LUMBER NOTES:

1. MEMBER GRADES SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:
   - GLU-LAM BEAMS . . . . 24F-V4 DF/DF
   - JOISTS . . . . DOUGLAS-FIR/LARCH #2
   - HEADERS . . . . DOUGLAS-FIR/LARCH #2
   - COLUMNS . . . . DOUGLAS-FIR/LARCH #2
   - STUDS NONBEARING WALLS . . . . DOUGLAS-FIR/LARCH #2
   - PRE-FAB JOISTS . . . . AS PER MANUFACTURER
   - SILL PLATES IN CONTACT WITH CONCRETE . . . . DOUGLAS-FIR/LARCH #2 TREATED FOR MOISTURE PROTECTION
2. WHERE NOT NOTED OTHERWISE, CONNECT ALL WOOD TO CONCRETE, WOOD TO STEEL AND WOOD TO WOOD (EXCEPT STUD TO PLATE) WITH SIMPSON STRONG-TIE OR EQUAL STRUCTURAL CONNECTORS. ANY OTHER SUBSTITUTION MUST BE APPROVED BY THE ENGINEER.
3. WHERE MULTIPLE SILL PLATES ARE USED, ANCHOR BOLTS SHALL EXTEND THROUGH ALL SILL PLATES.
4. BLOCK ALL HORIZONTAL EDGES OF PLYWOOD WALL SHEATHING WITH 2" NOMINAL BLOCKING. BLOCK EDGES OF PLYWOOD ON FLOORS AND ROOF AS DIRECTED ON DRAWINGS.
5. SOLID 2" NOMINAL BLOCKING SHALL BE PROVIDED AT ENDS OR POINTS OF SUPPORT OF ALL WOOD JOISTS. CROSS BRIDGING OF NOT LESS THAN 1"x3" MATERIAL SHALL BE PLACED IN ROWS BETWEEN SUPPORT POINTS NOT TO EXCEED 8'-0" APART, FOR SPANS OF 18'-0" AND GREATER.
6. ALL LEDGER BOLTS SHALL HAVE PLATE WASHERS WITH A MIN. DIA. EQUAL TO 3 TIMES THE BOLT DIA. UNLESS SHOWN OTHERWISE IN DETAILS.
7. MIN. NAILING SHALL BE AS PER SECTION 2304.10 OF THE INTERNATIONAL BUILDING CODE.
8. FASTENERS SUCH AS STAPLES, CAN ONLY BE SUBSTITUTED FOR NAILS AT A RATE EQUAL TO LOAD VALUES PROVIDED BY I.C.B.O. APPROVAL. SEE ATTACHED SCHEDULE.
9. JOISTS SHALL HAVE BRIDGING, BLOCKING AND NOTCHED BEARING PL AS RECOMMENDED BY THE MANUFACTURER WITH A MIN. OF ONE ROW OF BRACING AT MID SPAN MANUFACTURER SHALL SUPPLY AND CONTRACTOR SHALL INSTALL.
10. ALL PRE-MANUFACTURED WOOD PRODUCTS SHALL BE PROVIDED BY TRUSS JOIST, BOISE CASCADE CORP, OR LOUISIANA PACIFIC CORP. ANY OTHER SUBSTITUTION MUST BE APPROVED BY THE ENGINEER.
11. FASTENERS FOR PRESSURE PRESERVATIVE WOOD SHALL BE HOT-DIPPED, GALVANIZED STEEL OR STAINLESS STEEL.
12. BEAM SIZES ARE BASED ON A MIN. STRENGTH REQUIREMENTS. SIZES MAY BE INCREASED FOR ARCHITECTURAL OR CONSTRUCTION PURPOSES.
13. TYPICAL DOOR/WINDOW HEADERS TO BE (2) 2X8 UNLESS NOTED OTHERWISE.
14. 2-PLY AND 3-PLY PRE-ENGINEERED WOOD BEAMS SHALL BE NAILED TOGETHER AS PER MANUFACTURER'S SPECIFICATIONS. 4-PLY AND GREATER PRE-ENGINEERED WOOD BEAMS SHALL BE ATTACHED W/ (2) ROWS 1/2"Ø THRU-BOLTS @ 12" o.c., SPACED 2" FROM TOP AND BOTTOM OF BEAM. SEE MANUFACTURES SPECIFICATIONS FOR ALL OTHER CONNECTION CONDITIONS.
15. SOLID BLOCKING OR SQUASH BLOCKS REQUIRED IN JOIST SPACE AT ALL COLUMN LOCATIONS. CARRY ALL COLUMN LOADS DOWN TO FTG. OR FDN.
16. ROOF SHEATHING SHALL BE 15/32" APA RATED SHEATHING W/SPAN RATING OF 32/16. LAY SHEATHING WITH FACE GRAIN AT RIGHT ANGLES TO FRAMING WITH END JOINTS STAGGERED.
17. FLOOR SHEATHING SHALL BE 3/4" T&G WAFER BOARD GLUED & NAILED. GLUE SHALL CONFORM TO AFG-01 ACCORDING TO APA SPECIFICATIONS.
18. WALL SHEATHING SHALL BE 7/16" APA RATED SHEATHING. SEE SHEAR WALL SCHEDULE FOR MORE INFORMATION.
19. UNLESS NOTED OTHERWISE, 8d NAILS SHALL BE USED TO FASTEN ALL ROOF AND WALL SHEATHING, AND 10d NAILS SHALL BE USED TO FASTEN ALL FLOOR SHEATHING TO SUPPORTING FRAMING AS FOLLOWS.
   - A. BOUNDARY NAILING "BN": 4" O.C. AT ALL ROOF AND FLOOR SHEATHING INTO BEARING AND/OR SHEAR WALLS, TOP AND BOTTOM OF WALLS.
   - B. PANEL EDGE NAILING "EN": 6" O.C. AT ALL OTHER PLYWOOD PANEL EDGES.
   - C. PANEL FIELD NAILING "FN": 12" O.C. AT INTERIOR SUPPORTS IN FIELD OF PANEL.
20. BLOCK JOISTS, RAFTERS AND/OR TRUSSES SOLID AT ALL BEARING POINTS.
21. PROVIDE (2) 2x STUD COLUMN AT ALL BEAMS, HEADERS, AND GIRDER TRUSS BEARING LOCATIONS TYPICAL UNLESS NOTED OTHERWISE.
22. ALL BOLTS THRU WOOD SHALL BE ASTM A307 AND SHALL HAVE HARDENED WASHERS UNDER ASTM A563 HEAVY HEX NUTS AND BOLT HEADS.
23. UNLESS NOTED OTHERWISE, ALL WALL BOTTOM PLATES TO BE ANCHORED TO FOUNDATIONS OR FOOTINGS WITH 5/8" DIAMETER ANCHOR BOLTS AT 32" O.C. WITH 8" MIN. EMBEDMENT. WALL BOTTOM PLATES AT SHEAR WALLS SHALL INCLUDE 3"x3"x1/4" STEEL PLATE WASHERS. PROVIDE A ROUND CUT WASHER BETWEEN THE NUT OF THE ANCHOR BOLT AND THE PLATE WASHER.
24. UNLESS OTHERWISE NOTED, ALL BEARING WALL STUDS SHALL BE 2X6 SPACED AT 16" O.C. BLOCK ALL NON-SHEATHED BEARING WALLS AT 4'-0" O.C.
25. EXTERIOR WALLS SHALL HAVE DOUBLE 2x TOP PLATES SPLICED WITH A MIN. OF 48" OF OVERLAP AND SHALL BE CONNECTED WITH A MIN. OF (12) 16d NAILS.

ROOF TRUSS NOTES:

1. ROOF IS TO BE CONSTRUCTED OF A PRE-MANUFACTURED TRUSS SYSTEM DESIGNED BY TRUSS MANUFACTURER.
2. DESIGN TRUSSES TO LIMIT DEFLECTION TO SPAN (IN.) DIVIDED BY 240.
3. CHECK DIMENSIONS WITH ARCH. DRAWINGS. TRUSS MANUFACTURER IS RESPONSIBLE TO PROVIDE WEB AND CHORD MEMBERS TO SATISFY LOAD REQUIREMENTS.
4. SEE ARCHITECTURAL DRAWINGS FOR VAULTS, TRAY CEILINGS, CEILING HEIGHTS, ETC.
5. GIRDER TO GIRDER CONNECTIONS PER TRUSS MANUFACTURER.
6. TRUSS LAYOUT SHALL FOLLOW THE STRUCTURAL PLANS, OR TRUSS SHOP DRAWINGS NEED TO BE SUBMITTED TO REEVE AND ASSOCIATES FOR REVIEW.

REINFORCING STEEL NOTES:

1. ALL REINFORCING BARS SHALL CONFORM TO ASTM STANDARD A-615 GRADE 60. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM STANDARD A-185, SHALL BE SUPPLIED IN FLAT SHEETS AND SHALL HAVE A MIN. SIDE LAP OF 8 INCHES. ADEQUATELY TIE AND SUPPORT ALL REINFORCING STEEL AS SPECIFIED BY ACI 315 TO MAINTAIN EXACT REQUIRED POSITION. ALL FIELD BENT DOWELS SHALL BE GRADE 40 WITH SPACING INDICATED REDUCED BY 1/3.
2. REINFORCEMENT SHALL HAVE THE FOLLOWING CONCRETE COVERAGE:
   - A. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH. . . . . 3"
   - B. EXPOSED TO EARTH OR WEATHER:
     - #6 & LARGER. . . . . .2"
     - #5 & SMALLER. . . . . 1 1/2"
   - C. NOT EXPOSED TO WEATHER OR EARTH:
     - SLABS, WALLS, JOISTS, #11 & SMALLER . . . . 3/4"
     - BEAMS, COLUMNS: MAIN REINFORCING OR TIES . . . . 1 1/2"
   - D. SLAB ON GRADE:
     - PLACE REINFORCING AT CENTER OF SLAB UNLESS INDICATED OTHERWISE.
3. EXCEPT WHERE NOTED, CONTINUOUS REINFORCEMENT SHALL BE SPLICED AT POINTS OF MIN. STRESS
4. ALL VERTICAL REINFORCING SHALL BE DOWELED TO FOOTINGS OR STRUCTURE BELOW WITH DOWELS TO MATCH. SPLICE LENGTHS SHALL COMPLY WITH NOTE 3. DOWELS INTO FOOTINGS SHALL TERMINATE WITH A STANDARD HOOK, AND SHALL EXTEND TO WITHIN 4" OF THE BOTTOM OF THE FOOTING, BUT NOT MORE THAN 20" INTO FOOTING.
5. DO NOT WELD REINFORCING.
6. LAP LENGTHS IN CONCRETE SHALL BE AS FOLLOWS:

#3 = 22"	#6 = 43"
#4 = 29"	#7 = 63"
#5 = 36"	#8 = 72"

7. DO NOT SPLICE STIRRUPS AND TIES.
8. DO NOT SPLICE VERTICAL BARS IN RETAINING WALLS UNLESS SPECIFICALLY SHOWN.
9. PROVIDE CORNER BARS AT INTERSECTING WALL CORNERS USING THE SAME BAR SIZE AND SPACING AS THE HORIZONTAL WALL REINFORCEMENT.
10. HORIZONTAL WALL REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONSTRUCTION AND CONTROL JOINTS.

CONCRETE MASONRY NOTES:

1. CONCRETE MASONRY WALLS SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF f'm = 1500 p.s.i.
2. CONCRETE MASONRY UNITS SHALL BE LIGHT WEIGHT UNITS CONFORMING TO ASTM C90, GRADE N-1.
3. MORTAR SHALL BE TYPE "M" BELOW GRADE, TYPE "S" ABOVE GRADE CONFORMING TO ASTM C270. 1800psi MIN. COMPRESSIVE STRENGTH.
4. WHERE NO FOOTING OR FOUNDATION WALL IS SHOWN UNDER A MASONRY WALL, FLOORS UNDER SUCH WALL SHALL INCLUDE A THICKENED SLAB.
5. REINFORCED CONCRETE BLOCK WALLS
   - A. SEE PLANS FOR LOCATION OF REINFORCED WALLS.
   - B. WHEN PLACING ONE BAR IN A SINGLE CORE, PLACE IN CENTER UNLESS NOTED OTHERWISE.
   - C. WHEN PLACING TWO BARS IN A SINGLE CORE, PLACE BARS NEAR EACH FACE UNLESS NOTED OTHERWISE.
   - D. FILL BLOCK CORE AT VERTICAL STEEL WITH 2000 p.s.i. GROUT, RODDED OR VIBRATED IN PLACE. DO NOT USE PERMANENT REINFORCING STEEL TO CONSOLIDATE CONCRETE.
   - E. PLACEMENT OF CORE GROUT SHALL HAVE A MAXIMUM HEIGHT OF 4'-0" UNLESS CLEAN OUT HOLES ARE PROVIDED AT THE BOTTOM OF EACH GROUT LIFT, THEN A MAXIMUM HEIGHT OF 8'-0" BEFORE PLACEMENT OF CORE GROUT.
   - F. VERTICAL STEEL SHALL BE CONTINUOUS WITH REQD LAP AT SPLICES UNLESS NOTED OTHERWISE. SEE MASONRY SCHEDULE FOR LAP LENGTH.
6. GROUT STOP SHALL BE RESIN COATED FIBERGLASS MESH 1-1/2" NARROWER THAN WALL THICKNESS. OTHER GROUT STOP MATERIALS ARE NOT PERMITTED.
7. WRAP ALL OPENINGS WITH (2) #5 REBAR EXTENDING PAST THE EDGE OF OPENING BY 24" MIN. EACH DIRECTION.
8. ALL OPENINGS GREATER THAN 6'-0" SHALL HAVE MASONRY COLUMN ON EACH SIDE TO BE 8" X 16" W/ (4) #5 BARS VERTICAL TIED W/ #3 TIES @ 8" O.C. (UNLESS NOTED OTHERWISE).
9. UNLESS OTHERWISE NOTED MINIMUM REINFORCING IN ALL 8" MASONRY WALLS SHALL BE AS FOLLOWS:
   - A. VERTICAL: #5 BARS IN CELLS ADJACENT TO ALL OPENINGS, AT CORNERS AND AT A MAXIMUM SPACING OF 32" THROUGHOUT WALL. ALL VERTICAL REINFORCEMENT SHALL BE DOWELED TO FOUNDATION.
   - B. HORIZONTAL: (2) #4 BARS IN 8" DEEP "H" BLOCK BOND BEAM UNITS AT 48" O.C. AND AT FLOORS, ROOF AND TOP OF WALL. BOND BEAMS AT ROOF WILL SLOPE TO MATCH SLOPING ROOF.
10. UNLESS OTHERWISE NOTED MINIMUM REINFORCING IN ALL 12" MASONRY WALLS SHALL BE AS FOLLOWS:
    - A. VERTICAL: (2) #5 BARS IN CELLS ADJACENT TO ALL OPENINGS, AT CORNERS AND AT A MAXIMUM SPACING OF 32" THROUGHOUT WALL. ALL VERTICAL REINFORCEMENT SHALL BE DOWELED TO FOUNDATION.
    - B. HORIZONTAL: (2) #5 BARS IN 12" DEEP "H" BLOCK BOND BEAM UNITS AT 48" O.C. AND AT FLOORS, ROOF AND TOP OF WALL. BOND BEAMS AT ROOF WILL SLOPE TO MATCH SLOPING ROOF.
11. SOLID GROUTING OF MASONRY IS UNACCEPTABLE EXCEPT AS SPECIFICALLY NOTED ON PLANS AND SCHEDULES.
12. PROVIDE VERTICAL CONTROL JOINTS AT A MAXIMUM SPACING OF 40 FEET UNLESS OTHERWISE NOTED IN THE SPECIFICATIONS AND/OR ON ARCHITECTURAL ELEVATIONS AND AT ALL CHANGES IN WALL ELEVATION AND MASONRY THICKNESS. CONTROL JOINTS SHALL NOT BE LOCATED DIRECTLY OVER OR CLOSER THAN 24" TO WALL OPENINGS (DOORS, WINDOWS, MECHANICAL OPENINGS, ETC.).
13. HORIZONTAL REINFORCEMENT SHALL TERMINATE AT EACH SIDE OF CONTROL JOINTS EXCEPT AT FLOOR AND ROOF LEVEL BOND BEAMS AND AT TOP OF PARAPET.
14. MASONRY BAR LAPS SHALL BE PER "MASONRY BAR LAP SCHEDULE."
15. MASONRY STRENGTH VERIFICATION:
    - a. MASONRY STRENGTH, F'M SHALL BE VERIFIED USING THE “UNIT STRENGTH METHOD” PER IBC SECTION 2105.3.2.2.1 AND AS DESCRIBED BELOW:
    - i. PRIOR TO CONSTRUCTION, A LETTER OF STRENGTH CERTIFICATION FROM THE SUPPLIERS OF THE MASONRY UNITS AND GROUT SHALL BE SUBMITTED.
    - ii. DURING CONSTRUCTION, THE GROUT AND MORTAR SHALL BE TESTED FOR EVERY 5,000 SQUARE FEET OF MASONRY CONSTRUCTED.
    - b. THE CONTRACTOR HAS THE OPTION OF USING THE “MASONRY PRISM TEST METHOD” PER IBC SECTION 2105.2.2.2 IN LIEU OF THE “UNIT STRENGTH METHOD.”

GENERAL NOTES:

1. VISITS TO THE JOB SITE BY REPRESENTATIVES OF THE ENGINEER DO NOT SUBSTITUTE APPROVAL OF THE WORK PERFORMED BY THE CONTRACTOR OR HIS SUBCONTRACTORS AND ARE MERELY FOR THE PURPOSE OF OBSERVING THE WORK PERFORMED.
2. CONTRACTOR SHALL NOTIFY ENGINEER/ARCHITECT OF ANY DISCREPANCIES, OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS BEFORE PROCEEDING WITH ANY WORK INVOLVED. IN ALL CASES, UNLESS OTHERWISE DIRECTED, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN AND BE PERFORMED.
3. CONTRACTOR SHALL VERIFY ALL CONDITIONS, DIMENSIONS AND ELEVATIONS, ETC., AT THE SITE AND SHALL COORDINATE WORK PERFORMED BY ALL TRADES. SEE ARCHITECT'S PLANS FOR DIMENSIONS. DO NOT SCALE DRAWINGS
4. SHOP DRAWINGS SHALL BE REVIEWED BY THE ENGINEER/ARCHITECT PRIOR TO FABRICATION OR ERECTION FOR ANY PREFABRICATED OR MANUFACTURER-DESIGNED COMPONENTS AND SHALL BE STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THIS STRUCTURE RESIDES.
5. SIZES, LOCATIONS, LOADS, AND ANCHORAGES OF EQUIPMENT SHALL BE VERIFIED IN THE FIELD WITH EQUIPMENT MANUFACTURERS (SUPPLIERS) PRIOR TO FABRICATION OR INSTALLATION OF SUPPORTING STRUCTURES.
6. TEMPORARY BRACING SHALL BE PROVIDED WHEREVER NECESSARY TO TAKE CARE OF ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED, INCLUDING WIND. SUCH BRACING SHALL BE LEFT IN PLACE AS LONG AS MAY BE REQUIRED FOR SAFETY, OR UNTIL ALL THE STRUCTURAL ELEMENTS ARE INSTALLED.
7. DURING AND AFTER CONSTRUCTION THE CONTRACTOR AND/OR OWNER SHALL KEEP LOADS ON THE STRUCTURE WITHIN THE LIMITS OF THE DESIGN LOAD.
8. CONTRACTOR AND ALL SUBCONTRACTORS SHALL PERFORM THEIR TRADES AND DUTIES IN A MANNER CONFORMING TO THE PROCEDURES AND REQUIREMENTS AS STATED IN THE 2015 INTERNATIONAL BUILDING CODE, (OR LATEST ACCEPTED CODE ADOPTED BY THE LOCAL BUILDING OFFICIALS).
9. ANY SPECIAL INSPECTIONS REQUIRED BY THE BUILDING OFFICIAL OR THE INTERNATIONAL BUILDING CODE ARE THE RESPONSIBILITY OF THE OWNER.
10. CONTRACTOR SHALL BE RESPONSIBLE FOR SAFETY AND PROTECTION WITHIN AND ADJACENT TO THE JOB SITE.

FOOTINGS, FOUNDATIONS AND SLAB ON GRADE NOTES:

1. ALL FOOTING SIZES ARE BASED ON AN ALLOWABLE SOIL BEARING PRESSURE AS SHOWN IN THE DESIGN CRITERIA. ANY SOIL CONDITION ENCOUNTERED DURING EXCAVATION THAT IS CONTRARY TO THOSE USED FOR DESIGN OF FOOTINGS AS OUTLINED IN WORKING DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING.
2. CONTRACTOR IS TO VERIFY SOIL BEARING PRESSURE TO BE 1500 PSF. THE CONTRACTOR IS TO FOLLOW THE RECOMMENDATIONS OF THE SOILS ENGINEER.
3. ALL FOOTINGS SHALL BEAR ON UNDISTURBED NATIVE SOIL OR ENGINEERED GRANULAR FILL COMPACTED TO 95% OF MAX. DENSITY, BASED ON ASTM D 1557 METHOD OF COMPACTION. FILL SHALL BE PLACED IN LAYERS NOT TO EXCEED SIX INCHES IN DEPTH AFTER COMPACTION AND SHALL EXTEND DOWN TO IN-SITU SOILS. FILL SHALL BE COMPACTED UNDER ALL CONCRETE WORK ON THE SITE.
4. NO FOOTINGS SHALL BE PLACED IN WATER, SNOW, FROZEN GROUND, OR UNSTABLE SOILS.
5. ALL EXCAVATIONS ADJACENT TO AND BELOW FOOTING ELEVATION FOR OTHER TRADES SHALL BE ACCOMPLISHED PRIOR TO POURING ANY FOOTINGS.
6. CONTRACTOR SHALL BE RESPONSIBLE FOR LATERALLY SUPPORTING ALL RETAINING TYPE FOUNDATION WALLS WHILE COMPACTING BEHIND WALLS AND UNTIL ALL SUPPORTING MEMBERS HAVE BEEN PLACED (SUCH AS FLOOR).
7. ALL REINFORCEMENTS SHALL BE SECURELY TIED IN PLACE PRIOR TO POURING CONCRETE.
8. PROVIDE DOWELS IN FOOTING AND FOUNDATIONS TO MATCH ALL VERTICAL BARS IN WALLS AND COLUMNS ABOVE, UNLESS NOTED OTHERWISE.
9. PROVIDE CONTROL JOINTS IN SLABS AT A MAX. OF 15 FT. O.C. EACH WAY AND AS SHOWN ON PLANS. AT EXTERIOR SLABS AND GARAGE FLOORS POUR SLABS BETWEEN CONTROL JOINTS SO THAT ADJACENT POURS ARE STAGGERED AT LEAST TWO DAYS APART.
10. ALL EXTERIOR FOOTINGS MUST BEAR AT OR BELOW FROST DEPTH, MEASURED FROM LOWEST ADJACENT FINAL GRADE.
11. UNLESS NOTED OTHERWISE, ALL FOOTINGS AT COLUMNS TO BE CENTERED BELOW COLUMNS.
12. UNLESS NOTED OTHERWISE, ALL FOOTINGS SHALL HAVE VERTICAL FACES FORMED WITH STANDARD FORMING MATERIALS (WOOD, METAL, ETC.). WITH PRIOR APPROVAL OF ARCHITECT AND ENGINEER, CONCRETE FOR FOOTINGS CAN BE PLACED IN EXCAVATED "SOIL" FORMS PROVIDED THAT THE DIMENSIONS ARE INCREASED 3" ON EACH SIDE.
13. SLABS ON GRADE SHALL BE 4 INCHES THICK CONCRETE UNDERLAIN BY FREE DRAINING MATERIAL.

CONCRETE NOTES:

1. ALL COLUMNS, RETAINING WALLS AND ALL EXTERIOR FLATWORK, CURBS, GUTTERS, ETC., SHALL BE NORMAL WEIGHT CONCRETE WITH A COMPRESSIVE STRENGTH EQUAL TO AT LEAST 4,000 LBS. PER SQUARE INCH WITHIN 28 DAYS AFTER POURING.
2. ALL SUSPENDED SLABS AND BEAMS SHALL BE NORMAL WEIGHT CONCRETE WITH A COMPRESSIVE STRENGTH EQUAL TO AT LEAST 5,000 LBS. PER SQUARE INCH WITHIN 28 DAYS AFTER POURING.
3. ALL FOOTINGS, FOUNDATIONS, AND INTERIOR SLABS ON GRADE SHALL BE NORMAL WEIGHT CONCRETE WITH A COMPRESSIVE STRENGTH EQUAL TO A LEAST 3,000 LBS. PER SQUARE INCH WITHIN 28 DAYS AFTER POURING.
4. UNLESS OTHERWISE NOTED, ALL FOUNDATION WALL VERTICAL COLD JOINTS SHALL BE KEYED WITH A KEY 1-1/2" DEEP, A LENGTH 2" LESS THAN THE MEMBER, AND A WIDTH 1/2 OF THE MEMBER. REINFORCING SHALL BE CONTINUOUS THRU JOINT.
5. ALL OPENINGS IN CONCRETE WALLS SHALL BE REINFORCED WITH (2) #5 BARS EXTENDING 2'-0" MIN. BEYOND THE EDGE OF THE OPENING AT EACH FACE OF OPENING.
6. ALL CONCRETE WORK SHALL BE PLACED, CURED, STRIPPED, AND PROTECTED AS DIRECTED BY THE SPECIFICATIONS AND ACI STANDARDS AND PRACTICES.
7. BEFORE CONCRETE IS POURED CHECK WITH ALL TRADES TO INSURE PROPER PLACEMENT OF ALL OPENINGS, SLEEVES, CURBS, CONDUITS, BOLTS, INSERTS, ETC. RELATIVE TO WORK.
8. CONTRACTOR IS RESPONSIBLE FOR ALL SHORING AND FORMWORK.
9. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENT, CLIPS OR GROUNDS, REQUIRED TO BE ENCASED IN CONCRETE AND FLOOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.
10. FOR STEPS IN FOUNDATION GREATER THAN 2 FEET, WRAP CORNER W/(2) #4 BARS EXTENDING 18" EACH DIRECTION.
11. STRUCTURAL CONCRETE HAS BEEN DESIGNED AT 2,500 LBS. PER SQUARE INCH AND SPECIFIED AT A HIGHER STRENGTH CONCRETE AS STATED ABOVE. NO SPECIAL INSPECTIONS ARE REQUIRED PER IBC SECTION 1705.3.

DESIGN CRITERIA:

GOVERNING CODE			2015 IBC
RISK CATEGORY			I
EARTHQUAKE			
IMPORTANCE FACTOR	I_E	=	1.00
RESPONSE MODIFICATION COEFFICIENT	R	=	5.0
SPECTRAL RESPONSE COEFFICIENTS	S_S	=	0.802g
	S_1	=	0.268g
	S_{DS}	=	0.630g
	S_{D1}	=	0.333g
SEISMIC DESIGN CATEGORY			D
SOIL SITE CLASS			D (ASSUMED)
BASIC SEISMIC-FORCE-RESISTING SYSTEM			SPECIAL REINF. MAS. WALL
DESIGN BASE SHEAR			$V=C_S W$
ANALYSIS PROCEDURE			EQUIVALENT LATERAL FORCE PROCEDURE
WIND			
BASIC WIND SPEED (3 SECOND GUST)			115 MPH EXPOSURE C
SOIL			
FROST DEPTH			30" MIN.
SOIL BEARING PRESSURE			1500 PSF (ASSUMED)
SOIL REPORT BY:			
REPORT # :			
DATE:			
ROOF			
LIVE LOAD			20 PSF
SNOW GROUND			51 PSF
SNOW ROOF			40 PSF

LEGEND OF SYMBOLS AND ABBREVIATIONS

AB.	=	ANCHOR BOLT
ABV.	=	ABOVE
ARCH.	=	ARCHITECT
BN.	=	BOUNDARY NAILING
BLW.	=	BELOW
CL.	=	CENTERLINE
CMU.	=	CONCRETE MASONRY UNIT
COL.	=	COLUMN
CONC.	=	CONCRETE
CONT.	=	CONTINUOUS
DBA.	=	DEFORMED BAR ANCHOR
EN.	=	EDGE NAILING
EQ.	=	EQUAL
ELEV.	=	ELEVATION
EW.	=	EACH WAY
FDN.	=	FOUNDATION
FN.	=	FIELD NAILING
FTG.	=	FOOTING
GLB.	=	GLUELAM BEAM
HORIZ.	=	HORIZONTAL
IBC.	=	INTERNATIONAL BUILDING CODE
HSA.	=	HEADED STUD ANCHOR
LLH.	=	LONG LEG HORIZONTAL
LLV.	=	LONG LEG VERTICAL
MAX.	=	MAXIMUM
MECH.	=	MECHANICAL
MIN.	=	MINIMUM
OAE.	=	OR APPROVED EQUAL
O.C.	=	ON CENTER
OPP.	=	OPPOSITE
PSW.	=	PERFORATED SHEAR WALL
PL.	=	PLATE
PLM.	=	PARALLAM
REINF.	=	REINFORCEMENT
REQD.	=	REQUIRED
SCHED.	=	SCHEDULE
STRUCT.	=	STRUCTURAL
SW.	=	SHEAR WALL
SIM.	=	SIMILAR
SQ.	=	SQUARE
TYP.	=	TYPICAL
UNO.	=	UNLESS NOTED OTHERWISE
VERT.	=	VERTICAL

S —— S	FOOTING STEP
-- / S--	SECTION MARK SHEET NUMBER
◐ --	ELEVATION
△	HOLDOWN ANCHOR LOCATION
╱	HOLDOWN ANCHOR TYPE
▨	OVERBUILD AREA
⊠	DEPRESS FOUNDATION WALL AND POUR SLAB OVER
═	WOOD BEAM

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Revisions:

	Date:	Description:
1		
2		
3		
4		
5		
6		

WANGSGARD STORAGE
HUNTSVILLE, UTAH

GENERAL STRUCTURAL NOTES

STAMP IS NOT VALID IF SIGNATURE IS NOT SIGNED IN RED INK
PROFESSIONAL ENGINEER
JEFFERY M. TURVILLE
9412168
STATE OF UTAH
STRUCTURAL DRAWINGS ONLY

Project Info.
Engineer: J.M.G.
Drafter: A.W.B.
Begin Date: DECEMBER 1, 2017
Number: 6260-01

Sheet	S7
S1	Sheets

REQUIRED VERIFICATION AND INSPECTION OF MASONRY

LEVEL A QUALITY ASSURANCE

MINIMUM TESTS

NONE

MINIMUM INSPECTION

VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS

LEVEL B QUALITY ASSURANCE

MINIMUM TESTS

VERIFICATION OF SLUMP FLOW AND VSI AS DELIVERED TO THE SITE IN ACCORDANCE WITH ARTICLE 1.5 B.1.b.3 FOR SELF CONSOLIDATING GROUT

VERIFICATION OF f'm AND f'aac ACCORDANCE WITH SPECIFICATION ARTICLE 1.4 B PRIOR TO CONSTRUCTION, EXCEPT WHRE SPECIFICALLY EXEMPTED BY THE CODE.

MINIMUM INSPECTION

INSPECTION TASK	FREQUENCY OF INSPECTION	
	CONTINUOUS	PERIODIC
1. VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS	-	X
2. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:		
a. PROPORTIONS OF SITE-PREPARED MORTAR	-	X
b. CONSTRUCTION OF MORTAR JOINTS	-	X
c. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES	-	X
d. LOCATION OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES	-	X
e. PRESTRESSING TECHNIQUE	-	X
3. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:		
a. GROUT SPACE	-	X
b. GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS, AND ANCHORAGES	-	X
c. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES	-	X
d. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	-	X
e. CONSTRUCTION OF MORTAR JOINTS	-	X
4. VERIFY DURING CONSTRUCTION		
a. SIZE AND LOCATION OF STRUCTURAL ELEMENTS	-	X
b. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	-	X
c. WELDING OF REINFORCEMENT	X	-
d. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F (4.4°C)) OR HOT WEATHER (TEMPERATURE ABOVE 90°F (32.2°C))	-	X
e. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	X	-
f. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	X	-
5. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	-	X

LEVEL C QUALITY ASSURANCE

MINIMUM TESTS

VERIFICATION OF f'm AND f'aac IN ACCORDANCE WITH ARTICLE 1.4B PRIOR TO CONSTRUCTION AND FOR EVERY 5,000 SQ FT (464.5 m^2) DURING CONSTRUCTION

VERIFICATION OF PROPORTIONS OF MATERIALS IN PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT, AS DELIVERED TO THE SITE

VERIFICATION OF SLUMP FLOW AND VSI AS DELIVERED TO THE SITE IN ACCORDANCE WITH ARTICLE 1.5 B.1.b.3 FOR SELF CONSOLIDATING GROUT

MINIMUM INSPECTION

INSPECTION TASK	FREQUENCY OF INSPECTION	
	CONTINUOUS	PERIODIC
1. VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS	-	X
2. VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:		
a. PROPORTIONS OF SITE-PREPARED MORTAR	-	X
b. GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES	-	X
c. PLACEMENT OF MASONRY UNITS AND CONSTRUCTION OF MORTAR JOINTS	-	X
d. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES	X	-
e. GROUT SPACE PRIOR TO GROUTING	X	-
f. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	X	-
g. SIZE AND LOCATION OF STRUCTURAL ELEMENTS	-	X
h. TYPE, SIZE, AND LOCATION OF ANCHORS INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	X	-
i. WELDING OF REINFORCEMENT	X	-
j. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F (4.4°C)) OR HOT WEATHER (TEMPERATURE ABOVE 90°F (32.2°C))	-	X
k. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	X	-
5. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	X	-

REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS

VERIFICATION AND INSPECTION TASK	FREQUENCY OF INSPECTION	
	CONTINUOUS	PERIODIC
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	-	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	-
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	-	X

SIMPSON HOLDOWN SCHEDULE

HOLDOWN	MIN. POST	ANCHOR
HDU5	3"	5/8xREQD.

SPECIAL INSPECTION REQUIREMENTS

1. SPECIAL INSPECTIONS SHALL INCLUDE BUT ARE NOT LIMITED TO THE FOLLOWING:
 SPECIAL REINFORCED MASONRY WALLS
 SOILS
 SEE SPECIFIC DISCIPLINE DRAWINGS FOR ADDITIONAL DESIGNATED SEISMIC SYSTEMS REQUIRING SPECIAL INSPECTION WHICH ARE NOT CONTAINED IN THE STRUCTURAL DRAWINGS.
2. SPECIAL INSPECTION AND TESTING AS REQUIRED BY THE IBC SHALL BE PROVIDED BY AN INDEPENDENT AGENCY EMPLOYED BY THE OWNER UNLESS WAIVED BY THE BUILDING OFFICIAL. THE CONTRACTOR SHALL COORDINATE AND COOPERATE WITH THE REQUIRED INSPECTIONS/TESTS AS INDICATED BELOW, REFERRING TO THE IBC SECTION INDICATED AS APPROPRIATE.
3. SPECIAL INSPECTION REPORTS FROM THE INSPECTOR SHALL BE SENT TO THE ARCHITECT/ENGINEER AND BUILDING OFFICIAL.
 3.1. BRING ANY DISCREPANCIES TO THE CONTRACTOR'S IMMEDIATE ATTENTION.
 3.2. NOTIFY THE ENGINEER OF ANY NON-PASSING WORK THAT THE CONTRACTOR CANNOT READILY CORRECT.
 3.3. ANY UNCORRECTED DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER AND BUILDING OFFICIAL PRIOR TO COMPLETION OF THAT PHASE OF THE WORK.
 3.4. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTIONS OF ANY DISCREPANCIES SHALL BE PROVIDED.
4. QUALITY CONTROL SUBMITTALS FOR STRUCTURAL STEEL
 4.1. FURNISH WELDER QUALIFICATION RECORDS TO VERIFY PROJECT WELDERS ARE TESTED AND QUALIFIED IN ACCORDANCE WITH AWS D1.1 BEFORE WELDING STRUCTURAL OR MISCELLANEOUS STEELS, D1.3 BEFORE WELDING SHEET STEELS (10 GA AND THINNER), AND D1.4 BEFORE WELDING REINFORCING STEEL. SUBMIT DOCUMENTATION TO THE APPROVED INSPECTION AGENCY FOR REVIEW BEFORE WELDING.
 4.2. PROVIDE BOLT STORAGE AND INSTALLATION PROCEDURES TO THE APPROVED INSPECTION AGENCY FOR REVIEW.
5. SPECIAL INSPECTORS
 5.1. SPECIAL INSPECTORS SHALL BE QUALIFIED PERSONS WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
6. SECTION 1704: SPECIAL INSPECTIONS
 6.1. FABRICATORS (1704.2): ALL OFFSITE FABRICATION OF STRUCTURAL MEMBERS (STRUCTURAL STEEL, PRECAST CONCRETE, ETC.) SHALL BE SPECIAL INSPECTED AS REQUIRED BY THE IBC, EXCEPT WORK PERFORMED BY FABRICATORS REGISTERED AND APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION. APPROVED FABRICATORS SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO BUILDING OFFICIAL AT THE COMPLETION OF FABRICATION.
 6.2. SOILS (1704.7): EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT AND PROCEDURE AND LOAD-BEARING REQUIREMENTS SHALL BE SPECIAL INSPECTED TO VERIFY COMPLIANCE WITH THE APPROVED SOILS REPORT AND TABLE 1704.7. WHERE TOTAL DEPTH OF CONTROLLED FILL IS 12" OR LESS, THE SPECIAL INSPECTOR SHALL VERIFY THAT THE IN-PLACE DRY DENSITY OF THE COMPACTED FILL IS NOT LESS THAN THE 90% OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT ACCORDING TO ASTM D 1557.
 6.3. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INSPECTIONS PERTAINING TO EIFS (1704.14) OR SMOKE CONTROL SYSTEMS (1704.16).
 6.4. SPECIAL CASES (1704.15): SPECIAL INSPECT ALL POST-INSTALLED ANCHORS PER ICC REPORT.
7. SECTION 1707: SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE
 7.1. STRUCTURAL STEEL (1707.2): SEE SPECIAL INSPECTIONS FOR STEEL ABOVE.
 7.2. SEE SPECIFIC DISCIPLINE DRAWINGS FOR ANCHORAGE OF STORAGE RACKS (1707.5), ARCHITECTURAL COMPONENTS (1707.6), MECHANICAL/ELECTRICAL COMPONENTS (1707.7) AND DESIGNATED SEISMIC SYSTEM VERIFICATION (1707.8).
8. STRUCTURAL OBSERVATIONS FOR SEISMIC RESISTANCE: CONTRACTOR SHALL NOTIFY THE ENGINEER AT LEAST 2 DAYS PRIOR TO THE FOLLOWING STAGES OF CONSTRUCTION SO THE ENGINEER MAY HAVE THE OPPORTUNITY TO REVIEW THE WORK. OBSERVATION REPORTS SHALL BE SENT TO ARCHITECT, CONTRACTOR AND BUILDING OFFICIAL.
 8.1. INITIAL PLACING OF ANY CONCRETE, INCLUDING BUT NOT LIMITED TO: FOOTINGS, TILT WALLS, SLABS ON GRADE OR CONCRETE OVER STEEL DECK
 8.2. INITIAL GROUT POURS FOR MASONRY WALLS
 8.3. INITIAL ERECTION OF WOOD TRUSSES
 8.4. COMPLETION OF STRUCTURAL ROOF DECK

MINIMUM NAILING SCHEDULE

No.	CONNECTION	FASTENING						LOCATION
		NAILING			STAPLES			
		No.	SIZE	SPACING	No.	SIZE	SPACING	
1	JOIST TO SILL OR GIRDER	3	8d	--	3	3"-14 GA.	--	TOENAIL
2	BRIDGING TO JOIST	2	8d	--	2	3"-14 GA.	--	TOENAIL EA. END
3	BOTTOM PLATE TO JOIST OR BLOCKING	--	16d		--	3"-14 GA.	12" O.C.	TYP. FACE NAIL
4	BOTTOM PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL	3	16d	16" O.C.	4	3"-14 GA.	12" O.C.	BRACED WALL PANELS
5	TOP PLATE TO STUD	2	16d	--	3	3"-14 GA.	--	END NAIL
6	STUD TO BOTTOM PLATE	4	8d	--	3	3"-14 GA.	--	TOENAIL
6a	STUD TO BOTTOM PLATE (OPTIONAL)	2	16d	--	3	3"-14 GA.	--	END NAIL
7	DOUBLE STUDS	--	16d	16" O.C.	--	3"-14 GA.	8" O.C.	FACE NAIL
8	DOUBLE TOP PLATES	--	16d	16" O.C.	--	3"-14 GA.	12" O.C.	TYP. FACE NAIL
9	DOUBLE TOP PLATES LAP SPLICES	8	16d	--	12	3"-14 GA.	--	TYP. FACE NAIL
10	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	3	8d	--	3	3"-14 GA.	--	TOENAIL
11	RIM JOIST TO TOP PLATE	--	8d		--	3"-14 GA.	16" O.C.	TOENAIL
12	TOP PLATES, LAPS & INTERSECTIONS	2	16d	--	3	3"-14 GA.	--	FACE NAIL
13	CONTINUOUS HEADER, TWO PIECES	--	16d		--	--	--	ALONG EDGE
14	CEILING JOISTS TO PLATE	3	8d	--	5	3"-14 GA.	--	TOENAIL
15	CONTINUOUS HEADER TO STUD	4	16d	--	--	--	--	TOENAIL
16	CEILING JOISTS, LAPS OVER PARTITIONS	3	16d	--	4	3"-14 GA.	--	FACE NAIL
17	CEILING JOISTS TO PARALLEL RAFTERS	3	16d	--	4	3"-14 GA.	--	FACE NAIL
18	RAFTER TO PLATE	3	8d	--	3	3"-14 GA.	--	TOENAIL
19	BUILT-UP CORNER STUDS	--	16d	24" O.C.	--	3"-14 GA.	16" O.C.	FACE NAIL
20	BUILT-UP GIRDER AND BEAMS	--	20d	32" O.C.	--	3"-14 GA.	24" O.C.	FACE NAIL @ TOP & BOTTOM STAGGERED ON OPP. SIDES
20a	BUILT-UP GIRDER AND BEAMS (OPTIONAL)	2	20d	--	3	3"-14 GA.	--	FACE NAIL AT ENDS AND AT EACH SPLICE
21	COLLAR TIE TO RAFTER	3	10d	--	4	3"-14 GA.	--	FACE NAIL
22	JACK RAFTER TO HIP	3	10d	--	4	3"-14 GA.	--	TOENAIL
22a	JACK RAFTER TO HIP (OPTIONAL)	2	16d	--	3	3"-14 GA.	--	FACE NAIL
23	ROOF RAFTER TO 2x RIDGE BEAM	2	16d	--	3	3"-14 GA.	--	TOENAIL OR FACE NAIL
24	JOIST TO RIM JOIST	3	16d	--	5	3"-14 GA.	--	FACE NAIL
25	LEDGER STRIP	3	16d	--	4	3"-14 GA.	--	FACE NAIL

NOTES:
1. COMMON OR BOX NAILS ARE PERMITTED TO BE USED, EXCEPT WHERE OTHERWISE NOTED.
2. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16 INCH.
3. SEE IBC TABLE 2304.9.1 FOR ADDITIONAL NAILING REQUIREMENTS.

SHEAR WALL SCHEDULE

SYM.	SHEATHING		NAILING[3]				STUDS[4]			MIN.[10] SHEAR	ANCHOR[11] BOLT	ANCHOR BOLT SPACING	COMMENTS
			EDGE (E.N.)		FIELD (F.N.)		EDGE	FIELD					
	THICK.	TYPE[1]	SIZE	SPACING	SIZE	SPACING	SIZE	SIZE	SPACING				
SW-1	7/16"	OSB	8d	6" O.C.	8d	12" O.C.	2x	2x	16" O.C.	240 PLF	5/8"Øx10"	32" O.C.	-
SW-2	7/16"	OSB	8d	4" O.C.	8d	12" O.C.	3x	2x	16" O.C.	350 PLF	5/8"Øx10"	32" O.C.	-
SW-3	7/16"	OSB	8d	3" O.C.	8d	12" O.C.	3x[6]	2x	16" O.C.	450 PLF	5/8"Øx10"	16" O.C.	-
SW-4	7/16"	OSB	8d	2" O.C.	8d	12" O.C.	3x[6]	2x	16" O.C.	585 PLF	5/8"Øx10"	16" O.C.	-
SW-5	7/16"	OSB	8d	4" O.C.	8d	12" O.C.	3x[7]	2x	16" O.C.	700 PLF	3/4"Øx12"	16" O.C.	SHEATH BOTH SIDES. 3x SILL PL REQ.
SW-6	7/16"	OSB	8d	3" O.C.	8d	12" O.C.	3x[7]	2x	16" O.C.	900 PLF	3/4"Øx12"	16" O.C.	SHEATH BOTH SIDES. 3x SILL PL REQ.
SW-7	7/16"	OSB	8d	2" O.C.	8d	12" O.C.	3x[7]	2x	16" O.C.	1280 PLF	3/4"Øx12"	12" O.C.	SHEATH BOTH SIDES. 3x SILL PL REQ.

NOTES:
1. OSB SHEATHING SHALL BE TYPE C-D, C-C STRUCTURAL GRADE. ALL OTHER GRADES SHALL BE COVERED IN IBC SECTION 2303.15.
2. SHEATHING MAY BE INSTALLED ON EITHER SIDE OF WALL INDICATED, U.N.O.
3. SEE TABLE OF EQUIVALENT FASTENERS FOR APPROVED SUBSTITUTIONS.
4. STUDS SHALL BE DOUGLAS FIR-LARCH OR SOUTHERN PINE.
5. FASTENERS FOR PRESSURE PRESERVATIVE WOOD SHALL BE HOT-DIPPED, GALVANIZED STEEL OR STAINLESS STEEL.
6. (2) 2x NOMINAL STUDS MAY BE USED IN PLACE OF 3x NOMINAL STUDS PROVIDED THE (2) 2x NOMINAL STUDS ARE NAILED TOGETHER WITH 16d NAILS AT 3" O.C. STAGGER NAILING BETWEEN STUDS.
7. STUD MAY BE A 2x MINIMAL MEMBER PROVIDED PANEL JOINTS ON BOTH SIDES OF THE WALL ARE STAGGERED AND DO NOT SHARE THE SAME 2x NOMINAL STUD.
8. ALL HOLDOWNS MUST BE ANCHORED AS PER SIMPSON SPECS THROUGH A MIN. OF DOUBLE FULL LENGTH 2x STUDS. HOLDOWNS CAN NOT BE ANCHORED TO TRIMMERS OR CRIPPLES.
9. SIMPSON SET-XP ADHESIVE SYSTEM MAY BE USED AS PER MANUFACTURER'S SPECS TO ANCHOR BOLTS IN CONCRETE.
10. VALUES SHOWN ARE TO BE USED WHEN SEISMIC GOVERNS THE DESIGN AND MAY BE INCREASED 40% IF WIND GOVERNS.
11. USE "J" BOLTS W/ 3"x3"x1/4" STEEL PLATE WASHER AT EACH BOLT. PROVIDE A ROUND CUT WASHER BETWEEN THE NUT OF THE ANCHOR BOLT AND THE PLATE WASHER.

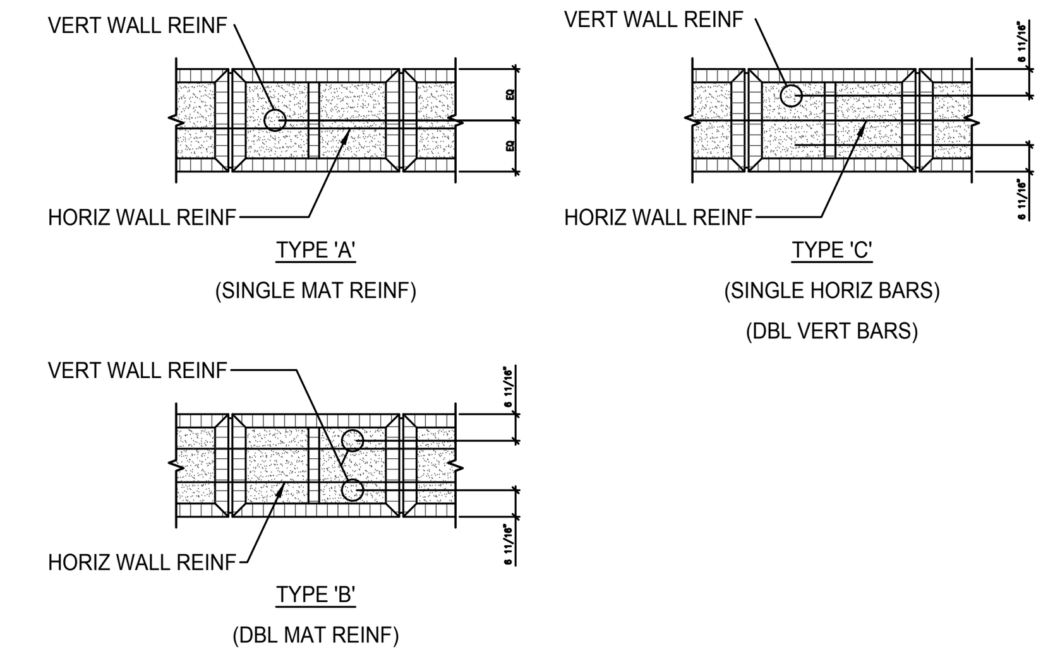
MASONRY WALL SCHEDULE

WALL MARK	THICKNESS	MATERIALS	SOLID GROUT	REINFORCING				SPECIAL INSPECTION REQUIRED
				VERTICAL	HORIZONTAL	TYPE	JOINT	
MW-1	8"	CMU 1500 PSI	NO	#4 @ 32"oc	#4 @ 48"oc	A	PER ARCH	YES

MASONRY WALL NOTES:
1. COORDINATE WITH ARCHITECTURAL DRAWINGS, MASONRY WALL FINISHES, TYPES OF MATERIAL, COURSING, ETC.
2. DO NOT SOLID GROUT WALLS UNLESS NOTED OTHERWISE.
3. ALL MASONRY BELOW GRADE SHALL BE GROUTED SOLID.
4. VERTICAL REINFORCING SHALL BE CENTERED IN THE WALL UNLESS NOTED OTHERWISE.
5. (1) VERTICAL BARS MINIMUM AT ALL CORNERS AND END OF WALLS.
6. HORIZONTAL WALL REINFORCING SHALL BE PLACED BETWEEN VERTICAL MASONRY COLUMN REINFORCING BARS.
7. HORIZONTAL WALL REINFORCING SHALL CONTINUE THRU MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
8. MASONRY WALLS NOT DESIGNATED ON THE PLANS SHALL BE REINFORCED AS FOLLOWS:

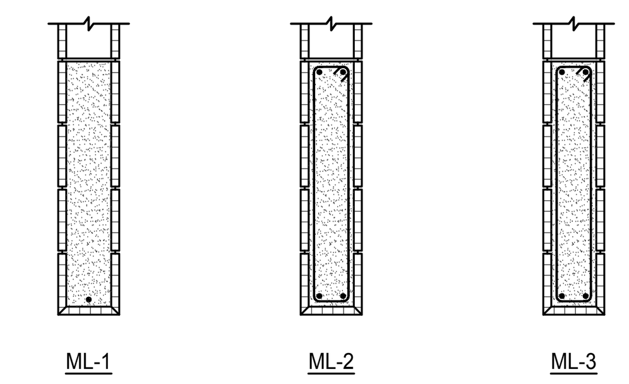
THICKNESS	VERTICAL REINFORCING	HORIZONTAL REINFORCING
6"	#5 BARS AT 32"oc	#4 BARS AT 48"oc
8"	#5 BARS AT 32"oc	#5 BARS AT 48"oc
10"	#6 BARS AT 32"oc	#6 BARS AT 48"oc
12"	#6 BARS AT 32"oc	(2) #5 BARS AT 48"oc

9. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



MASONRY LINTEL SCHEDULE

LINTEL MARK	LINTEL DEPTH	LINTEL SPAN MAXIMUM	REINFORCING		REMARKS
			HORIZONTAL	STIRRUPS	
ML-1	32"	10'-0"	#5 BAR	NONE	
ML-2	32"	10'-0"	(2) #5 BARS TOP & BOTTOM	#4 AT 24"oc	
ML-3	32"	12'-0"	(2) #6 BARS TOP & BOTTOM	#4 AT 24"oc	



MASONRY LINTEL NOTES:
1. LINTEL WIDTH AND MATERIAL TYPE SHALL BE THE SAME AS THE WALL IN WHICH THE LINTEL IS CONSTRUCTED.
2. GROUT MASONRY LINTELS MONOLITHICALLY WITH THE SUPPORT WALL OR COLUMN AT EACH END.
3. MASONRY LINTELS ML-1 THRU ML-4 SHALL BE USED OVER OPENINGS IN MASONRY WALLS WHEN A SPECIFIC MASONRY LINTEL IS NOT OTHERWISE SPECIFIED. WHEN A LINTEL IS SPECIFIED ON THE PLANS, THE MAXIMUM SPAN AS NOTED IN THIS SCHEDULE SHALL NOT APPLY. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SPECIFIED ON THE PLANS WHICH HAVE A SPAN GREATER THAN 10'-0".
4. MASONRY LINTELS ML-1 THRU ML-4 SHALL NOT BE LOCATED DIRECTLY BELOW FLOOR OR ROOF BEAMS OR GIRDERS UNLESS NOTED OTHERWISE ON THE PLANS. JOISTS SHALL NOT BEAR ON ANY LINTEL LESS THAN 16" DEEP. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SHOWN ON THE PLANS WHICH ARE LOCATED DIRECTLY BELOW FLOOR OR ROOF BEAMS OR GIRDERS.
5. EXTEND ALL HORIZONTAL REINFORCING BEYOND THE EDGE OF ALL OPENINGS. IF HORIZONTAL REINFORCING CANNOT EXTEND LAP SPLICE LENGTH BEYOND EDGE OF OPENING, PROVIDE 90° STANDARD HOOK.
6. SPLICE TOP BARS AT MIDSPAN OF LINTEL ONLY AND BOTTOM BARS OVER SUPPORTS ONLY.
7. HORIZONTAL WALL REINFORCING SHALL CONTINUE THRU MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
8. DOWEL VERTICAL REINFORCING OF WALL ABOVE LINTEL INTO THE FULL DEPTH OF LINTEL OR 48 BAR DIAMETERS, WHICHEVER IS LESS.
9. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

MASONRY REINFORCING BAR LAP SPLICE SCHEDULE

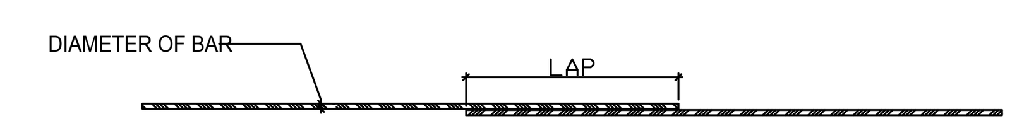
SINGLE BAR CENTERED IN CELL

REBAR SIZE	f'm = 1500 psi 8"	10"	12"	f'm = 1800 psi 8"	10"	12"	f'm = 2000 psi 8"	10"	12"	f'm = 2500 psi 8"	10"	12"	f'm = 3000 psi 8"	10"	12"	f'm = 3500 psi 8"	10"	12"
#3	16"			15"			15"			15"			15"			15"		
#4	22"			20"			20"			20"			20"			20"		
#5	26"			25"			25"			25"			25"			25"		
#6	43"	40"		40"	36"		38"	34"		34"	31"		31"	30"		30"		
#7	60"	46"		55"	42"		52"	40"		46"	36"		42"	35"		39"	35"	
#8	72"	72"	61"	72"	64"	56"	72"	61"	53"	71"	55"	47"	56"	50"	43"	60"	46"	40"
#9	82"	82"	74"	82"	82"	68"	82"	79"	64"	82"	71"	57"	82"	64"	52"	82"	60"	48"

FLUSH WALL PILASTER OR COLUMN, TWO BARS IN EACH CELL

REBAR SIZE	f'm = 1500 psi	f'm = 1800 psi	f'm = 2000 psi	f'm = 2500 psi	f'm = 3000 psi	f'm = 3500 psi
#3	16"	15"	15"	15"	15"	15"
#4	22"	20"	20"	20"	20"	20"
#5	32"	29"	28"	25"	25"	25"
#6	54"	54"	51"	46"	42"	39"
#7	63"	63"	63"	63"	57"	53"
#8	72"	72"	72"	72"	72"	72"
#9	82"	82"	82"	82"	82"	82"

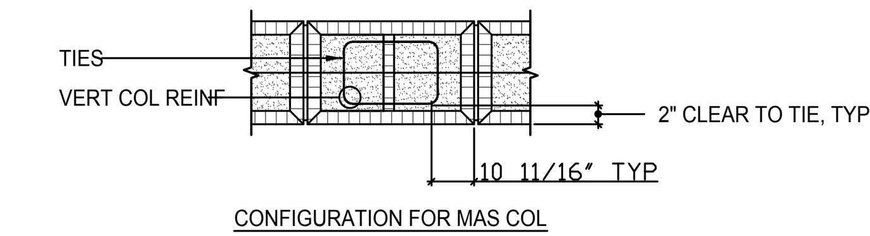
NOTES:
1. MECHANICAL SPLICES ARE REQUIRED FOR BARS IN MASONRY GREATER THAN #9 BARS.
2. MECHANICAL SPLICES MAY BE USED IN LIEU OF LAP SPLICES SHOWN.
3. MECHANICAL SPLICES SHALL DEVELOP 125% OF SPECIFIED YIELD STRENGTH OF BAR.
4. MASONRY DEVELOPMENT LENGTHS SHOWN SHALL BE INCREASED BY 50% WHERE REBAR IS COATED WITH EPOXY.



MASONRY COLUMN SCHEDULE

COLUMN MARK	COLUMN SIZE	REINFORCING		COMMENTS
		VERTICAL	TIES	
MC-1	WALL WIDTH x 16"	(4) #4	#3 AT 8"oc	
MC-2	WALL WIDTH x 24"	(6) #4	#3 AT 8"oc	
MC-3	WALL WIDTH x 32"	(8) #4	#3 AT 8"oc	
MC-4	WALL WIDTH x 40"	(10) #4	#3 AT 8"oc	
MC-5	WALL WIDTH x 48"	(12) #4	#3 AT 8"oc	

MASONRY COLUMN NOTES:
1. HORIZONTAL WALL REINFORCEMENT SHALL BE LOCATED TO THE INSIDE OF VERTICAL BARS. THE CENTERLINE OF VERTICAL BARS SHALL BE LOCATED 2 1/2" FROM THE FACE OF THE MASONRY.
2. UNLESS NOTED OTHERWISE, VERTICAL REINFORCING AND TIES SHALL EXTEND TO FULL WALL HEIGHT.
3. VERTICAL MASONRY COLUMN REINFORCING SHALL EXTEND INTO THE FOOTING AND TERMINATE WITH A STANDARD 90° HOOK. FOR CONCRETE FOUNDATION WALLS OVER 5'-0" TALL, VERTICAL COLUMN REINFORCING REINFORCING SHALL DOWEL 4'-0" MINIMUM INTO THE FOUNDATION WALL.
4. IN CONCRETE FOUNDATION WALLS, VERTICAL MASONRY COLUMN REINFORCING SHALL BE TIED WITH #3 TIES AT THE SAME SPACING AND CONFIGURATION AS MASONRY COLUMNS ABOVE.
5. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
6. SPACE TIES AT 48"oc FROM A POINT 24" ABOVE OPENINGS TO THE TOP OF THE WALL.

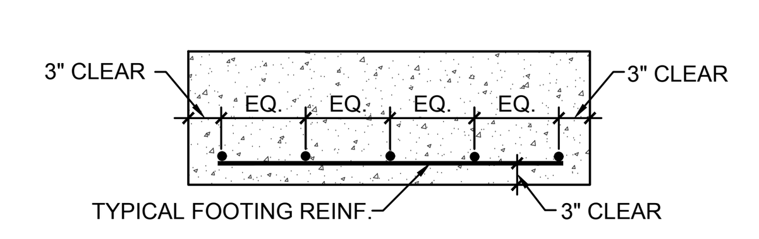


CONFIGURATION FOR MAS COL

CONCRETE FOOTING SCHEDULE

MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE NO	SIZE	LENGTH	SPACING	REINFORCING LENGTHWISE NO	SIZE	LENGTH	SPACING	REMARKS
FC2.0	2'-0"	CONT.	12"	-	#5	1'-6"	14"	3	#4	CONT.	EQ.	
FC2.5	2'-6"	CONT.	12"	-	#5	2'-0"	14"	3	#5	CONT.	EQ.	
FC3.0	3'-0"	CONT.	12"	-	#5	2'-6"	14"	3	#5	CONT.	EQ.	
FT1.5	1'-6"	CONT.	10"	-	-	-	-	2	#4	CONT.	-	
FS3.0	3'-0"	3'-0"	12"	3	#5	2'-6"	EQ.	3	#5	2'-6"	EQ.	
FS4.0	4'-0"	4'-0"	12"	4	#5	3'-6"	EQ.	4	#5	3'-6"	EQ.	
FS5.0	5'-0"	5'-0"	12"	5	#5	4'-6"	EQ.	5	#5	4'-6"	EQ.	
FS5.5	5'-6"	5'-6"	13"	6	#5	5'-0"	EQ.	6	#5	5'-0"	EQ.	
FS6.0	6'-0"	6'-0"	15"	7	#5	5'-6"	EQ.	7	#5	5'-6"	EQ.	
FS6.5	6'-6"	6'-6"	16"	8	#5	6'-0"	EQ.	8	#5	6'-0"	EQ.	
FS7.0	7'-0"	7'-0"	17"	6	#6	6'-6"	EQ.	6	#6	6'-6"	EQ.	

CONCRETE FOOTING NOTES:
1. PLACE ALL FOOTING REINFORCING IN BOTTOM OF FOOTING WITH 3" CLEAR CONCRETE COVER, UNLESS NOTED OTHERWISE.
2. TOP REINFORCING, WHERE SPECIFIED, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" MINIMUM CONCRETE COVER.
3. IF FOOTINGS ARE EARTH FORMED, FOOTING WIDTH AND LENGTH SHALL BE 6" WIDER AND LONGER THAN SCHEDULED.
4. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.
5. NOT ALL FOOTINGS ARE USED, SEE FOUNDATION PLAN FOR FOOTING MARKS.
6. RUN CONTINUOUS BARS IN 'FC' FOOTING THROUGH INTERSECTED 'FS' FOOTINGS.

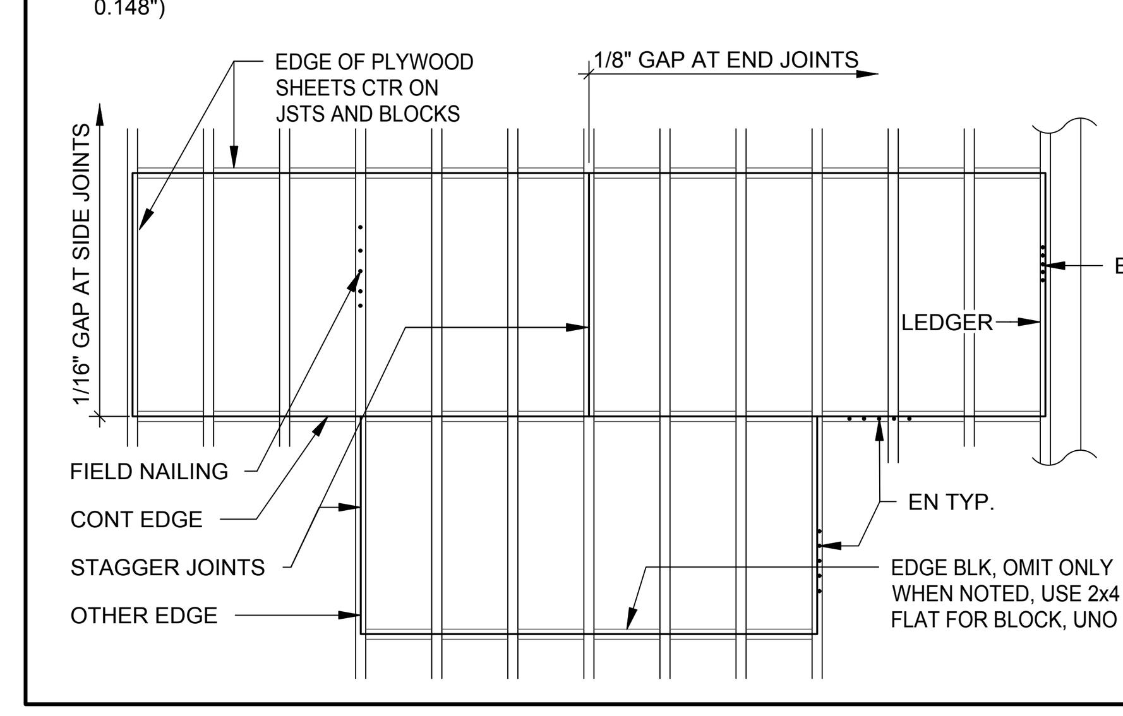


TYPICAL FOOTING SECTION

SHEATHING SCHEDULE AT ROOF

LOCATION	WOOD SHEATHING THICKNESS	NAIL SIZE	EDGE NAIL CONT EDGE	EDGE NAIL OTHER EDGE	FIELD NAIL	BOUNDARY NAIL	EDGE BLOCK
ROOF	15/32"	8D	6"	6"	12"	4"	YES

PLYWOOD SHEATHING NOTES:
1. MINIMUM NAIL PENETRATION INTO FRAMING 8d-1 1/2", 10d- 1 5/8".
2. USE SCREW SHANK NAILS AT FLOOR PLYWOOD.
3. USE COMMON NAILS. (8d DIAMETER = 0.131", 10d DIAMETER = 0.148")



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WANGSGARD STORAGE HUNTSVILLE, UTAH

GENERAL STRUCTURAL NOTES (CONT.)

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9412168
STATE OF UTAH
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Project Info.
Engineer: J.M.G.
Drafter: A.W.B.
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Number: 6260-01

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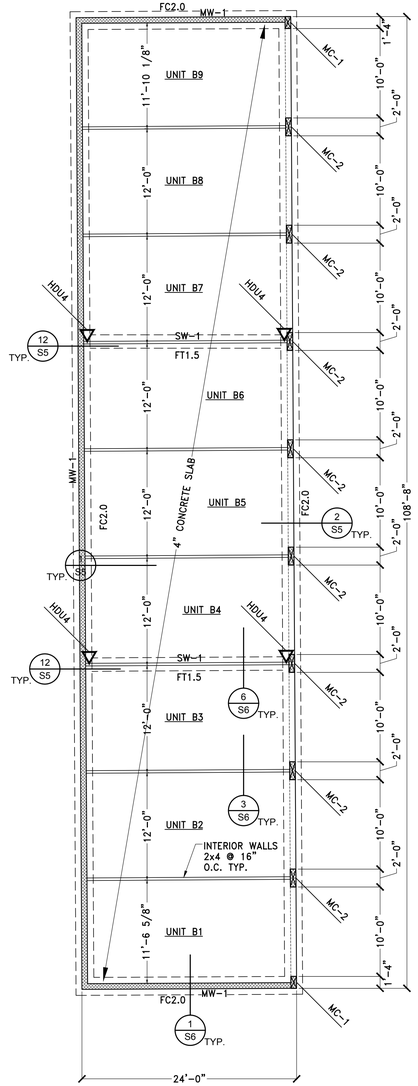
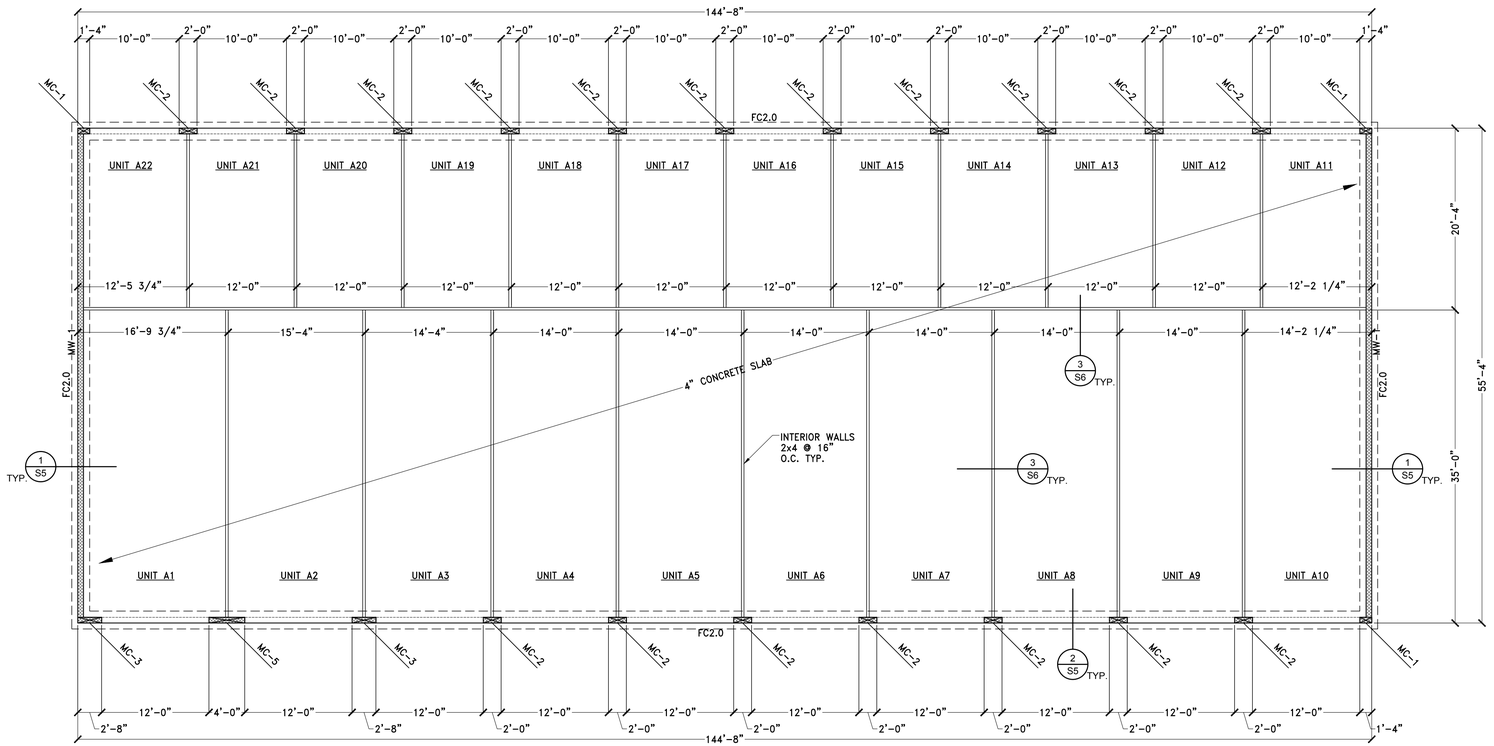


BUILDING B – FOOTING & FOUNDATION PLAN

SCALE: 1/8" = 1'-0"



BUILDING A – FOOTING & FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

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FOOTING & FOUNDATION PLAN

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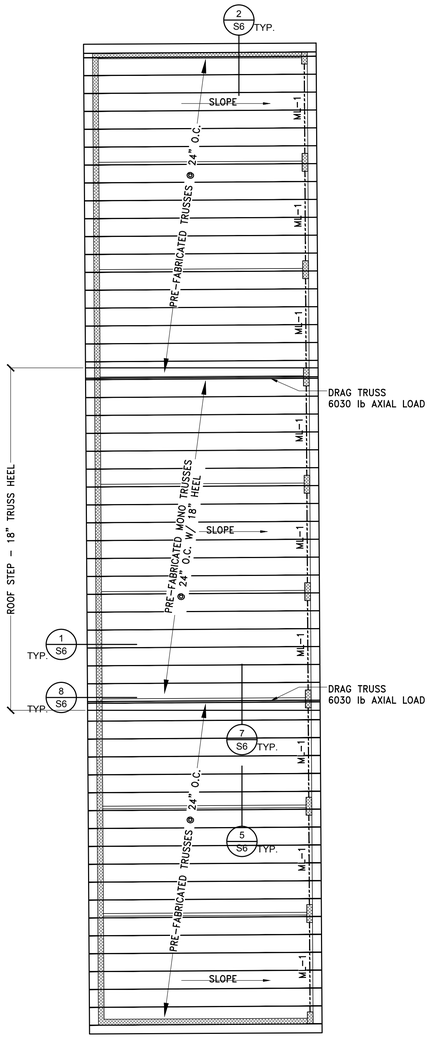
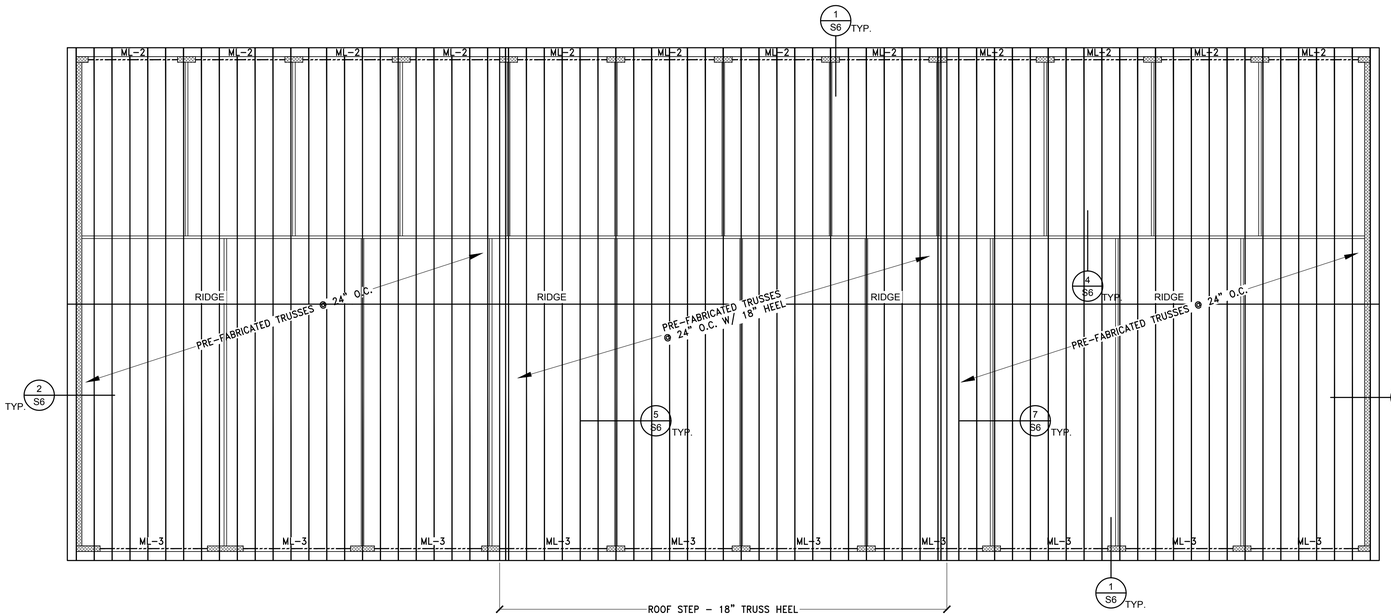
Sheet S7
S3 Sheets



BUILDING B – ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"



BUILDING A – ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

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WANGSGARD STORAGE
HUNTSVILLE, UTAH

ROOF FRAMING PLAN

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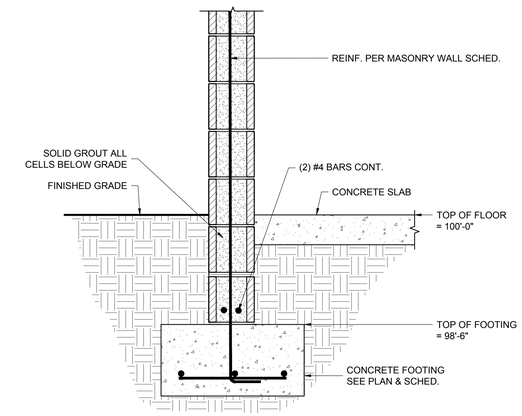
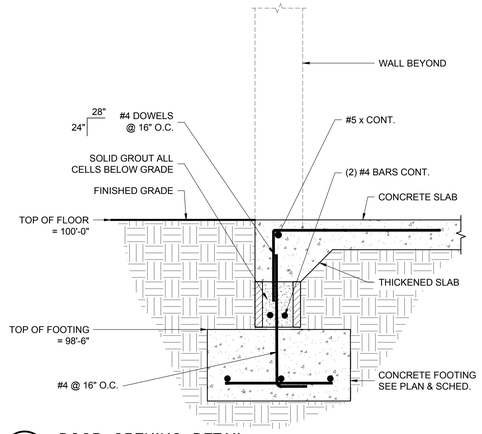
Sheet S4 | S7 Sheets



1 FOUNDATION WALL DETAIL
SCALE: NONE



2 DOOR OPENING DETAIL
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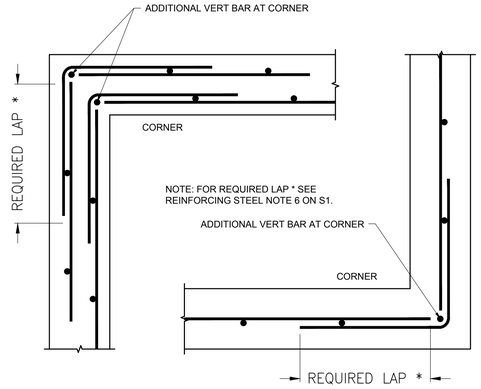
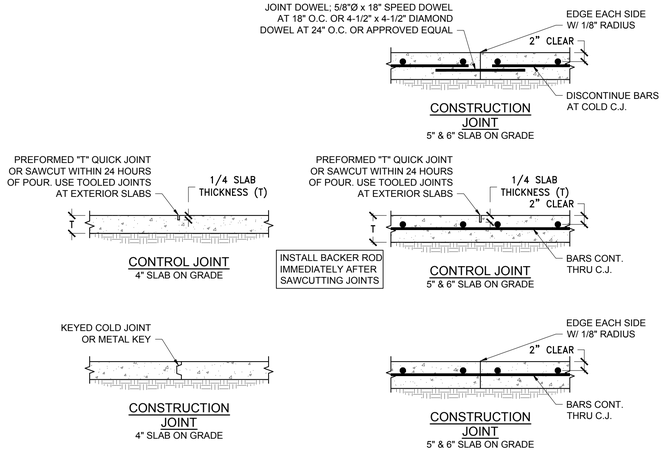


3 HORIZONTAL REINFORCEMENT (TYP.)
SCALE: NONE



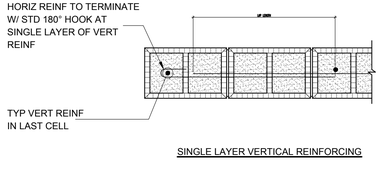
4 SLAB ON GRADE CONTROL JOINTS
SCALE: NONE

5 PIPE PARALLEL TO FOOTING DETAIL
SCALE: NONE

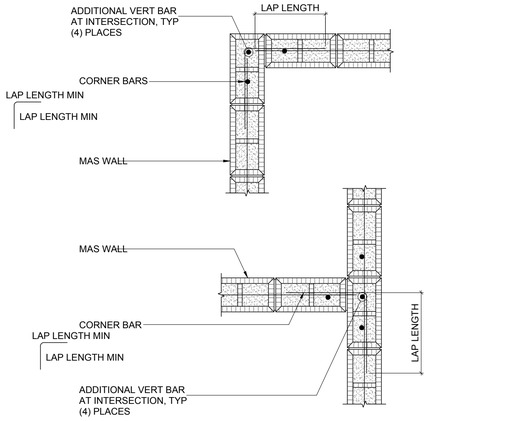
6 PIPE PERPENDICULAR TO FOOTING DETAIL
SCALE: NONE

7 TYPICAL MASONRY WALL RECESS DETAILS FOR SINGLE REINF. MASONRY WALLS (PLAN VIEW)
SCALE: NONE

8 TYPICAL CONTROL JOINT FOR SINGLE REINFORCED MASONRY WALLS (PLAN VIEW)
SCALE: NONE

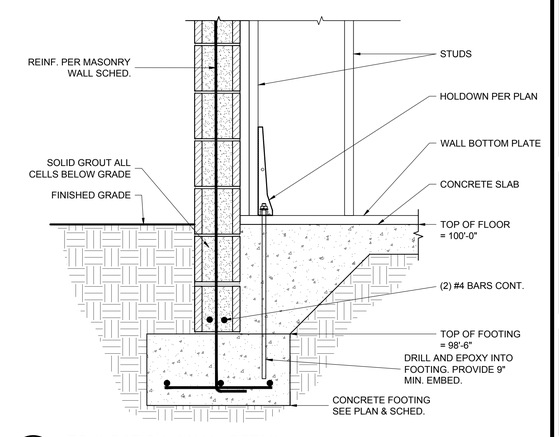


9 TYPICAL TERMINATION OF HORIZ. REINF. FOR SINGLE REINF MASONRY WALLS (PLAN VIEW)
SCALE: NONE



10 TYPICAL CORNER WALL REINFORCING FOR SINGLE REINFORCED MASONRY WALLS
SCALE: NONE

11 TYPICAL REINF. DETAIL FOR MISCELLANEOUS MASONRY WALL OPENINGS AND RECESSES
SCALE: NONE



12 FOUNDATION WALL DETAIL
SCALE: NONE

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STRUCTURAL DETAILS

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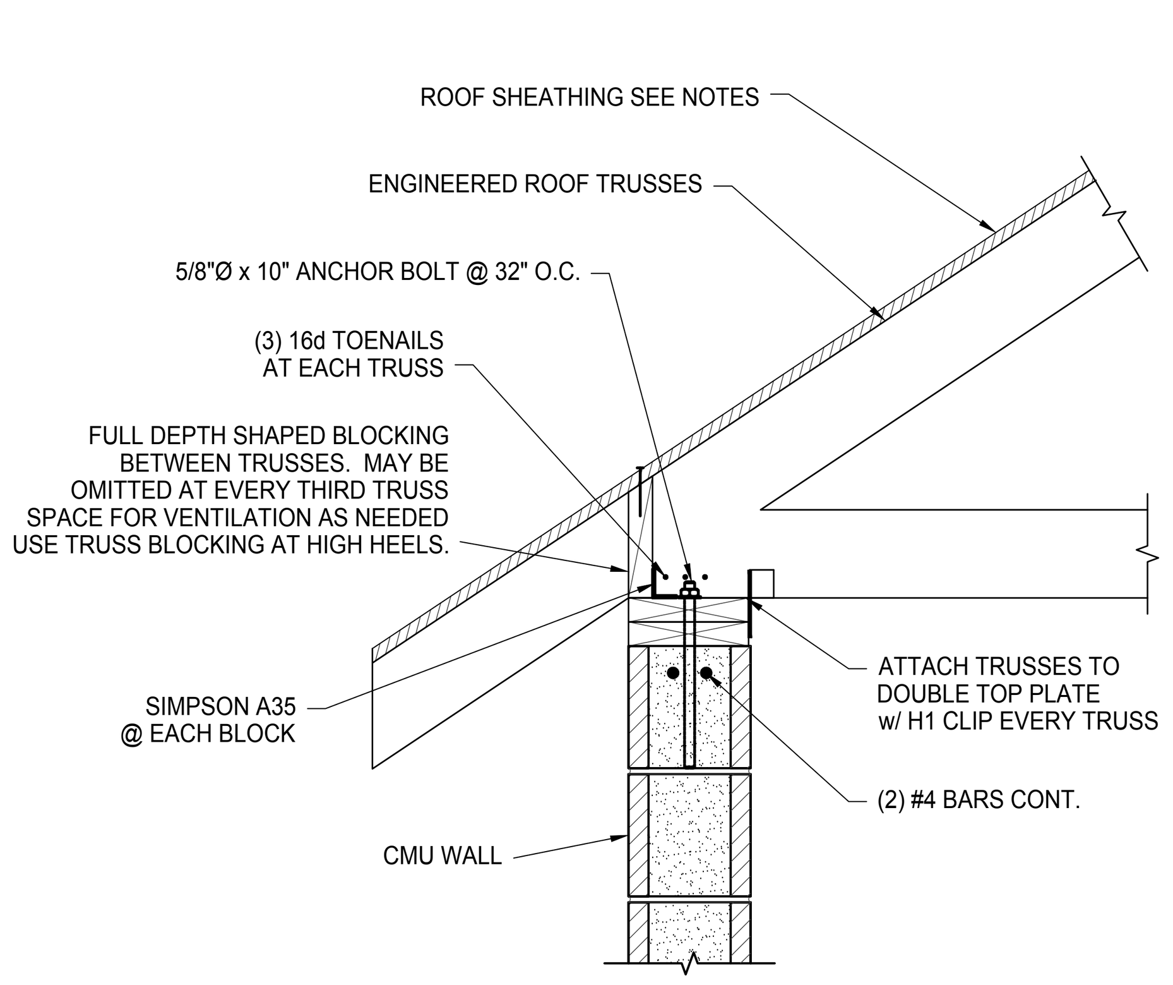
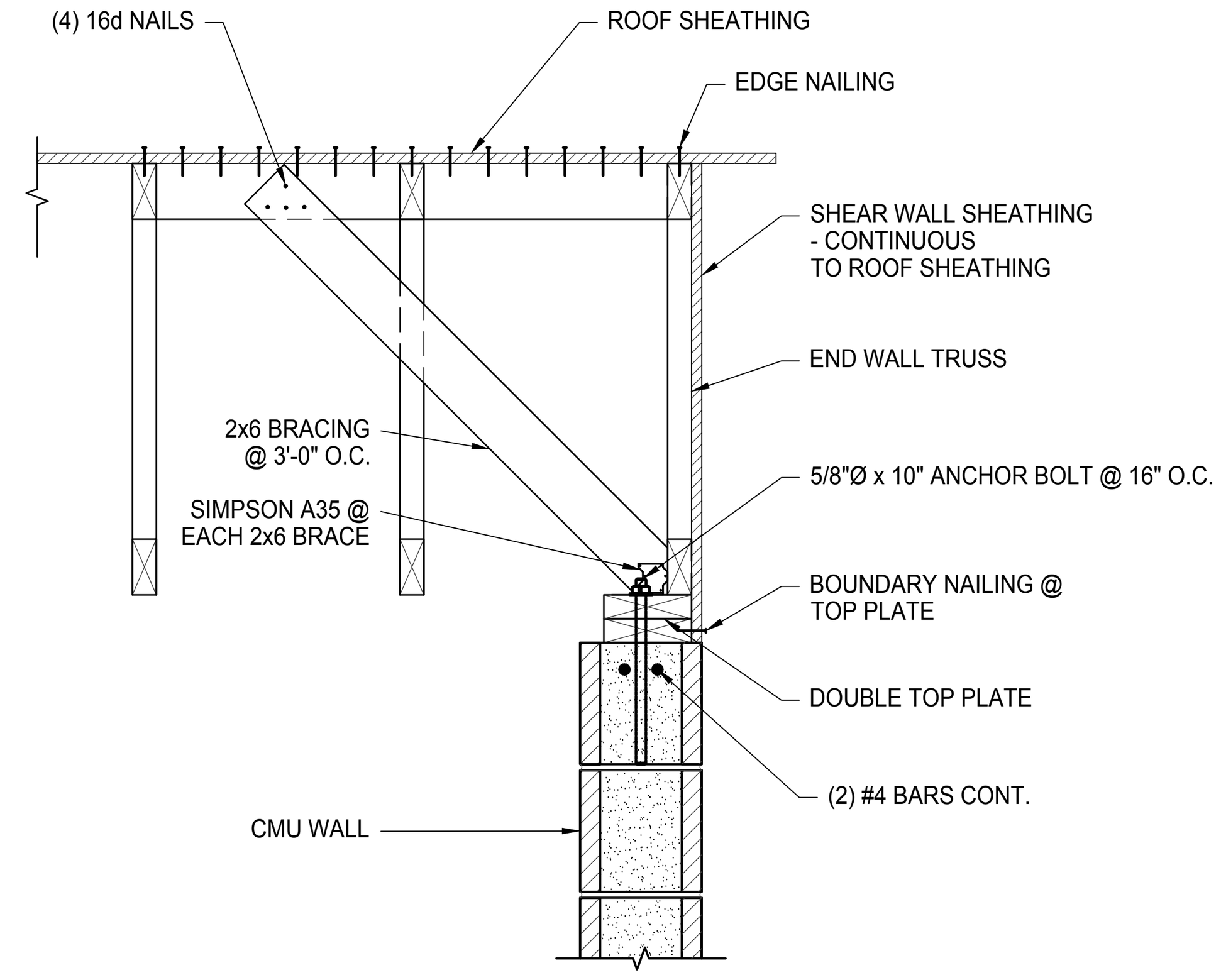
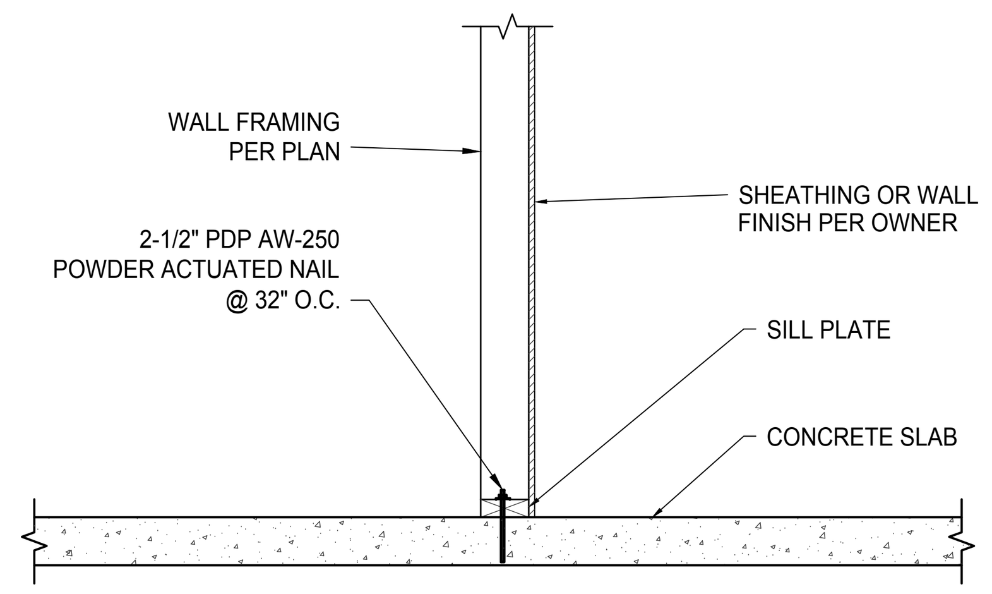


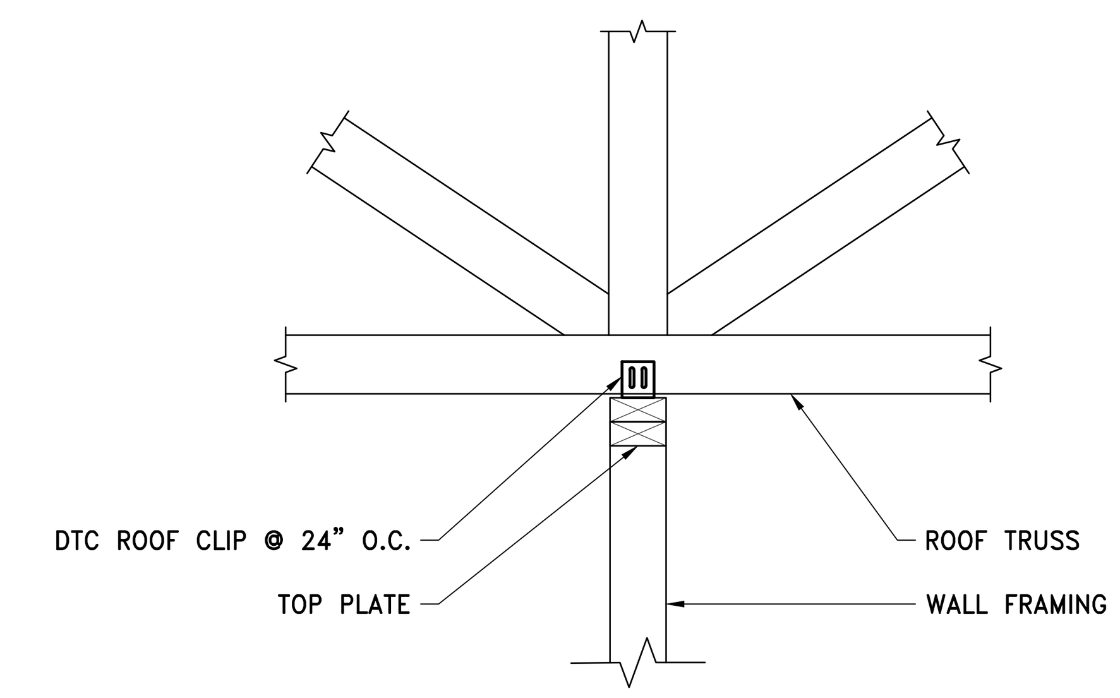
1 **TRUSS @ EXTERIOR WALL (TYP.)**
SCALE: NONE



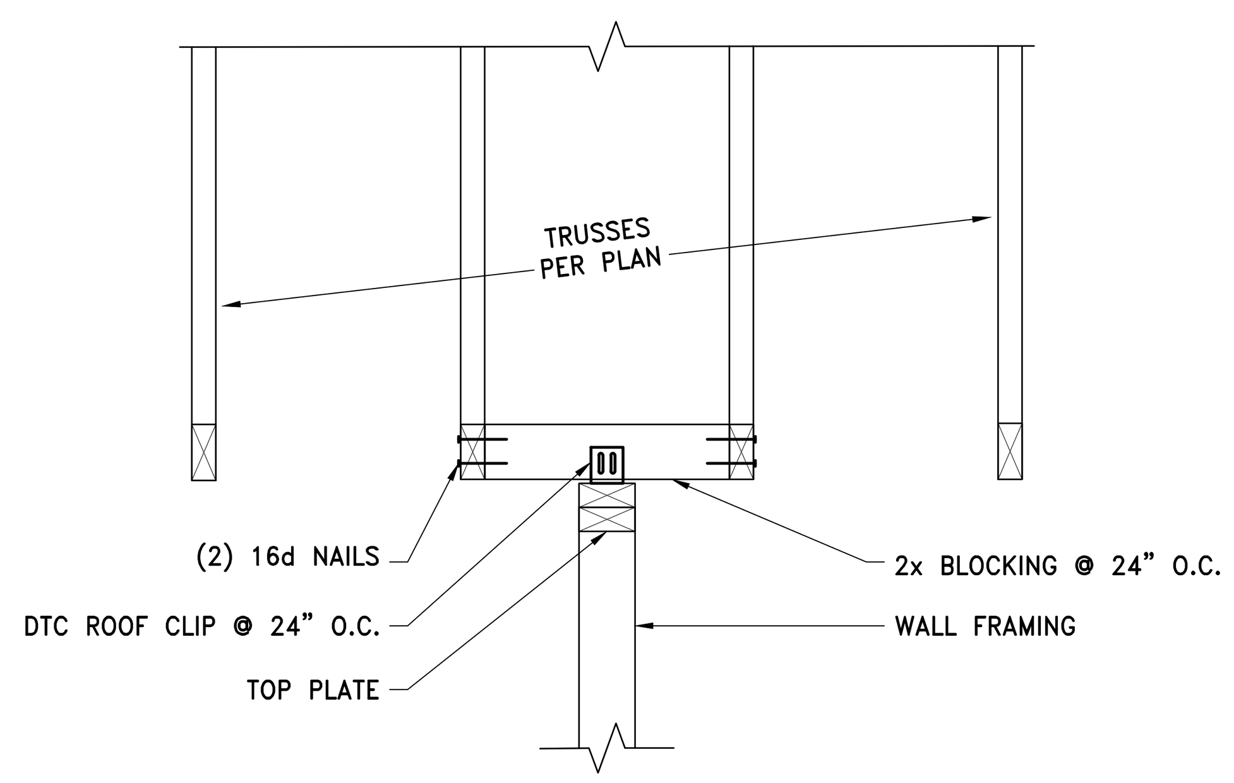
2 **TRUSS PARALLEL TO SHEAR WALL DETAIL**
SCALE: NONE



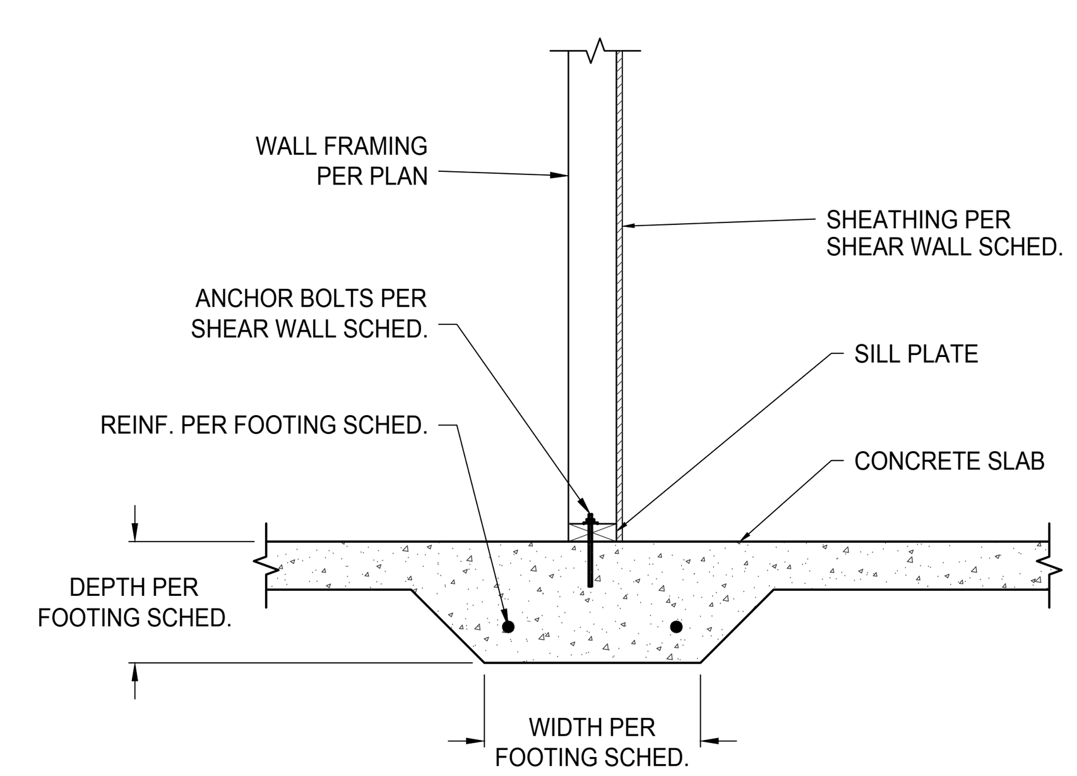
3 **WOOD FRAMED WALL CONNECTION TO CONCRETE SLAB**
SCALE: NONE



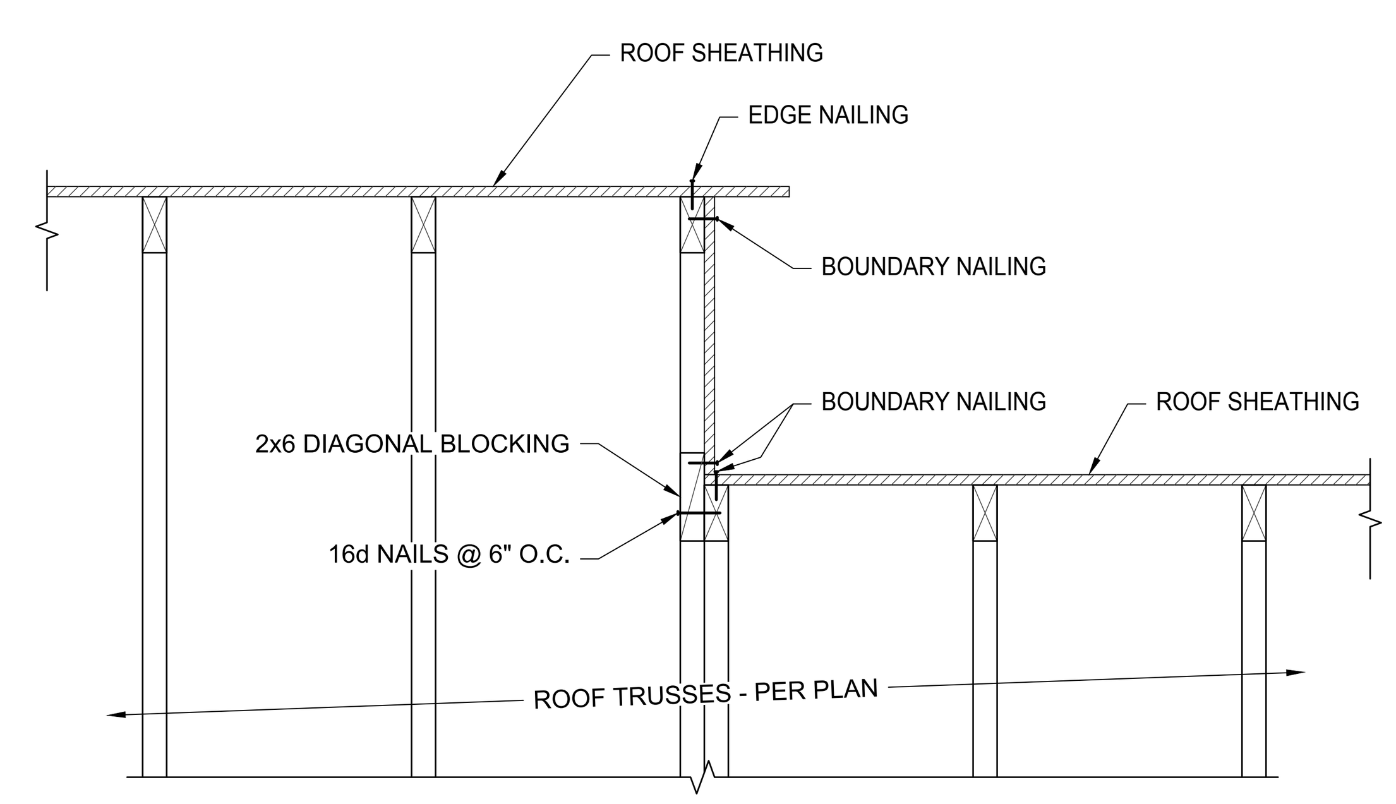
4 **WALL CONNECTION PERPENDICULAR TO TRUSS**
SCALE: NONE



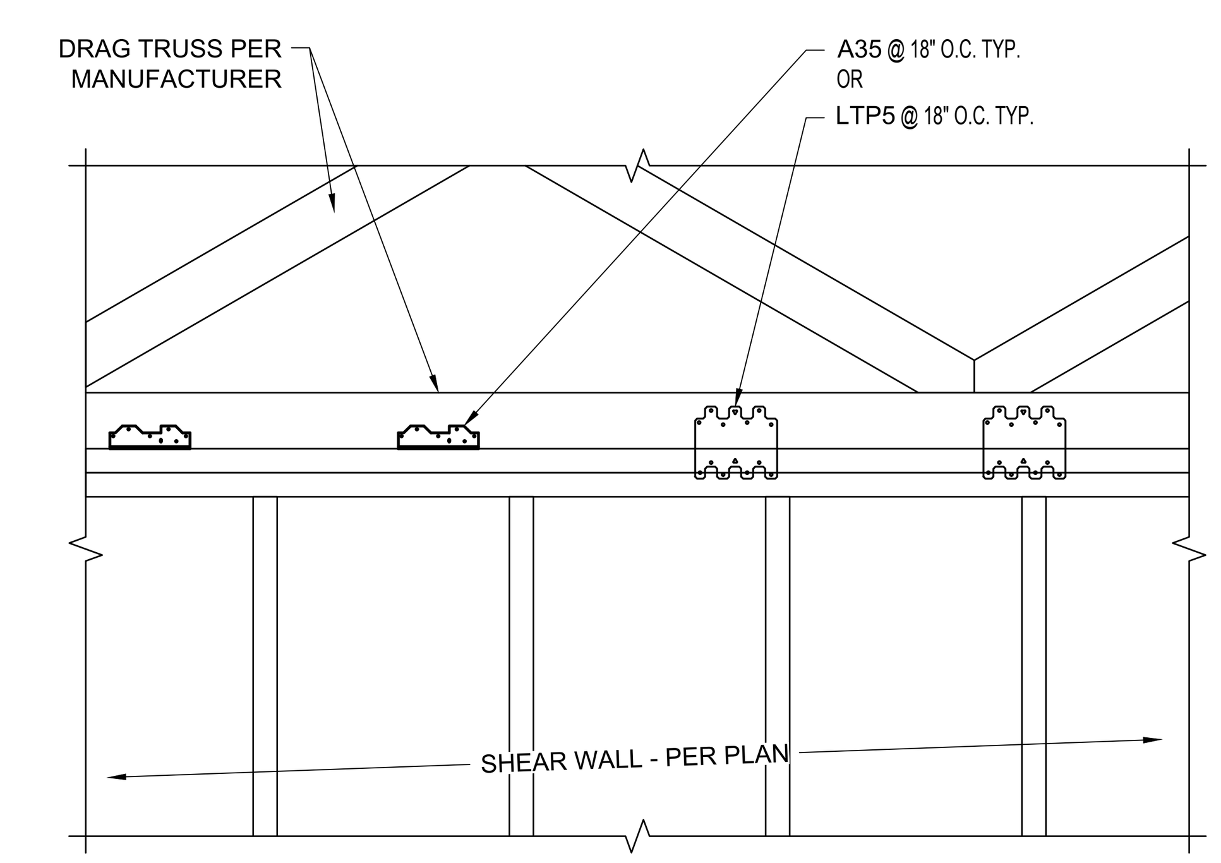
5 **WALL CONNECTION PARALLEL TO TRUSS**
SCALE: NONE



6 **WOOD FRAMED WALL SHEAR WALL DETAIL**
SCALE: NONE



7 **ROOF TRUSSES AT ROOF STEP DETAIL**
SCALE: NONE



8 **DRAG TRUSS PARALLEL TO SHEAR WALL**
SCALE: NONE

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WANGSGARD STORAGE HUNTSVILLE, UTAH

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S7 Sheets

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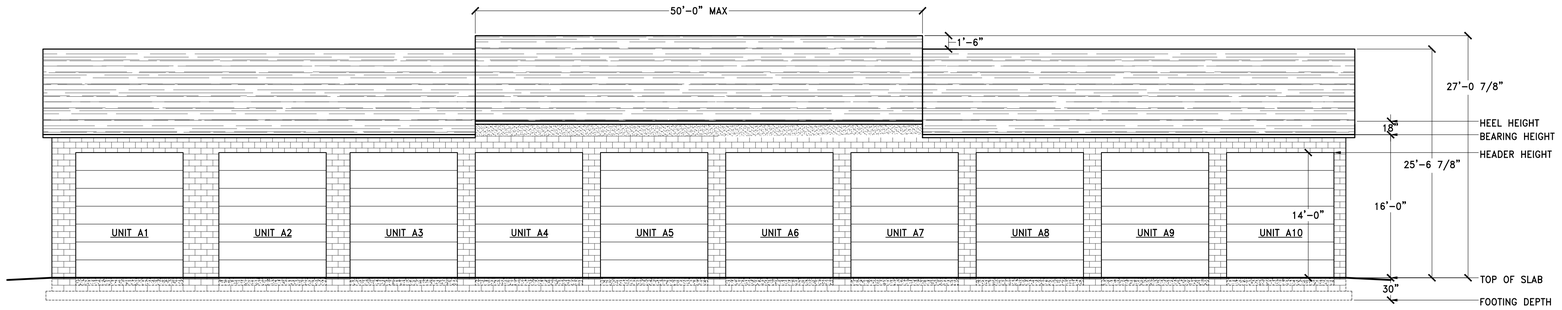
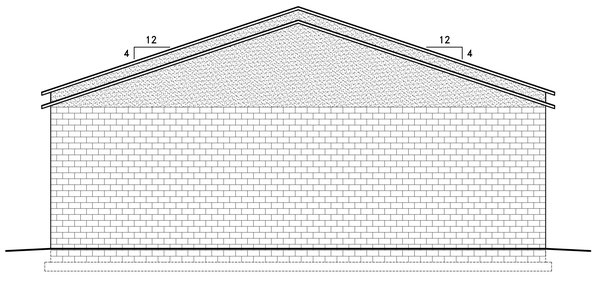


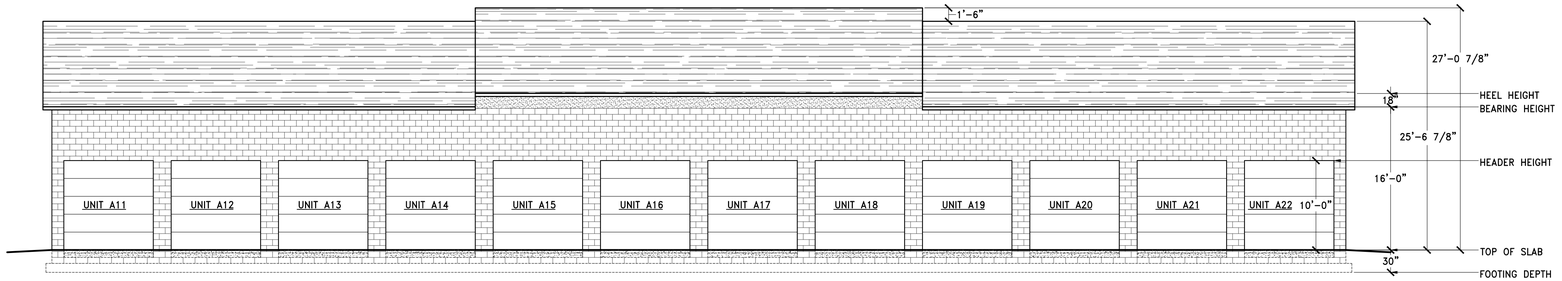
BUILDING A – WEST ELEVATION

SCALE: 1/8" = 1'-0"



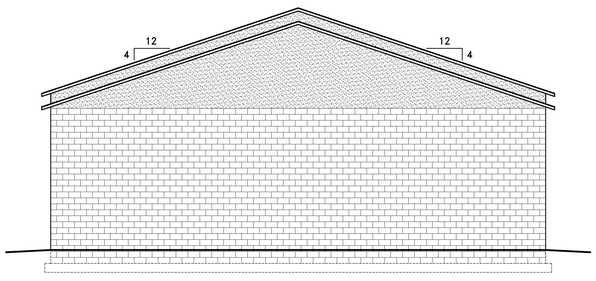
BUILDING A – SOUTH ELEVATION

SCALE: 1/8" = 1'-0"



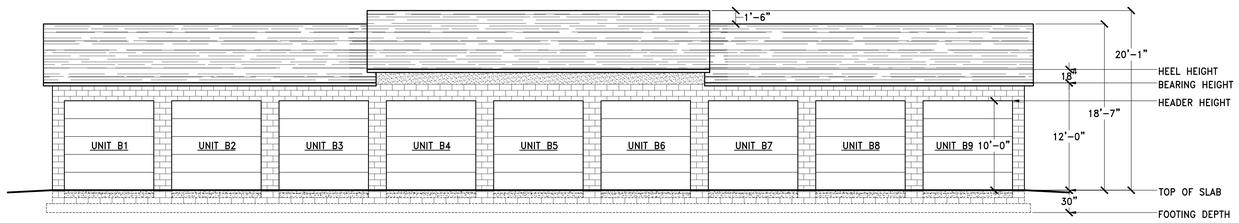
BUILDING A – EAST ELEVATION

SCALE: 1/8" = 1'-0"



BUILDING A – NORTH ELEVATION

SCALE: 1/8" = 1'-0"



BUILDING B – SOUTH ELEVATION

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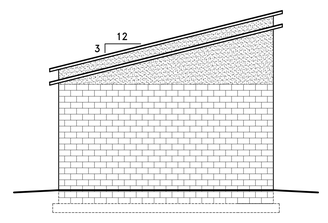

BUILDING B – EAST ELEVATION

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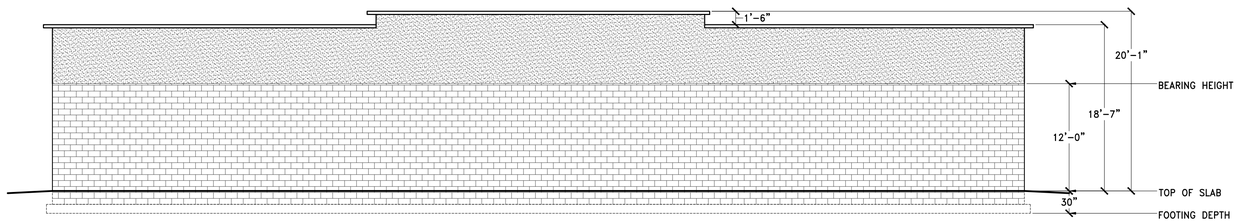
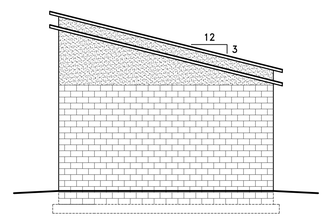


BUILDING B – NORTH ELEVATION

SCALE: 1/8" = 1'-0"



BUILDING B – WEST ELEVATION

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BUILDING ELEVATIONS

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