MINIMUN	<u>A DESIGN LOADS</u>		
1. DESIG	N LOADS ARE ALL DEAD LOADS PLUS THE FOLLOWING LIVE LOADS:		
1.1.	MAIN FLOORS @ PUBLIC AREAS	40	PSF
1.2.	DECKS	40	PSF
1.3.	PORCHES	40	PSF
1.4.	STAIRS	100	PSF
1.5.	STAIRS 1 & 2 FAMILY RESIDENCES	40	PSF
1.6.	PARTITION	20	PSF
	CORRIDORS		PSF
1.8.			PSF
1.9.			PSF
1.10.	HABITABLE ATTICS		PSF
1.11.	ELEVATED GARAGES: 3,000 LB POINT LOAD OVER $4\frac{1}{2}$ "x $4\frac{1}{2}$ " AREA AT ANY LOCAT	ON OR 40	PSF
2. WIND	LOADS		
2.1.	BASIC WIND SPEED 3 SECOND GUST	$V_{ult} = 115$	MPH
2.2.	IMPORTANCE FACTOR	1.0	
2.3.	WIND EXPOSURE CATEGORY	В	
2.4.	DESIGN WIND PRESSURE FOR COMPONENTS & CLADDING	20	PSF
3. SNOV	GROUND LOAD, pg	5	PSF

4. ALL DESIGN LOADS ARE IN ACCORDANCE WITH THE 2018 INTERNATIONAL BUILDING CODE (IBC) AND/OR 2018 INTERNATIONAL RESIDENTIAL CODE (IRC).

5. ALL LIVE LOADS SHALL BE INCREASED IF IT IS REQUIRED BY ANY RATING (i.e. UL555, ... etc.) ACCORDING TO THE RATINGS RECOMMENDATION. CONTACT STRUCTURAL ENGINEER IN CASE OF INCREASED LOADS. GENERAL STRUCTURAL NOTES

- 1. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, 10. ALL FOOTINGS AND FOUNDATIONS SHALL BE PLACED BELOW THE "FROST - DEPTH" OF THE CHASES, HANGERS, INSERTS, ANCHORS, HOLES AND OTHER ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS 11. IN THE PRESENCE OF THE GROUND WATER TABLE ABOVE ANY FOOTING OR FOUNDATION. DURING THE WORK. THE ENGINEER WILL NOT ADVISE ON NOR ISSUE DIRECTION AS TO SAFETY PRECAUTIONS AND PROGRAMS
- 3. THE STRUCTURAL DRAWINGS HERE IN REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR SHALL PROVIDE ALL 12. ALL STEEL EXPOSED TO WATER, MOISTURE, AND / OR CORROSIVES SHALL BE COVERED WIT TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL APPROPRIATE PROTECTIVE APPROVED COATING MATERIALS. ALL STRUCTURAL WORK AND CONNECTIONS HAVE BEEN COMPLETED. THE INVESTIGATION, DESIGN, SAFETY, ADEQUACY AND INSPECTION OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- 4. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR THE METHODS, TECHNIQUES, AND SEQUENCES OF THE PROCEDURES TO PERFORM THE WORK. THE SUPERVISION OF THE WORK IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- 5. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO APPROVAL BY THE ENGINEER
- 6. ALL STRUCTURAL SYSTEMS WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH THE SUPPLIER'S INSTRUCTIONS AND REQUIREMENTS.
- 7. LOADING APPLIED TO THE STRUCTURE DURING THE PROCESS OF CONSTRUCTION SHALL NOT EXCEED THE SAFE LOAD-CARRYING CAPACITY OF THE STRUCTURAL MEMBERS. THE LIVE LOADING USED IN THE DESIGN OF THIS STRUCTURE ARE INDICATED IN THE "DESIGN CRITERIA NOTES". DO NOT APPLY ANY CONSTRUCTION LOADS UNTIL STRUCTURAL FRAMING IS PROPERLY CONNECTED TOGETHER AND UNTIL ALL TEMPORARY BRACING IS IN PLACE.
- 8. ALL ASTM AND OTHER REFERENCES ARE PER THE LATEST EDITIONS OF THESE STANDARDS, UNLESS NOTED OTHERWISE. 3. UNLESS NOTED OTHERWISE, ALL CONNECTIONS SHALL BE SHEAR TYPE CONNECTIONS EXCI
- 9. SHOP DRAWINGS AND OTHER ITEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE GENERAL CONTRACTOR BEFORE SUBMITTAL. THE ENGINEER'S REVIEW IS TO BE FOR CONFORMANCE WITH THE DESIGN CONCEPT AND GENERAL COMPLIANCE WITH THE RELEVANT CONTRACT DOCUMENTS. THE ENGINEER'S REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW, CHECK AND COORDINATE THE SHOP DRAWINGS PRIOR TO SUBMISSIONS. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR THE ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, DIMENSIONS, ETC.
- 11. PROVIDE ADEQUATE AND PROPER FLASHING WHEREVER REQUIRED AGAINST WATER INTRUSION.
- 12. THE DESIGNS HEREIN BELONG TO THE STRUCTURAL ENGINEER OF RECORD. A LICENSE TO CONSTRUCT THIS BUILDING FROM THESE PLANS AT A SINGLE SITE IS GRANTED TO THE CONTRACTED CLIENT. LICENSEE LIMITS LIABILITY 6. UNLESS NOTED OTHERWISE, ALL STRUCTURAL STEEL PERMANENTLY EXPOSED TO VIEW SHALL OF THE STRUCTURAL ENGINEER OF RECORD TO THE TOTAL FEES PAID FOR WORK HEREIN. LICENSE IS NON-TRANSFERABLE. ANY BREACH OF THIS LICENSE SHALL ENTITLE THE STRUCTURAL ENGINEER OF RECORD TO PURSUE ANY AND ALL REMEDIES, AT LAW OR EQUITY, INCLUDING WITHOUT LIMITATION, INJUNCTIVE RELIEF TO PREVENT OR CEASE SUCH BREACH.
- 13. IT IS THE RESPONSIBILITY OF PURCHASER OF PLANS TO ENSURE THE FOLLOWING BEFORE CONSTRUCTION: 13.1. CONTRACTOR MUST VERIFY ALL DIMENSIONS PRIOR TO PROCEEDING WITH CONSTRUCTION. 13.2. CONTRACTOR MUST VERIFY COMPLIANCE WITH ALL LOCAL BUILDING CODES IN THE AREA THE PROJECT IS
- TO BE BUILT 13.3. ENGINEERING CONSULTANTS MUST INCORPORATE ACTUAL SITE CONDITIONS. 13.4. ANY MODIFICATIONS TO THESE DOCUMENTS MUST BE MADE BY THE STRUCTURAL ENGINEER OF RECORD.
- 13.5. PLANS INDICATE LOCATION ONLY. SITE CONDITIONS MUST BE VERIFIED BY OTHERS AND ACTUAL SITE CONDITION MUST BE INCORPORATED INTO ENGINEERING ASPECTS. 14. FOR ANY REASON, IF ANY PART OF THIS STRUCTURE (i.e. FLOORS, CEILINGS, ... etc.) IS DESIGNED BY OTHER PARTIES, 10. UNLESS NOTED OTHERWISE, ALL BEAM CONNECTIONS SHALL BE STANDARD FRAMED OR SE
- THE STRUCTURAL ENGINEER OF RECORD CLAIMS NO RESPONSIBILITY FOR, BUT NOT LIMITED TO, THE LATERAL RESISTANCE, STABILITY OF THE STRUCTURE, PROPER TRANSFER OF DESIGN LOADS, ANCHORAGE, HOLD DOWN, AND ANY OTHER ATTACHMENTS OR CONNECTION METHODS.
- 15. ALL SHOP DRAWINGS SUBMITTED FOR APPROVAL (IF INCLUDED IN THE CONTRACT) NEED TO BE SEALED, SIGNED, AND DATED BY A REGISTERED ENGINEER IN THE STATE THE PROJECT IS TO BE BUILT.

WOOD FRAMING NOTES

- 1. ALL WOOD FRAMING MATERIAL SHALL BE SURFACED DRY AND USED AT 19% MAXIMUM MOISTURE CONTENT. 2. ALL STUD AND WALL FRAMING SHALL BE EITHER NO. 2 GRADE SOUTHERN YELLOW PINE (SYP) OR NO. 2 GRADE
- SPRUCE-PINE-FIR (SPF). 3. ALL JOIST, RAFTER, AND MISCELLANEOUS FRAMING SHALL BE NO. 2 GRADE SOUTHERN YELLOW PINE. PROVIDE FULL-DEPTH (OR METAL) BRIDGING AT MID-SPAN AND AT A MAXIMUM SPACING OF 8'-0" O.C. IN BETWEEN.
- 4. ALL FRAMING EXPOSED TO THE WEATHER OR IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE-TREATED IN ACCORDANCE WITH THE AMERICAN WOOD PRESERVER'S ASSOCIATION SPECIFICATIONS. WHERE POSSIBLE, ALL CUTS AND HOLES SHOULD BE COMPLETED BEFORE TREATMENT. CUTS AND HOLES DUE TO ON-SITE FABRICATION SHALL BE BRUSHED WITH 2 COATS OF COPPER NAPHTHENATE SOLUTION CONTAINING A MINIMUM OF 2% METALLIC COPPER IN SOLUTION (PER AWPA STD. M4).
- 5. THE CONTRACTOR SHALL CAREFULLY SELECT LUMBER TO BE USED IN LOAD BEARING APPLICATIONS. THE LENGTH OF SPLIT ON THE WIDE FACE OF 2" NOMINAL LOAD BEARING FRAMING SHALL BE LIMITED TO THE SPLIT ON THE WIDE FACE OF 3" (NOMINAL) AND THICKER LUMBER SHALL BE LIMITED TO 1/2 OF THE NARROW FACE DIMENSION.
- 6. ALL NAILING NOT OTHERWISE INDICATED SHALL BE IN ACCORDANCE WITH TABLE R602.3.(1) OF THE IRC OR SCHEDULE 2304.9.1 OF THE IBC. NAILING SHALL NOT BE OVERDRIVEN.
- 7. PROVIDE DOUBLE JOISTS UNDER ALL PARTITIONS THAT RUN PARALLEL WITH JOISTS AND UNDER ALL CONCENTRATED LOADS FROM FRAMING ABOVE. 8. PROVIDE HEADER BEAMS OF THE SAME SIZE AS JOISTS OR RAFTERS TO FRAME AROUND OPENINGS IN THE

WASHERS FOR ALL BOLT HEADS AND NUTS IN CONTRACT WITH WOOD SURFACES.

- PLYWOOD DECK UNLESS NOTED OTHERWISE. 9. STRUCTURAL STEEL PLATE CONNECTORS SHALL CONFORM TO ASTM A-36 SPECIFICATIONS AND BE 1/4" THICK UNLESS OTHERWISE INDICATED. PROVIDE WASHERS 3/4" DIAMETER UNLESS OTHERWISE INDICATED. PROVIDE
- 10. BOLT HOLES SHALL BE CAREFULLY CENTERED AND DRILLED NOT MORE THAN 1/16" LARGER THAN THE BOLT DIAMETER. BOLTED CONNECTIONS SHALL BE SNUG - TIGHT BUT NOT TO THE EXTENT OF CRUSHING WOOD UNDER WASHERS
- 11. PRE-FABRICATED STRUCTURAL COMPOSITE LUMBER (LVL, PSL, LSL) HEADERS AND BEAMS SHALL BE MANUFACTURED BY "ILEVEL BY WEYERHAEUSER" (BOISE, IDAHO TEL: 888-453-8358) OR APPROVED EQUAL. CONNECTIONS AND ATTACHMENT OF THESE MEMBERS IS TO BE IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS UNLESS NOTED SPECIFICALLY ON PLAN. DO NOT CUT OR NOTCH MATERIAL WITHOUT THE MANUFACTURER'S APPROVAL.
- 12. PREFABRICATED METAL JOIST HANGERS, HURRICANE CLIPS, HOLD-DOWN ANCHORS, AND OTHER ACCESSORIES SHALL BE AS MANUFACTURED BY "SIMPSON STRONG-TIE COMPANY", TEL (800-999-5099), OR APPROVED EQUAL. INSTALL ALL ACCESSORIES PER THE MANUFACTURER'S REQUIREMENTS. ALL STEEL SHALL HAVE A MINIMUM THICKNESS OF 0.04 INCHES (PER ASTM A-446, GRADE A) AND BE GALVANIZED (COATING G60).
- 13. ALL HARDWARE AND FASTENERS USED FOR PRESSURE TREATED WOOD, TIMBER, AND LUMBER SHALL BE MADE FROM APPROVED CORROSIVE-RESISTANT MATERIALS.
- 14. ALL EXTERIOR WALLS TO BE CONSIDERED SHEAR WALLS, THEREFORE, ALL EXTERIOR WALLS TO BE FULLY SHEATHED AND FULLY BLOCKED AT ALL EDGES. ALL COLUMNS TO BE BRACED AT THE TOP AND BOTTOM. ALL CONTINUOUS 9. ALL JOISTS SUPPORTING NON-STACKING LOAD BEARING WALLS NEED TO BE REINFORCED COLUMNS TO BE BRACED AT EACH FLOOR LEVEL, UNLESS NOTED OTHERWISE.
- 15. ALL WOOD COLUMNS TO BE BRACED AT THE TOP AND THE BOTTOM. THEREFORE, ON ALL WOOD COLUMNS, USE THE APPLICABLE SIMPSON ABU POST BASE AND CC/ECC OR PC/EPC POST CAP.
- 16. NO PRESSURE TREATED LUMBER IS TO BE IN CONTACT WITH STEEL UNLESS STEEL IS H.D.G. OR OTHERWISE TREATED TO PREVENT STEEL CORROSION.

FOUNDATION NOTES

- . ALL FOOTINGS SHALL BEAR ON UNDISTURBED, FIRM, NATURAL SOIL OR ENGINEERED SOIL CAPABLE 1. CONCRETE MIXES SHALL BE DESIGNED PER ACI 301 USING PORTLAND CEMENT, AGGREGATES OF SUPPORTING A MINIMUM DESIGN BEARING PRESSURE OF 2,000 PSF UNLESS DATA TO SUBSTANTIATE THE USE OF A HIGHER VALUE ARE SUBMITTED AND APPROVED. ALL FOUND/ EXCAVATIONS SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER / TESTING AGENCY TO POURING FOUNDATION CONCRETE.
- 2. ALL FOUNDATION CONCRETE SHALL OBTAIN A 28 DAY COMPRESSIVE STRENGTH OF 3,000
- 3. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60.
- 4. UNLESS NOTED OTHERWISE, MINIMUM CONCRETE COVER SHALL BY PROVIDED IN ACCORE WITH ACI-318 SECTION 7.7.1.
- 5. ALL REINFORCING MARKED CONTINUOUS (CONT.) ON THE PLANS AND DETAILS SHALL BE AND/OR LAPPED A MINIMUM OF 48 TIMES THE BAR DIAMETER AT ALL SPLICES, CORNERS, A OTHER JUNCTIONS UNLESS OTHERWISE NOTED.
- 6. NO UNBALANCED BACK FILLING SHALL BE DONE AGAINST FOUNDATION WALLS UNLESS WA SECURELY BRACED AGAINST OVERTURNING EITHER BY TEMPORARY BRACING OR BY PERM CONSTRUCTION.
- 7. PRIOR TO COMMENCING ANY FOUNDATION WORK, COORDINATE WORK WITH ANY EXISTI UTILITIES. FOUNDATIONS SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES.
- 8. PROVIDE CONSTRUCTION JOINTS IN ALL CONCRETE WORK AS REQUIRED BY THE ACI CODI SHOWN ON THE INDIVIDUAL DETAILS.
- 9. PROVIDE PROPER AND ADEQUATE DRAINAGE BEHIND ANY TYPE OF RETAINING AND/OR B WALLS AS THE SITE CONDITIONS REQUIRE IN THE FIELD.
- GEOGRAPHIC AREA OF THE PROJECT.
- GENERAL CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF RECORD FOR ANY I REVISION

STRUCTURAL STEEL NOTES

- 1. ALL STRUCTURAL STEEL SHALL CONFORM TO THE THIRTEENTH EDITION OF THE "MANUAL OF S CONSTRUCTION" OF THE AISC.
- 2. UNLESS NOTED OTHERWISE, ALL MATERIALS SHALL BE IN ACCORDANCE WITH THE FOLLOWI SPECIFICATIONS:

Rolled Shapes

ANCHOR BOLTS

THREADED RODS

NONSHRINK GROUT

PLATES

STRUCTURAL TUBING A-500 (GRADE B) A-992 A-36 CONNECTION BOLTS A-325 (MIN. TYPE N) F1554

C-1107

16	KSI		
50	KSI		
36	KSI		
2	KSI		
36	KSI		
36	KSI		
		8,000 PSI	

NOTED OTHERWISE AND DESIGNED BY THE FABRICATOR FOR THE FACTORED SHEAR FORCE INDICATED ON PLAN IN ACCORDANCE WITH THE AISC SPECIFICATIONS FOR LOAD AND RE FACTOR DESIGN. MINIMUM BOLT DIAMETER SHALL BE 3/4". UNLESS NOTED OTHERWISE, ALL SHALL BE SHEAR/BEARING TYPE BOLTS AND BE "SNUG-TIGHT".

A-36

- 4. ALL WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1 USING E70XX ELECTRODES. UNL NOTED OTHERWISE, PROVIDE CONTINUOUS MINIMUM SIZED FILLET WELDS PER AWS REQUIR ALL FILLER MATERIAL SHALL HAVE A MINIMUM YIELD STRENGTH OF 58 KSI.
- 5. HOLES IN STEEL SHALL BE DRILLED OR PUNCHED. ALL SLOTTED HOLES SHALL BE PROVIDED SMOOTH EDGES. BURNING OF HOLES AND TORCH CUTTING AT THE SITE IS NOT PERMITTED.
- PAINTED WITH ONE COAT OF SSPC 15-68. TYPE 1 (RED OXIDE) PAINT.
- 7. THE STRUCTURAL STEEL ERECTOR SHALL PROVIDE ALL TEMPORARY GUYING AND BRACING 'GENERAL STRUCTURAL NOTES').
- 8. COLUMNS, ANCHOR BOLTS, BASE PLATES, ETC., HAVE BEEN DESIGNED FOR THE FINAL COM CONDITION AND HAVE NOT BEEN INVESTIGATED FOR POTENTIAL LOADING ENCOUNTERE STEEL ERECTION AND CONSTRUCTION. ANY INVESTIGATION OF THE COLUMNS, ANCHOR BASE PLATES, ETC. FOR ADEQUACY DURING THE STEEL ERECTION AND CONSTRUCTION PRO THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- 9. PROTECTIVE COATINGS DAMAGED DURING THE TRANSPORTING, ERECTING, AND FIELD WE PROCESSES SHALL BE REPAIRED IN THE FIELD TO MATCH THE SHOP APPLIED COATING.
- CONNECTIONS AS SHOWN IN THE AISC MANUAL OF STEEL CONSTRUCTION. UNLESS GREAT REACTIONS ARE INDICATED ON THE DRAWINGS, THE CONNECTIONS SHALL DEVELOP AT LE HALF OF THE TOTAL UNIFORM LOAD CAPACITY TABULATED IN THE TABLES OF THE MANUAL GIVEN SIZE AND SPAN OF THE BEAM IN QUESTION. IN NO CASE SHALL THE LENGTH OF THE F CONNECTIONS BE LESS THAN ONE HALF OF THE "T" DISTANCE OF THE BEAM WEB.
- 11. PROVIDE STIFFENER PLATES ON EACH SIDE OF THE WEB OF BEAM OR GIRDER AT POINTS OF CONCENTRATED LOADS OR SEATED BEAM BEARING LOCATIONS. MINIMUM STIFFENER THIC SHALL BE 1/2" UNLESS NOTED OTHERWISE.
- 12. ALL STEEL COMPONENTS IN CONTACT WITH EACH OTHER TO BE WELDED WITH THE LARGER WELD OR MIN. SIZED WELDS PER AISC REQUIREMENTS. WELD ALL AROUND EDGES AND PER OF ALL AFFECTED MEMBERS, UNLESS NOTED OTHERWISE ON THE INDIVIDUAL DETAILS.
- 13. ALL STEEL EXPOSED TO WATER, MOISTURE, AND / OR CORROSIVES SHALL BE COVERED WIT APPROPRIATE PROTECTIVE APPROVED COATING MATERIALS.
- 14. ALL SHOP DRAWINGS SUBMITTED FOR APPROVAL (IF INCLUDED IN THE CONTRACT) NEED T SEALED, SIGNED, AND DATED BY A REGISTERED ENGINEER IN THE STATE THE PROJECT IS TO
- 15. NO STEEL IS TO BE PLACED IN CONTACT OR "PACKED" WITH PRESSURE TREATED LUMBER. H.I BE UTILIZED AS NEEDED BUT ONLY WITH PRIOR E.O.R. APPROVAL.

PREFABRICATED WOOD JOIST NOTES

- 1. PREFABRICATED WOOD I-JOISTS SHALL BE DESIGNED AND FURNISHED IN ACCORDANCE CURRENT CODE-ACCEPTED EVALUATION REPORT. STRUCTURAL CAPACITIES AND DESIGN PROVISIONS SHALL BE ESTABLISHED AND MONITORED IN ACCORDANCE WITH ASTM D-505
- 2. WOOD I-JOISTS SHALL BE ERECTED IN ACCORDANCE WITH THE MANUFACTURER'S REQUIRI THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY AND PERMANENT BRACING AS REQU SAFE ERECTION AND PERFORMANCE AND PERFORMANCE OF THE JOISTS.
- 3. WOOD I-JOIST SHALL NOT BE CUT, NOTCHED, COPED, DRILLED, NOR OTHERWISE ALTERED WAY UNLESS SPECIFICALLY CONDUCTED IN ACCORDANCE WITH THE MANUFACTURER'S V REQUIREMENTS. DO NOT CUT FLANGES.
- 4. WOOD I-JOISTS SHALL BE PRODUCED BY A CODE ACCEPTED FABRICATOR WITH A MINIMU (5) YEARS EXPERIENCE PRODUCING PREFABRICATED WOOD I-JOISTS. QUALITY CONTROL AUDITED BY AN AGENCY ACCEPTED BY THE "BUILDING OFFICIALS & CODE ADMINISTRATO
- 5. WEB PANELS MUST BE JOINED WITH A MACHINE AND GLUED "V" JOINT TO FORM A CONTIN MEMBER. ALL JOINTS SHALL BE GLUED USING AN EXTERIOR TYPE ADHESIVE PER ASTM D 25
- 6. WOOD I-JOISTS SHALL BE STORED IN BUNDLES IN AN UPRIGHT POSITION AND AWAY FROM CONTACT. DAMAGE TO JOISTS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE SUPPLIER. FIELD REPAIR OR MODIFICATION OF JOISTS MUST NOT BE MADE WITHOUT THE W APPROVAL BY THE SUPPLIER, EXCEPT FOR TRIMMING TO CORRECT LENGTH.
- WOOD I-JOISTS SHALL BE CAREFULLY HANDLED TO PREVENT DAMAGE AND DISTORTION. JOIST SHALL BE ANCHORED AND BRACED AS IT IS ERECTED USING BLOCKING PANELS AND ANCHORAGE INDICATED (AND PER THE SUPPLIERS REQUIREMENTS). ERECTOR SHALL PROV SUPPLEMENTAL LATERAL BRACING OF THE TOP FLANGE UNTIL SHEATHING IS PROPERLY NA
- 8. AVOID ALL PLUMBING AND MECHANICAL, IF POSSIBLE. OTHERWISE ALL HOLES, NOTCHES, PENETRATIONS OR ALTERATIONS TO I-JOISTS OR FLOOR JOISTS ARE TO MEET THE MANUFAC SPECIFICATIONS AND REQUIREMENTS.
- SHEAR AND / OR BENDING ACCORDING TO THE MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS.
- 10. REFER TO THE FRAMING PLANS FOR ADDITIONAL NOTES.
- 11. ALL SHOP DRAWINGS SUBMITTED FOR APPROVAL (IF INCLUDED IN THE CONTRACT) NEED TO BE SEALED, SIGNED, AND DATED BY A REGISTERED ENGINEER IN THE STATE THE PROJECT IS TO BE BUILT.

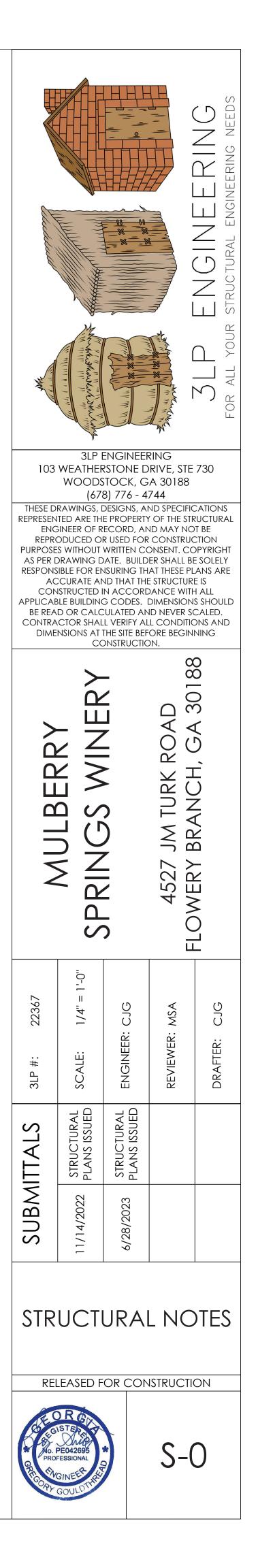
CAST-IN-PLACE CONCRETE NOTES

. CAPABLE DATION	1.	CONCRETE MIXES SHALL BE DESIGNED PER ACI 301 USING PORTLAND CEMENT, AGGREGATES AND ADMIXTURES CONFORMING TO ASTM REQUIREMENTS. CONCRETE SHALL BE READY-MIXED IN ACCORDANCE WITH ASTM REQUIREMENTS.		
CY PRIOR	2.	CONCRETE SHALL CONFORM TO THE FOLLOWING COMPRESSIVE STRENGTH, SLUMP AND WATER/CEMENT RATIO REQUIREMENTS:		
IO PSI.		CONCRETE MIN. f'c (28 DAYS) SLUMP* W/C RATIO COLUMNS 4,000 PSI 2" TO 4" .46		
RDANCE		ELEVATED SLABS 4,000 PSI 2" TO 4" .46 CONCRETE NOT NOTED 3,000 PSI 2" TO 4" .50 FOUNDATION 3,000 PSI 2" TO 4" .50 SLABS-ON-GRADE 3,000 PSI 2" TO 4" .50		
e bent , and any	3.	ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE."		
WALLS ARE MANENT	4.	ALL REINFORCING STEEL SHALL CONFORM TO ASTM REQUIREMENTS GRADE 60. ALL WELDING OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH AWS REQUIREMENTS. EPOXY COATED REINFORCING SHALL CONFORM TO ASTM REQUIREMENTS.		
STING	5.	ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM REQUIREMENTS.		
de or as	6.	ALL REINFORCING STEEL SHALL BE SET AND TIED IN PLACE PRIOR TO POURING OF CONCRETE, EXCEPT THAT VERTICAL DOWELS FOR MASONRY WALL REINFORCING MAY BE "FLOATED" IN PLACE. DO NOT FIELD BEND BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE UNLESS SPECIFICALLY INDICATED OR APPROVED BY THE ENGINEER.		
e basement	7.	REINFORCING STEEL, INCLUDING HOOKS AND BENDS, SHALL BE DETAILED IN ACCORDANCE WITH LATEST EDITION OF THE ACI 318. ALL REINFORCED STEEL INDICATED AS BEING CONTINUOUS ("CONT.") SHALL BE LAPPED WITH A TYPE 2 LAP SPLICE UNLESS NOTED OTHERWISE.		
N, THE	8.	UNLESS NOTED OTHERWISE, MINIMUM CONCRETE COVER SHALL BE PROVIDED IN ACCORDANCE WITH ACI-318 SECTION 7.7.1.		
y design /ith	9.	BAR SUPPORTS SHALL BE PROVIDED FOR ALL REINFORCING STEEL TO INSURE MINIMUM SUPPORT AND HOLDING BARS SHALL BE PER CONCRETE COVER. BAR SUPPORTS SHALL BE PLASTIC TIPPED OR STAINLESS STEEL.		
f steel wing astm	10.	 UNLESS NOTED OTHERWISE, ALL ONE WAY SLABS SHALL BE REINFORCED AS FOLLOWS: 10.1. BOTTOM REINFORCING 10.1.1. #4 @ 16" O.C. 10.1.2. BETWEEN SUPPORTS 10.2.1. #4 @ 12" O.C. 10.2.2. CENTERED ON SUPPORTS 10.3. TEMPERATURE REINFORCING 10.3.1. #4 @ 18" O.C. 10.3.2. TRANSVERSE BOTTOM 		
	11.	UNLESS OTHERWISE NOTED ON THE INDIVIDUAL DETAILS, ALL CONCRETE WALLS (OTHER THAN RETAINING WALLS AND BASEMENT WALLS) SHALL HAVE MINIMUM REINFORCEMENT AS FOLLOWS:		
		WALL THICKNESSHORIZONTALVERTICALLOCATION4" TO 6"#4 @ 16" O.C.#4 @ 16" O.C.CENTERED		
		8" #4 @ 12" O.C. #4 @ 12" O.C. CENTERED 10" #4 @ 16" O.C. #4 @ 16" O.C. EACH FACE		
CEPT AS CES RESISTANCE ALL BOLTS	12	12" #4 @ 12" O.C. #4 @ 12" O.C. EACH FACE ALL EDGES OF PERMANENTLY EXPOSED CONCRETE SURFACES SHALL BE CHAMFERED UNLESS		
NLESS IREMENTS.		NOTED OTHERWISE. THE CONTRACTOR SHALL PROVIDE DOCUMENTATION THAT ALL MATERIALS CONFORM TO THE		
) WITH	14.	QUALITY STANDARDS SPECIFIED IN THE APPLICABLE BUILDING CODE.		
d. Ll be shop		CONCRETE WORK. THE OWNER / CONTRACTOR WILL HIRE THE SPECIAL INSPECTOR TO PERFORM ALL REQUIRED SPECIAL INSPECTIONS.		
g (see		ALTERNATING LANE PATTERN. THE MAXIMUM LENGTH OF SLAB CAST IN ANY ONE CONTINUOUS POUR SHALL BE LIMITED TO 80 FEET.		
DMPLETED ED DURING 8 BOLTS,		FORM WORK SHALL REMAIN IN PLACE UNTIL CONCRETE HAS OBTAINED AT LEAST 90% OF ITS 28 DAY COMPRESSIVE STRENGTH. THE CONTRACTOR SHALL PROVIDE ALL SHORING AND RE-SHORING.		
rocess is		PROVIDE CONSTRUCTION JOINTS IN ALL CONCRETE WORK AS REQUIRED BY THE ACI CODE OR AS SHOWN ON THE INDIVIDUAL DETAILS.		
VELDING	18.	ALL STEEL EXPOSED TO WATER, MOISTURE, AND / OR CORROSIVES SHALL BE COVERED WITH APPROPRIATE PROTECTIVE APPROVED COATING MATERIALS.		
SEATED ATER LEAST ONE	<u>MA</u> 1.	ASONRY NOTES MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF		
L FOR THE E FRAME	2.	THE "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530) PUBLISHED BE THE AMERICAN CONCRETE INSTITUTE.		
df HICKNESS	3.	MINIMUM PRISM COMPRESSIVE STRENGTH (f 'm) SHALL BE 1,550 PSI AT AN AGE OF 28 DAYS, AS DETERMINED BY THE UNIT STRENGTH METHOD OF ACI 530. FILL ALL BOND BEAMS AND REINFORCED CELLS SOLIDLY WITH GROUT. GROUT SHALL CONFORM		
er of ¼" Perimeters	0.	TO ASTM REQUIREMENTS AND SHALL OBTAIN A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2,500 PSI.		
/ITH	4.	REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ASTM REQUIREMENTS, GRADE 60. SHOP FABRICATE REINFORCING BARS WHICH ARE SHOWN TO BE HOOKED OR BENT. PROVIDE A MINIMUM LAP OF 48 TIMES THE BAR DIAMETERS AT ALL SPLICES, UNLESS NOTED OTHERWISE.		
TO BE D BE BUILT. H.D.G. MAY	5.	THE USE OF MASONRY-CEMENT MORTAR IS STRICTLY PROHIBITED. MORTAR SHALL CONFORM TO ASTM REQUIREMENTS: ALL MORTAR SHALL MEET THE "PROPORTION SPECIFICATION" OF ASTM REQUIREMENTS AND BE MADE WITH PORTLAND CEMENT LIME (NON AIR-ENTRAINED).		
	6.	UNLESS NOTED OTHERWISE, ALL WALLS SHALL BE LAID IN RUNNING BOND. BOND CORNERS AND INTERSECTIONS OF LOAD BEARING WALLS.		
E WITH A	7.	VERTICAL REINFORCEMENT OF AT LEAST (1) #4 BAR SHALL BE PROVIDED AT CORNERS, WITHIN 16" OF EACH SIDE OPENINGS, WITHIN 8" OF THE ENDS OF WALLS, AND AT A MAXIMUM SPACING OF 10' ON CENTER. PROVIDE BARS AT ALL WALL CORNERS, INTERSECTIONS, AND OPENING EDGES.		
055. IREMENTS. QUIRED FOR	8.	PROVIDE REBAR DOWELS FROM FOUNDATIONS TO MATCH VERTICAL REINFORCING SIZE AND SPACING. DOWELS SHALL HAVE STANDARD 90 DEGREE HOOKS AND LAP WITH THE FIRST LIFT OF REINFORCING.		
d in any Written	9.	PROVIDE HORIZONTAL BOND WITH CONTINUOUS REINFORCING AS INDICATED. BOND BEAM REINFORCEMENT SHALL BE AT LEAST (1) #4 BAR SPACED NO MORE THAN 10' O.C. DISCONTINUE ALL HORIZONTAL REINFORCING AT CONTROL JOINTS EXCEPT FOR THE BOND BEAMS AT BEARING ELEVATIONS. INTERMEDIATE BOND BEAMS SHALL BE PROVIDED AS REQUIRED.		
AUM OF FIVE DL SHALL BE "ORS, INC."	10.	PROVIDE STANDARD 9 GAUGE HORIZONTAL JOINT REINFORCING AT 16 INCHES ON CENTER IN ALL WALLS. PROVIDE TRUSS TYPE JOINT REINFORCING FOR ALL CONCRETE MASONRY. COORDINATE BRICK TIE-BACK REQUIREMENTS WITH THE ARCHITECTURAL DRAWINGS. UNLESS NOTED OTHERWISE, STOP ALL HORIZONTAL JOINT REINFORCING AT CONTROL JOINTS.		
TINUOUS 2559.	11.	PROVIDE BOND BEAM LINTELS AND BRICK SHELF ANGLES ABOVE ALL WALL OPENINGS PER TYPICAL DETAILS. SEE ARCHITECTURAL DRAWINGS FOR ALL LOCATIONS OF WINDOW AND DOOR OPENINGS.		
m ground he joist written	12.	PROVIDE STEEL JOIST AND BEAM BEARING PLATES AND OTHER ACCESSORIES AS INDICATED. PROVIDE THREE COURSES OF SOLIDLY GROUTED CMU BELOW ALL BEAM BEARINGS OVER THE WIDTH OF 2'-8", CENTERED ON THE WALL, UNLESS NOTED OTHERWISE.		
. EACH ND OVIDE IAILED.	13.	PROVIDE CMU CONTROL JOINTS AS INDICATED, WITH ADDITIONAL JOINTS SO THAT THE SPACING BETWEEN JOINTS DOES NOT EXCEED A SPACING OF 3 x THE WALL HEIGHT (35 FEET MAXIMUM). WHERE BEAMS OR LINTELS BEAR AT CMU CONTROL JOIST OFFSET AND LAP THE VERTICAL		
es, Acturer's	14.	REINFORCING AS INDICATED. THE MASONRY CONTRACTOR SHALL PROVIDE ALL REQUIRED TEMPORARY WALL BRACING DURING CONSTRUCTION (SEE CENERAL STRUCTURAL NOTES)		
ED FOR	15.	DURING CONSTRUCTION (SEE GENERAL STRUCTURAL NOTES). PROVIDE CONSTRUCTION JOINTS IN ALL MASONRY WORK AS REQUIRED BY THE ACI CODE OR AS		
D	16.	SHOWN ON THE INDIVIDUAL DETAILS. ALL STEEL EXPOSED TO WATER, MOISTURE, AND / OR CORROSIVES SHALL BE COVERED WITH		
	17	APPROPRIATE PROTECTIVE APPROVED COATING MATERIALS.		

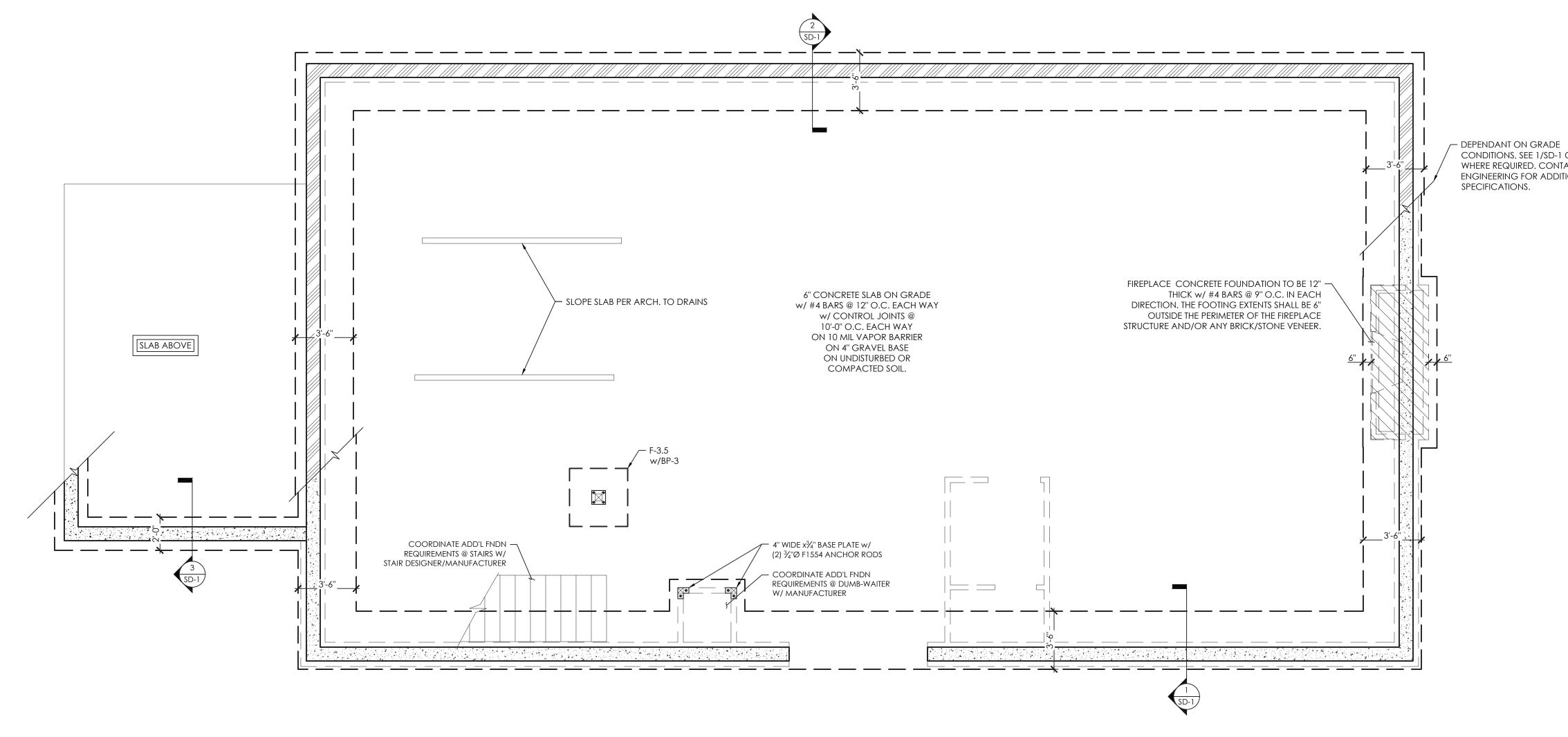
17. INSTALL VERTICAL MOVEMENT JOINT TO ISOLATE VENEER SUPPORTED BY WOOD FROM VENEER SUPPORTED BY VENEER SUPPORTED BY STEEL, MASONRY, OR CONCRETE WHERE APPLICABLE.

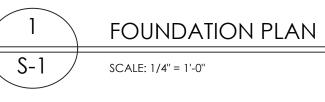
	ENER SCHEDULE FOR STRUCT	TABLE 2304.9.1 URAL MEMBERS (FOR USE ONL' HERWISE SPECIFIED WITHIN PLA	
1. JOIST TO S	CONNECTION SILL OR GIRDER	FASTENING ^{a,m} 3 - 8d COMMON (2½" x 0.131") 3 - 3" x 0.131" NAILS	LOCATION TOE NAIL
2. BRIDGING	G TO JOIST	3 - 3" 14 GAGE STAPLES 2 - 8d COMMON (2½" x 0.131") 2 - 3" x 0.131" NAILS 2 - 3" 14 GAGE STAPLES	TOE NAIL EACH END
	FLOOR OR LESS TO EACH JOIST	2 - 8d COMMON (2½" x 0.131")	FACE NAIL
	AN 1"x6" SUBFLOOR TO EACH JOIST	3 - 8d COMMON (2½" x 0.131") 2 - 16d COMMON (3½" x 0.162")	FACE NAIL BLIND & FACE NAIL
6. SOLE PLAT	te to joist or blocking	16d (3½" x 0.135") @ 16" O.C. 3" x 0.131" NAIL @ 8" O.C. 3" 14 GAGE STAPLES @ 12" O.C.	TYPICAL FACE NAIL
	TE TO JOIST OR BLOCKING AT WALL PANEL	3 - 16d (3½" x 0.135") @ 16" O.C. 4 - 3" x 0.131" NAILS @ 16" O.C. 4 - 3" 14 GAGE STAPLES @ 16" O.C.	BRACED WALL PANELS
7. TOP PLATE	e to stud	2 - 16d COMMON (2½" x 0.131") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	END NAIL
8. STUD TO S	OLE PLATE	4 - 8d COMMON (2½" x 0.131") 4 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES 2 - 16d COMMON (3½" x 0.162")	TOE NAIL
		3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES 16d (3½" x 0.135") @ 24" O.C.	END NAIL
9. DOUBLE S		3" x 0.131" NAIL @ 8" O.C. 3" 14 GAGE STAPLE @ 8" O.C. 16d (3½" x 0.135") @ 16" O.C.	FACE NAIL
		3" x 0.131" NAIL @ 12" O.C. 3" 14 GAGE STAPLE @ 12" O.C. 8 - 16d COMMON (3½" x 0.162")	TYPICAL FACE NAIL
	TOP PLATES	12 - 3" x 0.131" NAILS 12 - 3" 14 GAGE STAPLES 3 - 8d COMMON (2½" x 0.131")	LAP SPLICE
11. BLOCKIN PLATE	IG BETWEEN JOISTS OR RAFTERS TO TOP	3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOE NAIL
12. RIM JOIS	T TO TOP PLATE	8d (2½" x 0.131") @ 6" O.C. 3" x 0.131" NAIL @ 6" O.C. 3" 14 GAGE STAPLE @ 6" O.C.	TOE NAIL
13. TOP PLA	tes, laps and intersections	2 - 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
	JOUS HEADER, TWO PIECES	16d COMMON (3½" × 0.162") 3 - 8d COMMON (2½" × 0.131") 5 - 3" × 0.131" NAILS	16" O.C. ALONG EDGE
	JOUS HEADER TO STUD	5 - 3" 14 GAGE STAPLES 4 - 8d COMMON (2½" x 0.131")	TOE NAIL
	G JOISTS, LAPS OVER PARTITIONS 2308.10.4.1, TABLE 2308.10.4.1)	3 - 16d COMMON (3½" x 0.162") MIN. TABLE 2308.10.4.1 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
	G JOISTS TO PARALLEL RAFTERS 2308.10.4.1, TABLE 2308.10.4.1)	3 - 16d COMMON (3½" x 0.162") MIN. TABLE 2308.10.4.1 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
	TO PLATE 2308.10.1, TABLE 2308.10.1)	3 - 8d COMMON (2½" x 0.131") MIN. 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOE NAIL
20. 1" DIAGC	DNAL BRACE TO EACH STUD & PLATE	2 - 8d COMMON (2½" x 0.131") 2 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
	ATHING TO EACH BEARING HAN 1"x8" SHEATHING TO EACH BEARING	3 - 8d COMMON (2½" x 0.131") 3 - 8d COMMON (2½" x 0.131")	FACE NAIL FACE NAIL
23. BUILT-UP	CORNER STUDS	16d COMMON (3½" x 0.162") 3" x 0.131" NAILS 3" 14 GAGE STAPLES	24" O.C. 16" O.C. 16" O.C.
24. BUILT-UP	GIRDER AND BEAMS	20d COMMON (4" x 0.192") @ 32" O.C. 3" x 0.131" NAIL @ 24" O.C. 3" 14 GAGE STAPLE @ 24" O.C.	FACE NAIL AT TOP & BOTTOM, STAGGERED ON OPPOSITE SIDES
		2 - 20d COMMON (4" x 0.192") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL AT ENDS AND AT EACH SPLICE
25. 2" PLANK	TIE TO RAFTER	16d COMMON (3½" × 0.162") 3 - 10d COMMON (3" × 0.148") 4 - 3" × 0.131" NAILS	AT EACH BEARING
20. 0011/		4 - 3" 14 GAGE STAPLES 3 - 10d COMMON (3" x 0.148") 4 - 3" x 0.131" NAILS	TOE NAIL
27. JACK RA	FTER TO HIP	4 - 3" 14 GAGE STAPLES 2 - 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS	FACE NAIL
		3 - 3" 14 GAGE STAPLES 2 - 16d COMMON (3½" × 0.162") 3 - 3" × 0.131" NAILS	TOE NAIL
28. ROOF RA	AFTER TO 2-BY RIDGE BEAM	3 - 3" 14 GAGE STAPLES 2 - 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS	FACE NAIL
29. JOIST TO	BAND JOIST	3 - 3" 14 GAGE STAPLES 3 - 16d COMMON (3½" x 0.162") 4 - 3" x 0.131" NAILS 4 - 2" 14 CACE STAPLES	FACE NAIL
30. LEDGER	STRIP	4 - 3" 14 GAGE STAPLES 3 - 16d COMMON (3½" × 0.162") 4 - 3" × 0.131" NAILS	FACE NAIL AT EACH JOIST
PARTICLE	STRUCTURAL PANELS AND EBOARD ^D SUBFLOOR, ROOF & WALL NG (TO FRAMING)	4 - 3" 14 GAGE STAPLES 2" AND LESS 6d ^{C, 1} 2%" x 0.113" NAIL ⁿ 1%" 16 GAGE ^o	19061
	. ,	¹ % ₃₂ " TO ³ ⁄4" 8d ^d OR 6d ^e 2%" x 0.113" NAIL ^p 2" 16 GAGE ^p	
		7∕8" ΤΟ 1" 8d ^C 1∕4" ΤΟ 1∕4" 10d ^d OR 8d ^d	
	LOOR (COMBINATION SUBFLOOR - AYMENT TO FRAMING)	¾" AND LESS 6d ^{.e} ⅔" TO 1" 8d ^{.e} 1½" TO 1¼" 10d ^{.d} OR 8d. ^e	
32. PANEL SI	DING (TO FRAMING)	½" OR LESS 6d ^f ⅔" 8d ^f	
33. FIBERBOA	ard sheathing ^g	^h ² No. 11 GAGE ROOFING NAIL ⁶ d COMMON NAIL (2" _i x 0.113")	
		²⁵ / ₃₂ " No. 11 GAGE ROOFING NAIL ^h 8d COMMON NAIL (2½"x 0.113") No. 16 GAGE STAPLE	
34. INTERIOR		¼" 4d J ¾" 6d ^k	
FOR SI: 1	INCH = 25.4 mm.		

- FOR SI: 1 INCH = 25.4 mm. a. COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED. NAILS SPACED AT 6 INCHES ON CENTER AT EDGE, 12 INCHES AT INTERMEDIATE SUPPORTS EXCEPT 6 INCHES AT SUPPORTS WHERE SPANS ARE 48 INCHES OR MORE. FOR NAILING OF WOOD STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO SECTION 2305. NAILS FOR WALL SHEATHING ARE PERMITTED TO BE COMMON, BOX OR CASING.
- COMMON OR DEFORMED SHANK (6d 2" x 0.113"; 8d $2\frac{1}{2}$ " x 0.131"; 10d 3" x 0.148"). COMMON (6d - 2" x 0.113"; 8d - 2¹/₂" x 0.131"; 10d - 3" x 0.148").
- DEFORMED SHANK (6d 2" x 0.113"; 8d 2¹/₂" x 0.131"; 10d 3" x 0.148"). CORROSION-RESISTANT SIDING (6d - 17/8" x 0.106"; 8d - 23/8" x 0.128") OR CASING (6d - 2" x 0.099"; 8d - 22/2" x 0.113") NAIL
- FASTENERS SPACED 3 INCHES ON CENTER AT EXTERIOR EDGES AND 6 INCHES ON CENTER AT INTERMEDIATE SUPPORTS, WHEN USED AS STRUCTURAL SHEATHING. SPACING SHALL BE 6 INCHES ON CENTER ON THE EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS FOR NONSTRUCTURAL APPLICATIONS. Corrosion-resistant roofing nails with γ_6 -inch diameter head and 1½-inch length for γ_2 -inch sheathing
- AND 13/2-INCH LENGTH FOR 25/32-INCH SHATHING Corrosion-resistant staples with nominal γ_6 -inch crown and 1½-inch length for γ_2 -inch sheathing and
- 1½-INCH LENGTH FOR 25/32-INCH SHEATHING. PANEL SUPPORTS AT 16 INCHES (20 INCHES IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, UNLESS OTHERWISE MARKED.)
- CASING ($1\frac{1}{2}$ " x 0.080") OR FINISH ($1\frac{1}{2}$ " x 0.072") NAILS SPACED AT 6 INCHES ON PANEL EDGES, 12 INCHES AT NTERMEDIATE SUPPORTS PANEL SUPPORT AT 24 INCHES. CASING OR FINISH NAILS SPACED AT 6 INCHES ON PANEL EDGES, 12 INCHES AT
- INTERMEDIATE SUPPORTS.
- FOR ROOF SHEATHING APPLICATIONS, 8d NAILS ($2'_2$ " x 0.113") ARE THE MINIMUM REQUIRED FOR WOOD STRUCTURAL STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF \mathcal{V}_{4} -INCH.
- FOR ROOF SHEATHING APPLICATIONS, FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE
- FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS FOR SUBFLOOR AND WALL
- SHEATHING AND 3 INCHES ON CENTER AT EDGES, 6 INCHES AT INTERMEDIATE SUPPORTS FOR ROOF SHEATHING. FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS.



IG EDGE TTOP & GGERED E SIDES h splice AT EACH





FOUNDATION LE	GEND
FOOTING OR FOUNDATION	
15" CONC. FOUNDATION WALL	
12" CONC. FOUNDATION WALL	
STEM WALL	
BASEMENT FRAMED WALL ABOVE	
COLUMN OR STUD PACK ABOVE	\boxtimes
VENEER ABOVE	
MASONRY CHIMNEY/ FIREPLACE BY OTHERS	
BOTTOM OF FOOTING AT LOWER ELEVATION THAN OTHER 12" THICK FOOTINGS	

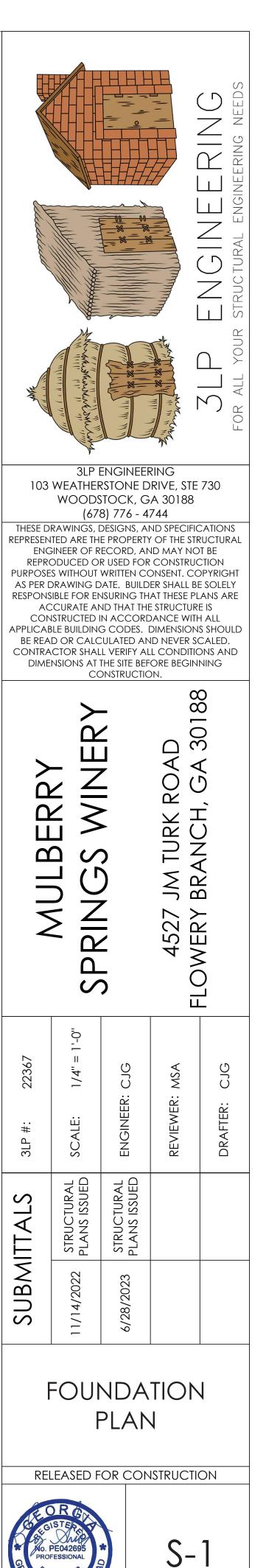
	FOOTING SCHEDULE
F-3.5	3'-6''x3'-6''x12'' DEEP FOOTING W/ #4 BARS @ 9'' O.C. E.W.

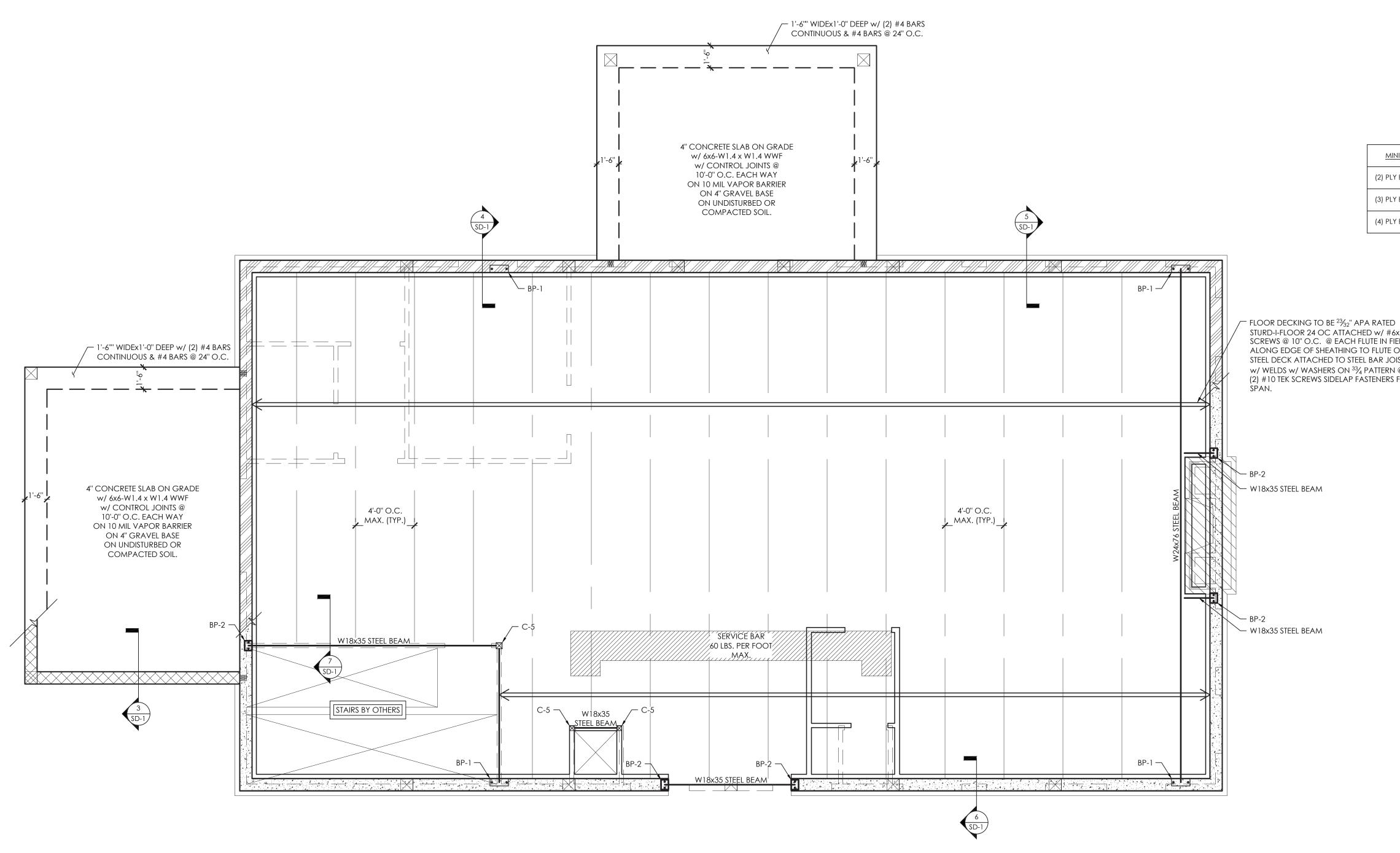
	BASE PLATE SCHEDULE	
BP-1	6"x15"x½" BASE PLATE W/ (2) ½"Ø THREADED RODS THROUGH BOTTOM FLANGE OF STEEL BEAM	
BP-2	4"x8"x½" BASE PLATE W/ (2) ½"Ø THREADED RODS THROUGH BOTTOM FLANGE OF STEEL BEAM	
BP-3	10"x10"x ¾" BASE PLATE w/ (4) ¾"Ø F1554 ANCHOR RODS	

CONDITIONS, SEE 1/SD-1 OR 2/SD-1 WHERE REQUIRED. CONTACT 3LP ENGINEERING FOR ADDITIONAL

FOUNDATION NOTES:

- 1. FOUNDATION DESIGNED BASED ON ASSUMED 2000 PSF ALLOWABLE SOIL
- BEARING CAPACITY. 2. ALL FOUNDATION WALLS TO BE CONTINUOUS FROM FOOTING TO FLOOR
- SYSTEM (UNLESS NOTED OTHERWISE).
 CONTRACTOR TO PROVIDE TEMPORARY SHORING TO BRACE FOUNDATION WALLS WHILE BACK FILLING.
- 4. SOLE / SILL PLATES TO BE ANCHORED TO THE FOUNDATION WITH $\frac{1}{2}$ "Ø ANCHOR BOLTS @ A MAXIMUM OF 6'-0" O.C. MINIMUM (2) BOLTS PER PLATE SECTION AND (1) BOLT WITHIN 12" FROM END OF PLATE SECTION. MINIMUM
- 7" EMBEDMENT INTO MASONRY OR CONCRETE. 5. EXTERIOR GRADES ARE TO BE A MIN. OF 6" BELOW FINISH FLOOR AND
- PROVIDE A 6% SLOPE OF GRADE AWAY FROM BUILDING EXTERIOR. 6. SEE SHEET S-0 FOR ADDITIONAL NOTES.





STRUCTURAL CONNECTORS IN CONTACT WITH PRESSURE TREATED LUMBER OR EXPOSED TO EXTERIOR TO BE PROTECTED FROM WEATHERING ELEMENTS. THEREFORE, Z-MAX COATING, HOT DIPPED GALVANIZED, STAINLESS STEEL MATERIAL OR SIMILAR IS REQUIRED.

STRUCTURAL FRAMING MEMBERS EXPOSED TO EXTERIOR TO BE PROTECTED FROM WEATHERING ELEMENTS.



SCALE: 1/4" = 1'-0"

<u>1ST LEVEL FRAMING</u>	LEGEND
15" CONC. FOUNDATION WALL	
12" CONC. FOUNDATION WALL	
STEM WALL	
BASEMENT FRAMED WALL	
1ST LEVEL WALLS ABOVE	
headers or beams	
BAR JOISTS	
COLUMN OR STUD PACK ABOVE	\boxtimes
FOOTING OR FOUNDATION	
VENEER BELOW	
VENEER ABOVE	
MASONRY CHIMNEY/ FIREPLACE BY OTHERS	
DIRECTION OF STEEL DECK	\iff

	12" CONC. FOUNDATION WALL]
	STEM WALL	
	BASEMENT FRAMED WALL	
	1ST LEVEL WALLS ABOVE	
	HEADERS OR BEAMS	
	BAR JOISTS	
	COLUMN OR STUD PACK ABOVE	
Γ		

FLOOR DES	IGN LOADS
LIVE LOAD	100 PSF
DEAD LOAD	10 PSF

	COLUMN SCHEDULE *
C-1	10x10 SYP #2 P.T. COLUMN
C-2	(2) 2x6 SPF #2 STUD PACK
C-3	(3) 2x6 SPF #2 STUD PACK
C-4	HSS 4"x4"x¼" STEEL COLUMN
C-5	HSS 5"x5"x¼" STEEL COLUMN

* # OF STUDS IN STUD PACKS INDICATE REQUIRED MIN. # OF JACK STUDS (U.N.O.)

BASE PLATE SCHEDULE

	6"x15"x ¹ / ₂ " BASE PLATE W/
BP-1	(2) ½"Ø THREADED RODS THROUGH
	BOTTOM FLANGE OF STEEL BEAM
BP-2	4"x8"x1/2" BASE PLATE W/
	(2) ½"Ø THREADED RODS THROUGH
	BOTTOM FLANGE OF STEEL BEAM
	10"x10"x ¾" BASE PLATE
BP-3	w/ (4) ³ / ₄ "Ø F1554 ANCHOR RODS
1	

MINIMUM PACKED STUD SCHEDULE *			
(2) PLY BEAM	(2) 2x6 SPF #2		
(3) PLY BEAM	(3) 2x6 SPF #2		
(4) PLY BEAM	(4) 2x6 SPF #2		

STURD-I-FLOOR 24 OC ATTACHED w/ #6x11/2" BULGE SCREWS @ 10" O.C. @ EACH FLUTE IN FIELD & 6" O.C. ALONG EDGE OF SHEATHING TO FLUTE ON 1.5C22 STEEL DECK ATTACHED TO STEEL BAR JOISTS & BEAMS w/ WELDS w/ WASHERS ON ³³/₄ PATTERN @ SUPPORTS & (2) #10 TEK SCREWS SIDELAP FASTENERS FOR EACH

1ST FLOOR FRAMING NOTES:

- 1. STEEL BAR JOISTS TO BE 24LH08 (220/200) @ 4'-0" O.C. MAX w/ BRIDGING PER STEEL JOIST INSTITUTE (SJI) RECOMMENDATIONS (UNLESS NOTED OTHERWISE).
- 2. FLOOR DECKING TO BE $^{23}_{32}$ " APA RATED STURD-I-FLOOR 24 OC ATTACHED w/ $\#6x1\frac{1}{2}$ " BULGE SCREWS @ 10" O.C. @ EACH FLUTE IN FIELD & 6" O.C. ALONG EDGE OF SHEATHING TO FLUTE.
- 3. WHERE JOISTS ARE PARALLEL TO EXTERIOR WALLS, PROVIDE FULL DEPTH BRIDGING @ 24" O.C. BETWEEN FIRST (2) BAYS TO BRACE WALL.
- 4. BRIDGING REQUIRED AT ALL SUPPORT, CONCENTRATED LOAD LOCATIONS, AND BELOW WALLS.
- 5. THE ENDS OF ALL BEAMS AND JOISTS ARE TO BE RESTRAINED TO PREVENT ROTATION. ALL FLUSH BEAMS ARE TO BE CONTINUOUSLY BRACED ALONG THE SIDES. ALL DROPPED BEAMS ARE TO BE CONTINUOUSLY BRACED ALONG THE TOP FACE.
- 6. IN FLOOR CAVITIES, PROVIDE BLOCKING UNDER ALL CONCENTRATED LOADS AND AT ALL BEAMS & HEADERS.
- 7. WHERE REQUIRED, PROVIDE ADEQUATE AND PROPER FLASHING AGAINST WATER INTRUSION (TYP.).
- Ζ _____ \mathbf{Y} Ζ _____ \bigcirc \geq 3LP ENGINEERING 103 WEATHERSTONE DRIVE, STE 730 WOODSTOCK, GA 30188 (678) 776 - 4744 THESE DRAWINGS, DESIGNS, AND SPECIFICATIONS REPRESENTED ARE THE PROPERTY OF THE STRUCTURAL ENGINEER OF RECORD, AND MAY NOT BE REPRODUCED OR USED FOR CONSTRUCTION PURPOSES WITHOUT WRITTEN CONSENT. COPYRIGHT AS PER DRAWING DATE. BUILDER SHALL BE SOLELY RESPONSIBLE FOR ENSURING THAT THESE PLANS ARE ACCURATE AND THAT THE STRUCTURE IS CONSTRUCTED IN ACCORDANCE WITH ALL APPLICABLE BUILDING CODES. DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED. CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE BEFORE BEGINNING CONSTRUCTION. ∞ \bigcirc \mathbf{N} ΟĒ 111 O U R N \leq Ř CH, ΞŽ \mathbf{m} $\boldsymbol{\mathcal{O}}$ - ML BRA VUL R N N 4527 WERY Δ FLO \mathbf{S} -|--22367 II Q () 1/4" \odot \cup SCALE: :# ЗLР R E ίΠ \Box SUED TURAL ISSUED **SUBMITTALS** STRUC⁻ PLANS | IRUC ANS PL S 22 3 11/14/ 6 1ST LEVEL FRAMING PLAN

RELEASED FOR CONSTRUCTION



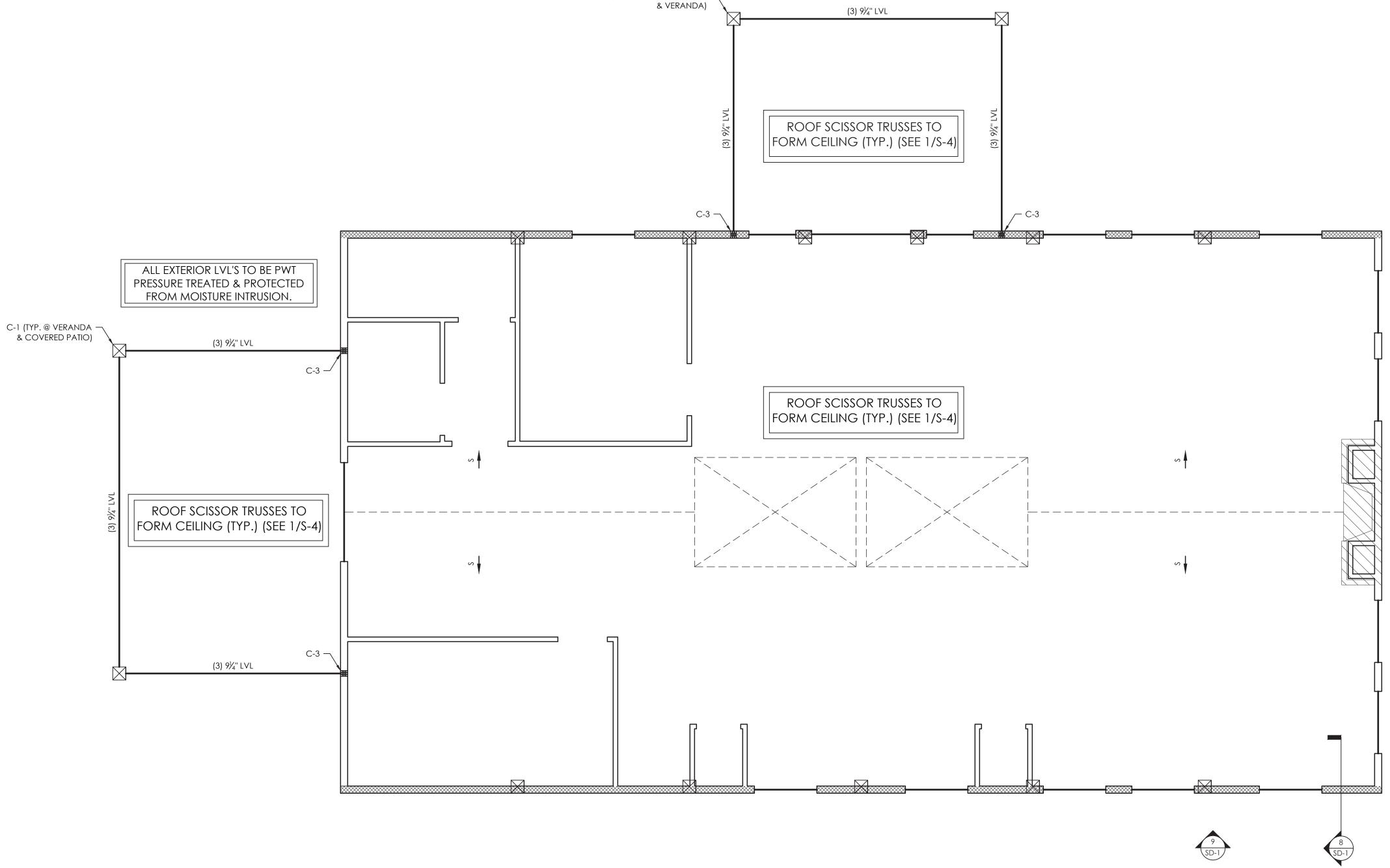
S-2

SIMILAR IS REQUIRED. STRUCTURAL FRAMING MEMBERS EXPOSED TO EXTERIOR TO BE PROTECTED FROM WEATHERING

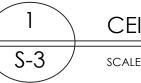
ELEMENTS.

PRESSURE TREATED LUMBER OR EXPOSED TO EXTERIOR TO BE PROTECTED FROM WEATHERING ELEMENTS. THEREFORE, Z-MAX COATING, HOT DIPPED GALVANIZED, STAINLESS STEEL MATERIAL OR

STRUCTURAL CONNECTORS IN CONTACT WITH



C-1 (TYP. @ COVERED PATIO -



CEILING LEVEL FRAMING PLAN

SCALE: 1/4" = 1'-0"

ROOF SCISSOR TRUSS DESIGN LOAD				
TOP CHORD LIVE LOAD	20 PSF			
TOP CHORD DEAD LOAD	10 PSF			
BOTTOM CHORD DEAD LOAD	10 PSF			

CEILING FRAMING LEGEND			
1ST LEVEL WALLS			
LOAD BEARING WALLS FOR TRUSS ATTACHMENT			
headers or beams			
VENEER BELOW			
MASONRY CHIMNEY/ FIREPLACE BY OTHERS			
MAJOR CEILING LINES			
SLOPED CEILING	S		

MINIMUM PACKED STUD SCHEDULE *			
(2) PLY BEAM (2) 2x6 SPF #2			
(3) PLY BEAM	(3) 2x6 SPF #2		
(4) PLY BEAM	(4) 2x6 SPF #2		

* FOR USE WHERE MEMBER SUPPORTS ARE NOT OTHERWISE CALLED OUT ON PLAN

COLUMN SCHEDULE *		
C-1	10x10 SYP #2 P.T. COLUMN	
C-2	(2) 2x6 SPF #2 STUD PACK	
C-3	(3) 2x6 SPF #2 STUD PACK	
C-4	HSS 4"x4'x¼" STEEL COLUMN	
C-5	HSS 5''x5''x¼'' STEEL COLUMN	

* # OF STUDS IN STUD PACKS INDICATE REQUIRED MIN. # OF JACK STUDS (U.N.O.)

NOTE TO ROOF SCISSOR TRUSSES MANUFACTURER:

- <u>CONTACT 3LP ENGINEERING FOR ADD'L LOAD DIAGRAMS OF</u> TRUSSES, GIRDER TRUSSES, & SPECIAL TRUSSES REQ'D. ON A CASE BY CASE BASIS. DIAGRAMS TO BE CREATED @ OWNER EXPENSE W/
- PRIOR OWNER APPROVAL. SHOP DRAWINGS ARE TO BE PROVIDED FOR 3LP ENGINEERING TO
- REVIEW PRIOR TO CONSTRUCTION @ OWNERS EXPENSE. • TRUSS MANUFACTURER IS RESPONSIBLE FOR THE DESIGN AND
- SPECIFICATION OF ALL HANGERS ASSOCIATED WITH TRUSS SUPPORT.

ALL TRUSSES BY OTHERS. TRUSS DESIGNER/MANUFACTURER TO SUBMIT FINAL LAYOUT AND/OR SHOP DRAWINGS TO 3LP ENGINEERING FOR REVIEW PRIOR TO CONSTRUCTION. COORDINATE w/ ARCH. (TYP.)

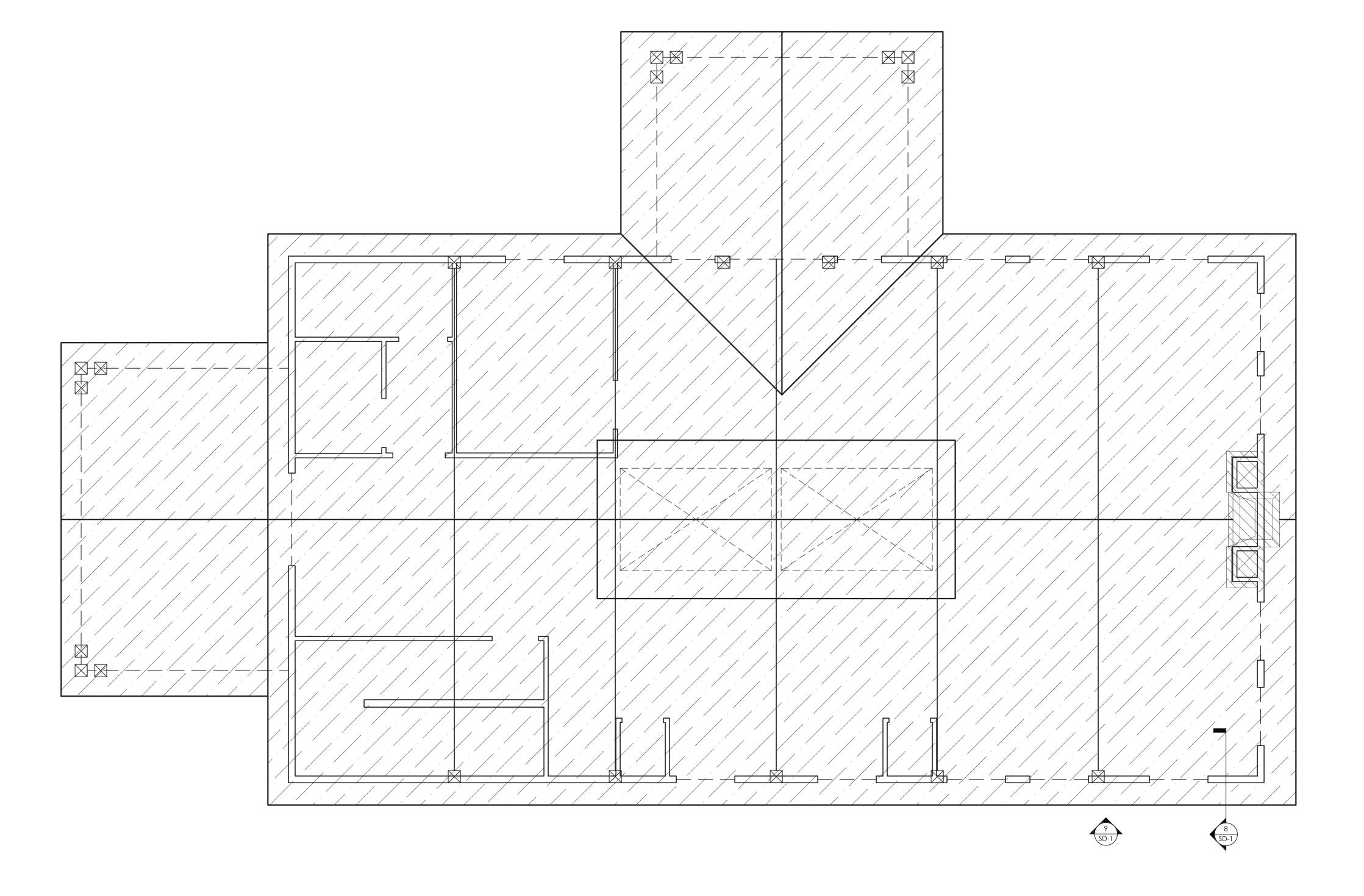
CEILING FRAMING NOTES:

- 1. ALL CEILINGS TO BE FORMED BY ROOF SCISSOR TRUSSES. SEE 1/S-4. 2. THE ENDS OF ALL BEAMS AND JOISTS ARE TO BE RESTRAINED TO PREVENT ROTATION. ALL FLUSH BEAMS ARE TO BE CONTINUOUSLY BRACED ALONG THE SIDES. ALL DROPPED BEAMS ARE TO BE CONTINUOUSLY BRACED
- ALONG THE TOP FACE. 3. USE APPROVED SIMPSON HANGERS W/ MAX. ATTACHMENT ON ALL WOOD BEAM / JOIST CONNECTIONS.
- 4. IN CEILING CAVITIES, PROVIDE BLOCKING UNDER ALL CONCENTRATED
- LOADS AND AT ALL BEAMS & HEADERS. 5. WHERE REQUIRED, PROVIDE ADEQUATE AND PROPER FLASHING AGAINST
- WATER INTRUSION (TYP.). 6. All LVL's REFERENCED ON PLAN ARE TO BE $1\frac{3}{4}$ " PER PLY AND ARE TO HAVE
- MINIMUM 2.0E MODULUS OF ELASTICITY (UNLESS NOTED OTHERWISE) 7. ALL SIDE LOADED LVL'S ARE TO HAVE MAXIMUM RECOMMENDED CONNECTION BETWEEN PLIES PER LVL MANUFACTURER SPECIFIER'S GUIDE.

1ST LEVEL WALL (BELOW CEILING) FRAMING NOTES:

- 1. LOAD BEARING WALLS TO BE 2x6 SPF #2 STUDS @ 16" O.C. W/ 10'-0"
- MAXIMUM STUD HEIGHT (UNLESS NOTED OTHERWISE). 2. WINDOW AND DOOR HEADERS IN LOAD BEARING WALLS TO BE (3) 2x10
- SYP #2 W/ (1) JACK STUDS & (1) KING STUDS ON EACH END (UNLESS NOTED OTHERWISE).
- 3. EXTERIOR WINDOW AND DOOR HEADERS NOT BRACED AGAINST LATERAL LOADING BY ADJACENT FRAMING ARE TO HAVE (1) 2x6 SPF #2 NAILED TO TOP AND BOTTOM OF HEADER ATTACHED TO ADJACENT JACK & DOUBLE KING STUDS USING SIMPSON A34 FRAMING ANGLES. (APPLIES TO HEADER/BEAM LENGTHS \geq 4'-0")
- 4. ALL STUDS TO BE CONTINUOUS BETWEEN DIAPHRAGMS. STUDS IN GABLE-END WALLS NOT BRACED BY A CEILING SYSTEM MUST BE CONTINUOUS FROM FLOOR TO ROOF. 5. ALL LOAD BEARING WALLS TO BE BLOCKED AT 5'-0" O.C. MAX.
- 6. EXTERIOR WALLS TO BE FULLY SHEATHED W/ $\frac{15}{32}$ APA RATED SHEATHING ATTACHED W/ 10d NAILS @ 6" O.C. AT PANEL EDGES & 12" O.C. AT INTERMEDIATE MEMBERS. PROVIDE BLOCKING BETWEEN STUDS AT PANEL EDGES.
- 7. ALL COLUMNS TO BE BRACED AT TOP AND BOTTOM. ALL CONTINUOUS COLUMNS TO BE BRACED AT EACH FLOOR LEVEL.
- 8. USE APPROVED SIMPSON POST BASE & POST CAPS ON ALL WOOD COLUMNS.

THE PARENCE DRIVE, STE 730 WOODSTOCK, GA 30188					
(678) 776 - 4744 THESE DRAWINGS, DESIGNS, AND SPECIFICATIONS REPRESENTED ARE THE PROPERTY OF THE STRUCTURAL ENGINEER OF RECORD, AND MAY NOT BE REPRODUCED OR USED FOR CONSTRUCTION PURPOSES WITHOUT WRITTEN CONSENT. COPYRIGHT AS PER DRAWING DATE. BUILDER SHALL BE SOLELY RESPONSIBLE FOR ENSURING THAT THESE PLANS ARE ACCURATE AND THAT THE STRUCTURE IS CONSTRUCTED IN ACCORDANCE WITH ALL APPLICABLE BUILDING CODES. DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED. CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE BEFORE BEGINNING CONSTRUCTION. RESPONCE WITH ALL APPLICABLE BUILDING CODES. DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED. CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE BEFORE BEGINNING CONSTRUCTION. RESPONCE OF BEGINNING CONSTRUCTION. CONTRUCTION. CONTRUCTION. CONTRUCTION. RESPONCE OF STRUCTURES CONTRUCTION. CONTRUC					
3LP #: 22367	SCALE: 1/4" = 1'-0"	ENGINEER: CJG	REVIEWER: MSA	DRAFTER: CJG	
SUBMITTALS	2022 STRUCTURAL PLANS ISSUED	023 STRUCTURAL			
CEILING LEVEL FRAMING PLAN RELEASED FOR CONSTRUCTION					



ROOF FRAMING PLAN S-4

SCALE: 1/4" = 1'-0"

<u>ROOF SCISSOR TRUSS DESIGN LOAD</u>				
TOP CHORD LIVE LOAD	20 PSF			
TOP CHORD DEAD LOAD	10 PSF			
BOTTOM CHORD DEAD LOAD	10 PSF			

EGEND	ROOF FRAMING L
	ROOF LINES
	1ST LEVEL WALLS
	HEADERS OR BEAMS BELOW
	ROOF SCISSOR TRUSSES
	(BY OTHERS)
	ROOF TO BE FORMED BY SCISSOR TRUSSES (BY OTHERS)
	VENEER BELOW
	MASONRY CHIMNEY/ FIREPLACE BY OTHERS

NOTE TO ROOF SCISSOR TRUSSES MANUFACTURER:

- <u>CONTACT 3LP ENGINEERING FOR ADD'L LOAD DIAGRAMS OF</u> <u>TRUSSES, GIRDER TRUSSES, & SPECIAL TRUSSES REQ'D. ON A CASE BY</u> <u>CASE BASIS. DIAGRAMS TO BE CREATED @ OWNER EXPENSE W/</u> <u>PRIOR OWNER APPROVAL.</u>
 SHOP DRAWINGS ARE TO BE PROVIDED FOR 3LP ENGINEERING TO <u>DRAWINGS ARE TO BE PROVIDED FOR 3LP ENGINEERING TO</u>
- REVIEW PRIOR TO CONSTRUCTION @ OWNERS EXPENSE.
- TRUSS MANUFACTURER IS RESPONSIBLE FOR THE DESIGN AND SPECIFICATION OF ALL HANGERS ASSOCIATED WITH TRUSS SUPPORT.

ALL TRUSSES BY OTHERS. TRUSS DESIGNER/MANUFACTURER TO SUBMIT FINAL LAYOUT AND/OR SHOP DRAWINGS TO 3LP ENGINEERING FOR REVIEW PRIOR TO CONSTRUCTION. COORDINATE w/ ARCH. (TYP.)

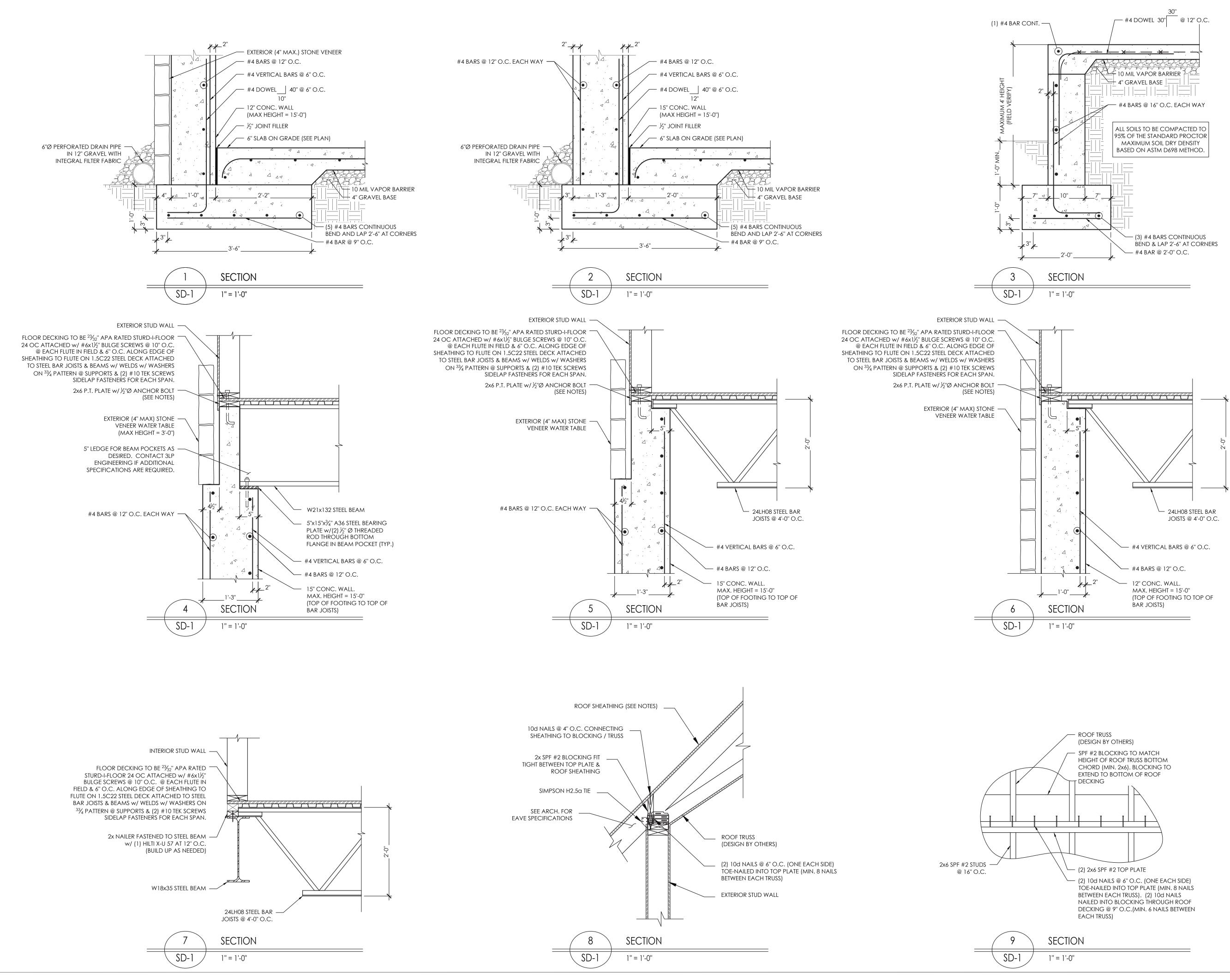
COORD. W/ MEP FOR MECHANICAL ALLOWANCES IN TRUSS PROFILE

ROOF SCISSOR TRUSSES NOTES:

- 1. ALL ROOF SCISSOR TRUSSES BY OTHERS (COORD. DEPTH, SLOPE OF TOP
- CHORDS AND OPENING W/ ARCH. & MEP) 2. ALL PRE-ENGINEERED TRUSSES SHALL BE DESIGNED BY A PROFESSIONAL
- ENGINEER REGISTERED IN THE STATE OF GEORGIA. 3. TRUSS DESIGNER TO COORDINATE WITH ARCHITECT TO MEET DESIRED
- DEFLECTION CRITERIA.
- 4. STUDS IN BEARING WALLS ARE TO BE PLACED DIRECTLY BELOW TRUSSES AND SHOULD BE CONTINUOUS BETWEEN DIAPHRAGMS TO FOUNDATIONS. 5. ALL BOLTS, HANGERS, STRAPS, SHEATHING, ETC. REQUIRED FOR
- CONNECTIONS BETWEEN PRE-ENGINEERED TRUSSES SHALL BE DESIGNED AND SPECIFIED BY TRUSS DESIGN ENGINEER.
- 6. PRE-ENGINEERED METAL PLATE CONNECTED WOOD TRUSSES SHALL BE BRACED IN ACCORDANCE WITH BSCI 1-08 AND RELATED SUMMARY SHEETS. 7. ALL PRE-ENGINEERED TRUSS SHOP DRAWINGS SHALL BE AVAILABLE ON THE JOB SITE DURING THE TIMES OF INSPECTION AND SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED AND APPROVED BY THE
- PROJECT STRUCTURAL ENGINEER OF RECORD. 8. TRUSS MANUFACTURER TO PROVIDE APPROPRIATE OUT-OF-PLANE LOAD BRACING FOR ALL TRUSSES.
- 9. ROOF DECKING TO BE $\frac{5}{3}$ " APA RATED $\frac{24}{6}$ Sheathing attached w/ 10d NAILS @ 4" O.C. AT SUPPORTED EDGES & 12" O.C. AT INTERMEDIATE MEMBERS.
- 10. WHERE TRUSSES ARE PARALLEL TO EXTERIOR WALLS, PROVIDE FULL DEPTH BLOCKING @ 16" O.C. BETWEEN FIRST (2) BAYS TO BRACE WALL.
- 11. REFER TO 2018 INTERNATIONAL BUILDING CODE (IBC) TABLE 1607.1 "MINIMUM UNIFORMLY DISTRIBUTED LOADS AND MINIMUM CONCENTRATED loads". 12. ONLY BRACE TRUSSES ON FRAMING MEMBERS & LOAD BEARING WALLS
- shown.
- 13. ALL STUDS TO BE CONTINUOUS BETWEEN DIAPHRAGMS. 14. ALL COLUMNS TO BE BRACED AT TOP AND BOTTOM. ALL CONTINUOUS
- COLUMNS TO BE BRACED AT EACH FLOOR LEVEL. 15. USE APPROVED SIMPSON POST BASE & POST CAPS ON ALL WOOD COLUMNS.
- 16. WHERE REQUIRED, PROVIDE ADEQUATE AND PROPER FLASHING AGAINST WATER INTRUSION (TYP.).

The structural engineering needs						
3LP ENGINEERING 103 WEATHERSTONE DRIVE, STE 730 WOODSTOCK, GA 30188 (678) 776 - 4744 THESE DRAWINGS, DESIGNS, AND SPECIFICATIONS REPRESENTED ARE THE PROPERTY OF THE STRUCTURAL ENGINEER OF RECORD, AND MAY NOT BE REPRODUCED OR USED FOR CONSTRUCTION PURPOSES WITHOUT WRITTEN CONSENT. COPYRIGHT AS PER DRAWING DATE. BUILDER SHALL BE SOLELY RESPONSIBLE FOR ENSURING THAT THESE PLANS ARE ACCURATE AND THAT THE STRUCTURE IS CONSTRUCTED IN ACCORDANCE WITH ALL APPLICABLE BUILDING CODES. DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED. CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE BEFORE BEGINNING						
SPRINGS WINERY			4527 JM TURK ROAD	BRANCI		
3LP #: 22367	SCALE: 1/4" = 1'-0"	ENGINEER: CJG	REVIEWER: MSA		DRAFTER: CJG	
SUBMITTALS	11/14/2022 STRUCTURAL PLANS ISSUED	6/28/2023 STRUCTURAL				
ROOF FRAMING PLAN RELEASED FOR CONSTRUCTION						
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Ζ _____ \mathbf{Y} \angle _____ \bigcirc \geq 3LP ENGINEERING 103 WEATHERSTONE DRIVE, STE 730 WOODSTOCK, GA 30188 (678) 776 - 4744 THESE DRAWINGS, DESIGNS, AND SPECIFICATIONS REPRESENTED ARE THE PROPERTY OF THE STRUCTURAL ENGINEER OF RECORD, AND MAY NOT BE REPRODUCED OR USED FOR CONSTRUCTION PURPOSES WITHOUT WRITTEN CONSENT. COPYRIGHT AS PER DRAWING DATE. BUILDER SHALL BE SOLELY RESPONSIBLE FOR ENSURING THAT THESE PLANS ARE ACCURATE AND THAT THE STRUCTURE IS CONSTRUCTED IN ACCORDANCE WITH ALL APPLICABLE BUILDING CODES. DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED. CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE BEFORE BEGINNING CONSTRUCTION. Ω \bigcirc $\mathbf{\mathcal{L}}$ $\Box \omega$ Ζ ОU Ŕ \sim \sim > CH 11 \Box Z \mathbf{m} ()JUL JM BR/ R N N \succ 4527 WER Δ FLO $\boldsymbol{\mathcal{S}}$ -0 _ 22367 11 Q () 1/4" \odot \odot # 3LP SC/ ВШ \square STURAL ISSUED TURAL ISSUED **SUBMITTALS** STRUC' PLANS 'RUC ANS г Г 22 4 1/11 STRUCTURAL DETAILS RELEASED FOR CONSTRUCTION SD-