETAILS
ES-1.1 SITE ELECTRICAL PLAN
ES-1.2 ENLARGED DEMOLITION SITE ELECTRICAL
ES-1.3 ENLARGED SITE ELECTRICAL PLAN

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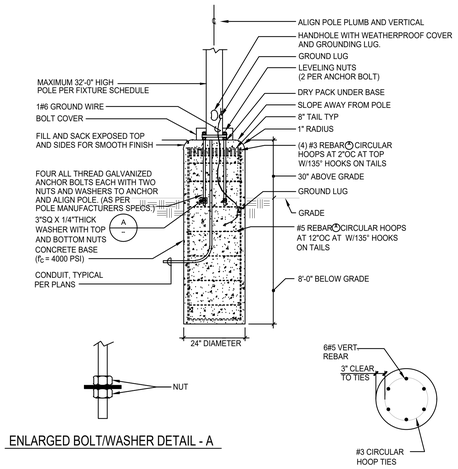
FIXTURE SCHEDULE

IMAGE	TYPE	DESCRIPTION	MANUFACTURER		LAMP				POWER				MOUNTING	NOTES
			NAME	CATALOG NO.	TYPE	LUMENS	CCT	CRI (MIN)	VOLTAGE	INPUT WATTAGE	BALLAST TYPE	BALLAST DIM TYPE	TYPE	
	SL1	LED POST TOP LIGHT FIXTURE WITH DIA-CAST ALUMINUM HOUSINGFULLY GASKETED, INTEGRAL HEAT SINK, STAINLESS STEEL HARDWARE, IP66 RATED, EDGE-LIT, TYPE V WIDE LIGHT DISTRIBUTION, 120/277V, MOUNTED ON 12-FOOT HIGH, ROUND STEEL POLE, CUSTOM COLOR FINISH TO MATCH EXISTING POST TOP LIGHT FIXTURES ON SITE.	KIM LIGHTING	UR20 EDGE-LIT SERIES OR EQUAL	LED	15,075	4K	80	277	135	INTERNAL DRIVER/POWER SUPPLY	NON-DIMMING	POLE	N/A
	SL2	SAME AS TYPE SL1 EXCEPT WITH TWIN HEADS.				30,150				270				
	SL3	IN-GRADE LED UP LIGHT FIXTURE WITH THERMAL PLASTIC HOUSING. STAINLESS STEEL FACE TRIM. CLEAR TEMPERED GLASS LENS, IP68 RATED, BLUETOOTH CONTROLLABLE	KIM LIGHTING	LTV81SS SERIES OR EQUAL	LED	3,835	4K	80	277	44	INTERNAL DRIVER/POWER SUPPLY	NON-DIMMING	FLUSH IN GRADE	N/A
	SL4	LED SINGLE HEAD PARKING LOT LUMINAIRE WITH SINGLE PIECE DIE CAST ALUMINUM HOUSING, ACRYLIC LENS, TYPE 4 LIGHT DISTRIBUTION, INTEGRAL MOTION DENSOR, MOUNTED ON 17'-6" HIGH SQUARE SENSOR, MOUNTED ON 17'-6" HIGH SQUARE STEEL POLE, CUSTOM COLOR AS SELECTED BY ARCHITECT.	LITHONIA LIGHTING	DSXO-LES-P2 SERIES OR EQUAL	LED	6,272	4K	80	120	45	INTERNAL DRIVER/POWER SUPPLY	0-10V	POLE	N/A

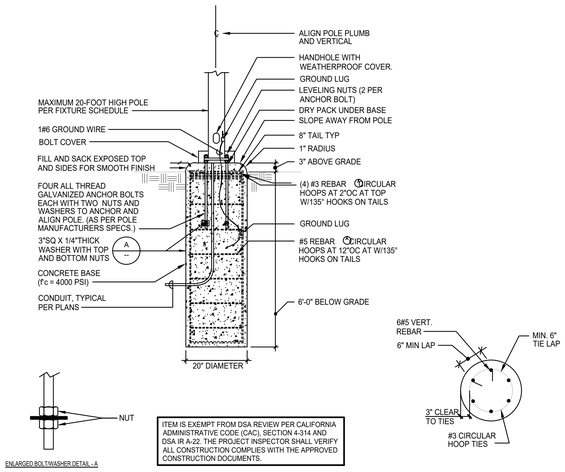
RAISED POLE BASE DETAIL

SCALE NONE | 3



SEMI-FLUSH POLE BASE DETAIL

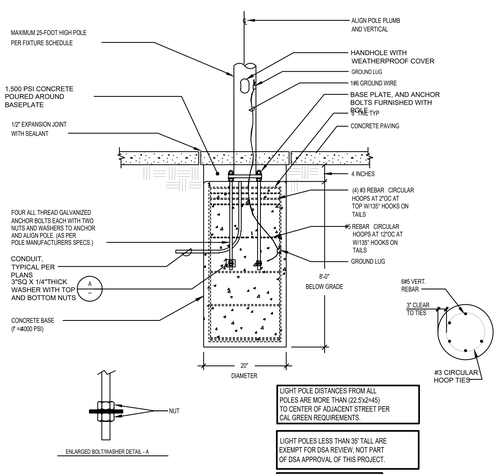
SCALE NONE | 2



ITEM IS EXEMPT FROM DSA REVIEW PER CALIFORNIA ADMINISTRATIVE CODE (CAC), SECTION 4-314 AND DSA IR A-22. THE PROJECT INSPECTOR SHALL VERIFY ALL CONSTRUCTION COMPLIES WITH THE APPROVED CONSTRUCTION DOCUMENTS.

FLUSH POLE BASE DETAIL

SCALE NONE | 1



LIGHT POLE DISTANCES FROM ALL POLES ARE MORE THAN (22.5)x(45) TO CENTER OF ADJACENT STREET PER CAL GREEN REQUIREMENTS.

LIGHT POLES LESS THAN 35' TALL ARE EXEMPT FOR DSA REVIEW, NOT PART OF DSA APPROVAL OF THIS PROJECT.

NOT PART OF DSA SUBMITTAL

FIXTURE NOTES

(NOTE: REFER TO SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS)

1. FIXTURES LOCATED OUTDOORS SHALL BE RATED FOR STARTING AND OPERATING TEMPERATURES BELOW 0-DEGREES FAHRENHEIT.
2. FIXTURES WITH THE SAME TYPE # SHALL BE THE PRODUCTS OF THE SAME MANUFACTURER, (I.E., TYPE #1, 1A, 1B, ETC., SHALL BE THE SAME MANUFACTURER).
3. THE CONTRACTOR SHALL VERIFY ACTUAL CEILING AND WALL CONSTRUCTION TYPE AS DEFINED ON THE ARCHITECTURAL DRAWINGS AND FURNISH LIGHTING FIXTURES WITH THE CORRECT AND COMPLETE MOUNTING HARDWARE AND MOUNTING DEVICES TO ACCOMMODATE BUILDING CONSTRUCTION AT EACH INSTALL LOCATION, WHETHER OR NOT SUCH VARIATIONS ARE INDICATED BY THE FIXTURE CATALOG NUMBER.
4. THE CONTRACTOR SHALL VERIFY DEPTH OF ALL RECESSED LIGHTING FIXTURES WITH ARCHITECTURAL DRAWINGS PRIOR TO ORDERING FIXTURES. ANY DISCREPANCIES THAT WILL CAUSE RECESSED FIXTURES NOT TO FIT INTO CEILING/WALL SPACES SHALL BE REPORTED TO THE OWNER'S REPRESENTATIVE PRIOR TO SUBMITTING SHOP DRAWINGS AND PRIOR TO ORDERING FIXTURES.
5. LIGHT FIXTURES RECESSED IN CEILING OR WALL WITH A ONE HOUR OR MORE FIRE RATING BUILDING CONSTRUCTION, EACH FIXTURE SHALL BE ENCLOSED IN A BOX WHICH HAS A FIRE RATING EQUAL TO THAT OF THE BUILDING CONSTRUCTION. PROVIDE MINIMUM OF 3" CLEAR FROM ALL SIDES AND TOP OF RECESSED LIGHT FIXTURES.
6. WALL AND CEILING INSULATION SHALL BE INSTALLED TO ALLOW 3" MINIMUM CLEARANCE FROM BOTTOM, SIDES AND TOP OF RECESSED LIGHT FIXTURES.
7. VERIFY MOUNTING HEIGHT OF ALL WALL MOUNTED FIXTURES WITH ARCHITECT PRIOR TO ROUGH-IN.
8. REFER TO ARCHITECTURAL REFLECTED CEILING PLANS AND WALL ELEVATIONS FOR EXACT INSTALL LOCATION OF ALL FIXTURES.
9. VERIFY VOLTAGE BEING SUPPLIED TO FIXTURES PRIOR TO SUBMITTING SHOP DRAWINGS AND PRIOR TO ORDERING. FIXTURE VOLTAGE SHALL MATCH BRANCH CIRCUITS CONNECTING TO RESPECTIVE FIXTURE.
10. SUSPENDED MOUNT LIGHT FIXTURES THAT MAY STRIKE STRUCTURAL ELEMENTS, WALL OR MECHANICAL DUCT WORK IF SWIVELED AT +45-DEGREES SHALL BE SWAY BRACED WITH AIR CRAFT CABLE TO PREVENT STRIKING SAID APPURTENANCES DURING SEISMIC EVENTS, AS REQUIRED.
11. OCCUPANCY MOTION SENSOR SYSTEM SHALL BE PROVIDED IN EVERY ROOM/SPACE LOCATION THROUGHOUT THE FACILITY AND AS DESCRIBED IN THE SPECIFICATIONS, WHETHER SYMBOLS ARE SHOWN OR NOT SHOWN ON THE PLANS.
12. PROVIDE TESTING CERTIFICATION AND COMMISSIONING OF LIGHTING FIXTURES, INSTALLATION, LIGHTING CONTROL SYSTEM AND LIGHTING SYSTEM OPERATION.
13. CONTRACTOR SHALL COORDINATE INSTALLATION OF RECESSED LIGHTING FIXTURES IN HARD LID OR STUCCO CEILING AREAS WITH FRAMING CONTRACTOR.

ADDITIONAL SUBMITTAL REQUIREMENTS FOR LED LIGHT FIXTURES

PART 1. THE MANUFACTURER AND MODEL NUMBER OF THE LED LIGHT FIXTURE INDICATED IN THE FIXTURE SCHEDULE HAS BEEN SPECIFIED BECAUSE THE PERFORMANCE OF THE FIXTURE HAS BEEN ANALYZED IN DETAIL AND DETERMINED TO ADEQUATELY MEET THE REQUIREMENTS OF THE PROJECT.FIXTURES OTHER THAN THE SPECIFIED ITEM MAY BE PROPOSED AS EQUAL PROVIDED CONTRACTOR COMPLIES WITH THE FOLLOWING ADDITIONAL REQUIREMENTS:

A. AS PART OF THE SHOP DRAWINGS SUBMITTAL PROCESS, CONTRACTOR SHALL PROVIDE DOCUMENTATION FOR APPROVAL IN ACCORDANCE WITH PART 2 BELOW. ALL SUBMITTALS SHALL BE MADE IN A TIMELY MANNER TO ALLOW AT LEAST TWO WEEKS OF ENGINEERING REVIEW WITHOUT JEOPARDIZING THE PROJECT SCHEDULE.

B. CONTRACTOR SHALL NOT ACQUIRE THE PROPOSED EQUAL LIGHT FIXTURES BEFORE OBTAINING WRITTEN APPROVAL OF SUBSTITUTION TEN (10) DAYS BEFORE THE BID.

C. CONTRACTOR SHALL REMOVE AND REPLACE WITHOUT CHANGE TO CONTRACT PRICE, ANY INSTALLED LIGHT FIXTURE WHICH DO NOT PERFORM AS INDICATED IN THE CONTRACTOR'S APPROVED SUBMITTALS.

PART 2. SHOULD THE CONTRACTOR CHOOSE TO PROPOSE AN LED LIGHT LIGHT FIXTURE OTHER THAN THE SPECIFIED ITEM, CONTRACTOR SHALL PREPARE AND SUBMIT FOR APPROVAL OF THE FOLLOWING ITEMS IN ADDITION TO MANUFACTURERS SPECIFICATION SHEETS:

A. FOR INTERIOR FIXTURES, SCALED DRAWINGS OF EACH ROOM WHERE THE FIXTURE(S) ARE TO BE INSTALLED SHOWING COMPUTER GENERATED POINT-BY POINT CALCULATIONS OF THE MAINTAINED HORIZONTAL FOOTCANDLE LEVELS AT 30-INCHES ABOVE THE FLOOR.FOR CLASSROOMS, SUBMITTALS SHALL ALSO INDICATE VERTICAL FOOTCANDLE LEVELS ON THE PRIMARY TEACHING WALL WHITE BOARD IN EACH ROOM. THE CALCULATIONS SHALL BE BASED ON THE IES LIGHT DISTRIBUTION FOR THE FIXTURES PROPOSED USING 80% CEILING REFLECTANCE, 20% WALL REFLECTANCE, AND 10% FLOOR REFLECTANCE..

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01/05/2026

REGISTERED ARCHITECT — RUSSELL CLARKE — No. 10-31-27 — STATE OF CALIFORNIA

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FBA Engineering
Consulting Electrical Engineers
150 Paularino Avenue Suite A120
Costa Mesa, CA 92626
949.852.9995 ♥ 949.852.1657 (fax)
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VALLEY VIEW HIGH SCHOOL MODERNIZATION
24100 Delphinium Ave, Moreno Valley, CA 92553
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LIGHTING FIXTURE SCHEDULE AND DETAILS

E-0.2

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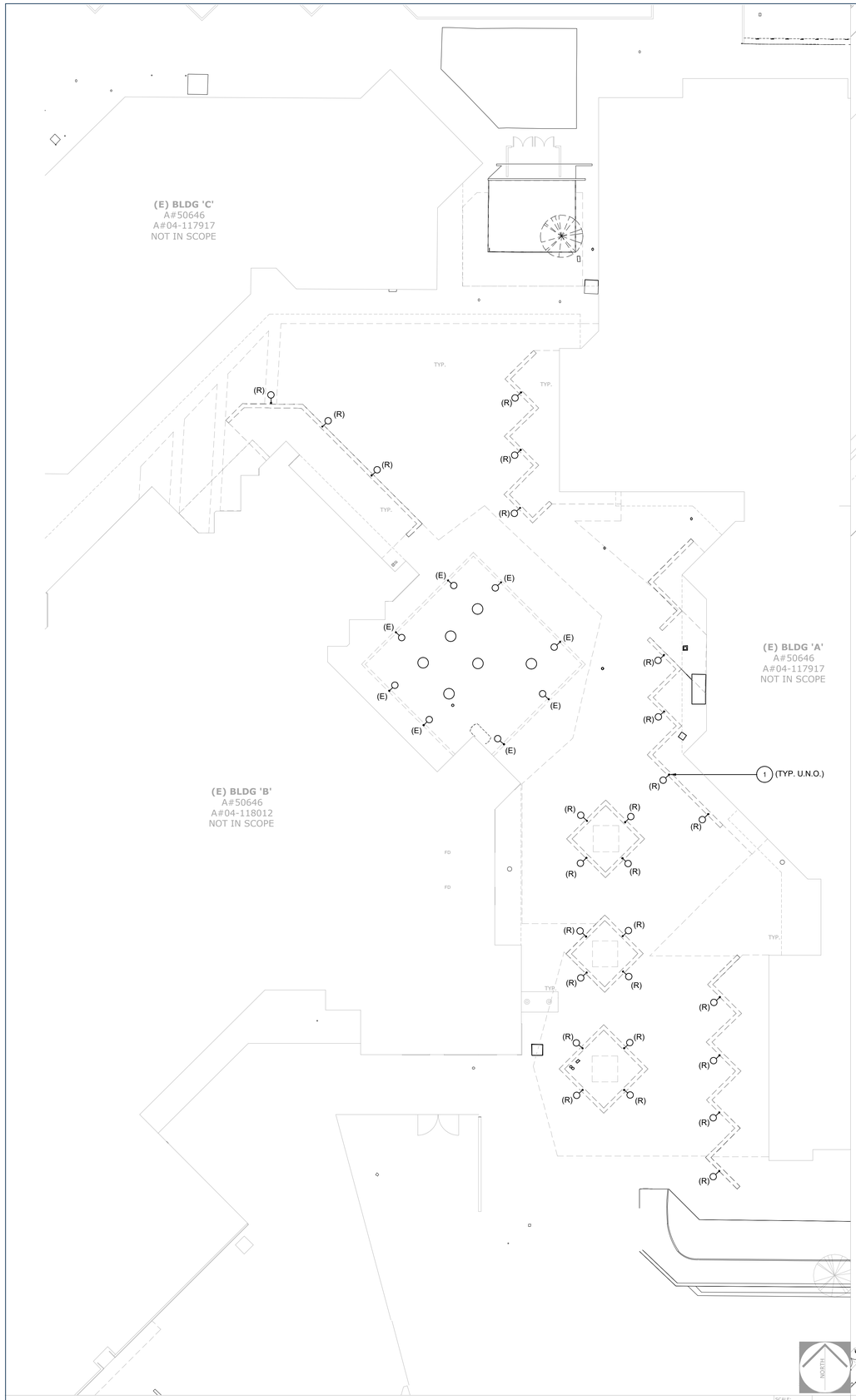
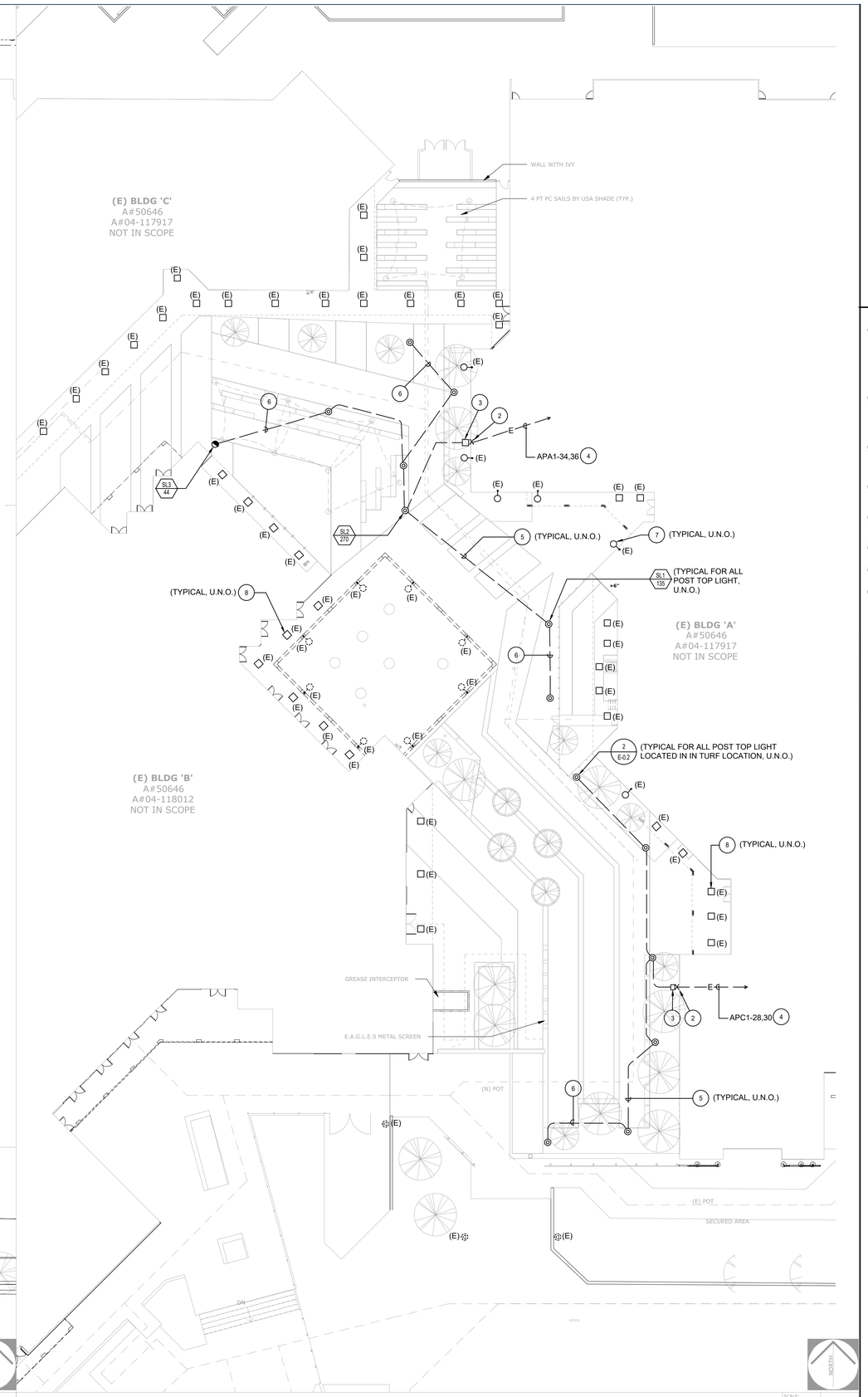


SITE ELECTRICAL DEMOLITION PLAN

SCALE: 1" = 20'-0" 2



SITE ELECTRICAL PLAN - NEW

SCALE: 1" = 20'-0" 1

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Costa Mesa, CA 92626
949.852.9995 ♥ 949.852.1657 (fax)
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PLAN NOTES

1. EXISTING FLUSH IN WALL LOW LEVEL LIGHT FIXTURE SHALL BE DISCONNECTED AND REMOVED INCLUDING ALL ASSOCIATED BRANCH CIRCUIT WIRING BACK TO EXISTING SOURCE. EXISTING UNDERGROUND CONDUIT SHALL BE INTERCEPTED AND REUSED FOR ROUTING NEW BRANCH CIRCUIT WIRING AS INDICATED. REFER TO PLAN NOTE 2 THIS SHEET FOR ADDITIONAL INFORMATION.
2. EXISTING UNDERGROUND BRANCH SITE LIGHTING CIRCUIT CONDUIT AND WIRING SHALL BE INTERCEPTED AND EXTENDED AS INDICATED. VERIFY THE EXACT LOCATION OF EXISTING UNDERGROUND CONDUIT IN FIELD PRIOR TO ELECTRICAL ROUGH-IN WORK.
3. PROVIDE 18" X 30" X 24" D. CONCRETE PULLBOX WITH BOLT-DOWN TRAFFIC RATED COVER ENGRAVED: "SITE LIGHTS". INSTALL PULLBOX LID AND COVER TO BE FLUSH WITH THE ADJACENT FINISH GRADE.
4. PROVIDE 4 # 10, 1 # 10 GRD IN EXISTING CONDUIT AND EXTEND BACK TO EXISTING PANEL FOR TERMINATION OF NEW BRANCH CIRCUIT WIRING.
5. PROVIDE 4 # 10, 1 # 10 GRD – 3/4" C. EXTEND THE ENTIRE LENGTH OF CIRCUITS.
6. PROVIDE 2 # 10, 1 # 10 GRD – 3/4" C.
7. EXISTING WALL MOUNTED EXTERIOR LIGHT FIXTURE TO REMAIN.
8. EXISTING RECESSED DOWNLIGHT FIXTURE IN EXTERIOR SOFFIT TO REMAIN.

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SITE ELECTRICAL PLANS

ES-1.1

14812

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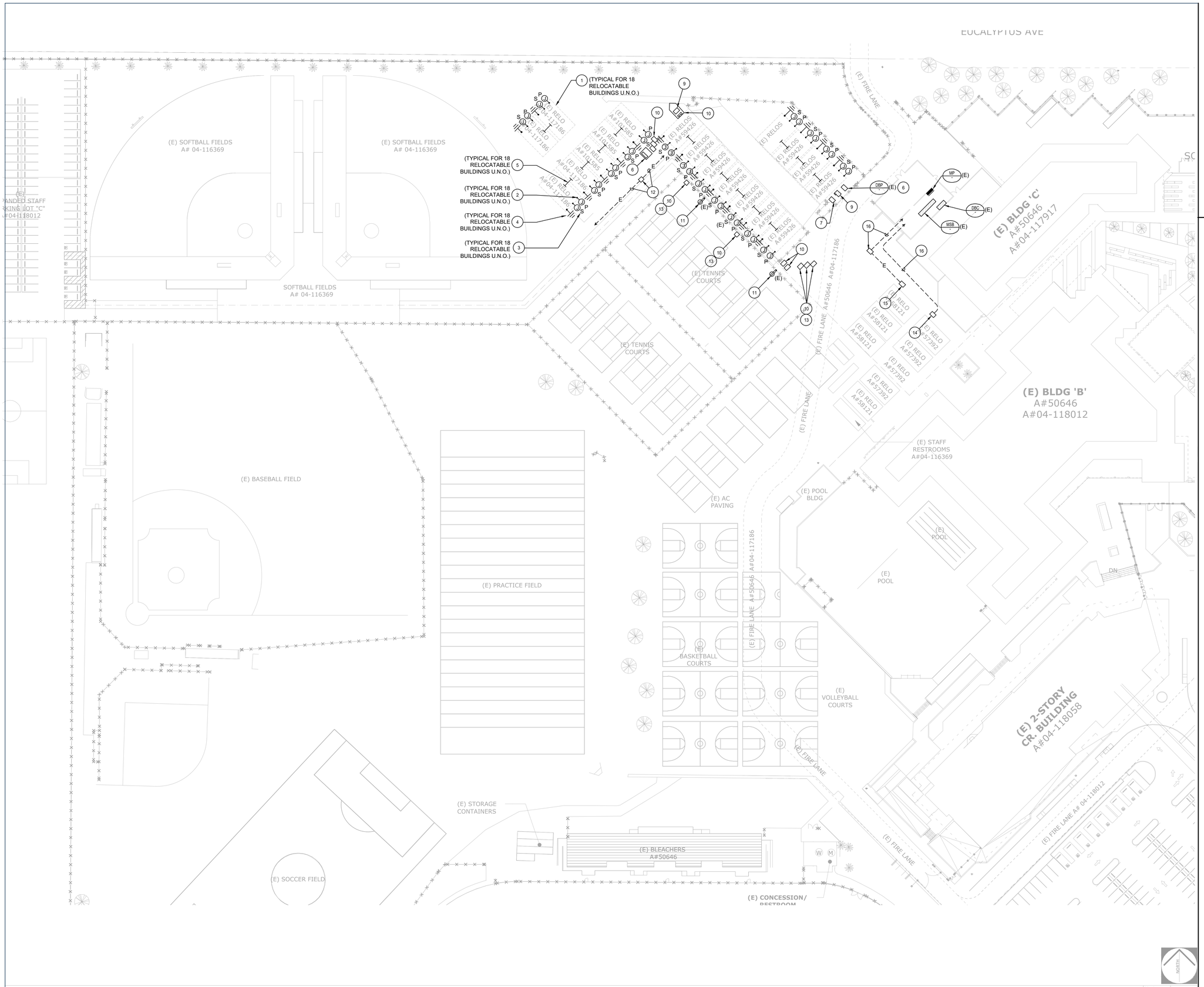

ENLARGED DEMOLITION SITE ELECTRICAL PLAN SCALE: 1" =40'-0" 1

PLAN NOTES

1. EXISTING RELOCATABLE BUILDING SHALL BE DISCONNECTED AND REMOVED FROM SITE. REFER TO ARCHITECTURAL PLANS FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
2. UNCOUPLE AND DISCONNECT EXISTING POWER PULLBOX AND CONDUIT FROM RELOCATABLE BUILDING TO BE REMOVED FROM SITE. REMOVE ALL EXISTING SITE CONDUITS AND CONDUCTORS EXTENDING UNDERGROUND BACK TO EXISTING POWER SOURCE.
3. UNCOUPLE AND DISCONNECT EXISTING SIGNAL SYSTEMS PULLBOXES AND CONDUITS FROM EXISTING RELOCATABLE BUILDING TO BE REMOVED FROM SITE. REMOVE ALL EXISTING SITE CONDUITS AND CONDUCTORS EXTENDING UNDERGROUND BETWEEN THE RELOCATABLE BUILDINGS TO BE REMOVED FROM SITE.
4. DISCONNECT AND REMOVE EXISTING RELOCATABLE BUILDING GROUNDING SYSTEM INCLUDING CONDUIT, WIRE, PULLBOX, AND GROUND ROD.
5. DISCONNECT AND REMOVE EXISTING CONDUIT SLEEVES EXTENDING BETWEEN RELOCATABLE BUILDINGS ABOVE ACCESSIBLE CEILING INCLUDING ALL CONDUCTORS ROUTING EXPOSED THROUGH THE CEILING SPACES AND IN BETWEEN THE RELOCATABLE BUILDINGS. PROVIDE SEALS FOR ALL WALL PENETRATION OPENINGS.
6. EXISTING DISTRIBUTION SWITCHBOARD SHALL BE DISCONNECTED AND REMOVED INCLUDING ALL RELATED SITE CONDUIT, CONDUCTORS, CONCRETE PAD, AND MOUNTING ASSEMBLY.
7. EXISTING TRANSFORMER SERVING DISTRIBUTION SWITCHBOARD "DBP" SHALL BE DISCONNECTED AND REMOVED INCLUDING RELATED CONDUIT, CONDUCTORS, CONCRETE PAD, GROUNDING SYSTEM, AND MOUNTING ASSEMBLY.
8. EXISTING FLOOR STANDING SIGNAL SYSTEM DISTRIBUTION ENCLOSURE SHALL BE DISCONNECTED AND REMOVED INCLUDING RELATED CONDUIT, CONDUCTORS, CONCRETE PAD, AND MOUNTING ASSEMBLY.
9. EXISTING DATA NETWORK SYSTEM RITTAL CABINET SHALL BE DISCONNECTED AND REMOVED INCLUDING RELATED SITE CONDUIT, CONDUCTORS, CONCRETE PAD, AND MOUNTING ASSEMBLY.
10. EXISTING IN-GRADE CONCRETE PULLBOXES FOR POWER, SIGNAL, AND FIRE ALARM SYSTEMS INCLUDING UNDERGROUND SITE CONDUITS WITH WIRING SERVING PORTABLE BUILDINGS TO BE REMOVED FROM SITE, SHALL BE DISCONNECTED AND REMOVED INCLUDING RAISED CONCRETE PAD. AND WIRING TO THE SOFTBALL FIELDS SHALL REMAIN. PROTECT IN PLACE. ADJUST EXISTING PULLBOX LID AND COVER TO BE FLUSH WITH THE NEW FINISH GRADE.
11. EXISTING TENNIS COURT RECEPTACLE OUTLETS INCLUDING RELATED UNDERGROUND POWER CONDUITS WITH WIRING EXTENDING TO PANEL "MP" LOCATED IN MAIN ELECTRICAL ROOM SHALL REMAIN. PROTECT IN PLACE.
12. EXISTING UNDERGROUND POWER FEEDER WITH CONDUCTORS EXTENDING FROM EXISTING DISTRIBUTION SWITCHBOARD "DBP" TO PANELBOARD "SB" LOCATED AT THE SOFTBALL FIELD. REFER TO SHEET ES-1.3 FOR WORK TO INTERCEPT AND REROUTE THE POWER FEDER TO NEW POWER SOURCE.
13. EXISTING PULLBOXES USED FOR EXTENDING UNDERGROUND POWER FEEDER TO THE SOFTBALL FILED SHALL REMAIN. PROTECT IN PLACE. ADJUST EXISTING PULLBOX LID AND COVER TO BE FLUSH WITH THE NEW FINISH GRADE.
14. EXISTING DISTRIBUTION SWITCHBOARD SERVING RELOCATABLE BUILDINGS THAT ARE REMAINING ON SITE, SHALL REMAIN. PROTECT IN PLACE.
15. EXISTING FLOOR STANDING SIGNAL SYSTEMS TERMINAL CABINET SERVING RELOCATABLE BUILDINGS THAT ARE REMAING ON SITE, SHALL REMAIN. PROTECT IN PLACE.
16. EXISTING UNDERGROUND POWER AND SIGNAL SYSTEMS CONDUITS WITH CONDUCTORS AND RELATED PULLBOXES SERVING RELOCATABLE BUILDINGS THAT ARE REMAING ON SITE, SHALL REMAIN. PROTECT IN PLACE.

REGISTERED PROFESSIONAL ENGINEER STEPHEN R. ZAUNER No. E 10372 Exp. 09-30-26 ELECTRICAL STATE OF CALIFORNIA
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REGISTERED ARCHITECT ROBERT CLARKE No. 11546 Exp. 10-31-27 STATE OF CALIFORNIA

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Consulting Electrical Engineers
150 Paularino Avenue Suite A120
Costa Mesa, CA 92626
949.852.9995 ♦ 949.852.1657 (fax)
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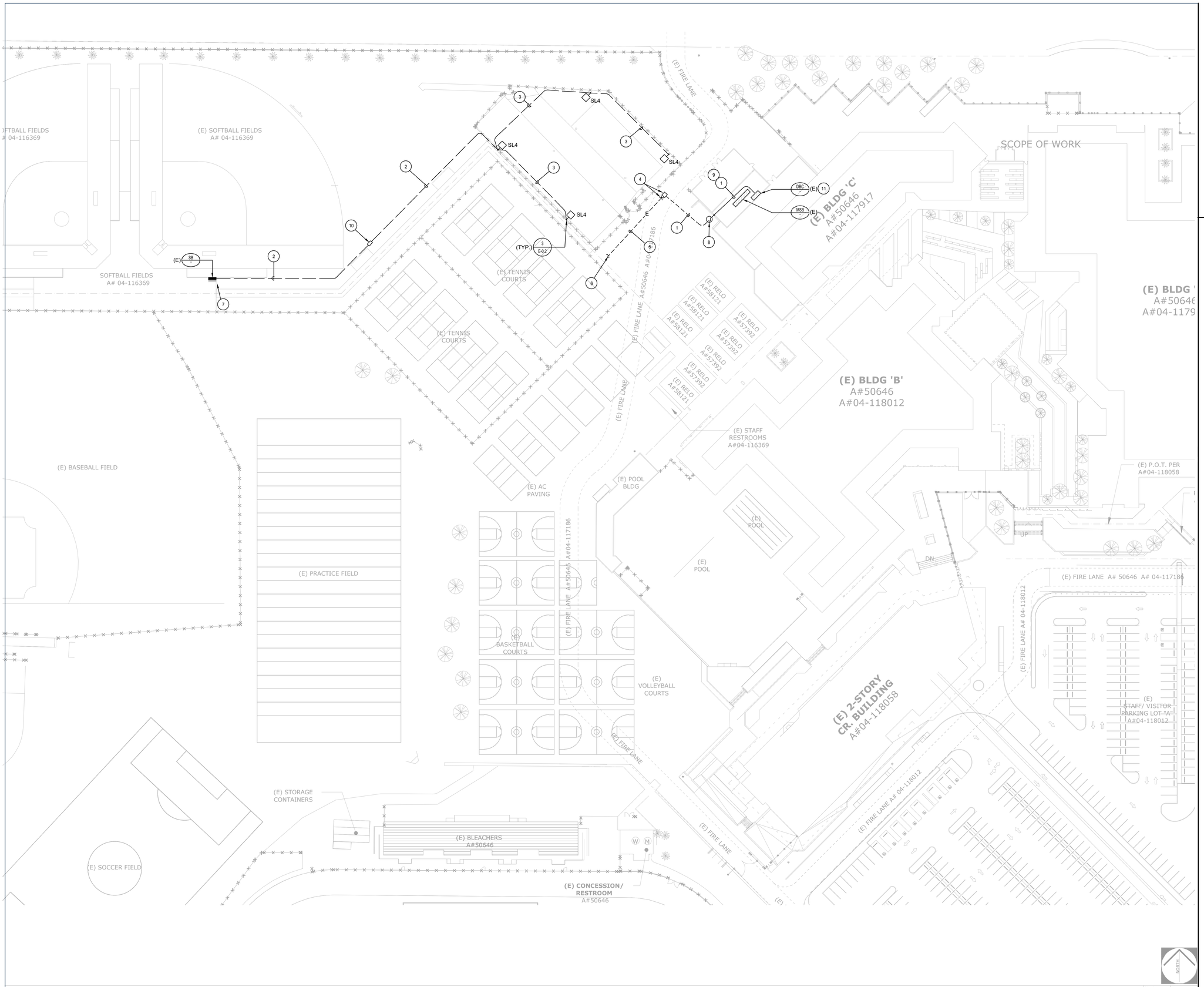
VALLEY VIEW HIGH SCHOOL MODERNIZATION
24100 Delphinium Ave, Moreno Valley, CA 92553
MORENO VALLEY UNIFIED SCHOOL DISTRICT

ENLARGED DEMOLITION SITE ELECTRICAL PLAN

ES-1.2

14812

FBA Engineering / Plot Date: 1/5/2026 3:21 PM / Plotted by: Blake Meech / Drawing Location: I:\874\991\15\ES-1.2_874991\15_ENLARGED DEMOLITION SITE ELECTRICAL PLAN.dwg



ENLARGED SITE ELECTRICAL PLAN | SCALE: 1" =40'-0" | 1

PLAN NOTES

1. PROVIDE 4 # 1/0, 1 # 6 GRD – 2" C.
2. PROVIDE 2 # 8, 1 # 10 GRD – 1" C.
3. PROVIDE 2 # 10, 1 # 10 GRD – 3/4" C.
4. PROVIDE 2' X 3' CONCRETE INTERCEPT TYPE PULLBOX WITH BOLT-DOWN TRAFFIC RATED COVER ENGRAVED: "ELECTRIC" AND DEPTH AS REQUIRED IN ORDER TO INTERCEPT THE EXISTING UNDERGROUND POWER FEEDER EXTENDING TO PANEL "SB" AT THE SOFTBALL FIELD. INSTALL PULLBOX LID AND COVER TO BE FLUSH WITH THE ADJACENT FINISH GRADE. PROVIDE WEATHERPROOF SPLICE BETWEEN NEW AND EXISTING CONDUCTORS AND EXTEND WITH NEW CONDUIT AND CONDUCTORS AS INDICATED.
5. EXISTING POWER FEEDER EXTENDING TO PANEL "SB" AT THE SOFTBALL FIELDS SHALL REMAIN.
6. REFER TO SHEET ES-1.2 FOR CONTINUATION.
7. INDICATED CONDUIT SHALL RISE UP FROM BELOW GRADE AND TERMINATE AT EXISTING PANEL. EXTEND CONDUCTORS AND TERMINATE ON TO EXISTING SPARE 20A/1P CUIT BREAKER IN PANEL TO SERVE THE NEW SITE LIGHTS. IF A SPARE DOES NOT EXIST, PROVIDE NEW 20A/1P CIRCUIT BREAKER INCLUDING MOUN TING HARDWARE TO MATCH EXISTING CIRCUIT BREAKERS CURRENT USED. PROVIDE NEW CONTROL RELAY AND MODIFY THE EXISTING LIGHTING CONTROL SYSTEM TO PROGRAM AND OPERATE THE NEW SITE LIGHTS ON THE SAME SCHEDULE AS THE EXISTING PARKING LOT LIGHTS.
8. INDICATED CONDUIT SHALL RISE UP FROM BELOW GRADE AND ROUTE EXPOSED ON SIDE OF EXTERIOR WALL TO AN ELEVATION BELOW THE CEILINGH INSIDE THE ELECTRICAL ROOM. PROVIDE WEATHERPROOF LB FITTING AND PENETRATE THE EXISTING WALL IN ORDER TO EXTEND CONDUIT INTO BUILDING AND TERMINNATE AS INDICATED. PAINT THE EXPOSED CONDUIT TO MATCH THE SURROUNDING SURFACE COLOR. SEAL WALL PENETRATION.
9. INICATED CONDUIT SHALL ROUTE EXPOSED INSIDE ELECTRICAL ROOM AND EXTEND AS INDICATED.
10. PROVIDE 2' X 3' X 3' D. CONCRETE PULLBOX WITH BOLT DOWN TRAFFIC RATED COVER ENGRAVED: "ELECTRIC". INSTALL PULLBOX LID AND COVER TO BE FLUSH WITH THE ADJACENT FINISH GRADE.
11. PROVIDE NEW 100A/3P CIRCUIT BREAKER INCLUDING ALL REQUIRED MOUNTING HARDWARE IN SPACE OF EXISTING DISTRIBUTION SWITHBOARD AND CONNECT TO NEW FEEDER CONDUCTORS TO RESTORE POWER TO EXISTING PANEL "SB". NEW CIRCUIT BREAKER SHALL MEET OR EXCEED THE A.I.C. RATING OF EXISTING CIRCUIT BREAKERS CURRENTLY USED.

REGISTERED PROFESSIONAL ENGINEER STEPHEN R. ZAUNBRECHER No. E 10372 Exp. 09-30-26 ELECTRICAL STATE OF CALIFORNIA
01/05/2026

LICENSED ARCHITECT BLAKE A. CLARKE No. 31546 Exp. 10-31-27 STATE OF CALIFORNIA

AGENCY APPROVAL
File No: A#

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 04-123574 INC:
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 01/15/2026

FBA Engineering
Consulting Electrical Engineers
150 Paularino Avenue Suite A120
Costa Mesa, CA 92626
949.852.9995 ♥ 949.852.1657 (fax)
fbaeng.com
FBA Job Number: 874.991.15

CONSULTANT BRANDING

RUHNAU CLARKE ARCHITECTS

PROJECT No. :

DRAWN BY: CHECKED BY:
DELTA # ____ DATE ____ ☐ ADD ☐ AFO ☐ CCD ☐ REV
DELTA # ____ DATE ____ ☐ ADD ☐ AFO ☐ CCD ☐ REV
DELTA # ____ DATE ____ ☐ ADD ☐ AFO ☐ CCD ☐ REV
DELTA # ____ DATE ____ ☐ ADD ☐ AFO ☐ CCD ☐ REV

RUHNAUCLARKE.COM
3775 TENTH STREET, RIVERSIDE CALIFORNIA 92501 (951) 684 4664 / 5751 PALMER WAY, SUITE C, CARLSBAD CALIFORNIA 92010 (760) 438 5899

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24100 Delphinium Ave, Moreno Valley, CA 92553
MORENO VALLEY UNIFIED SCHOOL DISTRICT

ENLARGED SITE ELECTRICAL PLAN

ES-1.3

14812

FBA Engineering / Plot Date: 1/5/2026 3:21 PM / Plotted by: Blake Meech / Drawing Location: I:\874991\15\ES-1.3_874991\5_ENLARGED SITE ELECTRICAL PLAN.dwg

USASHADE
& Fabric Structures, Inc.

FABRIC SHADE STRUCTURE

DSA P.C. 04-123501

GENERAL NOTES:

1. ALL WORK SHALL CONFORM TO THE 2022 EDITION OF THE TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR).
2. ALL WORK SHALL BE IN COMPLIANCE WITH CFC CHAPTER 33 - FIRE SAFETY DURING CONSTRUCTION AND DEMOLITION.
3. SEE INDIVIDUAL STRUCTURAL DRAWINGS FOR SPECIFIC DESIGN NOTES AND LOADING.
4. PRIOR TO SUBMITTAL ARCHITECT OF RECORD SHALL IDENTIFY PC MODEL(S) SELECTED BY END USER ON SHEETS T-1.0 AND T-2.0 BY CHECKING THE APPROPRIATE BOX ASSOCIATED WITH SELECTED PC MODEL(S). EXCLUDE SHEETS FOR MODELS NOT SELECTED.

PLANS FOR SPECIFIC APPLICATION SHALL INCLUDE THE FOLLOWING:

1. COMPLETE SCOPE OF WORK INCLUDING THE SHADE STRUCTURE MODEL NUMBER, P.C. NUMBER, AND SPECIFIC SIZE OF THE SHADE STRUCTURE(S).
2. PROVIDE A CODE ANALYSIS, INCLUDING ACTUAL SHADE STRUCTURE AREA (SQ. FT.), OCCUPANCY TYPE (A-3), AND TYPE OF CONSTRUCTIONS (V-B). INDICATE OCCUPANT LOAD FACTOR (2022 CBC, SECTION 1004).
3. ACTUAL DIMENSIONS OF SHADE STRUCTURES.
4. DIMENSIONS FROM ADJACENT STRUCTURES AND PROXIMITY OF ASSUMED OR ACTUAL PROPERTY LINES.
5. INDICATE LOCATIONS OF FIRE EXTINGUISHERS WITHIN 75 FEET.
6. SHOW LOCATION OF AUDIBLE FIRE ALARM.
7. ALL SADDLES, CLAMPS AND FITTINGS SHALL CONFORM TO THE GUIDELINES AS SPECIFIED IN APPENDICES "A, B, & C", RESPECTIVELY, IN ASCE/SEI 19-16, "STRUCTURAL APPLICATIONS OF STEEL CABLES FOR BUILDINGS."
8. ARCHITECTS OF RECORD TO DETERMINE IF SPECIFIC SITE IS LOCATED IN A MAPPED GEOLOGIC HAZARD ZONE. GEOHAZARD REPORTS REQUIREMENTS SHALL COMPLY WITH DSA IR A-4.
9. ARCHITECTS OF RECORD TO DETERMINE IF SPECIFIC SITE IS LOCATED IN A MAPPED FIRE HAZARD SEVERITY ZONE OR WILDLAND INTERFACE AREA.

FOR SNOW LOAD MODELS ONLY:

10. INDICATE DIMENSIONS FROM THE ROOF TO THE HIGHER STRUCTURE OR TERRAIN FEATURE. MINIMUM DIMENSION OF 20'-0" FOR SNOW LOAD MODEL (ASCE 7-16).
11. ACTUAL SITE ELEVATION (FEET) TO DETERMINE IF THE SITE OCCURS AT OR BELOW THE UPPER ELEVATION LIMIT FOR THE GROUND SNOW LOAD SHOWN IN ASCE 7-16.

P.C. NOTES

PLANS FOR SPECIFIC APPLICATION SHALL INCLUDE THE FOLLOWING:

LIST OF APPLICABLE CODES:

- 2022 CALIFORNIA ADMINISTRATIVE CODE (CAC), PART 1, TITLE 24 C.C.R.
- 2022 CALIFORNIA BUILDING CODE (CBC), PART 2, TITLE 24 C.C.R.
- 2022 CALIFORNIA ELECTRICAL CODE (CEC) , PART 3, TITLE 24 C.C.R.
- 2022 CALIFORNIA MECHANICAL CODE (CMC), PART 4, TITLE 24 C.C.R.
- 2022 CALIFORNIA PLUMBING CODE (CPC), PART 5, TITLE 24 C.C.R.
- 2022 CALIFORNIA ENERGY CODE (CEC), PART 6, TITLE 24 C.C.R.
- 2022 CALIFORNIA FIRE CODE, PART 9, TITLE 24 C.C.R.
- 2022 CALIFORNIA EXISTING BUILDING CODE (CEBC), PART 10, TITLE 24 C.C.R.
- 2022 CALIFORNIA GREEN BUILDING STANDARDS CODE (CALGREEN), PART 11, TITLE 24 C.C.R.
- 2022 CALIFORNIA REFERENCED STANDARDS CODE, PART 12, TITLE 24 C.C.R.
- TITLE 19 C.C.R., PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS

APPLICABLE STANDARDS:

FOR A LIST OF APPLICABLE STANDARDS, INCLUDING CALIFORNIA AMENDMENTS TO THE NFPA STANDARDS, REFER TO CBC CHAPTER 35 AND CFC CHAPTER 80.

APPLICABLE CODES

SITE SPECIFIC PARAMETERS

INSTRUCTIONS: DESIGN PROFFESIONAL SHALL CHECK THE APPROPRIATE SELECTION BOXES BELOW AND ENTER THE DESIGN PARAMETERS APPLICABLE TO THE SPECIFIC PROJECT SITE

SEISMIC

SELECT ONE

☒ DESIGN BASED ON SITE CLASS $D_{default}$
NO GEOTECHNICAL INVESTIGATION REQUIRED
Ss = 2.99g Fa = 1.2

☐ DESIGN BASED ON SITE CLASS DETERMINED PER CHAPTER 20 OF ASCE 7-16
GEOTECHNICAL INVESTIGATION PROVIDED
SITE CLASS: ☐ C ☐ D
Ss = _____ Fa = _____ PER ASCE 7-16 SUPPL 3, TABLE 11.4-1

☐ DESIGN BASED ON SITE CLASS SPECIFIC GROUND MOTION HAZARD ANALYSIS
PER CHAPTER 21 OF ASCE 7-16
SHORT-PERIOD DESIGN SPECTRAL RESPONSE PARAMETER, S_{DS}, SHALL BE AS SPECIFIED IN GEOTECHNICAL INVESTIGATION
CGS APPROVAL REQUIRED
NOT ELIGIBLE FOR OTC REVIEW
SITE CLASS: ☐ C ☐ D

S_{DS} = 2/3 Fa Ss = 1.99 ≤ 2.0
Cs = 1.6 USED IN DESIGN
SEISMIC DESIGN CATEGORY: ☒ D ☐ E

CODE ANALYSIS			
OCCUPANCY GROUP	OCCUPANT LOAD FACTOR	TOTAL OCCUPANT LOAD	SHADE STRUCTURE AREA (ft^2)

SITE SPECIFIC PARAMETERS

MANUFACTURER:

USA SHADE & FABRIC STRUCTURES
2580 ESTERS BOUVLEVARD, SUITE 100
DFW AIRPORT, TEXAS 75261
PH. 800-966-5005
W. www.usa-shade.com

ARCHITECT:

HIGGINSON ARCHITECTS, INC.
DAVID HIGGINSON, AIA, PRINCIPAL ARCHITECT
34247 YUCAIPA BOULEVARD, SUITE D
YUCAIPA, CALIFORNIA 92399
PH. 909-499-0058
E. dhigginson@higginsonarchitects.com
W. www.higginsonarchitects.com

LICENSED ARCHITECT
DAVID HIGGINSON
C19168
10-31-25
RENEWAL DATE
STATE OF CALIFORNIA

8/19/24

STRUCTURAL ENGINEER:

MARK LOWE, S.E.
c/o USA SHADE AND FABRIC STRUCTURES

REGISTERED PROFESSIONAL ENGINEER
MARK ALBERT LOWE
No. 3693
STRUCTURAL
STATE OF CALIFORNIA

8/23/2024

ARCHITECT / ENGINEER

✓	SHEET NO.	SHEET DESCRIPTION	UNIT STRUCTURE TYPE	MAX. UNIT SIZE	UNIT MODEL NUMBER
X	T-1.0	TITLE SHEET			
X	T-2.0	UNIT SELECTION			
X	T-3.0	T&I FORMS			
	1.1-1000	PRODUCT INFORMATION	HIP	20' x 30' x 15'	DSA4012030-22
	1.2-2000	REACTIONS	HIP	20' x 30' x 15'	DSA4012030-22
	2.1-1000	PRODUCT INFORMATION	HIP	30' x 30' x 15'	DSA4013030-22
	2.2-2000	REACTIONS	HIP	30' x 30' x 15'	DSA4013030-22
	3.1-1000	PRODUCT INFORMATION	HIP	30' x 40' x 15'	DSA4013040-22
	3.2-2000	REACTIONS	HIP	30' x 40' x 15'	DSA4013040-22
	4.1-1000	PRODUCT INFORMATION	HIP	40' x 40' x 15'	DSA4014040-22
	4.2-2000	REACTIONS	HIP	40' x 40' x 15'	DSA4014040-22
	5.1-1000	PRODUCT INFORMATION	HIP	20' x 30' x 12'	DSA401203012-22
	5.2-2000	REACTIONS	HIP	20' x 30' x 12'	DSA401203012-22
	6.1-1000	PRODUCT INFORMATION	HIP	30' x 30' x 12'	DSA401303012-22
	6.2-2000	REACTIONS	HIP	30' x 30' x 12'	DSA401303012-22
	7.1-1000	PRODUCT INFORMATION	HIP	30' x 40' x 12'	DSA401304012-22
	7.2-2000	REACTIONS	HIP	30' x 40' x 12'	DSA401304012-22
	8.1-1000	PRODUCT INFORMATION	HIP (20 psf SNOW LOAD)	20' x 30' x 15'	DSA401S2030-22
	8.2-2000	REACTIONS	HIP (20 psf SNOW LOAD)	20' x 30' x 15'	DSA401S2030-22
	9.1-1000	PRODUCT INFORMATION	JOINED HIPS	VARIES	DSA401J-22
	9.2-1001	DETAILS	JOINED HIPS	VARIES	DSA401J-22
	9.3-2000	REACTIONS	JOINED HIPS	VARIES	DSA401J-22
	10.1-1000	PRODUCT INFORMATION	QUAD JOINED HIPS	VARIES	DSA401Q-22
	10.2-1001	DETAILS	QUAD JOINED HIPS	VARIES	DSA401Q-22
	10.3-2000	REACTIONS	QUAD JOINED HIPS	VARIES	DSA401Q-22
	11.1-1000	PRODUCT INFORMATION	FULL CANTILEVER HIP SINGLE	20' x 30' x 15'	DSA2022030-22
	11.2-2000	REACTIONS	FULL CANTILEVER HIP SINGLE	20' x 30' x 15'	DSA2022030-22
	12.1-1000	PRODUCT INFORMATION	FULL CANTILEVER HIP JOINED	20' x 200' x 15'	DSA3022060-22
	12.2-2000	REACTIONS	FULL CANTILEVER HIP JOINED	20' x 200' x 15'	DSA3022060-22
	13.1-1000	PRODUCT INFORMATION	SINGLE POST PYRAMID	14' x 14' x 12'	DSA1031414-22
	13.2-2000	REACTIONS	SINGLE POST PYRAMID	14' x 14' x 12'	DSA1031414-22
	14.1-1000	PRODUCT INFORMATION	SINGLE POST PYRAMID	20' x 20' x 12'	DSA1032020-22
	14.2-2000	REACTIONS	SINGLE POST PYRAMID	20' x 20' x 12'	DSA1032020-22
	15.1-1000	PRODUCT INFORMATION	SINGLE POST PYRAMID CANTILEVER	14' x 14' x 12'	DSA1241414-22
	15.2-2000	REACTIONS	SINGLE POST PYRAMID CANTILEVER	14' x 14' x 12'	DSA1241414-22
	16.1-1000	PRODUCT INFORMATION	SINGLE POST PYRAMID CANTILEVER	20' x 20' x 12'	DSA1242020-22
	16.2-2000	REACTIONS	SINGLE POST PYRAMID CANTILEVER	20' x 20' x 12'	DSA1242020-22
	17.1-1000	PRODUCT INFORMATION	MARINER PEAK	30' x 30' x 15'	DSA4073030-22
	17.2-2000	REACTIONS	MARINER PEAK	30' x 30' x 15'	DSA4073030-22
	18.1-1000	PRODUCT INFORMATION	MARINER PEAK	30' x 40' x 18'	DSA4073040-22
	18.2-2000	REACTIONS	MARINER PEAK	30' x 40' x 18'	DSA4073040-22
	19.1-1000	PRODUCT INFORMATION	MARINER PEAK JOINED	30' x 133' x 15'	DSA407J3060-22
	19.2-2000	REACTIONS	MARINER PEAK JOINED	30' x 133' x 15'	DSA407J3060-22
	20.1-1000	PRODUCT INFORMATION	MARINER PEAK QUAD	60' x 60' x 15'	DSA407Q6060-22
	20.2-2000	REACTIONS	MARINER PEAK QUAD	60' x 60' x 15'	DSA407Q6060-22
	21.1-1000	PRODUCT INFORMATION	TRI TRUSS HIP SINGLE WIDE	20' x 30' x 15'	DSA2062030-22
	21.2-2000	REACTIONS	TRI TRUSS HIP SINGLE WIDE	20' x 30' x 15'	DSA2062030-22
	22.1-1000	PRODUCT INFORMATION	TRI TRUSS HIP JOINED	20' x 200' x 15'	DSA3052060-22
	22.2-2000	REACTIONS	TRI TRUSS HIP JOINED	20' x 200' x 15'	DSA3052060-22
	23.1-1000	PRODUCT INFORMATION	TENSION SAILS THREE POINT	30' x 133' x 15'	DSA30730-22
	23.2-2000	REACTIONS	TENSION SAILS THREE POINT	30' x 133' x 15'	DSA30730-22
	24.1-1000	PRODUCT INFORMATION	TENSIONS SAILS FOUR POINT	20' x 200' x 15'	DSA4182020-22
	24.2-2000	REACTIONS	TENSIONS SAILS FOUR POINT	20' x 200' x 15'	DSA4182020-22
X	25.1-1000	PRODUCT INFORMATION	TENSIONS SAILS FOUR POINT	30' x 133' x 15'	DSA4183030-22
X	25.2-2000	REACTIONS	TENSIONS SAILS FOUR POINT	30' x 133' x 15'	DSA4183030-22
	26.1-1000	PRODUCT INFORMATION	TRIANGLE	25' x 25' x 15'	DSA30125-22
	26.2-2000	REACTIONS	TRIANGLE	25' x 25' x 15'	DSA30125-22
	27.1-1000	PRODUCT INFORMATION	TRIANGLE	40' x 40' x 15'	DSA30140-22
	27.2-2000	REACTIONS	TRIANGLE	40' x 40' x 15'	DSA30140-22
	28.1-1000	PRODUCT INFORMATION	HEXAGON	Ø40' X 15'	DSA60340-22
	28.2-2000	REACTIONS	HEXAGON	Ø40' X 15'	DSA60340-22
	29.1-1000	PRODUCT INFORMATION	HEXAGON	Ø60' X 15'	DSA60360-22
	29.2-2000	REACTIONS	HEXAGON	Ø60' X 15'	DSA60360-22

TOTAL SHEET COUNT: 63 SHEETS

SHEET INDEX

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 04-123574 INC:
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 01/15/2026

THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN

USASHADE & Fabric Structures®

CORPORATE HEADQUARTERS
2580 ESTERS BLVD. SUITE 100
DFW AIRPORT, TX, 75261
800-966-5005

CERTIFICATIONS:
IAS CERTIFICATION No: FA-428
CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:
Moreno Valley U.S.D.

PROJECT NAME:
Valley View High School

LOCATION:
13135 Nason Street
Moreno Valley, CA 92555

MODEL NUMBER:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 04-123501 PC
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑ CG ☐
DATE: 12/11/2024

STRUCTURE TYPE:

SCALE : VARIES

DRAWING SIZE:
D

PRE-CHECK (PC) DOCUMENT
Code : 2022 CBC
A separate project application for construction is required.

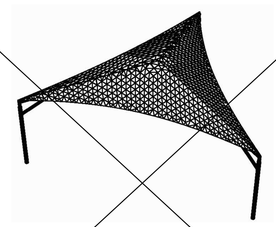
Eng. By :	DWH	8/19/23
Design By :	DWH	8/19/23
Approved By :	DWH	8/19/23

DRAWING DESCRIPTION:

DWG. TITLE SHEET

SHEET T-1.0

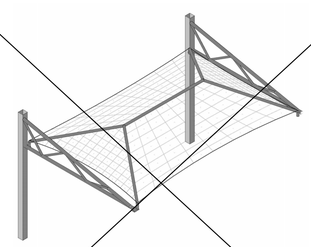
REV.



STRUCTURE MODEL:	DSA30125-22	SEE SHEET 26.1-1000
MAX. SIZE:	25' x 25' x 15'	
MAX. AREA:	271 SQ. FT.	
MAX. OCCUPANCY:	18 PERSONS	
STRUCTURE MODEL:	DSA30140-22	SEE SHEET 27.1-1000
MAX. SIZE:	40' x 40' x 15'	
MAX. AREA:	692 SQ. FT.	
MAX. OCCUPANCY:	46 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

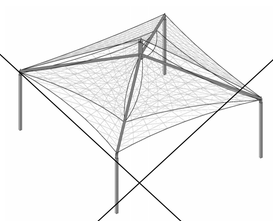
TRIANGLE



STRUCTURE MODEL:	DSA2062030-22	SEE SHEET 21.1-1000
MAX. SIZE:	20' x 30' x 15'	
MAX. AREA:	600 SQ. FT.	
MAX. OCCUPANCY:	40 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

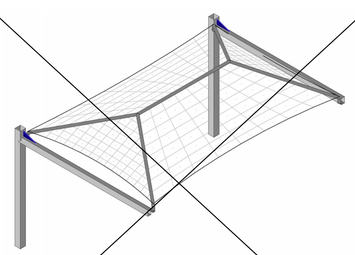
TRI-TRUSS HIP SINGLE WIDE



STRUCTURE MODEL:	DSA4073030-22	SEE SHEET 17.1-1000
MAX. SIZE:	30' x 30' x 15'	
MAX. AREA:	900 SQ. FT.	
MAX. OCCUPANCY:	60 PERSONS	
STRUCTURE MODEL:	DSA4073040-22	SEE SHEET 18.1-1000
MAX. SIZE:	30' x 40' x 15'	
MAX. AREA:	1,200 SQ. FT.	
MAX. OCCUPANCY:	80 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

MARINER PEAK



STRUCTURE MODEL:	DSA2022030-22	SEE SHEET 11.1-1000
MAX. SIZE:	20' x 30' x 15'	
MAX. AREA:	600 SQ. FT.	
MAX. OCCUPANCY:	40 PERSONS	

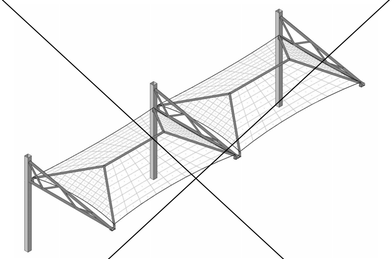
FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

FULL CANTILEVER HIP SINGLE

STRUCTURE MODEL:	DSA60340-22	SEE SHEET 28.1-1000
MAX. SIZE:	Ø40' x 15'	
MAX. AREA:	1,040 SQ. FT.	
MAX. OCCUPANCY:	69 PERSONS	
STRUCTURE MODEL:	DSA60360-22	SEE SHEET 29.1-1000
MAX. SIZE:	Ø60' x 15'	
MAX. AREA:	2,338 SQ. FT.	
MAX. OCCUPANCY:	156 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

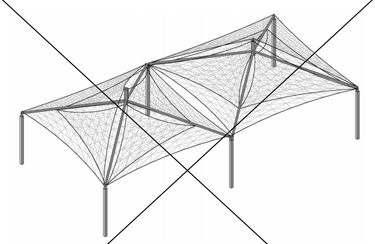
HEXAGON



STRUCTURE MODEL:	DSA3052060-22	SEE SHEET 22.1-1000
MAX. SIZE:	20' x 200' x 15'	
MAX. AREA:	4,000 SQ. FT.	
MAX. OCCUPANCY:	266 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

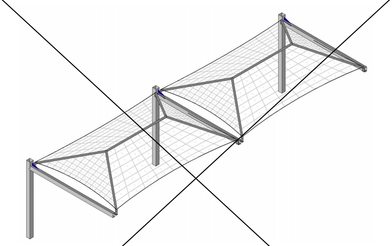
TRI-TRUSS HIP JOINED



STRUCTURE MODEL:	DSA407J3060-22	SEE SHEET 19.1-1000
MAX. SIZE:	30' x 133' x 15'	
MAX. AREA:	3,990 SQ. FT.	
MAX. OCCUPANCY:	266 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

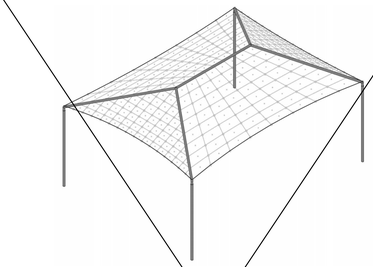
MARINER PEAK JOINED



STRUCTURE MODEL:	DSA3022060-22	SEE SHEET 12.1-1000
MAX. SIZE:	20' x 200' x 15'	
MAX. AREA:	4,000 SQ. FT.	
MAX. OCCUPANCY:	266 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

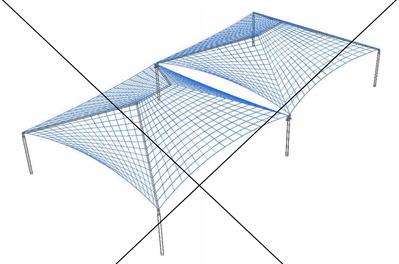
FULL CANTILEVER HIP JOINED



STRUCTURE MODEL:	DSA4012030-22	SEE SHEET 1.1-1000
MAX. SIZE:	20' x 30' x 15'	
MAX. AREA:	600 SQ. FT.	
MAX. OCCUPANCY:	40 PERSONS	
STRUCTURE MODEL:	DSA4013030-22	SEE SHEET 2.1-1000
MAX. SIZE:	30' x 30' x 15'	
MAX. AREA:	900 SQ. FT.	
MAX. OCCUPANCY:	60 PERSONS	
STRUCTURE MODEL:	DSA4013040-22	SEE SHEET 3.1-1000
MAX. SIZE:	30' x 40' x 15'	
MAX. AREA:	1,200 SQ. FT.	
MAX. OCCUPANCY:	80 PERSONS	
STRUCTURE MODEL:	DSA4014040-22	SEE SHEET 4.1-1000
MAX. SIZE:	40' x 40' x 15'	
MAX. AREA:	1,600 SQ. FT.	
MAX. OCCUPANCY:	106 PERSONS	
STRUCTURE MODEL:	DSA401203012-22	SEE SHEET 5.1-1000
MAX. SIZE:	20' x 30' x 12'	
MAX. AREA:	600 SQ. FT.	
MAX. OCCUPANCY:	40 PERSONS	
STRUCTURE MODEL:	DSA401303012-22	SEE SHEET 6.1-1000
MAX. SIZE:	30' x 30' x 12'	
MAX. AREA:	900 SQ. FT.	
MAX. OCCUPANCY:	60 PERSONS	
STRUCTURE MODEL:	DSA401304012-22	SEE SHEET 7.1-1000
MAX. SIZE:	30' x 40' x 12'	
MAX. AREA:	1,200 SQ. FT.	
MAX. OCCUPANCY:	80 PERSONS	
STRUCTURE MODEL:	DSA401S2030-22 (20 psf SNOW LOAD)	SEE SHEET 8.1-1000
MAX. SIZE:	20' x 30' x 15'	
MAX. AREA:	600 SQ. FT.	
MAX. OCCUPANCY:	40 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

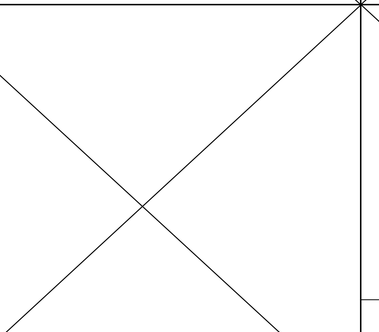
HIP



STRUCTURE MODEL:	DSA401J-22	SEE SHEET 9.1-1000
MAX. SIZE:	VARIES, SEE JOINED HIP UNIT SHEET	
MAX. AREA:	VARIES	
MAX. OCCUPANCY:	VARIES	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

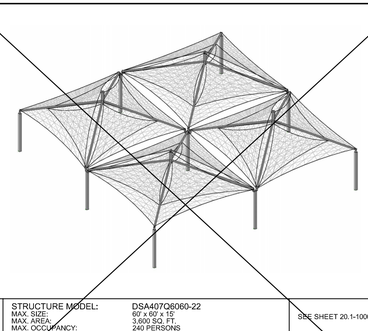
JOINED HIP



STRUCTURE MODEL:	DSA30730-22	SEE SHEET 23.1-1000
MAX. SIZE:	30' x 133' x 15'	
MAX. AREA:	3,990 SQ. FT.	
MAX. OCCUPANCY:	266 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

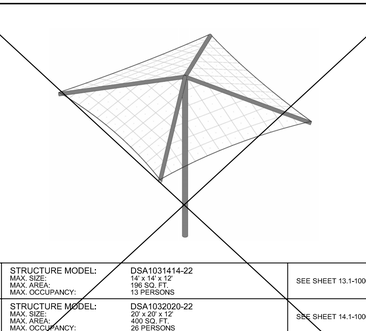
TENSIONS SAILS THREE-POINT



STRUCTURE MODEL:	DSA407Q6060-22	SEE SHEET 20.1-1000
MAX. SIZE:	60' x 60' x 15'	
MAX. AREA:	3,600 SQ. FT.	
MAX. OCCUPANCY:	240 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

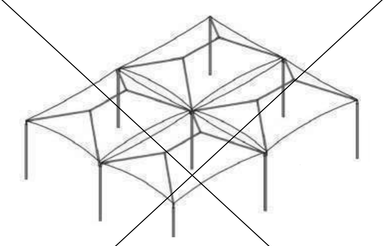
MARINER PEAK QUAD



STRUCTURE MODEL:	DSA1031414-22	SEE SHEET 13.1-1000
MAX. SIZE:	14' x 14' x 12'	
MAX. AREA:	196 SQ. FT.	
MAX. OCCUPANCY:	13 PERSONS	
STRUCTURE MODEL:	DSA1032020-22	SEE SHEET 14.1-1000
MAX. SIZE:	20' x 20' x 12'	
MAX. AREA:	400 SQ. FT.	
MAX. OCCUPANCY:	26 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

SINGLE POST PYRAMID

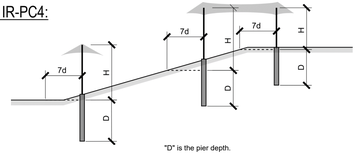


STRUCTURE MODEL:	DSA401Q-22	SEE SHEET 10.1-1000
MAX. SIZE:	VARIES, SEE QUAD HIP UNIT SHEET	
MAX. AREA:	VARIES	
MAX. OCCUPANCY:	VARIES	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

QUAD HIP

PER IR-PC4:



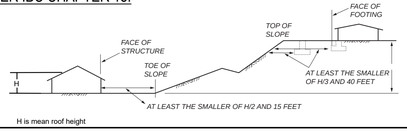
"D" is the pier depth.

"d" is the pier diameter.

"7d" is the horizontal plane projected from 7 times d.

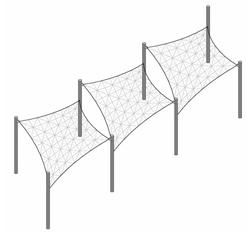
"H" is the theoretical column height (column height plus part of projected height)

PER IBC CHAPTER 18:



H is mean roof height

PC SLOPE DESIGN PARAMETERS

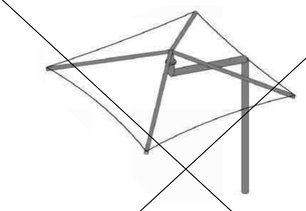


STRUCTURE MODEL:	DSA4182020-22	SEE SHEET 24.1-1000
MAX. SIZE:	20' x 200' x 15'	
MAX. AREA:	4,000 SQ. FT.	
MAX. OCCUPANCY:	266 PERSONS	
X STRUCTURE MODEL:	DSA4183030-22	SEE SHEET 25.1-1000
MAX. SIZE:	30' x 133' x 15'	
MAX. AREA:	3,990 SQ. FT.	
MAX. OCCUPANCY:	266 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

TENSIONS SAILS FOUR-POINT

NOT USED



STRUCTURE MODEL:	DSA1241414-22	SEE SHEET 15.1-1000
MAX. SIZE:	14' x 14' x 12'	
MAX. AREA:	196 SQ. FT.	
MAX. OCCUPANCY:	13 PERSONS	
STRUCTURE MODEL:	DSA1242020-22	SEE SHEET 16.1-1000
MAX. SIZE:	20' x 20' x 12'	
MAX. AREA:	400 SQ. FT.	
MAX. OCCUPANCY:	26 PERSONS	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0

SINGLE POST PYRAMID CANTILEVER

LICENSED ARCHITECT
DAVID HIGGINSON
C19168
10-31-25
RENEWAL DATE
STATE OF CALIFORNIA

8/19/24

REGISTERED PROFESSIONAL ENGINEER
MARK ALBERT LOWE
No. 3693
STRUCTURAL
STATE OF CALIFORNIA

8/23/2024

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 04-123574 INC:
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 01/15/2026

THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN

USASHADE & Fabric Structures®

CORPORATE HEADQUARTERS
2580 ESTERS BLVD. SUITE 100
DFW AIRPORT, TX, 75261
800-966-5005

CERTIFICATIONS:

IAS CERTIFICATION No: FA-428

CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:

Moreno Valley U.S.D.

PROJECT NAME:

Valley View High School

LOCATION:

13135 Nason Street
Moreno Valley, CA 92555

MODEL NUMBER:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 04-123501 PC
REVIEWED FOR
SS ☑ FLS ☑ ACS ☒ CG ☐
DATE: 12/11/2024

STRUCTURE TYPE:

SCALE : VARIES

DRAWING SIZE:
D

PRE-CHECK (PC) DOCUMENT
Code : 2022 CBC
A separate project application for construction is required.

Eng. By :	**DWH**	**8/19/23**
Design By :	**DWH**	**8/19/23**
Approved By :	**DWH**	**8/19/23**

DRAWING DESCRIPTION:

DWG.
UNIT SELECTION

SHEET

REV.

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS, 2022 CBC

General

Application Number: 04-123501	**School Name:** PCC FABRIC SHADE STRUCTURES	**School District:** USA SHADE AND FABRIC STRUCTURES
DSA File Number: PC-140	**Increment Number:**	**Date Created:** 2024-08-06 15:16:59

IMPORTANT: This form is only a summary list of structural tests and some of the special inspections required for the project. Generally, the structural tests and special inspections noted on this form are those that will be performed by the Geotechnical Engineer of Record, Laboratory of Record, or Special Inspector. The actual complete test and inspection program must be performed as detailed on the DSA approved documents. The appendix at the bottom of this form identifies work NOT subject to DSA requirements for special inspection or structural testing. The project inspector is responsible for providing inspection of all facets of construction, including but not limited to, special inspections not listed on this form such as structural wood framing, high-load wood diaphragms, cold-formed steel framing, anchorage of non-structural components, etc., per Title 24, Part 2, Chapter 17A (2022 CBC).

****NOTE:** Undefined section and table references found in this document are from the CBC, or California Building Code.

KEY TO COLUMNS

1. TYPE	2. PERFORMED BY
Continuous – Indicates that a continuous special inspection is required **Periodic –** Indicates that a periodic special inspection is required **Test –** Indicates that a test is required	**GE (Geotechnical Engineer) –** Indicates that the special inspection shall be performed by a registered geotechnical engineer or his or her authorized representative. **LOR (Laboratory of Record) –** Indicates that the test or special inspection shall be performed by a testing laboratory accepted in the DSA Laboratory Evaluation and Acceptance (LEA) Program. See CAC Section 4-335. **PI (Project Inspector) –** Indicates that the special inspection may be performed by a project inspector when specifically approved by DSA. **SI (Special Inspection) –** Indicates that the special inspection shall be performed by an appropriately qualified/approved special inspector.

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2022 CBC

Table 1705A.6, Table 1705A.7, Table 1705A.8

Application Number: 04-123501	**School Name:** PCC FABRIC SHADE STRUCTURES	**School District:** USA SHADE AND FABRIC STRUCTURES
DSA File Number: PC-140	**Increment Number:**	**Date Created:** 2024-08-06 15:16:59

Geotechnical Reports: Project does NOT have and does NOT require a geotechnical report

	S1. GENERAL:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Verify that: • Site has been prepared properly prior to placement of controlled fill and/or excavations for foundations. • Foundation excavations are extended to proper depth and have reached proper material. • Materials below footings must not contain loose material, mud, organic silt, organic clays, or peat.	**See Notes**	**PI**	Refer to specific items identified in the Appendix listing exemptions for limitations. Placement of controlled fill exceeding 12" depth under foundations and/or within the building envelope is not permitted without a geotechnical report.

	S2. SOIL COMPACTION AND FILL:		see T&I Note #7 for additional information	
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Perform classification and testing of fill materials.	**Test**	**LOR***	* Under the supervision of the geotechnical engineer.
☐	**b.** Verify use of proper materials, densities and inspect lift thicknesses, placement and compaction during placement of fill.	**Continuous**	**LOR***	* Under the supervision of a geotechnical engineer or LOR's engineering manager. Refer to specific items identified in the Appendix listing exemptions for limitations.
☐	**c.** Compaction testing.	**Test**	**LOR***	* Under the supervision of a geotechnical engineer or LOR's engineering manager. Refer to specific items identified in the Appendix listing exemptions for limitations.

	S3. DRIVEN DEEP FOUNDATIONS (PILES):			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Verify pile materials, sizes and lengths comply with the requirements.	**Continuous**	**GE***	* By geotechnical engineer or his or her qualified representative.

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2022 CBC

Table 1705A.6, Table 1705A.7, Table 1705A.8

Application Number: 04-123501	**School Name:** PCC FABRIC SHADE STRUCTURES	**School District:** USA SHADE AND FABRIC STRUCTURES
DSA File Number: PC-140	**Increment Number:**	**Date Created:** 2024-08-06 15:16:59

	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**b.** Determine capacities of test piles and conduct additional load tests as required.	**Test**	**LOR***	* Under the supervision of the geotechnical engineer.
☐	**c.** Inspect driving operations and maintain complete and accurate records for each pile.	**Continuous**	**GE***	* By geotechnical engineer or his or her qualified representative.
☐	**d.** Verify locations of piles and their plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and record any pile damage.	**Continuous**	**GE***	* By geotechnical engineer or his or her qualified representative.
☐	**e.** Steel piles.	Provide tests and inspections per STEEL section below.		
☐	**f.** Concrete piles and concrete filled piles.	Provide tests and inspections per CONCRETE section below.		
☐	**g.** For specialty piles, perform additional inspections as determined by the registered design professional in responsible charge.	*	*	* As defined on drawings or specifications.

	S4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS):		see T&I Note #8 for additional information	
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Note**
☑	**a.** Inspect drilling operations and maintain complete and accurate records for each pier.	**Continuous**	**PI**	Continuous inspection to be provided by project inspector. Refer to specific items identified in the Appendix listing exemptions for limitations.
☑	**b.** Verify pier locations, diameters, plumbness and lengths Record concrete or grout volumes.	**Continuous**	**PI**	Continuous inspection to be provided by project inspector. Refer to specific items identified in the Appendix listing exemptions for limitations.
☑	**c.** Concrete piers.	Provide tests and inspections per CONCRETE section below.		

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2022 CBC

Table 1705A.6, Table 1705A.7, Table 1705A.8

Application Number: 04-123501	**School Name:** PCC FABRIC SHADE STRUCTURES	**School District:** USA SHADE AND FABRIC STRUCTURES
DSA File Number: PC-140	**Increment Number:**	**Date Created:** 2024-08-06 15:16:59

	S5. RETAINING WALLS:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Placement, compaction and inspection of backfill.	**Continuous**	**GE***	**1705A.6.1.** * By geotechnical engineer or his or her qualified representative. (See section S2 above).
☐	**b.** Placement of soil reinforcement and/or drainage devices.	**Continuous**	**GE***	* By geotechnical engineer or his or her qualified representative.
☐	**c.** Segmental retaining walls; inspect placement of units, dowels, connectors, etc.	**Continuous**	**GE***	* By geotechnical engineer or his or her qualified representative. See DSA IR 18-2.
☐	**d.** Concrete retaining walls.	Provide tests and inspections per CONCRETE section below.		
☐	**e.** Masonry retaining walls.	Provide tests and inspections per MASONRY section below.		

	S6. OTHER SOILS:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Soil Improvements	**Test**	**GE***	Submit a comprehensive report documenting final soil improvements constructed, construction observation and the results of the confirmation testing and analysis to CGS (California Geological Survey) for final acceptance. * By geotechnical engineer or his or her qualified representative.
☐	**b.** Inspection of Soil Improvements	**Continuous**	**GE***	* By geotechnical engineer or his or her qualified representative.
☐	**c.**			

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (CONCRETE), 2022 CBC

Table 1705A.3; ACI 318-19 Sections 26.12 & 26.13

Application Number: 04-123501	**School Name:** PCC FABRIC SHADE STRUCTURES	**School District:** USA SHADE AND FABRIC STRUCTURES
DSA File Number: PC-140	**Increment Number:**	**Date Created:** 2024-08-06 15:16:59

	C1. CAST-IN-PLACE CONCRETE			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Verify use of required design mix.	**Continuous**	**SI**	**Table 1705A.3 Item 5, 1910A.1.**
☑	**b.** Identify, sample, and test reinforcing steel.	**Test**	**LOR**	**1910A.2;** ACI 318-19 Ch.20 and Section 26.6.1.2; DSA IR 17-10. (See Appendix (end of this form) for exemptions.)
☑	**c.** During concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	**Test**	**LOR**	**Table 1705A.3 Item 6**; ACI 318-19 Sections 26.5 & 26.12.
☑	**d.** Test concrete (f'c).	**Test**	**LOR**	**1905A.1.17**; ACI 318-19 Section 26.12.
☑	**e.** Batch plant inspection: **Not Required**	**See Notes**	**SI**	Default of '**Continuous**' per **1705A.3.3**. If approved by DSA, batch plant inspection may be reduced to '**Periodic**' subject to requirements in Section **1705A.3.3.1.**, or not required per **1705A.3.3.2.** See IR 17-13. (See Appendix (end of this form) for exemptions.)
☐	**f.** Welding of reinforcing steel.	Provide special inspection per STEEL, Category S/A4(d) & (e) and/or S/A5(g) & (h) below.		

	C2. PRESTRESSED / POST-TENSIONED CONCRETE (IN ADDITION TO SECTION C1):			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Sample and test prestressing tendons and anchorages.	**Test**	**LOR**	**1705A.3.4, 1910A.3**
☐	**b.** Inspect placement of prestressing tendons.	**Periodic**	**SI**	**1705A.3.4, Table 1705A.3 Items 1 & 9.**

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (CONCRETE), 2022 CBC

Table 1705A.3; ACI 318-19 Sections 26.12 & 26.13

Application Number: 04-123501	**School Name:** PCC FABRIC SHADE STRUCTURES	**School District:** USA SHADE AND FABRIC STRUCTURES
DSA File Number: PC-140	**Increment Number:**	**Date Created:** 2024-08-06 15:16:59

	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**c.** Verify in-situ concrete strength prior to stressing of post-tensioning tendons.	**Periodic**	**SI**	**Table 1705A.3 Item 13.** Special inspector to verify specified concrete strength test prior to stressing.
☐	**d.** Inspect application of post-tensioning or prestressing forces and grouting of bonded prestressing tendons.	**Continuous**	**SI**	**1705A.3.4, Table 1705A.3 Item 9**; ACI 318-19 Section 26.13

	C3. PRECAST CONCRETE (IN ADDITION TO SECTION C1):			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Inspect fabrication of precast concrete members.	**Continuous**	**SI**	ACI 318-19 Section 26.13, and PCI MNL-128 and -130.
☐	**b.** Inspect erection of precast concrete members.	**Periodic**	**SI***	**Table 1705A.3 Item 10.** * May be performed by PI when specifically approved by DSA.
☐	**c.** For precast concrete diaphragm connections or reinforcement at joints classified as moderate or high deformability elements (MDE or HDE) in structures assigned to Seismic Design Category D, E or F, inspect such connections and reinforcement in the field for: 1. Installation of the embedded parts 2. Completion of the continuity of reinforcement across joints. 3. Completion of connections in the field.	**Continuous**	**SI**	Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5
☐	**d.** Inspect installation tolerances of precast concrete diaphragm connections for compliance with ACI 550.5.	**Periodic**	**SI**	Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (CONCRETE), 2022 CBC

Table 1705A.3; ACI 318-19 Sections 26.12 & 26.13

Application Number: 04-123501	**School Name:** PCC FABRIC SHADE STRUCTURES	**School District:** USA SHADE AND FABRIC STRUCTURES
DSA File Number: PC-140	**Increment Number:**	**Date Created:** 2024-08-06 15:16:59

	C4. SHOTCRETE (IN ADDITION TO SECTION C1):			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Inspect shotcrete placement for proper application techniques.	**Continuous**	**SI**	**1705A.3.9, Table 1705A.3 Item 7, 1908A.1, 1908A.2, 1908A.3.** See ACI 506.2-13 Section 3.4, ACI 506R-16.
☐	**b.** Sample and test shotcrete (f'c).	**Test**	**LOR**	**1908A.2, 1705A.3.9**

	C5. POST-INSTALLED ANCHORS:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Inspect installation of post-installed anchors	**See Notes**	**SI***	**1617A.1.19, Table 1705A.3 Item 4a (Continuous) & 4b (Periodic), 1705A.3.8** (See Appendix (end of this form) for exemptions). ACI 318-19 Section 26.13. * May be performed by the project inspector when specifically approved by DSA.
☐	**b.** Test post-installed anchors.	**Test**	**LOR**	**1910A.5.** (See Appendix (end of this form) for exemptions.)

	C6. OTHER CONCRETE:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.**			

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMNINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

Application Number: 04-123501	**School Name:** PCC FABRIC SHADE STRUCTURES	**School District:** USA SHADE AND FABRIC STRUCTURES
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	S/A1. STRUCTURAL STEEL, COLD-FORMED STEEL AND ALUMINUM USED FOR STRUCTURAL PURPOSES			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Verify identification of all materials and: • Mill certificates indicate material properties that comply with requirements. •Material sizes, types and grades comply with requirements.	**Periodic**	*	**Table 1705A.2.1 Item 3a–3c**, 2202A.1; AISI S100-20 Section A3.1 & A3.2, AISI S240-20 Section A3 & A5, AISI S220-20 Sections A4 & A6. * By special inspector or qualified technician when performed off-site.
☑	**b.** Test unidentified materials	**Test**	**LOR**	**2202A.1.**
☑	**c.** Examine seam welds of HSS shapes	**Periodic**	**SI**	DSA IR 17-3.
☑	**d.** Verify and document steel fabrication per DSA-approved construction documents.	**Periodic**	**SI**	Not applicable to cold-formed steel light-frame construction, except for trusses (**1705A.2.4**).
☐	**e.** Buckling restrained braces.	**Test**	**LOR**	Testing and special inspections in accordance with IR 22-4.

	S/A2. HIGH-STRENGTH BOLTS:		see Testing Matrix table for additional information	
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Verify identification markings and manufacturer's certificates of compliance conform to ASTM standards specified in the DSA-approved documents.	**Periodic**	**SI**	**Table 1705A.2.1 Items 1a & 1b, 2202A.1;** AISC 360-16 Section A3.3, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9.
☑	**b.** Test high-strength bolts, nuts and washers.	**Test**	**LOR**	**Table 1705A.2.1 Item 1c, 2213A.1**; RCSC 2014 Section 7.2; DSA IR 17-8.
☑	**c.** Bearing-type ("snug tight") connections.	**Periodic**	**SI**	**Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2**; AISC 360-16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Section 9.1; DSA IR 17-9.
☑	**d.** Pretensioned and slip-critical connections.	*	**SI**	**Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2;** AISC 360-16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9 **"Continuous" or "Periodic" depends on the tightening method used.

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1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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	S/A3. WELDING:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Verify weld filler material identification markings per AWS designation listed on the DSA-approved documents and the WPS.	**Periodic**	**SI**	**1705A.2.5, Table 1705A.2.1 Items 4 & 5**; AWS D1.1 and AWS D1.8 for structural steel; AWS D1.2 for Aluminum; AWS D1.3 for cold-formed steel; AWS D1.4 for reinforcing steel; DSA IR 17-3.
☑	b. Verify weld filler material manufacturer's certificate of compliance.	**Periodic**	**SI**	DSA IR 17-3.
☑	**c.** Verify WPS, welder qualifications and equipment.	**Periodic**	**SI**	DSA IR 17-3.

	S/A4. SHOP WELDING (IN ADDITION TO SECTION S/A3):			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Inspect groove welds, multi-pass fillet welds, single pass fillet welds > 5/16", plug and slot welds.	**Continuous**	**SI**	**Table 1705A.2.1 Items 5a.1–4**; AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.
☑	b. Inspect single-pass fillet welds ≤ 5/16", floor and roof deck welds.	**Periodic**	**SI**	**1705A.2.2, Table 1705A.2.1 Items 5a.5 & 5a.6;** AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.
☐	**c.** Inspect welding of stairs and railing systems.	**Periodic**	**SI**	**1705A.2.1**; AISC 360-16 (and AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3.
☐	**d.** Verification of reinforcing steel weldability other than ASTM A706.	**Periodic**	**SI**	**1705A.3.1**; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.
☐	**e.** Inspect welding of reinforcing steel.	**Continuous**	**SI**	**Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8;** AWS D1.4; DSA IR 17-3.

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1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
	S/A5. FIELD WELDING (IN ADDITION TO SECTION S/A3):			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Inspect groove welds, multi-pass fillet welds, single pass fillet welds > 5/16", plug and slot welds.	**Continuous**	**SI**	**Table 1705A.2.1 Items 5a.1–4**; AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.
☐	b. Inspect single-pass fillet welds ≤ 5/16".	**Periodic**	**SI**	**Table 1705A.2.1 Item 5a.5;** AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.
☐	**c.** Inspect end-welded studs (ASTM A-108) installation (including bend test).	**Periodic**	**SI**	**2213A.2**; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1; DSA IR 17-3.
☐	**d.** Inspect floor and roof deck welds.	**Periodic**	**SI**	**1705A.2.2, Table 1705A.2.1 Item 5a.6;** AISC 360-16 (AISC 341-16 as applicable); AWS D1.3; DSA IR 17-3.
☐	**e.** Inspect welding of structural cold-formed steel.	**Periodic**	**SI***	**1705A.2.5; AWS D1.3; DSA IR 17-3.** The quality control provisions of AISI S240-20 Chapter D shall also apply. * May be performed by the project inspector when specifically approved by DSA.
☐	**f.** Inspect welding of stairs and railing systems.	**Periodic**	**SI***	**1705A.2.1**; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3. * May be performed by the project inspector when specifically approved by DSA.
☐	**g.** Verification of reinforcing steel weldability.	**Periodic**	**SI**	**1705A.3.1**; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.
☐	**h.** Inspect welding of reinforcing steel.	**Continuous**	**SI**	**Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8;** AWS D1.4; DSA IR 17-3.

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1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
	S/A6. NONDESTRUCTIVE TESTING:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Ultrasonic	**Test**	**LOR**	**1705A.2.1, 1705A.2.5;** AISC 341-16 J6.2, AISC 360-16 N5.5; AWS D1.1, AWS D1.8; DSA IR 17-2.
☐	**b.** Magnetic Particle	**Test**	**LOR**	**1705A.2.1, 1705A.2.5;** AISC 341-16 J6.2, AISC 360-16 N5.5; AWS D1.1, AWS D1.8; DSA IR 17-2.
☐	**c.**	**Test**	**LOR**	

	S/A7. STEEL JOISTS AND TRUSSES:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Verify size, type and grade for all chord and web members as well as connectors and weld filler material; verify joist profile, dimensions and camber (if applicable); verify all weld locations, lengths and profiles; mark or tag each joist.	**Continuous**	**SI**	**1705A.2.3, Table 1705A.2.3**; AWS D1.1; DSA IR 22-3 for steel joists only. **1705A.2.4;** AWS D1.3 for cold-formed steel trusses.

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1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
	S/A8. SPRAYED FIRE-RESISTANT MATERIALS:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Examine structural steel surface conditions, inspect application, take samples, measure thickness and verify compliance of all aspects of application with DSA-approved documents.	**Periodic**	**SI**	**1705A.15, 1705A.15.1, 1705A.15.2, 1705A.15.3, 1705A.15.4, 1705A.15.5, 1705A.15.6.**
☐	**b.** Test density.	**Test**	**LOR**	**1705A.15.1, 1705A.15.5, ASTM E605**
☐	**c.** Bond strength adhesion/cohesion.	**Test**	**LOR**	**1705A.15.1, 1705A.15.6, ASTM E736**

	S/A9. ANCHOR BOLTS AND ANCHOR RODS:		see T&I Note #9 for additional information	
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☑	**a.** Anchor Bolts and Anchor Rods	**Test**	**LOR**	Identify, sample and test anchor bolts and anchor rods **not** meeting exemptions identified in Section 1 of IR 17-11.
☑	**b.** Threaded rod not used for foundation anchorage.	**Test**	**LOR**	Identify, sample and test threaded rods **not** meeting exemptions identified in Section 1 of IR 17-11.

	S/A10. STORAGE RACK SYSTEMS:			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.** Materials used, to verify compliance with one or more of the material test reports in accordance with the approved construction documents.	**Periodic**	**SI**	Table 1705A.13.7
☐	**b.** Fabricated storage rack elements.	**Periodic**	**SI**	1704A.2.5; Table 1705A.13.7

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1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**c.** Storage rack anchorage installation.	**Periodic**	**SI**	ANSI/MH16.1 Section 7.3.2; Table 1705A.13.7
☐	**d.** Completed storage rack system to indicate compliance with the approved construction documents.	**Periodic**	**SI***	Table 1705A.13.7; * May be preformed by the project inspector when specifically approved by DSA.

	S/A11. Other Steel			
	Test or Special Inspection	**Type**	**Performed By**	**Code References and Notes**
☐	**a.**			

GENERAL DSA-103 NOTES:

1. THE SAMPLE DSA-103 FORM PROVIDED ON THIS SHEET IS FOR ILLUSTRATIVE PURPOSES ONLY TO ASSIST IN THE COMPLETION OF SPECIFIC DSA-103 FORMS FOR FUTURE PROJECTS.
2. A CURRENT DSA-103 FORM IS TO BE COMPLETED FOR EACH APPLICATION THAT THIS P.C. DOCUMENT IS BEING INCORPORATED INTO AND ALL SAMPLE DSA-103 SHEETS ARE TO BE CROSSED OUT ON THIS SHEET

TESTING AND INSPECTION NOTES:

1. THE PROJECT INSPECTOR AND TESTING AGENCY SHALL BE SELECTED BY THE SCHOOL DISTRICT AND APPROVED BY DSA AND THE ARCHITECT OF RECORD.
2. THE SITE PROJECT INSPECTOR SHALL BE CLASS 2. IF THE TOTAL AREA OF THE SHADE STRUCTURE(S) IS LESS THAN 2,000 SQ. FT. WITH NO FIELD WELDING OR HIGH-STRENGTH FASTENERS THEN THE SITE PROJECT INSPECTOR MAY BE A CLASS 3.
3. THE COSTS OF THE PROJECT INSPECTOR AND TESTING AGENCY SHALL BE BORN BY THE SCHOOL DISTRICT.
4. COPIES OF THE VERIFIED REPORTS SHALL BE SENT TO DSA, THE ARCHITECT, THE SCHOOL DISTRICT, THE CONTRACTOR, AND THE PROJECT INSPECTOR.
5. THE IN-PLANT INSPECTOR SHALL BE A WELDING SPECIAL INSPECTOR FOR MATERIAL VERIFICATION AND WELDING.
6. PER 2022 CBC, SECTION 1705A.3.3, BATCH PLANT INSPECTION MAY BE WAIVED WHEN THE FOLLOWING REQUIREMENTS ARE MET:
   - 6.1. A LICENSED WEIGHMASTER SHALL POSITIVELY IDENTIFY QUANTITY OF MATERIALS AND CERTIFY EACH LOAD BY A BATCH TICKET.
   - 6.2. BATCH TICKETS, INCLUDING MATERIAL QUANTITIES AND WEIGHTS SHALL ACCOMPANY THE LOAD, SHALL BE TRANSMITTED TO THE INSPECTOR OF RECORD BY THE TRUCK DRIVER WITH LOAD IDENTIFIED THEREON. THE LOAD SHALL NOT BE PLACED WITHOUT A BATCH TICKET IDENTIFYING THE MIX. THE INSPECTOR OF RECORD SHALL KEEP A DAILY RECORD OF PLACEMENTS, IDENTIFYING EACH TRUCK, IT'S LOAD, TIME OF RECEIPT AT THE JOBSITE, AND APPROXIMATE LOCATION OF DEPOSIT IN THE STRUCTURE AND SHALL MAINTAIN A COPY OF THE DAILY REC

GENERAL NOTES

1.- SPECIAL INSPECTION REQUIREMENTS SHALL FOLLOW THE ATTACHED SAMPLE TEST AND INSPECTION LIST (T & I LIST) APPROVED BY DSA. THE SHOP WELDING INSPECTION SHALL INCLUDE WELDING OF ALL STEEL MEMBERS AND IDENTIFICATION OF STEEL THROUGH MILL CERTIFICATE OR MATERIAL TESTING, UNCERTIFIED STEEL SHALL BE TESTED TO THE REQUIREMENTS OF CBC 2022 CHAPTER 17A. THE FIELD SPECIAL INSPECTION SHALL INCLUDE COMPRESSION CYLINDER TESTS FOR THE CONCRETE FOUNDATION.

2.- STRUCTURE SHALL BE IN THE LOCATION SHOWN ON THE SITE SPECIFIC DSA APPLICATION DRAWING.

3.- FOUNDATION DESIGN BASED ON CBC 2022, TABLE 1806.A.2, SOIL CLASS 5 (ALLOWABLE FOUNDATION PRESSURE 1500 PSF)

4.- DESIGN PER FOLLOWING CODES: CBC 2022 (CHAPTER 35), ASCE 7-16, AISC 360-16, AISC 341-16, ACI 318-19, ASCE 55-16 & ASCE 19-16

STRUCTURAL STEEL
1.- FABRICATION OF THE STEEL STRUCTURES SHALL BE PERFORMED BY SHADE STRUCTURES OR AN AUTHORIZED LICENSEE. MATERIAL TESTING (OR MILL CERTIFICATES) AND INSPECTION OF WELDING SHALL BE CONDUCTED PER CBC 2022 SECTIONS 1704A, 1705A, 1705A.2, AND TABLE 1705A.2.1.

2.- ONLY CALIFORNIA LICENSED CONTRACTORS AUTHORIZED BY SHADE STRUCTURES SHALL INSTALL THE SHADE STRUCTURES.

3.- ALL WORK SHALL CONFORM TO CBC 2022 EDITION, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR).

4.- ALL STRUCTURAL SHAPES SHALL BE COLD FORMED HSS ASTM A500 GRADE C, UNLESS OTHERWISE NOTED. TYPICAL MECHANICAL PROPERTIES ACHIEVED FOR HSS PRODUCTS:

SQUARE AND RECTANGULAR	50,000 PSI YIELD STRESS	/	62,000 PSI TENSILE STRESS
ROUND PIPE	50,000 PSI YIELD STRESS	/	62,000 PSI TENSILE STRESS

5.- ALL PLATES PRODUCTS SHALL COMPLY WITH ASTM A572 GRADE 50.

6.- STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH A.I.S.C. SPECIFICATIONS.

7.- ALL WELDING TO CONFORM WITH AMERICAN WELDING SOCIETY STANDARDS AND SHALL BE INSPECTED BY AN AWS/CWI INSPECTOR. AWS D1.1 FOR HOT ROLLED. AWS D1.3 FOR SHEET/COLD FORMED. AWS D1.8 SEISMIC SUPPLEMENT.

8.- ALL FULL PENETRATION WELD SHALL BE CONTINUOUSLY INSPECTED PER AWS D1.1 & D1.8.

9.- SHOP CONNECTIONS SHALL BE WELDED UNLESS NOTED OTHERWISE. ALL FILLET WELDS SHALL BE A MINIMUM OF 3/16" ER70SX ELECTRODES UNLESS OTHERWISE NOTED. GMAW IS ACCEPTABLE.

10.- ALL STRUCTURAL STEEL (ITEMS FROM NOTE 4) SHALL BE POWDER COATED WITH ONE SHOP COAT (2.5 MILS MIN.) OF ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT, OR EQUIVALENT PAINT SYSTEM. THIS COAT IS A WEATHER RESISTANT POWDER COATING BASED ON POLYESTER TGIC (MANUFACTURED BY SHERWIN WILLIAMS, ASKO NOBEL, PPG OR TIGER DRYLAC). TO ACHIEVE OPTIMUM ADHESION, IT IS RECOMMENDED THAT THE PROPER TREATMENT AND DRYING TAKE PLACE BEFORE COATING. POLYESTER POWDER (TGIC) SPECIFICATIONS SHALL BE AS FOLLOWS:
- PENCIL HARDNESS (ASTM D-3363). - HUMIDITY (ASTM D-2247).
- SOLVENT RESISTANCE (PCI METHOD) - 50 DBL RUBS SL. SOFTNESS.

11.- ALL EXPOSED STEEL FASTENERS SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM), HOT DIP GALVANIZED (ASTM A153, CLASS D MINIMUM OR ASTM F2329) AS APPLICABLE, OR PROTECTED WITH CORROSION PREVENTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RED RUST IN MINIMUM 1,000 HOURS OF EXPOSURE IN SALT SPRAY TEST PER ASTM B117. ZINC-PLATED FASTENERS DO NOT COMPLY WITH THIS REQUIREMENT.

CONCRETE SPECIFICATION
1.- CONCRETE SHALL BE SAMPLED AND TESTED PER CBC 2022 SECTION 1903A & SHALL BE INSPECTED PER SECTION 1903A.

2.- CONCRETE TO BE F'c= 4500 PSI, TYPE V CEMENT PLUS POZZOLAN OR SLAG CEMENT, MAXIMUM WATER/CEMENT RATIO OF 0.45, PER ACI 318-19 CHAPTER 19. (NO ADMIXTURES CONTAINING CALCIUM CHLORIDE WILL BE USED.) REINFORCING STEEL SHALL CONFORM TO ASTM A-615 GRADE 60 AND TO BE Fy= 60000 PSI . MIN. GR. 60.

3.- ALL ANCHOR BOLTS SET IN NEW CONCRETE (WHEN APPLICABLE) SHALL COMPLY WITH ASTM F-1554 GRADE 36 (GALVANIZED PER ASTM A153, CLASS D MINIMUM OR ASTM F2329).
ANCHOR BOLT'S DIAMETER NEEDS TO BE AS FOLLOW:
A) ANCHOR BOLT Ø1 1/4"

4.- CERTIFIED MILL TEST REPORTS ARE TO BE PROVIDED FOR EACH SHIPMENT OF REINFORCEMENT.

5.- ALL NON-SHRINK GROUT SHALL HAVE A MINIMUM 28 DAYS COMPRESSIVE STRENGTH OF 5000 PSI, AND SHALL COMPLY THE REQUIREMENTS OF ASTM C109, ASTM C939, ASTM C1090, ASTM C1107, WHEN APPLICABLE.

6.- CONCRETE EXPOSED TO FREEZING-AND-THAWING CYCLES SHALL BE AIR ENTRAINED PER ACI 318 SECTION 19.3.3.

FABRIC SPECIFICATION
1.- FABRIC SHALL BE MANUFACTURED BY MULTIKNIT LTD., WHICH MEETS THE SPECIFICATIONS LISTED ON PAGE 2000, AND SHALL BE FABRICATED FROM POLYETHYLENE MATERIALS. MINIMUM SEAM LENGTH 3/4".

2.- THE FABRIC SHALL RETAIN 80% OF ITS TENSILE AND TEARING STRENGTH AFTER ULTRAVIOLET EXPOSURE PER ASTM G53 USING A 313 NM LIGHT SOURCE FOR 500 HOURS WHILE MOISTENED FOR 1 HOUR EVERY 12 HOURS.

3.- PROVIDE CERTIFICATION BY MANUFACTURER AND STATE FIRE MARSHAL TO SCHOOL'S DISTRICT INSPECTOR OF RECORD AT SITE SPECIFIC INSTALLATION. COPY OF FIRE CERTIFICATION SHALL BE SENT TO DSA.

4.- FABRIC SHALL REQUIRE ANNUAL INSPECTION AND MAINTENANCE BY THE DISTRICT. FIRE TEST ON FABRIC: NFPA 701 TEST 2 AND ASTM E 84 EXTENDED 30 MINUTES TEST. FLAME SPREAD INDEX (FSI): 10. SMOKE DEVELOPED INDEX (SDI): 50. FABRIC IS ACCEPTABLE FOR USE IN WILDLIFE URBAN INTERFACE AREA.

5.- FABRIC TOP NEEDS TO BE REMOVED IF SNOW EXCEEDING 5 PSF ARE ANTICIPATED, FABRIC TOP NEEDS TO BE REMOVED IF WINDS EXCEEDING 115 MPH ARE ANTICIPATED.

6.- A VISUAL INSPECTION LOOKING FOR TEAR AND ABNORMAL WEAR IN FABRIC MATERIAL AND THREAD IS REQUIRED PRIOR TO RE-INSTALLATION. USA SHADE & FABRIC STRUCTURES SHALL BE NOTIFIED IF SIGNIFICANT DAMAGE IS PRESENT BEFORE RE-INSTALLATION.

AIRCRAFT CABLE
1.- FOR FABRIC ATTACHMENT USE 1/2" 6x19 GALV. CABLE PER ASTM A1023/A1023M, WITH A BREAKING STRENGTH VALUE OF 20,700 LBS. CABLE SHALL BE TENSIONED TO 300 LBS MINIMUM AND 500 LBS MAXIMUM.THE MAXIMUM CALCULATED CABLE ALLOWABLE CAPACITY IS Sa=7056 LB.

2.- CABLES SHALL BE FED THROUGH THE FABRIC SLEEVES AROUND THE PERIMETER OF THE CANOPY AND TENSIONED UNTIL THE FABRIC PANELS (DESIGNED PURPOSELY UNDERSIZED) REACH A TAUT APPEARANCE. ANY LONG TERM CABLE SAG SHALL BE MINIMIZED DURING THE MAINTENANCE RE-TIGHTING VISITS AS REQUIRED.

MAXIMUM OCCUPANT LOAD (PER CBC 2022 TABLE 1604A.5)	
-K-12:	250 PERSONS
-PUBLIC ASSEMBLY:	300 PERSONS
-EDUCATIONAL OCCUPANCIES ABOVE 12TH GRADE:	500 PERSONS

NOTE: THE MINIMUM CLEARANCE REQUIRED BETWEEN DRILLED PIERS WHEN PLACING MULTIPLE OPEN FABRIC SHADE STRUCTURES ADJACENT TO EACH OTHER, FROM CENTER TO CENTER, IS THREE TIMES THE LEAST HORIZONTAL DIMENSION OF THE PIER PER CBC 2022 SEC. 1810A.2.5.

FOOTPRINT CONFIGURATION

1.- THE STRUCTURE CAN BE A SINGLE 4 POST TENSION SAIL.

2.- THE STRUCTURE CAN BE PLACED FOLLOWING A CURVED CONFIGURATION AS LONG AS THE MAXIMUM DIMENSIONS ARE NOT EXCEEDED.

CBC PC DESIGN NOTES

BUILDING CODE	CBC 2022 (BASED ON IBC 2021)
FLOOR LIVE LOAD	N/A
ROOF LIVE LOAD	RLL 5 PSF

ALLOWABLE SOIL PRESSURE:	
DL + LL (CONC FTG)	1500 PSF
DL + LL + SEISMIC (CONC FTG)	1500 PSF
LATERAL BEARING DESIGN VALUE	100 PSF/FT BELOW NATURAL GRADE, PER TABLE 1806A.2

TWO TIMES THE TABULAR VALUE IS USED (200 PSF/FT)
PER CBC SECTION 1806A.3.4.
ALLOWABLE PIER FRICTIONAL RESISTANCE 250 PSF MAXIMUM
BASED ON SECTION 1810A.3.3.1.4 (ONE-SIXTH OF THE BEARING VALUE).
UPLIFT FRICTIONAL RESISTANCE HAVE A SAFETY FACTOR OF 3.

ROOF SNOW LOAD	5 PSF
ICE LOAD	ZERO PSF
FLOOD HAZARD AREA	ZONE X

WHEN A SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS NEEDED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED IN THE PC ARE STILL APPLICABLE.

WIND DESIGN DIRECTIONAL PROCEDURE: ASCE 7-16, SECTION 27.3.2
NOTE: WIND DESIGN IS LIMITED TO UNOBSTRUCTED CLEAR FLOW CONDITION

-BASIC DESIGN WIND SPEED (3 SEC GUST)	V	115 MPH
-ASD WIND LOAD (CBC 2022 SEC. 1603A.1.4)	V_{ASD}	90 MPH
-WIND EXPOSURE FACTOR	C	
-TOPOGRAPHIC FACTOR	Kzt	1
-RISK CATEGORY	II	
-VELOCITY PRESSURE EXPOSURE COEFFICIENT	Kz	0.88
-VELOCITY PRESSURE	qz	25.32 PSF

SEISMIC DESIGN:		
-SITE CLASS	D	

NOTE: UNLESS A SITE-SPECIFIC GROUND MOTION HAZARD ANALYSIS IS PERFORMED, THE SM1 VALUE INCREASED BY 50% SHALL BE LESS THAN THE DESIGN CRITERIA STATED HEREIN.

	SS	3.00g
	S1	1.389g
-SPECTRAL RESPONSE COEFFICIENTS	SDS	2.00
	SD1	1.39

-LATERAL FORCE RESISTING SYSTEM G.2 ORDINARY CANTILEVERED COLUMN SYSTEM.

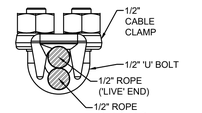
-SEISMIC IMPORTANCE FACTOR	Ie	1.0
-DESIGN BASE SHEAR AT BASE	V	22638 LB
-SEISMIC RESPONSE COEFFICIENTS	Cs	1.6
-RESPONSE MODIFICATION FACTOR	R	1.25
-ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE	
-RISK CATEGORY	II	
-SEISMIC DESIGN CATEGORY		E
-SITE COEFFICIENT CATEGORY	Fa	1.2
	Fv	1.5
-REDUNDANCY FACTOR	ρ	1.3

GEOHAZARD REPORT IS NOT REQUIRED FOR OPEN FABRIC STRUCTURES 1,600 SQF OR LESS COMPLYING WITH THE REQUIREMENTS OF IR A-4 SECTION 3.1.1. OPEN FABRIC SHADE STRUCTURES GREATER THAN 1,600 SQUARE FEET UP TO A MAXIMUM OF 4,000 SQUARE FEET AND COMPLYING WITH THE REQUIREMENTS NOTED IN IR A-4 SECTION 3.1.1 DO NOT REQUIRE A GEOHAZARD REPORT PROVIDED A GEOTECHNICAL REPORT INDICATES THAT NO LIQUEFACTION POTENTIAL EXISTS.

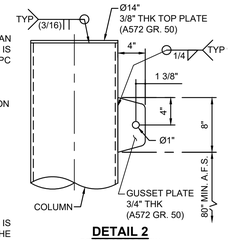
ARCHITECT OF RECORD TO DETERMINE IF SPECIFIC SITE IS IN GEOLOGIC HAZARD ZONE. GEOHAZARD REPORT REQUIREMENTS PER DSA IR A-4.

PC OPTIONS SHALL NOT INCLUDE LIQUEFIABLE SOIL (EXCEPTION: OPEN FABRIC SHADE STRUCTURES 1,600 SQUARE FEET OR LESS COMPLYING WITH REQUIREMENTS OF IR A-4 SECTION 3.1.1). IF STRUCTURE IS LOCATED IN AN AREA WITH LIQUEFIABLE SOIL OR SITE CLASS F, OVER-THE-COUNTER SUBMITTAL IS NOT ALLOWED AND REGULAR PROJECT SUBMITTAL IS REQUIRED. IF SITE IS NOT IN A MAPPED LIQUEFACTION HAZARD ZONE, IT MAY BE PRESUMED THAT NO LIQUEFACTION HAZARD EXISTS ON THAT SITE UNLESS A SITE-SPECIFIC GEOTECHNICAL REPORT IDENTIFIES SUCH HAZARD.

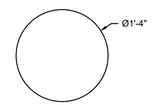
MINIMUM FOUNDATION SETBACK LIMIT IN ADJACENT SLOPE: THE DEPTH OF REQUIRED PIER EMBEDMENT SHALL START FROM AN ELEVATION THAT CORRESPONDS WITH A HORIZONTAL CLEAR DISTANCE OF 24'-6" FEET THAT INTERSECT WITH THE SLOPE (DAYLIGHTING). IF SETBACK LIMITS ARE SMALLER THAN CBC REQUIRES, A SITE-SPECIFIC SOILS REPORT IS REQUIRED.



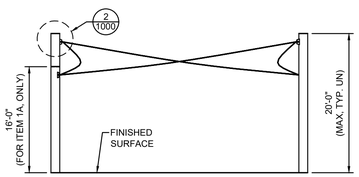
VIEW A-A
REFER TO DETAIL 1



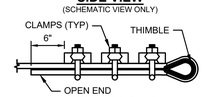
DETAIL 2
REFER TO SIDE VIEW



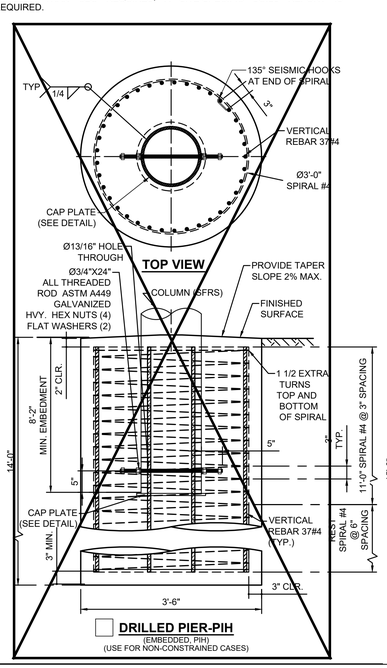
CAP PLATE
(3/8" THK)
(BOTTOM OF PIH COLUMNS)
(A572 GR. 50)



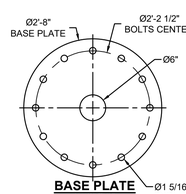
SIDE VIEW
(SCHEMATIC VIEW ONLY)



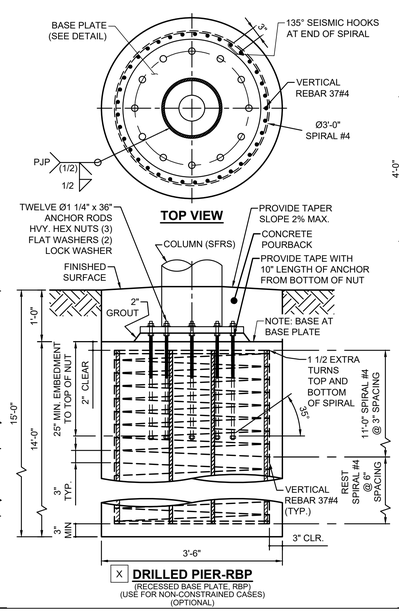
CABLE ASSEMBLY
(SEE SHEET 2000 FOR ADDITIONAL INFO)



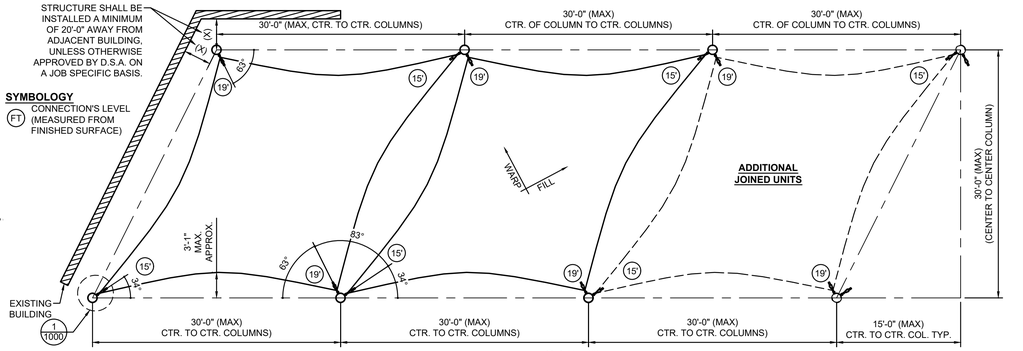
DRILLED PIER-PIH
(EMBEDDED, PIH)
(USE FOR NON-CONSTRAINED CASES)



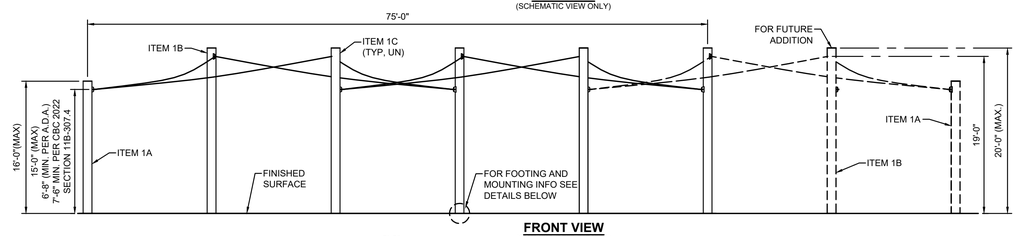
BASE PLATE
(1 1/2" THK)
(TYP. FOR RBP COLUMNS)



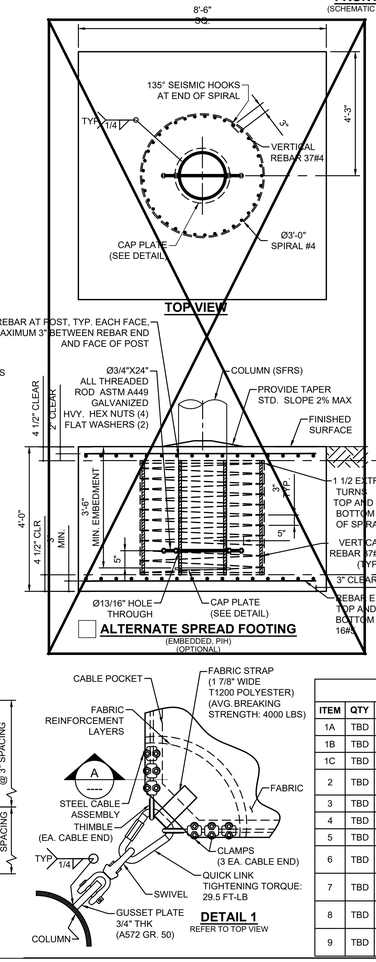
X DRILLED PIER-RBP
(RECESSED BASE PLATE, RBP)
(USE FOR NON-CONSTRAINED CASES)
(OPTIONAL)

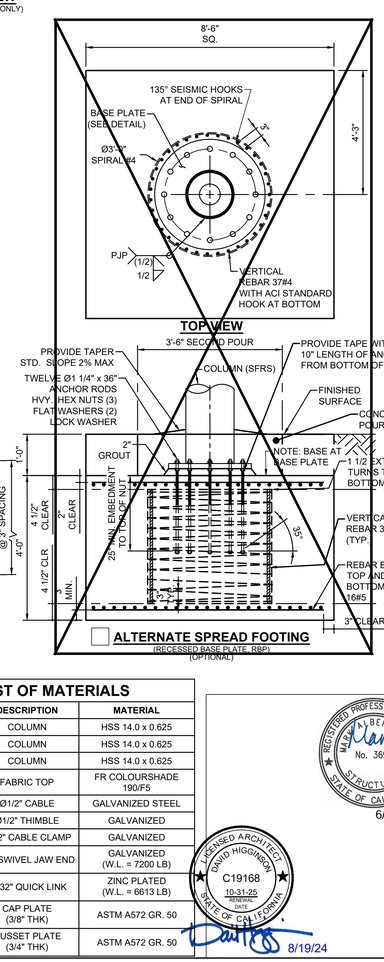


TOP VIEW
(SCHEMATIC VIEW ONLY)



FRONT VIEW
(SCHEMATIC VIEW ONLY)





DETAIL 1
REFER TO TOP VIEW

LIST OF MATERIALS

ITEM	QTY	DESCRIPTION	MATERIAL
1A	TBD	COLUMN	HSS 14.0 x 0.625
1B	TBD	COLUMN	HSS 14.0 x 0.625
1C	TBD	COLUMN	HSS 14.0 x 0.625
2	TBD	FABRIC TOP	FR COLOURSHADE 190/F5
3	TBD	Ø1/2" CABLE	GALVANIZED STEEL
4	TBD	Ø1/2" THIMBLE	GALVANIZED
5	TBD	Ø1/2" CABLE CLAMP	GALVANIZED
6	TBD	3/4" SWIVEL JAW END	GALVANIZED (W.L. = 7200 LB)
7	TBD	25/32" QUICK LINK	ZINC PLATED (W.L. = 6613 LB)
8	TBD	CAP PLATE (3/8" THK)	ASTM A572 GR. 50
9	TBD	GUSSET PLATE (3/4" THK)	ASTM A572 GR. 50

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 04-123574 INC:
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 01/15/2026

THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN PERMISSION.

USASHADE & Fabric Structures
CORPORATE HEADQUARTERS
2580 ESTERS BLVD. SUITE 100
DFW AIRPORT, TX, 75261
800-966-5005

CERTIFICATIONS:
IAS CERTIFICATION No: FA-428
CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:
Moreno Valley U.S.D.

PROJECT NAME:
Valley View High School

LOCATION:
13135 Nason Street
Moreno Valley, CA 92555

MODEL NUMBER:
DSA4183030-22

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 04-123501 PC
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑ CG ☐
DATE: 12/11/2024

STRUCTURE TYPE:
TENSION SAILS DSA

SIZE: MAXIMUM
30' x 133' MAX. x 15'e

SCALE : NONE

DRAWING SIZE:
D

PRE-CHECK (PC) DOCUMENT
Code : 2022 CBC
A separate project application for construction is required.

Eng. By :	HH	12/01/22
Design By :	OS	12/01/22
Approved By :	MB	12/01/22

REGISTERED PROFESSIONAL ENGINEER MARK ALBERT BERRY No. 3693 STRUCTURAL STATE OF CALIFORNIA
6/28/2024

LICENSED ARCHITECT DAVID HIGGINSON C19168 10-31-25 RENEWAL DATE STATE OF CALIFORNIA
8/19/24

DRAWING DESCRIPTION:
PRODUCT INFORMATION

DWG.
DSA4183030-22

SHEET
25.1-1000

REV.
NC

Multiknit INTERNATIONAL

190/F5 Fire rated specifications

Standard range

Revision 0 28-Oct-12

Colour	Shade %	UV Block %	Average GSM	Average Warp break strength kgs	Average Elongation %	Average Weft break strength kgs	Average Elongation %	Average Burst Kpa	Average Burst to Mass ratio
Desert Sand	80	92	185	50	40	72	73	156	0.84
Blue	80	85	185	50	40	72	73	156	0.84
Brown	85		185	50	40	72	73	156	0.84
Green	80	85	185	50	40	72	73	156	0.84
Red	80	86	185	50	40	72	73	156	0.84
Silver	80	81	185	50	40	72	73	156	0.84
Terracotta	75	82	185	50	40	72	73	156	0.84
Yellow	80	89	185	50	40	72	73	156	0.84
				110 LB		159 LB		3258 PSF	

CONVERSION TO IMPERIAL UNITS:
185 GSM = .0378 psf
50 KGS = 110 Lb
72 KGS = 159 Lb
156 Kpa = 3258 psf

Notes:
- 190/F5 conforms to The California State Fire Marshal Title 19 Test for Small scale Fabrics
- Tear tests are done using a 50mm wide strip and a cross head speed of 500mm/min
- This report has been compiled using the mean results from all tests conducted on the given sample by our Quality Control Laboratory, the information provided is considered to be a good reflection of the relevant properties of the fabric tested. These results must only be used as an indication of the quality and characteristics of the fabric tested.
Company cannot be held responsible or liable in any way whatsoever should this information differ to that of a registered testing institution.

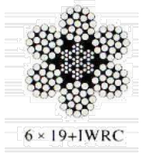
PP
Deon Joubert
General Manager - Multiknit (Pty) Ltd

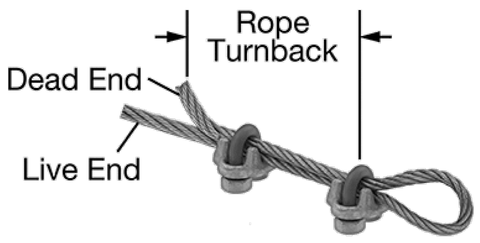
Tommy Rogers
Managing Director - Multiknit (Pty) Ltd

GALVANIZED IWRC
6 X 19 IWRC

IMPROVED PLOW STEEL / EXTRA IMPROVED PLOW STEEL

NOMINAL DIAMETER	MIN. BREAKING STRENGTH		WEIGHT	STOCK NUMBER
	IPS	EIPS*		
INCH	LBS	LBS	LBS/FT	6X19
1/4"	5,300	6,120	0.105	J42
5/16"	8,240	9,480	0.164	K42
3/8"	11,800	13,600	0.236	L42
7/16"	16,000	18,360	0.320	M42
1/2"	20,700	24,000	0.420	N42
9/16"	26,100	30,200	0.530	O42
5/8"	32,200	37,000	0.660	A42
3/4"	46,000	53,000	0.950	Q42
7/8"	62,200	71,600	1.290	R42
1"	80,800	93,000	1.680	S42
1 1/8"	101,800	117,000	2.130	T42
1 1/4"	125,000	143,800	2.630	U42
1 3/8"	150,400	172,800	3.180	V42
1 1/2"	178,000	206,000	3.780	W42





FORGED WIRE ROPE CLAMP

FITTING TYPE: ROPE CLAMP
FABRICATION: FORGED
MATERIAL: GALVANIZED STEEL
FOR WIRE ROPE DIAMETER: 1/2"
NUMBER OF CLAMPS REQUIRED: 3
ROPE TURNBACK: 11 1/2"
FOR WIRE ROPE CONSTRUCTION: 6 × 19
ATTACHMENT TYPE: LOOP
CLAMP
WIDTH: 2 5/16"
HEIGHT: 2 3/8"
THICKNESS: 1 15/16"
REQUIRED INSTALLATION TOOL: TORQUE WRENCH
REQUIRED TORQUE: 65 FT.-LBS.
CAPACITY: 80% OF THE ROPE'S CAPACITY
SPECIFICATIONS MET ASME B30.26, FED. SPEC. FF-C-450

CAL FIRE

FLAME RETARDANT

Fabric Registration

LICENSE NUMBER: F-052001

COLOURSHADE 190/F5

Product Marketed by:

MULTIKNIT (PTY) LTD
BOX 798 WHITE RIVER 1240
MPUMALANGA SOUTH AFRICA, ,

Issue Date : 05/08/2023
Expiration Date : 06/30/2024

This product meets the minimum requirements of flame resistance established by the California State Fire Marshal for products identified in Section 13115, California Health and Safety Code. The scope of the approved use of this product is provided in the current edition of the CALIFORNIA APPROVED LIST OF FLAME RETARDANT CHEMICALS AND FABRICS, GENERAL AND LIMITED APPLICATIONS CONCERNS published by the California State Fire Marshal.

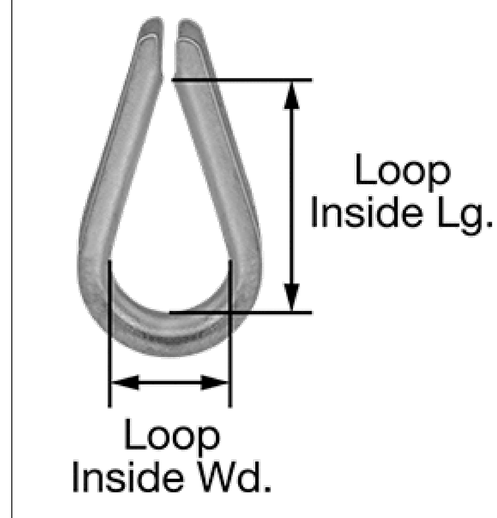
Issued By Cortney Walker
Fire Engineering License Manager
Fire Engineering & Investigations Division

Reviewed and Approved By Patricia Setter
Deputy State Fire Marshal III
Fire Engineering & Investigations Division

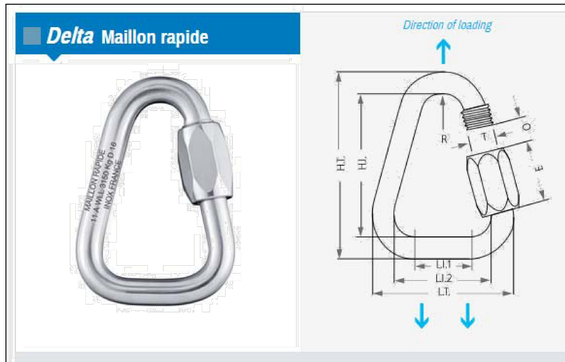
OFFICE OF THE STATE FIRE MARSHAL

Please visit calfire.govmotus.org for more information on Licensing and Permitting with CAL FIRE

Page 1 of 1



Delta Maillon rapide



Delta shape: developed after the ever-increasing development of webbing-fitted systems; perfect for webbing uphold onto its lower flat part.

Zinc plated steel | Other materials and dimensions | Stainless steel | Zicral

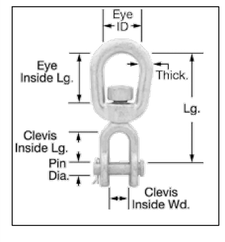
Reference	Diameter mm	Diameter inches	L.T.	L.I.1	L.I.2	H.T.	H.I.	O	E	R	T	Weight g	WLL kg	BL kg	Quote Qty
MRDZ02.5	2,5	3/32"	22	10	17	27	22	3,5	8	3,5	3,5	3	25	125	
MRDZ03.0	3	7/64"	27	12,5	21	30	24	4	9	4,25	4	6	40	200	
MRDZ03.5	3,5	1/8"	31	14	24	36	29	5	11	5	5	9	70	350	
MRDZ04.0	4	5/32"	35,5	16	27,5	40	32	5,5	12,5	5,75	6	14	100	500	
MRDZ05.0	5	3/16"	40	17	30	48	38	6,5	16	6,5	7	23	150	750	
MRDZ06.0	6	1/4"	47	20,5	35	56	44	7,5	19	7,25	9	39	250	1250	
MRDZ07.0	7	9/32"	51	21	37	63	49	8,5	21,5	8	10	58	400	2000	
MRDZ08.0	8	5/16"	56	22,5	40	73	57	10	24	8,85	11	88	550	2750	
MRDZ09.0	9	3/8"	60	23	42	78	60	11	26	9,5	12	115	700	3500	
MRDZ10.0	10	7/16"	66	25,5	46	87	67	12	29	10,25	13	153	900	4500	
MRDZ12.0	12	1/2"	75	27,5	51	104	80	15	33	11,75	15	256	1100	5500	
MRDZ14.0	14	9/16"	85	30,5	57	123	95	17	38,5	13,25	17	404	1800	9000	
MRDZ16.0	16	5/8"	93	31,5	61	138	106	19	45	14,75	19	612	2200	11000	
MRDZ18.0	18	11/16"	102	32,5	66	155	119	23	52	16,25	22	845	2600	13000	
MRDZ20.0	20	25/32"	112	31,5	72	176	136	24	60	17,75	24	1185	3000	15000	

*WLL: Working load limit.
*BL: Breaking load.

Other materials and dimensions | Stainless steel | Zicral

25/32 QUICK LINK UNITS CONVERSION

LT	LI1	LI2	HT	HI	O	E	R	T	WEIGHT	WORKING LOAD	BREAKING LOAD
[in]	[in]	[in]	[in]	[in]	[in]	[in]	[in]	[in]	[lb]	[lb]	[lb]
4.409	1.240	2.835	6.929	5.354	0.945	2.362	0.699	0.945	2.61	6613	33069



EYE-TO-CLEVIS SWIVEL
MATERIAL: GALVANIZED STEEL
LENGTH: 5 7/8"
EYE:
THICKNESS: 3/4"
INSIDE LENGTH: 1 3/4"
ID: 2"
CLEVIS
INSIDE WIDTH: 1 1/8"
INSIDE LENGTH: 1 3/4"
PIN DIAMETER: 3/4"
PIN TYPE: COTTER
CAPACITY: 7,200 LBS.
FABRICATION: FORGED
SPECIFICATIONS MET FED. SPEC. RR-C-271
FITTING TYPE: SWIVEL
ATTACHMENT TYPE: EYE-TO-CLEVIS

WIRE ROPE THIMBLE
FITTING TYPE: THIMBLE
MATERIAL: GALVANIZED STEEL
FOR WIRE ROPE DIAMETER" 1/2"
LOOP
INSIDE LENGTH: 1 7/8"
INSIDE WIDTH: 1 1/8"
SPECIFICATIONS MET FED. SPEC. FF-T-276B

REGISTERED PROFESSIONAL ENGINEER ALBERT W. CLARK JR. No. 3693 STRUCTURAL STATE OF CALIFORNIA
6/28/2024

LICENSED ARCHITECT DAVID HIGGINSON C19168 10-31-25 RENEWAL DATE STATE OF CALIFORNIA
8/19/24

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 04-123574 INC:
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 01/15/2026

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USASHADE & Fabric Structures
CORPORATE HEADQUARTERS
2580 ESTERS BLVD. SUITE 100
DFW AIRPORT, TX, 75261
800-966-5005

CERTIFICATIONS:
IAS CERTIFICATION No: FA-428
CLARK COUNTY MANUFACTURER CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:
Moreno Valley U.S.D.

PROJECT NAME:
Valley View High School

LOCATION:
13135 Nason Street
Moreno Valley, CA 92555

MODEL NUMBER:
DSA4183030-22

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 04-123501 PC
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑ CG ☐
DATE: 12/11/2024

STRUCTURE TYPE:
TENSION SAILS
DSA

SIZE: **MAXIMUM**
30' x 133' MAX. x 15'e

SCALE : NONE

DRAWING SIZE:
D

PRE-CHECK (PC) DOCUMENT
Code : 2022 CBC
A separate project application for construction is required.

Eng. By :	HH	12/01/22
Design By :	OS	12/01/22
Approved By :	MB	12/01/22

DRAWING DESCRIPTION:
SPECIFICATIONS

DWG. **DSA4183030-22**

SHEET **25.2-2000**

REV. **NC**