

**Addendum # 01**

Date: 18 July 2025

**PROJECT:**

IH LRH OR 7 Expansion  
Logan Regional Hospital  
1400 N 500 E, Logan, UT 84341

**OWNER:**

Intermountain Healthcare  
Milt White, Construction Project Manager  
435-770-9328

**ARCHITECT:**

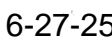
Method Studio  
360 W Aspen Ave.  
Salt Lake City, UT 84101  
801-532-4422

*This Addendum forms a part of the Contract Documents and modifies the original contract documents. Receipt of this Addendum must be acknowledged by the Contractor and Owner.*

**Structural Clarifications:**

- 1) S103A – Roof Framing Plan
  - a) Updated Structural annotation and dimension
- 2) S310 – Structural Details
  - a) Updated detail 10

**END OF ADDENDUM – 01**



THE DESIGNS SHOWN AND DESCRIBED HEREIN INCLUDING ALL TECHNICAL DRAWINGS, GRAPHIC REPRESENTATIONS & MODELS THEREOF, ARE PROPRIETARY & CAN NOT BE COPIED, DUPLICATED, OR COMMERCIALY EXPLOITED IN WHOLE OR IN PART WITHOUT THE SOLE AND EXPRESS WRITTEN PERMISSION FROM METHOD STUDIO INC.

LRH OR 7  
EXPANSION

project#: 24.0520      DS#: 4090125  
date: 05 JUNE 2025

revisions :

1 Addendum 01	26 June 2025
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# GENERAL STRUCTURAL NOTES

S001

CD PROGRESS SET

1. IN ALL CASES, "CONTRACTOR" SHALL REFER TO THE CONTRACTOR OR SUB-CONTRACTOR RESPONSIBLE FOR THE TRADE SPECIFICALLY REFERRED TO IN THE NOTES (A, B, STEEL, ETC.) UNLESS OTHERWISE SPECIFIED. CONTRACTOR SHALL MEET ALL NOTE REQUIREMENTS AND SHALL INCLUDE THE COSTS ASSOCIATED WITH THESE REQUIREMENTS IN HIS/BIDDER THE TOTAL CONTRACT PRICE. CONTRACTOR SHALL DESIGNATE A CONSTRUCTION MANAGER, IS ULTIMATELY RESPONSIBLE FOR COMPLIANCE WITH ALL NOTE REQUIREMENTS.
2. THE CONTRACTOR SHALL PERFORM HIS/HER TRADE AND DUTIES IN A MANNER CONFORMING TO THE PROCEDURES AND REQUIREMENTS AS STATED IN THE 2021 INTERNATIONAL BUILDING CODE (IBC), AND/OR LATEST CODE ADOPTED BY THE LOCAL BUILDING OFFICIAL, AND ALL OTHER ORDINANCES.
3. THE GENERAL CONTRACTOR, OR PROJECT MANAGER, SHALL COORDINATE THE WORK PERFORMED BY ALL TRADES.
4. THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND/OR ARCHITECT OF ANY DISCREPANCIES, OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR THE SPECIFICATIONS BEFORE PROCEEDING WITH ANY WORK THEREON. IN THE EVENT OF SUCH DISCREPANCIES, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN AND BE FOLLOWED.
5. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS, DIRECTIONS, SLOPES AND ELEVATIONS ETC. AT THE JOB SITE AND SHALL COORDINATE THESE WITH THE ARCHITECT AND WITH ALL TRADES/STRUCTURAL DRAWINGS. ALL WORK SHALL BE SCALED FOR DIMENSIONS.
6. VISITS TO THE JOB SITE BY REPRESENTATIVES OF THE ENGINEER DO NOT CONSTITUTE APPROVAL OF THE WORK PERFORMED BY THE CONTRACTOR OR HIS SUBCONTRACTORS; THEY ARE MERELY FOR THE PURPOSE OF OBSERVATION.
7. SHOP DRAWINGS FOR ANY FABRICATED COMPONENTS OR COMPONENTS DESIGNED-BY-MANUFACTURER SHALL BE APPROVED BY THE ENGINEER AND ARCHITECT PRIOR TO CONSTRUCTION. SHOP DRAWINGS SHALL BE STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT.
8. THE CONTRACTOR SHALL VERIFY SIZES, LOCATIONS, LOADS, AND EQUIPMENT ANCHORAGE IN THE FIELD WITH THE EQUIPMENT MANUFACTURER (OR SUPPLIER) PRIOR TO FABRICATION OR INSTALLATION OF SUPPORTING STRUCTURES.
9. TEMPORARY SHORING (BRACING) SHALL BE PROVIDED WHERE NECESSARY. SHORING SHALL BE DESIGNED AND CONSTRUCTED BY THE CONTRACTOR (OR SUB-CONTRACTOR) (i.e. WIND) SHORING SHALL REMAIN IN PLACE AS LONG AS MAY BE REQUIRED FOR SAFETY OR UNTIL ALL THE STRUCTURAL ELEMENTS ARE COMPLETED. ALL SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.
10. DURING AND AFTER CONSTRUCTION, THE CONTRACTOR AND OWNER SHALL KEEP LOADS ON THE STRUCTURE WITHIN THE LIMITS OF THE DESIGN LOADS FOR THE OCCUPANCY. SEE STRUCTURAL PLANS AND CALCULATIONS FOR STRUCTURAL DESIGN, LOADINGS AND CRITICAL DETAILS.
11. ANY SPECIFIC NOTATION OR REQUIREMENT BY CONSULTING DOCUMENTS, OR BY THE BUILDING OFFICIAL, OR BY THE IBC, IS THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE ON BEHALF OF THE OWNER.
12. CONTRACTOR SHALL BE RESPONSIBLE FOR SAFETY AND PROTECTION WITHIN AND ADJACENT TO THE JOB SITE.
13. PRIOR APPROVAL, IN WRITING, FROM THE ENGINEER IS REQUIRED FOR ANY DEVIATION FROM THE STRUCTURAL PLANS AND/OR CONSTRUCTION DOCUMENTS. OPTIONAL MEMBER SUBSTITUTIONS, OR ADDITIONAL CONNECTIONS, OR ANY OTHER APPROVAL OF THE ENGINEER, ARCHITECT AND OWNER. FAILURE TO FOLLOW PLANS AND CONSTRUCTION DOCUMENTS CONSTITUTES CHANGE IN PROJECT SCOPE.
14. SEE STRUCTURAL PLANS FOR ADDITIONAL STRUCTURAL NOTES AND REQUIREMENTS.
15. THE ENGINEER RESERVES THE RIGHT TO REQUEST REPLACEMENT OF ANY PORTION OF THE STRUCTURE DEVIATING FROM THE PLANS WHERE WRITTEN PRIOR APPROVAL HAS NOT BEEN OBTAINED. REPLACEMENT OF ANY PORTION OF THE STRUCTURE PRIOR TO CONSTRUCTION OF THE CHANGED PORTION HAS NOT HAPPENED.
16. ALL SITE WORK, GRADING, COMPACTION AND BACKFILL, ETC. SHALL BE DONE IN COMPLIANCE WITH A GEOTECHNICAL REPORT SPECIFIC TO THE SITE. IT IS THE GENERAL RESPONSIBILITY OF THE CONTRACTOR TO OBTAIN AND REVIEW THE REPORT. PRIOR TO CONSTRUCTION, IF NECESSARY, AND SUBMIT A COPY TO THE ENGINEER FOR VERIFICATION.
17. ALL ANCHORING ADHESIVE SHALL BE SIMPSON SET-3G EPOXY OR HILTI HY-200V3 ADHESIVE. ANCHORS SHALL BE INSTALLED PER MANUFACTURERS INSTRUCTIONS. EPOXIED ANCHORS SHALL NOT BE INSTALLED IN CONCRETE LESS THAN 21 DAYS OLD.
18. ALL NON-EPOXIED POST-INSTALLED ANCHORS TO BE SIMPSON STRONG-BOLT 2 WEDGE ANCHORS OR EPOXIED ANCHORS SHALL BE INSTALLED PER MANUFACTURERS INSTRUCTIONS. KWIK-BOLT T22 ANCHORS MECHANICAL ANCHORS SHALL NOT BE INSTALLED IN CONCRETE LESS THAN 7 DAYS OLD.
19. FASTENERS AND ANCHOR BOLTS USED IN PRESERVATIVE-TREATED WOOD SHALL BE HOT DIPPED ZINC COATED GALVANIZED STEEL. THE COATING WEIGHTS SHALL BE IN

1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. ALL WORK SHALL BE IN STRICT ACCORDANCE WITH THE 2021 IBC, ACI 318, AND LOCAL ORDINANCES.
3. CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS PRIOR TO PLACING CONCRETE.
4. CONTRACTOR SHALL COORDINATE WITH MECHANICAL, ELECTRICAL, AND ARCHITECTURAL PRIOR TO PLACING CONCRETE. PROVIDE SLEEVES, BLOCK OUTS, ETC. AS REQUIRED.
5. CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER PLACEMENT OF ALL ANCHOR BOLTS, SLEEVES, AND BLOCK OUTS. ETC. INSTALL PER MANUFACTURERS SPECIFICATIONS.
6. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL FORM WORK, POUR STOPS, ETC. ETC TO CONSTRUCT ALL CONCRETE WORK. SUCH FORM WORK IS NOT NECESSARILY SHOWN ON THE STRUCTURAL PLANS OR DETAILS. THE CONTRACTOR SHALL SPECIFY ALL FORM WORK AND SHALL INCLUDE THE COST FOR SUCH IN HIS/her ORIGINAL BID.
7. CONTRACTOR SHALL PROVIDE ALL SHORING AS REQUIRED.
8. SEE FOUNDATION PLAN FOR ADDITIONAL NOTES AND REQUIREMENTS.
9. **CONCRETE & REINFORCEMENT**
10. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI IN 28 DAYS UNLESS NOTED OTHERWISE. FOOTINGS AND INTERIOR SLABS MAY HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.
11. SEE PROJECT SPECIFICATIONS FOR CONCRETE DESIGN REQUIREMENTS.
12. ALL REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO THE STANDARD SPECIFICATIONS ASTM A615 GRADE 60. REINFORCING STEEL SHALL BE PROPERLY TIED TO PLACE PRIOR TO PLACING CONCRETE.
13. ALL REINFORCING STEEL SHALL BE DETAILLED AND PLACED IN ACCORDANCE WITH THE ACI DETAILING MANUAL AND ACI STANDARDS (LATEST EDITION).
14. ALL SPLICES IN CONTINUOUS CONCRETE REINFORCING BARS SHALL LAP A MINIMUM OF 40 BAR DIAMETERS. ALL SPLICES SHALL BE MADE IN A COMPRESSION ZONE UNLESS NOTED OTHERWISE. CONTINUOUS REINFORCING STEEL TERMINATE WITH A 90 DEG. BEND OR WITH SEPARATE CORNER BARS.

1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. ALL WORK TO BE IN STRICT ACCORDANCE WITH THE 2021 IBC, AISC, AND LOCAL ORDINANCES.
3. ALL DIMENSIONS AND CONDITIONS SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND ERECTION.
4. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
5. SEE ARCHITECTURAL SHEETS FOR DECK BEARING ELEVATIONS. STRUCTURAL STEEL DETAILER SHALL DETERMINE ALL BEARING PLATE ELEVATIONS FROM ARCHITECTURAL DECK ELEVATIONS.
6. SEE ARCHITECTURAL SHEETS FOR ADDITIONAL DIMENSIONS.
7. SEE ARCHITECTURAL FOR ACCESS HATCHES, DRAFT STOPS, ETC.
8. SUBMIT SHOP DRAWINGS OF ALL STRUCTURAL STEEL, STEEL DECKING & MISCELLANEOUS STEEL TO ENGINEER FOR APPROVAL PRIOR TO FABRICATION.
9. SEE FRAMING PLANS FOR ADDITIONAL NOTES AND REQUIREMENTS.
10. STRUCTURAL STEEL
11. ALL WIDE FLANGE MEMBERS TO BE MANUFACTURED UNDER ASTM A992.
12. ALL STRUCTURAL PLATES, CHANNELS & ANGLES TO BE MANUFACTURED UNDER ASTM A36.
13. ALL HSS MEMBERS TO BE MANUFACTURED UNDER ASTM A500 GRADE C.
14. ALL PIPE COLUMNS TO BE MANUFACTURED UNDER ASTM A500 GRADE C.
15. ALL BOLTS FOR STEEL TO STEEL CONNECTIONS TO BE 3/4" DIA. MIN. A325-N HIGH STRENGTH BOLTS, UNLESS NOTED OTHERWISE. BOLT EMBEDDED IN CONCRETE OR MASONRY SHALL BE #16A GRADE 60 UNLESS NOTED OTHERWISE.
16. ALL WELDING AND BOLTING TO MEET APPROVAL OF SPECIAL INSPECTOR AS REQUIRED BY FIELDING OFFICIAL.
17. ALL STEEL SHALL BE PROPERLY PRIMED EXCEPT AREAS THAT REQUIRE FIELD WELDING (ø TOP OF BEAMS).
18. ANY BEARING PLATES NOT DETAILLED SHALL BE: SAME THICKNESS AS FLANGE OF MEMBER, WIDTH SHALL BE WIDTH OF MEMBER PLUS 4" AND DEPTH SHALL BE 6" MIN. BEARING SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO FABRICATION AND INSTALLATION.
19. SEE ARCHITECTURAL, MECHANICAL & ELECTRICAL FOR ADDITIONAL STEEL MEMBERS (BRACKETS, ANGLES, ETC.) REQUIRED.
20. STEEL MEMBERS SHALL NOT BE CUT, DRILLED OR TORCHED FOR PIPES, ETC. UNLESS SPECIFICALLY DETAILLED.
21. ANY MODIFICATION OF STRUCTURAL MEMBERS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS IS NOT PERMITTED WITHOUT PRIOR APPROVAL.
22. ANY CONNECTIONS NOT DETAILED ON STRUCTURAL PLANS SHALL BE PROVIDED BY THE STEEL DETAILER. SHOP DRAWINGS FOR ALL FABRICATED STEEL CONNECTIONS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO FABRICATION AND INSTALLATION.

1. ALL PRODUCTS, DETAILING, FABRICATION AND INSTALLATION SHALL MEET THE REQUIREMENTS OF AISC "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AND THE 2021 INTERNATIONAL BUILDING CODE.
2. ALL STEEL STUDS SHALL BE THE TYPE, SIZE AND GAUGE SHOWN ON THE PLANS.
3. ALL LIGHT GAUGE STEEL STUDS, JOISTS, TRACKS AND COMPONENTS SHALL BE MADE FROM ZINC COATED (550 GRAIN) STEEL MEETING THE REQUIREMENTS OF ASTM A575 GAUGE:

GRADE:	
97 MIL, 68 MIL, 54 MIL, .....	F.Y (MIN) 50 ksi
43 MIL, 33 MIL, .....	F.Y (MIN) 33 ksi
- NOTE: GRADE 50 STEEL TO CONFORM TO ASTM A570 REQUIREMENTS.  
GRADE 33 STEEL TO CONFORM TO ASTM 611 GRADE C REQUIREMENTS.
4. ALL STUDS, TRACK AND ACCESSORIES SHALL BE GALVANIZED OR PRIMED W RUST-INHIBITIVE PAINT, MEETING THE PERFORMANCE REQUIREMENTS OF TT-P-4362.
5. THE PHYSICAL AND STRUCTURAL PROPERTIES LISTED BY AISI SHALL BE CONSIDERED THE MINIMUM FOR ALL FRAMING MEMBERS.
6. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO SUBSTRUCTURE MEMBERS. STEEL REQUIRED FOR AN ANGULAR FIT AGAINST BRUTTING MEMBERS. MEMBERS SHALL BE HELD POSITIVELY IN PLACE UNTIL PROPERLY FASTENED.
7. AXIALLY LOADED STUDS SHALL BE INSTALLED IN A MANNER WHICH WILL ASSURE THAT ENDS OF THE STUDS ARE POSITIONED AGAINST THE INSIDE TRACK WEB. PRIOR TO STUD AND TRACK ATTACHMENT.
8. TRACKS SHALL BE SECURELY ANCHORED TO CONCRETE SUPPORTING STRUCTURE W/ HILTI XU ANCHORING MEMBERS. STEEL WASHER AT 16" O.C. COMPLETE UNIFORM AND LEVEL BEARING SUPPORT SHALL BE PROVIDED FOR THE BOTTOM TRACK. ALL OTHER TYPES OF CONNECTIONS SEE PLANS.
9. AT TRACK BUTT JOINTS, ABUTTING PICES OF TRACK SHALL BE SECURELY ANCHORED TO A TRUSS MEMBER, EITHER, OR THEY SHALL BE BUTT WELDED OR SPICED TOGETHER.
10. TEMPORARILY BRACING SHALL BE PROVIDED UNTIL ERECTION IS COMPLETED.
11. WALL STUD BRIDGING SHALL BE INSTALLED IN A MANNER TO PROVIDE RESISTANCE TO BOTH MINOR AXIS BENDING AND ROTATION BRIDGING ROLLS SHALL BE EQUALLY SPACED NOT TO EXCEED 6' ON CENTER SPACING, UNLESS CONTINUOUS SHEATHING IS PRESENT ON BOTH SIDES OF STUDS FROM TRACK TO TRACK #8.
12. ALL CONNECTORS SHALL BE FIELD SCREWED USING #8, #10 SELF TAPPING SCREWS, OR SHOP WELD STIFFENERS. USE (2) SCREWS MINIMUM FOR EACH CONNECTION. ALL WELDING SHALL CONFORM TO AWS D1.3.
13. ALL SCREWS SHALL HAVE AN EDGE DISTANCE OF 1 1/2" (MIN) U.N.O. AND SHALL BE SPACED MIN. (4") SCREW DIMENSIONS.
14. TORCH CUTTING OF MEMBERS OR HOLES IS NOT PERMITTED.
15. CONTRACTOR MAY SUBSTITUTE MEMBERS OF GREATER STRENGTH THAN SHOWN SUBJECT TO APPROVAL FROM ENGINEER OF RECORD. ALTERNATE CONNECTIONS MAY BE USED UPON REVIEW AND APPROVAL OF ENGINEER.
16. ALL EXTERIOR WALLS TO BE FRAMED W/ 6008162x43 STUDS AT 16" O.C. PROVIDE 2" MINIMUM STANDARD CONNECTIONS AND FRAMING. SPLICES IN AXIALLY LOADED STUDS SHALL NOT BE PERMITTED.
17. PROVIDE SLIP TRACK TYPE CONNECTION ON UNDERSIDE OF ALL STEEL BEAMS.
18. ALL LIGHTWEIGHT STEEL FRAMING SHALL CONFORM TO ASTM A446.
19. CONTRACTOR SHALL PROVIDE ALL ACCESSORIES INCLUDING BUT NOT LIMITED TO TRACKS, SLIP, WEB STIFFENERS, CONNECTIONS AND FRAMING. CONTRACTOR TO COMPLETE A PROPER INSTALLATION AS RECOMMENDED BY THE MANUFACTURER.

1. THE CONTRACTOR SHALL SUBMIT THE FOLLOWING DOCUMENTS TO THE ARCHITECT AND ENGINEER OF RECORD FOR REVIEW AND APPROVAL. THE DOCUMENTS MUST BE PREPARED AND STAMPED BY AN ENGINEER LICENSED IN THE STATE OF UTAH. THE DOCUMENTS MAY BE SUBMITTED AFTER THE BUILDING PERMIT IS ISSUED, BUT MUST BE SUBMITTED AND APPROVED PRIOR TO COMMENCING FABRICATION OR CONSTRUCTION OF THE COMPONENTS.

1. CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS PRIOR TO STARTING DEMOLITION.
2. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AS REQUIRED PRIOR TO ANY PARTIAL DEMOLITION OF EXISTING STRUCTURE.
3. ANY DAMAGE TO THE EXISTING STRUCTURE AS A RESULT OF THE CONTRACTOR OR HIS SUBS, SHALL BE REPAIRED AND/OR REPLACED AT THE CONTRACTORS EXPENSE.
4. CONTRACTOR SHALL PREVENT DEBRIS FROM ENTERING OTHER AREAS OF STRUCTURE WHICH ARE NOT BEING REMOVED.
5. ALL MATERIALS FROM DEMOLITION, SHALL BE REMOVED FROM THE SITE AND PROPERLY DISPOSED OF BY THE CONTRACTOR.
6. THE CONTRACTOR SHALL PROVIDE BARRICADES, WARNING SIGNS, ETC., AS REQUIRED, FOR PROJECT.
7. THE CONTRACTOR SHALL FIELD VERIFY ALL STRUCTURAL CONDITIONS, NEW AND EXISTING AND NOTIFY THE PROJECT ENGINEER OF ANY DISCREPANCIES OR ADDITIONAL ASSISTANCE THAT MAY BE REQUIRED.
8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SAFETY OF ALL WORKMEN, TENANTS, AND PUBLIC AT THE JOB SITE.
9. CONTRACTOR SHALL COORDINATE ALL PHASES OF DEMOLITION AND WORK WITH THE OWNER AND THE ARCHITECT.
10. CONTRACTOR SHALL PROTECT EXISTING TREES, AND OTHER VEGETATION DURING DEMOLITION AND RECONSTRUCTION. SOME TREES AND OTHER VEGETATION MAY HAVE TO BE TEMPORARILY RELOCATED, OR REPLACED.
11. CONTRACTOR SHALL PROPERLY LOCATE AND MARK ALL UTILITIES PRIOR TO DEMOLITION.
12. RELOCATE SPRINKLER LINES AND OTHER UTILITY LINES AS REQUIRED, COORDINATE WITH BUILDING MANAGER PROVIDE AS-BUILT DRAWINGS.

STRUCTURAL OBSERVATIONS SHALL BE MADE BY A REPRESENTATIVE OF DYNAMIC STRUCTURES AT THE FOLLOWING STAGES.

- a. AFTER STEEL ERECTION
- b. AT COMPLETION OF STRUCTURAL COMPONENTS
- c. AT COMPLETION OF 4-WAY PRIOR TO CLOSING THE CEILING
- d.
- e.

AT THE CONCLUSION OF THE WORK INCLUDED IN THE PERMIT, THE STRUCTURAL OBSERVER SHALL SUBMIT TO THE BUILDING OFFICIAL A WRITTEN STATEMENT THAT THE STRUCTURAL OBSERVER HAS MADE AND IDENTIFY ANY REPORTED DEFICIENCIES WHICH, TO THE BEST OF THE STRUCTURAL OBSERVER'S KNOWLEDGE, HAVE NOT BEEN RESOLVED.

GOVERNING BUILDING CODE: 2021 INTERNATIONAL BUILDING CODE (IBC)	
2. FLOOR LIVE LOADING:	
a. HOSPITALS; CORRIDORS ABOVE 1st FLOOR	80 psf
b. HOSPITAL OPERATING ROOMS; LABORATORIES	60 psf
c. HOSPITALS; PATIENT ROOMS	40 psf
3. ROOF LIVE LOADING:	
a. ROOF LIVE LOAD	20 psf
b. ROOF SNOW LOAD	30 psf
1. GROUND SNOW LOAD, Po	43 psf
2. SNOW EXPOSURE FACTOR, Ce	1.0
3. IMPORTANCE FACTOR, I	1.2
3. THERMAL FACTOR, Ct	1.0
4. FLOOR DEAD LOADS:	
a. SELF WEIGHT	57 psf
b. SUPERIMPOSED	20 psf
5. ROOF DEAD LOADS:	
a. FLAT ROOF	25 psf
6. EARTHQUAKE:	
a. RISK CATEGORY	IV
b. SEISMIC DESIGN CATEGORY	D
c. SPECTRAL RESPONSE ACCELERATIONS:	
Ss = 1.04g	
Sus = 0.75g	
Si = 0.34g	
S01 = 0.45g	
d. SOIL SITE CLASS	D
Fa = 1.08	
Fv = 1.95	
e. IMPORTANCE FACTOR, I	1.5
f. DESIGN BASE SHEAR	N/A
g. SEISMIC RESPONSE COEFFICIENT, Cs	0.334
h. ANALYSIS PROCEDURE	EQUIV. LATERAL FORCE
i. BASIC SEISMIC FORCE RESISTING SYSTEM	OMF
j. RESPONSE MODIFICATION FACTOR, R	3.5
7. WIND:	
a. BASIC WIND SPEED (3 SECOND GUST)	N/A



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1 Addendum 01

1 Addendum 01 26 June 2025

title:

# SPECIAL INSPECTION SHEET

sheet:

S002

CD PROGRESS SET

## SPECIAL INSPECTION SCHEDULE

SOILS (IBC 1705.6)				
REQ'D	TASK	INSPECTION FREQUENCY		COMMENTS:
		CONT.	PERIODIC	
	VERIFY ADEQUATE MATERIALS BELOW FOOTINGS		◆	PRIOR TO PLACEMENT OF CONCRETE.
	EXCAVATION EXTEND TO PROPER DEPTH AND MATERIALS		◆	PRIOR TO PLACEMENT OF COMPACTED FILL OR CONCRETE.
	CLASSIFICATION AND TESTING OF FILL MATERIALS		◆	CHECK CLASSIFICATION AND GRADATIONS AT EACH LIFT, BUT NOT LESS THAN ONCE FOR EACH 10,000 FT OF SURFACE AREA.
	VERIFY PROPER FILL MATERIALS, LIFT THICKNESSES AND IN-PLACE DENSITIES	◆		
	VERIFY PROPERLY PREPARED SITE AND SUBGRADE		◆	PRIOR TO PLACEMENT OF CONCRETE.

CONCRETE CONSTRUCTION (IBC 1705.3)				
REQ'D	TASK	INSPECTION FREQUENCY		COMMENTS:
		CONT.	PERIODIC	
X	REINFORCING STEEL PLACEMENT		◆	VERIFY SIZE, CLEARANCES, SPLICES AND PROPER TIES.
	REINFORCING BAR WELDING a. WELDABILITY OF NON ASTM A706 BARS b. SINGLE PASS FILLED WELDS < ⅜ c. ALL OTHER WELDS	◆	◆	
X	CAST IN ANCHORS		◆	VERIFY MIX DESIGN MEETS STRENGTH AND EXPOSURE REQUIREMENTS LISTED ON APPROVED PLANS.
X	POST-INSTALLED ANCHORS a. ADHESIVE ANCHORS INSTALLED HORIZ. or UPWARDLY INCLINED RESISTING SUSTAINED TENSION LOADS b. POST INSTALLED ANCHORS NOT DEFINED IN a.	◆	◆	IN ACCORDANCE WITH APPROVED ICC-ES REPORT. PERIODIC INSPECTIONS ALLOWED IF STATED IN ES REPORT.
X	VERIFY REQUIRED DESIGN MIX		◆	VERIFY MIX DESIGN MEETS STRENGTH AND EXPOSURE REQUIREMENTS LISTED ON APPROVED PLANS.
	SLUMP, AIR + TEMPERATURE TESTS. PREPARE STRENGTH TEST SAMPLES	◆		
	CONCRETE PLACEMENT	◆		INCLUDES SAMPLING FOR AIR, SLUMP, STRENGTH AND TEMPERATURE TECHNIQUES.
	CURING TEMPERATURE MAINTENANCE		◆	
	PRESTRESSED CONCRETE a. PRESTRESSING FORCES b. GROUTING OF BONDED TENDONS	◆ ◆		
	ERECTION OF PRECAST MEMBERS		◆	
	POST-TENSIONED CONCRETE STRENGTH		◆	
	INSPECT FORMWORK		◆	

COLD-FORMED STEEL CONSTRUCTION (IBC 1705.11.2 & 1705.12.3)			
REQ'D	TASK	INSPECTION FREQUENCY CONT.      PERIODIC	COMMENTS:
	COMPONENTS OF WIND AND SEISMIC-FORCE RESISTING SYSTEMS	◆	VERIFY PROPER SCREW ATTACHMENT, BOLTING AND ANCHORING OF SHEAR WALLS, BRACES AND HOLD-DOWNS HAVING A FASTENER SPACING $\leq 4"$ O.C.
	FIELD WELDING OF ELEMENTS OF MAIN LATERAL FORCE RESISTING SYSTEM.	◆	

OTHER THAN STRUCTURAL STEEL (IBC 1705.2.2)				
REQ'D	TASK	INSPECTION FREQUENCY		COMMENTS:
		CONT.	PERIODIC	
	STEEL ROOF & FLOOR DECK:			
	MATERIAL VERIFICATION OF STEEL DECK		◆	IDENTIFICATION MARKINGS PER APPLICABLE ASTM STANDARD
	ROOF AND DECK WELDS		◆	VERIFY THAT WELDS CONFORM TO AWS D1.3.
	WELDING OF REINFORCING STEEL:			
	VERIFICATION OF WELDABILITY (EXCEPT A706 BAR)		◆	VERIFY MATERIAL IS ABLE TO CONFORM TO AWS D1.4.

INSTALLATION OF OPEN-WEB STEEL JOISTS AND GIRDERS (IBC 1705.2.3)				
REQ'D	TASK	INSPECTION CONT.	FREQUENCY PERIODIC	COMMENTS:
	END CONNECTIONS		◆	S.J. 2207.1
	BRIDGING - HORIZONTAL OR DIAGONAL		◆	S.J. 2207.1
	a. STANDARD BRIDGING		◆	
	b. NON-STANDARD BRIDGING		◆	

MASONRY CONSTRUCTION (IBC 1705.4)				
REQD	TASK	INSPECTION FREQUENCY		COMMENTS:
		CONT.	PERIODIC	
	MINIMUM TESTING (TMS - 402/602-16):			
	VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) FOR SELF-CONSOLIDATING GROUT. VERIFICATION OF $F_{cr}$		◆	COMPRESSIVE STRENGTH TESTS PER ASTM C 1019 FOR SLUMP FLOW AND ASTM C 1611 FOR VSI.
			◆	DETERMINE COMPRESSIVE STRENGTH PER UNIT STRENGTH" OR "PRISM TEST AS SPECIFIED IN ARTICLE 1.4.B OF ACI 530.1 PRIOR TO CONSTRUCTION.
	PRIOR TO CONSTRUCTION (TMS - 402/602-16):			
	REVIEW MATERIAL CERTIFICATES, MIX DESIGNS, TEST RESULTS AND CONSTRUCTION PROCEDURES		◆	VERIFY MATERIALS CONFORM TO APPROVED CONSTRUCTION DOCUMENTS, MIX DESIGN, TEST RESULTS, MATERIAL CERTIFICATES, AND CONSTRUCTION PROCEDURES SHOULD BE SUBMITTED FOR REVIEW. MORTAR MIX DESIGNS SHALL CONFORM TO ASTM C 270 WHILE GROUT SHALL CONFORM TO ASTM C 476. MATERIAL CERTIFICATES SHALL BE PROVIDED FOR THE FOLLOWING: REINFORCEMENT; ANCHORS, TIES, FASTENERS, AND METAL ACCESSORIES; MASONRY UNITS; MORTAR AND GROUT MATERIALS. REVIEW COLD-WEATHER OR HOT-WEATHER CONSTRUCTION PROCEDURES.
	AS CONSTRUCTION BEGINS (TMS - 402/602-16):			
	PROPORTIONS OF SITE-PREPARED MORTAR		◆	VERIFY THAT MORTAR IS TYPE AND COLOR SPECIFIED ON APPROVED PLANS, IT CONFORMS TO ASTM C 270, AND IS MIXED PER ARTICLE 2.6.A OF ACI 530.1.
	CONSTRUCTION OF MORTAR JOINTS		◆	VERIFY MORTAR JOINTS MEET ARTICLE 3.3.B OF ACI 530.1.1
	GRADE AND SIZE OF PRE-STRESSING TENDONS AND ANCHORAGES		◆	VERIFY THAT PRE-STRESSING TENDONS CONFORM TO REQUIREMENTS OF ARTICLE 2.4.B AND 2.4.H OF ACI530.1
	LOCATION OF REINFORCEMENT, CONNECTORS AND ANCHORAGES.		◆	VERIFY REINFORCEMENT IS PLACED IN ACCORDANCE WITH ARTICLE 3.4 OF 530.1.
	PRE-STRESSING TECHNIQUE		◆	VERIFY PRE-STRESSING TECHNIQUE CONFORMS TO ARTICLE 3.6.B OF ACI 530.1
	PROPERTIES OF THIN BED MORTAR FOR AAC MASONRY	◆	◆	VERIFY REINFORCEMENT IS PLACED IN ACCORDANCE WITH ARTICLE 3.4 OF 530.1.
	PRIOR TO GROUTING (TMS - 402/602-16):			
	GROUT SPACE		◆	VERIFY GROUT SPACE IS FREE OF MORTAR DROPPINGS, DEBRIS, LOOSE AGGREGATE, AND OTHER DELETERIOUS MATERIALS AND THAT CLEANOUTS ARE PROVIDED PER ARTICLE 3.2.D AND 3.2.F OF ACI 530.1.
	GRADE, TYPE AND SIZE OF REINFORCEMENT, ANCHOR BOLTS AND ANCHORAGES.		◆	VERIFY REINFORCEMENT, JOINT REINFORCEMENT, ANCHOR BOLTS AND VENEER ANCHORS COMPLY WITH APPROVED PLANS AND SECTIONS 1.6 OF ACI 530.
	PLACEMENT OF REINFORCEMENT, CONNECTORS AND ANCHORAGES.		◆	VERIFY REINFORCEMENT, JOINT REINFORCEMENT, ANCHOR BOLTS AND VENEER ANCHORS ARE INSTALLED PER APPROVED PLANS AND ARTICLES 3.2.E, 3.4, 3.4, AND 3.6.A OF ACI 530.1.
	PROPORTIONS OF SITE-PREPARED GROUT.		◆	VERIFY GROUT PROPORTIONS MEET ASTM C 476 AND A SLUMP BETWEEN 8-11 INCHES. SELF-CONSOLIDATED GROUT SHALL NOT BE PROPORTIONED ON SITE.
	CONSTRUCTION OF MORTAR JOINTS		◆	VERIFY MORTAR JOINTS PLACED IN ACCORDANCE WITH ARTICLE 3.3.B OF ACI 530.1.
	DURING CONSTRUCTION (TMS - 402/602-16):			
	SIZE AND LOCATION OF STRUCTURAL ELEMENTS		◆	VERIFY LOCATIONS OF STRUCTURAL ELEMENTS PER APPROVED PLANS AND CONFIRM TOLERANCES MEET ARTICLE 3.3.F OF ACI 530.1.
	TYPE, SIZE AND LOCATION OF ANCHORS, FRAMES, ETC.		◆	VERIFY CORRECT ANCHORAGES AND CONNECTIONS ARE PROVIDED PER APPROVED PLANS AND SECTIONS 1.16.4.3 AND 1.17.1 OF ACI 530.
	WELDING OF REINFORCEMENT	◆		VERIFY CONFORMANCE WITH SECTIONS 2.1.7.7.2, 3.3.3.4 (c) AND 8.3.3.4 (b) OF ACI 530
	APPLICATION AND MEASUREMENT OF PRE-STRESSING FORCE	◆		VERIFY CONFORMANCE WITH ARTICLE 3.6.B OF ACI 530.1
	PLACEMENT OF GROUT	◆		
	PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (<40 F) OR HOT WEATHER (>90 F)		◆	VERIFY COLD-WEATHER CONSTRUCTION COMPLIES WITH ARTICLE 1.8.C OF ACI 530.1 AND HOT WEATHER CONSTRUCTION PER ARTICLE 1.8.D OF ACI 530.1.
	PLACEMENT OF GROUT AND PRE-STRESSING GROUT FOR BONDED TENDONS	◆		VERIFY COMPLIANCE WITH ARTICLE 3.5, 3.6.C OF ACI 530.1
	OBSERVATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND / OR PRISMS.		◆	CONFIRM SPECIMENS/ PRISMS ARE PERFORMED AS REQUIRED BY ARTICLE 1.4 OF ACI 530.1.

WOOD CONSTRUCTION (IBC 1705.11.1)				
REQD	TASK	INSPECTION FREQUENCY		COMMENTS:
		CONT.	PERIODIC	
	COMPONENTS OF WIND AND SEISMIC-FORCE RESISTING SYSTEMS		◆	VERIFY PROPER SCREW ATTACHMENT, BOLTING AND ANCHORING OF SHEAR WALLS, BRACES AND HOLDOWNS HAVING A FASTENER SPACING < 4" O.C.
	FIELD GLUING OF MAIN LATERAL FORCE RESISTING SYSTEM	◆		

## STATEMENT OF SPECIAL INSPECTIONS

1. THE PROJECT OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED BELOW. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. THESE INSPECTIONS SHALL BE CONDUCTED AT THE INSPECTIONS REQUIRED BY THE BUILDING DEPARTMENT OF THE LOCAL JURISDICTION.
2. SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INCLUDE THE WORK INSPECTED, THE DATE OF INSPECTION, AND THE INSPECTION CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE DESIGN CONSULTANT. DISCREPANCIES THAT ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT A PHASE OF THE WORK. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL PRIOR TO THE START OF WORK.
3. SPECIAL INSPECTIONS FOR EACH TASK SHALL BE CARRIED OUT IN COMPLIANCE WITH REQUIREMENTS PER THE CURRENT IBC AND OTHER MATERIAL STANDARDS
- FABRICATION SHOP REQUIREMENTS
4. WHERE FABRICATION OF STRUCTURAL LOAD BEARING MEMBERS AND ASSEMBLIES IS BEING PERFORMED ON THE PREMISES OF A FABRICATOR SHOP, SPECIAL INSPECTIONS REQUIRED BELOW SHALL BE PROVIDED IN THE SHOP DURING THE FABRICATION PROCESS. THIS REQUIREMENT MAY BE EXCEPTED IF THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. A CERTIFICATE SHALL BE REQUIRED TO VERIFY SUCH APPROVAL. AT COMPLETION OF THE FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS COMPLETED AND INSPECTED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DRAWINGS.

REQ'D	TASK	INSPECTION TYPE		COMMENTS:
		Q.C.	Q.A.	
PRIOR TO WELDING (TABLE N5.4-1, AISC 360-16):				
X	VERIFY WELDING PROCEDURES	P	P	
X	MANUFACTURER CERTIFICATIONS	P	P	
X	MATERIAL IDENTIFICATION	O	O	VERIFY TYPE AND GRADE OF MATERIAL.
X	WELDER IDENTIFICATION	O	O	VERIFY THERE IS A SYSTEM IN PLACE TO IDENTIFY THE WELDER WHO HAS WELDED A JOINT OR MEMBER.
X	FIT-UP GROOVE WELDS	O	O	VERIFY JOINT PREPARATION, DIMENSIONS, CLEANLINESS, TACKING AND BACKING.
X	ACCESS HOLES	O	O	VERIFY CONFIGURATION AND FINISH.
X	FIT-UP FILLET WELDS	O	O	VERIFY ALIGNMENT, GAPS AT ROOT, CLEANLINESS OF STEEL SURFACES, TACK WELD QUALITY AND LOCATION.
X	CHECK WELDING EQUIPMENT	O	O	
DURING WELDING (TABLE N5.4-2, AISC 360-16):				
X	USE OF QUALIFIED WELDERS	O	O	VERIFY THAT WELDERS ARE APPROPRIATELY QUALIFIED.
X	CONTROL AND HANDLING OF WELDING CONSUMABLES	O	O	VERIFY PACKAGING AND EXPOSURE CONTROL.
X	CRACKED TACK WELDS	O	O	VERIFY WELDING IS NOT OVER A CRACKED TACK WELD.
X	ENVIRONMENTAL CONDITIONS	O	O	VERIFY WIND SPEED IS WITHIN LIMITS AS WELL AS PRECIPITATION AND TEMPERATURE.
X	WPS FOLLOWED	O	O	VERIFY ITEMS SUCH AS WELDING EQUIPMENT SETTINGS, TRAVEL SPEED, WELDING MATERIALS, SHIELDING GAS TYPE,FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED, AND PROPER POSITION.
X	WELDING TECHNIQUES	O	O	VERIFY INTERPASS AND FINAL CLEANING, EACH PASS IS WITHIN PROFILE LIMITATIONS, AND QUALITY OF EACH PASS.
AFTER WELDING (TABLE N5.4-3, AISC 360-16):				
X	WELDS CLEANED	O	O	VERIFY THAT WELDS HAVE BEEN PROPERLY CLEANED.
X	SIZE, LENGTH AND LOCATION OF WELDS	P	P	
X	WELDS MEET VISUAL ACCEPTANCE CRITERIA	P	P	
X	ARC STRIKES	P	P	
PRIOR TO BOLTING (TABLE N5.6-1 AISC 360-16):				
X	MANUFACTURERS CERTIFICATIONS FOR FASTENERS	O	P	
X	FASTENERS MARKED w/ ASTM REQUIREMENTS	O	O	
X	PROPER FASTENERS SELECTED FOR DETAIL	O	O	
X	PROPER PROCEDURE FOR DETAIL	O	O	
X	CONNECTING ELEMENTS	O	O	
X	PRE-INSTALLATION VERIFICATION TESTING	P	O	
X	PROPER STORAGE OF FASTENERS	O	O	
DURING BOLTING (TABLE N5.6-2 AISC 360-16):				
X	FASTENER ASSEMBLIES	O	O	
	JOINTS SNUG TIGHT PRIOR TO PRETENSIONING	O	O	
X	PROPER WRENCH USAGE	O	O	
	FASTENERS PRETENSIONED	O	O	
AFTER BOLTING (TABLE N5.6-3, AISC 360-16):				
X	STRUCTURAL STEEL DETAILS	P	P	

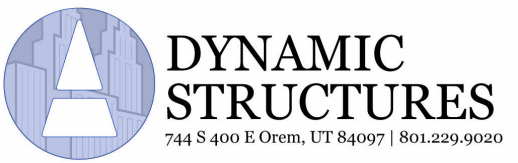
O- OBSERVE THESE ITEMS ON A RANDOM BASIS.

**P- PERFORM THESE TASKS FOR EACH WELDED / BOLTED JOINT OR MEMBER**  
(AISC 360-10 N5.4)

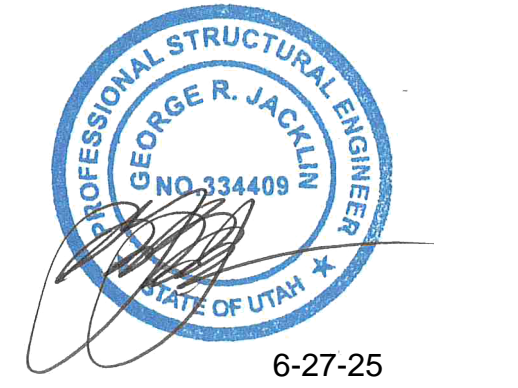




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EXPANSION**

Logan Regional Hospital  
1400 N 500 E  
Logan, UT 84341

project#: 24.0520 DS#: 4090125  
date: 05 JUNE 2025

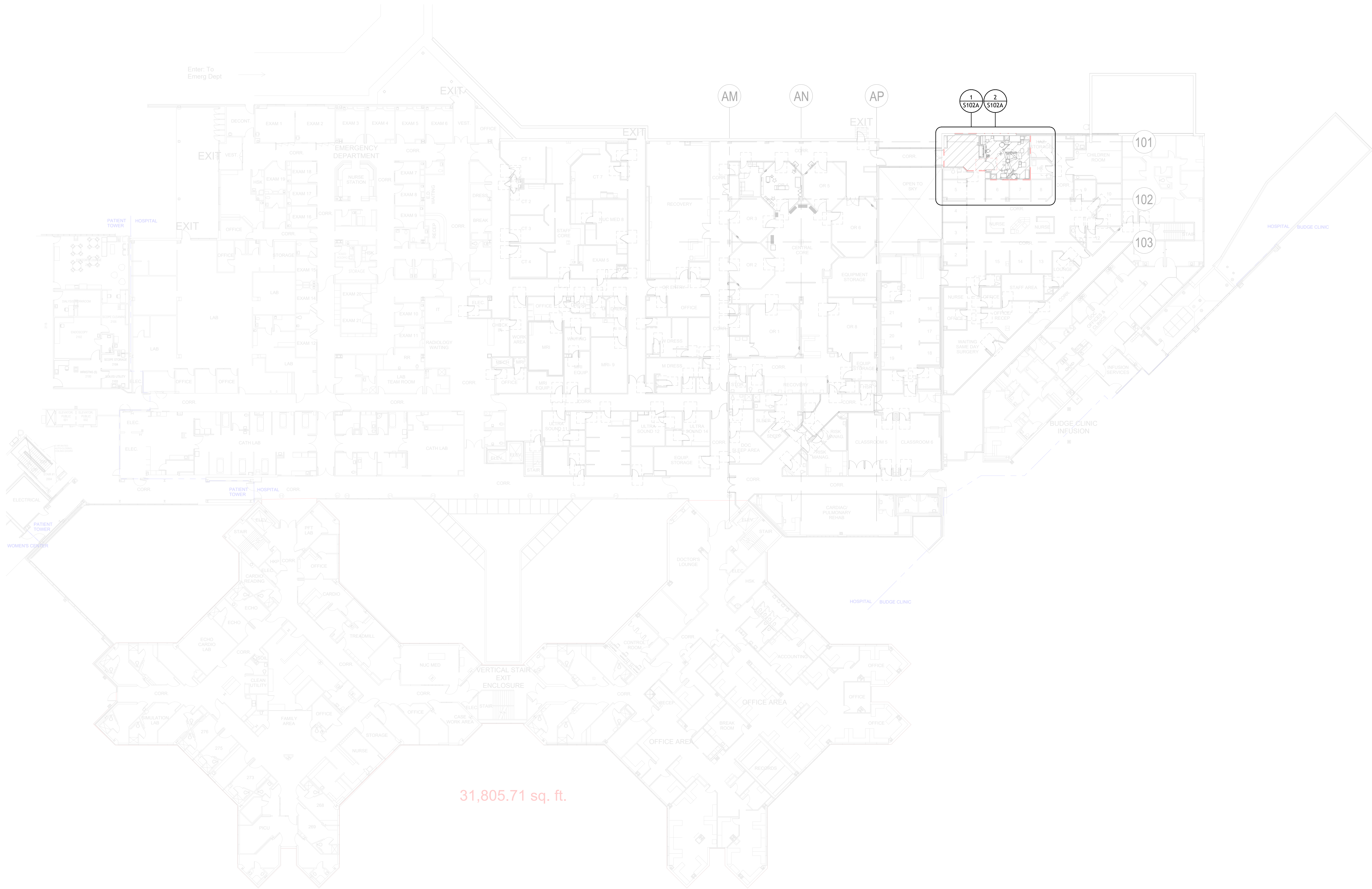
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1 Addendum 01 26 June 2025

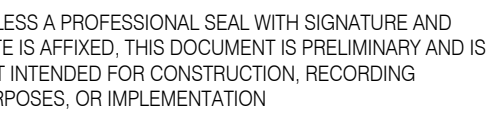
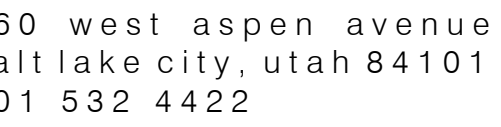
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LEVEL 2**

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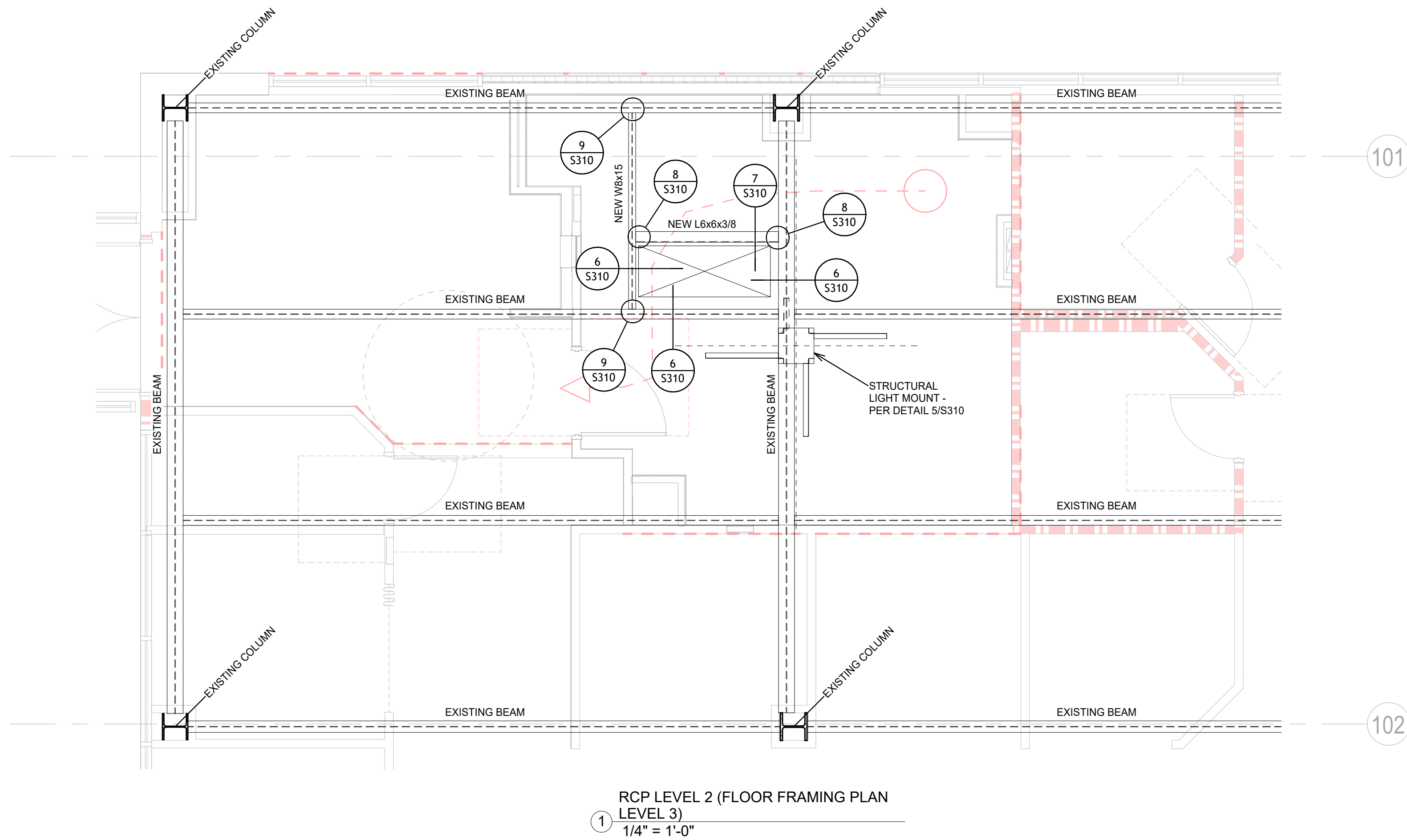
1 Addendum 01 26 June 2025

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LEVEL 2

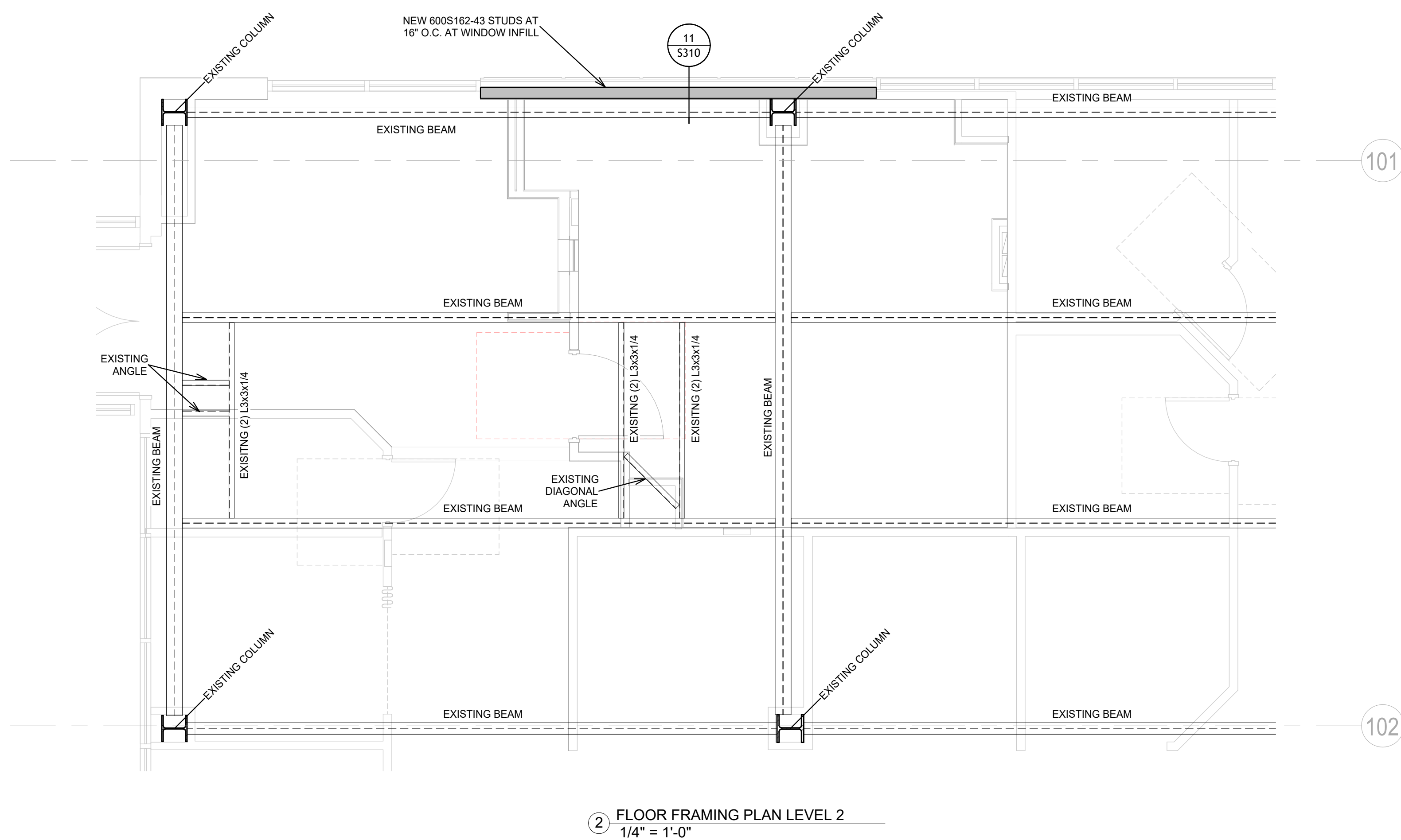
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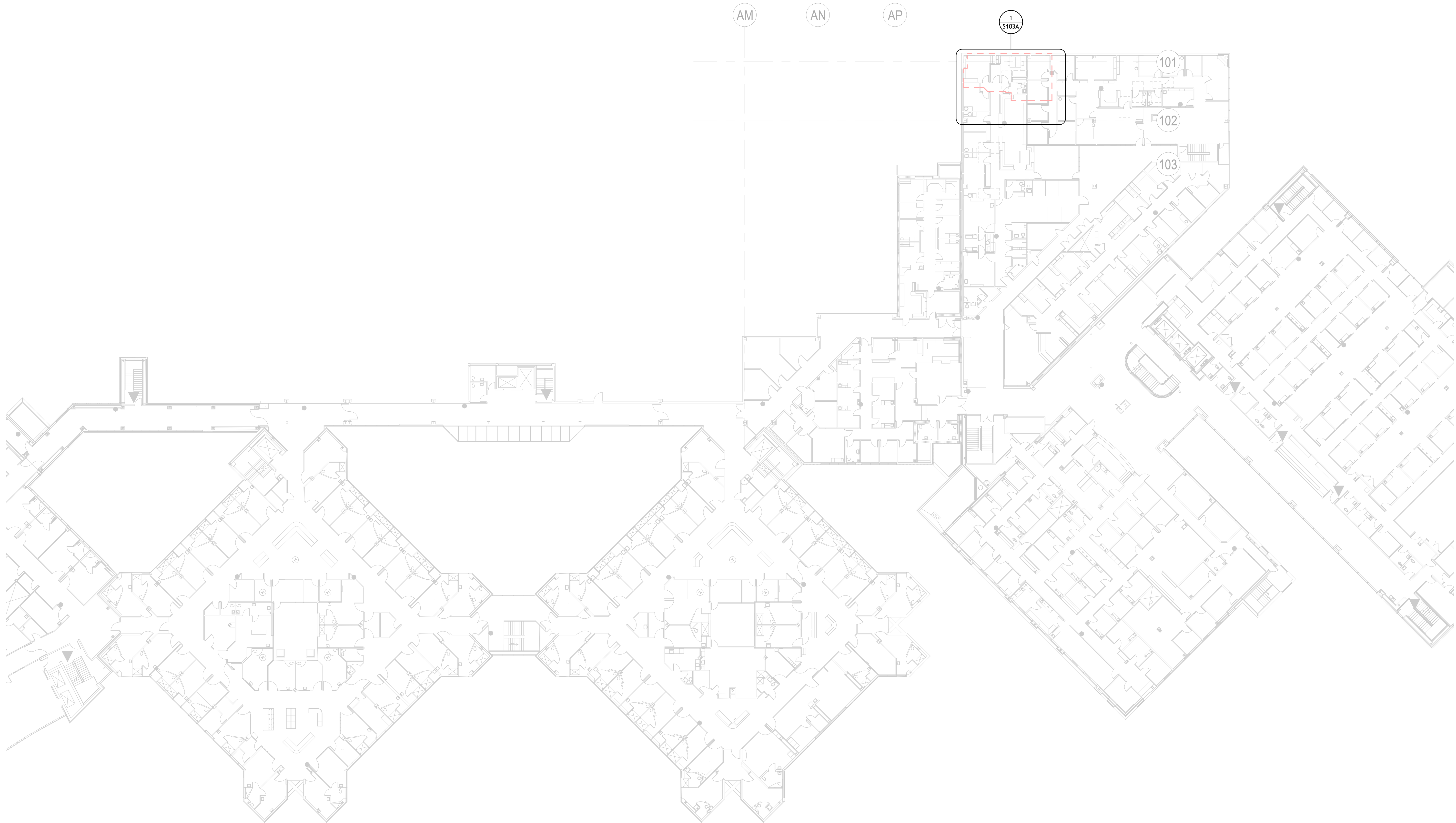


- ## STRUCTURAL NOTES
- ① ALL OF THE BEAMS SHOWN IN THE PLANS ARE EXISTING BEAMS w/ NO ADJUSTMENTS NEEDED - BEAMS SHOWN ARE LOCATED BENEATH THE 3rd Floor.
  - ② EXISTING SLAB IS ASSUMED TO BE 8" TOTAL THICKNESS - CONTRACTOR SHALL VERIFY SLAB THICKNESS IF ACTUAL THICKNESS DIFFERS, CONTACT STRUCTURAL ENGINEER FOR NECESSARY DESIGN REVISIONS
  - ③ CONTRACTOR IS RESPONSIBLE FOR ALL NON-STANDARD ANCHORING. EQUIPMENT SUPPLIER MUST INDICATE WHICH ANCHORS ARE NON-STANDARD (SEE DETAIL 2/ S310)
  - ④ SEE EQUIPMENT INSTALLATION DOCUMENTS FOR STANDARD ANCHORING SPECIFICATIONS AND REQUIREMENTS (SEE DETAIL 1/ S310 FOR STANDARD EQUIPMENT ANCHOR LIMITS)
  - ⑤ ALL POST INSTALLED ANCHORS ARE REQUIRED TO HAVE SPECIAL INSPECTIONS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING WITH THE OWNERS SPECIAL INSPECTOR.
  - ⑥ DEMOLITION OF ALL CONCRETE NEEDED TO BE IN A NON-EVASIVE WAY (NO IMPACT)
  - ⑦ CONTRACTOR CAN CORE DRILL UP TO TWO 6" HOLES IN THE EXISTING CONCRETE FILLED WITH EPOXY (SEE 3/ S310). NO HOLES CAN BE CLOSER THAN 4" HOLE CORE DIAMETERS AWAY FROM ANOTHER IN ANY DIRECTION. STRUCTURAL ENGINEER TO PROVIDE LOCATION AND DEPTH OF CORE DRILLING TO MAKE SURE NO ADDITIONAL STRUCTURAL REQUIREMENTS ARE NEEDED.
  - ⑧ EXISTING MOMENT FRAME PROTECTED ZONE PLEASE CONSULT THE S.E.R. BEFORE ATTACHING ANY THING TO THIS SECTION OF THE BEAM



- ### STRUCTURAL NOTES
- ① ALL OF THE BEAMS SHOWN IN THE PLANS ARE EXISTING BEAMS w/ NO ADJUSTMENTS NEEDED - BEAMS SHOWN ARE LOCATED BENEATH THE 2nd FLOOR
  - ② EXISTING SLAB IS ASSUMED TO BE 6" TOTAL THICKNESS - CONTRACTOR SHALL VERIFY SLAB THICKNESS. IF ACTUAL THICKNESS DIFFERS, CONTACT STRUCTURAL ENGINEER FOR NECESSARY DESIGN REVISIONS
  - ③ CONTRACTOR IS RESPONSIBLE FOR ALL NON-STANDARD ANCHORING. EQUIPMENT SUPPLIER MUST INDICATE WHICH ANCHORS ARE NON-STANDARD (SEE DETAIL 2' S310).
  - ④ SEE EQUIPMENT INSTALLATION DOCUMENTS FOR STANDARD ANCHORING SPECIFICATIONS AND REQUIREMENTS (SEE DETAIL 1' S310 FOR STANDARD EQUIPMENT ANCHOR LIMITS).
  - ⑤ ALL POST INSTALLED ANCHORS ARE REQUIRED TO HAVE SPECIAL INSPECTIONS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING WITH THE OWNER'S SPECIAL INSPECTOR.
  - ⑥ DEMOLITION OF ALL CONCRETE NEEDED TO BE IN A NON-EVASIVE WAY (NO IMPACT)
  - ⑦ CONTRACTOR CAN CORE DRILL UP TO 6" HOLE IN THE EXISTING CONCRETE FILLED METAL DECKING (SEE 3' S310). NO TWO HOLES CAN BE CLOSER THAN 4" HOLE CORE DIMENSIONS AWAY FROM ANOTHER IN ANY DIRECTION. (STRUCTURAL REVIEWER WILL PROVIDE LOCATIONS PRIOR TO CONTRACTOR CORE DRILLING TO MAKE SURE NO ADDITIONAL STRUCTURAL REQUIREMENTS ARE NEEDED).
  - ⑧ ALL EXISTING HOLES NOT BEING USED NEED TO BE FILLED WITH CONCRETE OR NON-SHRINK GROUT SEE 4' S310

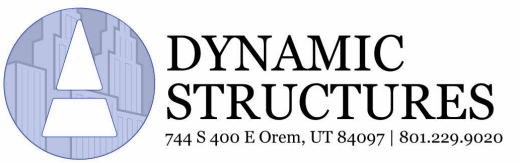




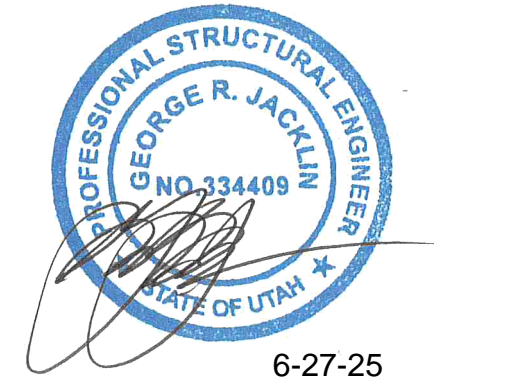
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Logan, UT 84341

project#: 24.0520 DS#: 4090125  
date: 05 JUNE 2025

revisions :  
1 Addendum 01 26 June 2025

title:  
**OVERALL PLAN  
LEVEL 3**

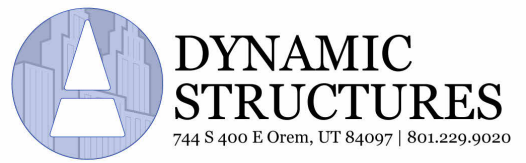
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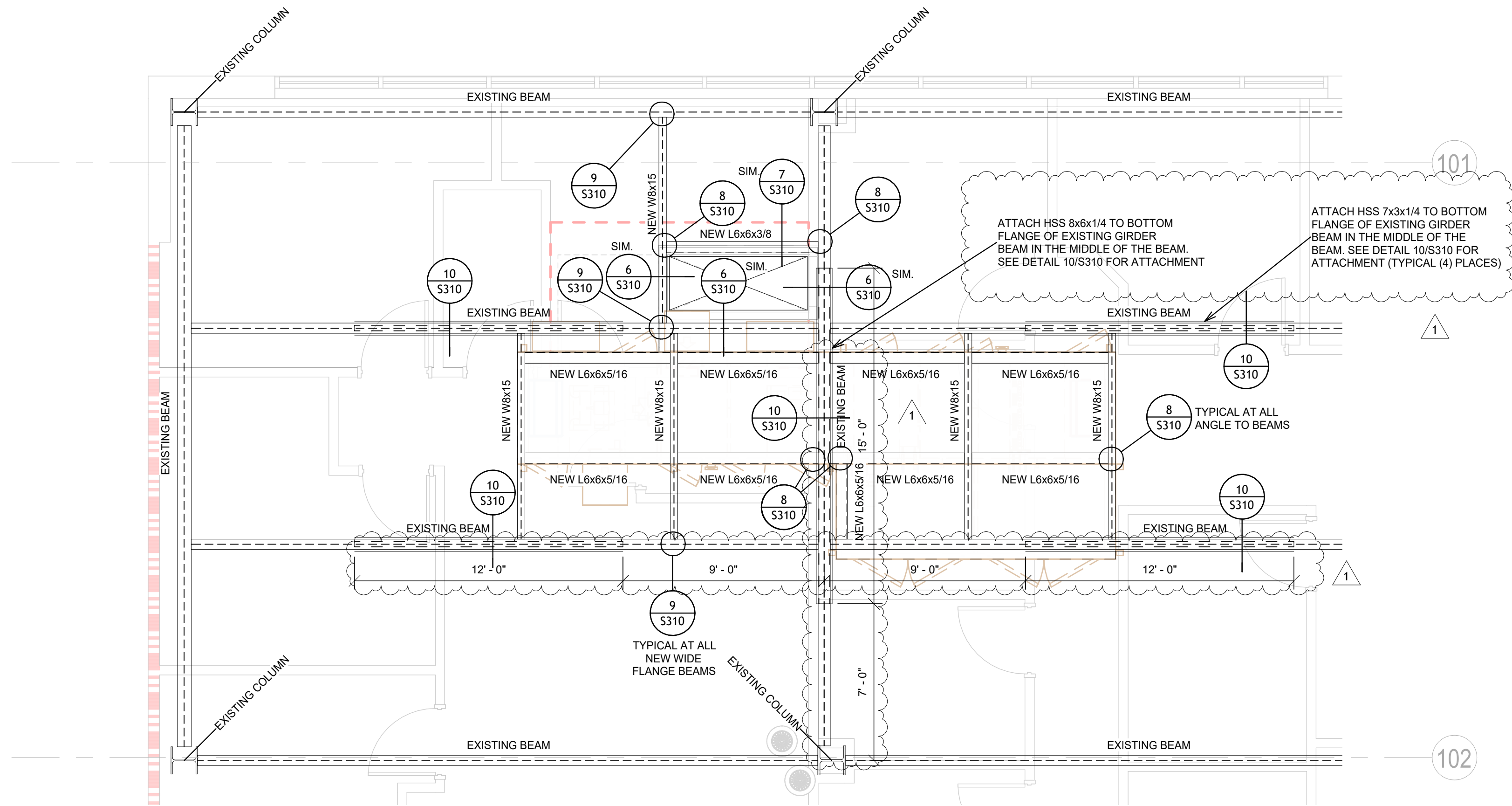
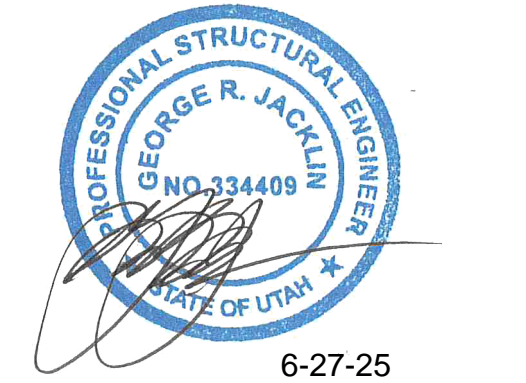
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STRUCTURAL NOTES

- ALL OF THE BEAMS SHOWN IN THE PLANS ARE EXISTING ROOF BEAMS W/ NO ADJUSTMENTS NEEDED - BEAMS SHOWN ARE LOCATED ABOVE THE FLOOR SHOWN
- EXISTING ROOF DECKING IS ASSUMED TO BE 3" TOTAL THICKNESS - CONTRACTOR SHALL VERIFY EXISTING CONDITIONS. IF CONDITIONS DIFFER, CONTACT STRUCTURAL ENGINEER FOR NECESSARY DESIGN REVISIONS
- CONTRACTOR CAN DRILL UP TO AN 8" HOLE IN THE EXISTING METAL DECKING (SEE 3/ S310). NO TWO HOLES CAN BE CLOSER THAN (4) HOLE CORE DIAMETERS AWAY FROM ANOTHER IN ANY DIRECTION. (STRUCTURAL ENGINEER TO REVIEW HOLE LOCATIONS PRIOR TO CONTRACTOR CORE DRILLING TO MAKE SURE NO ADDITIONAL STRUCTURAL REQUIREMENTS ARE NEEDED)

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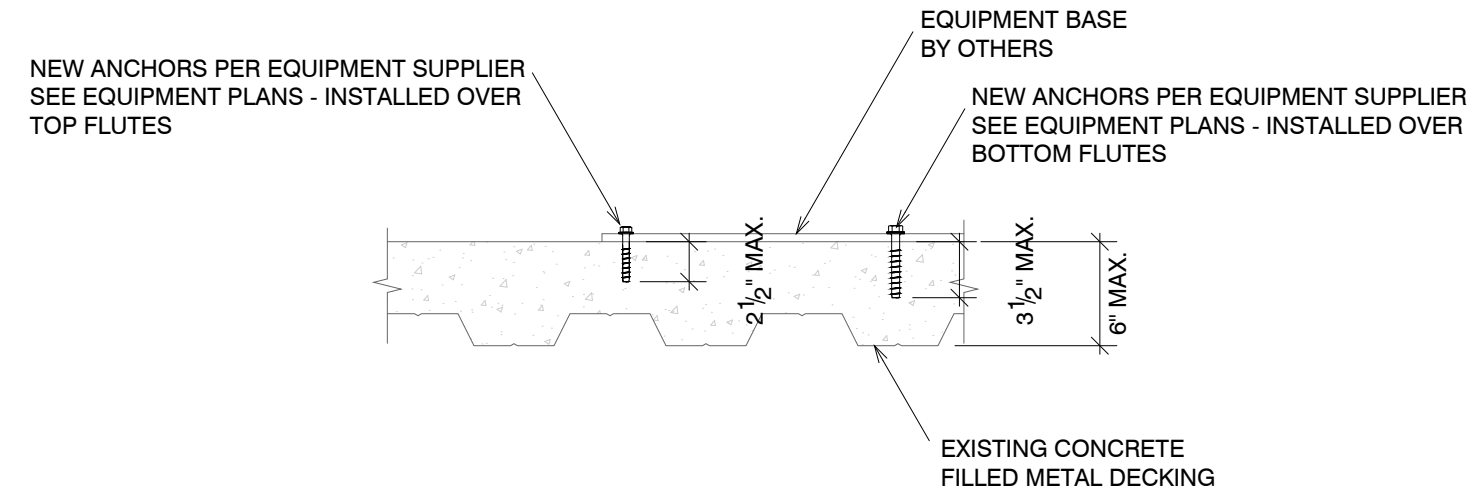
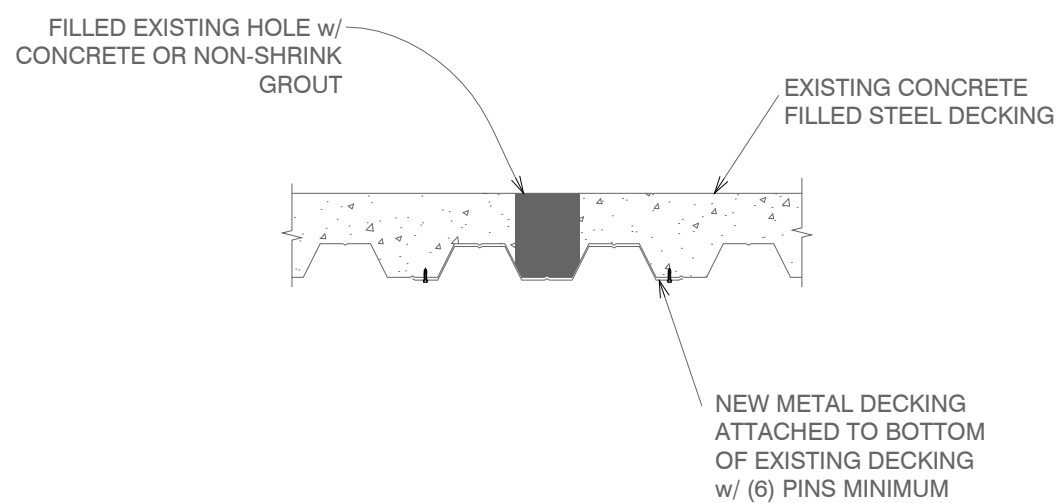
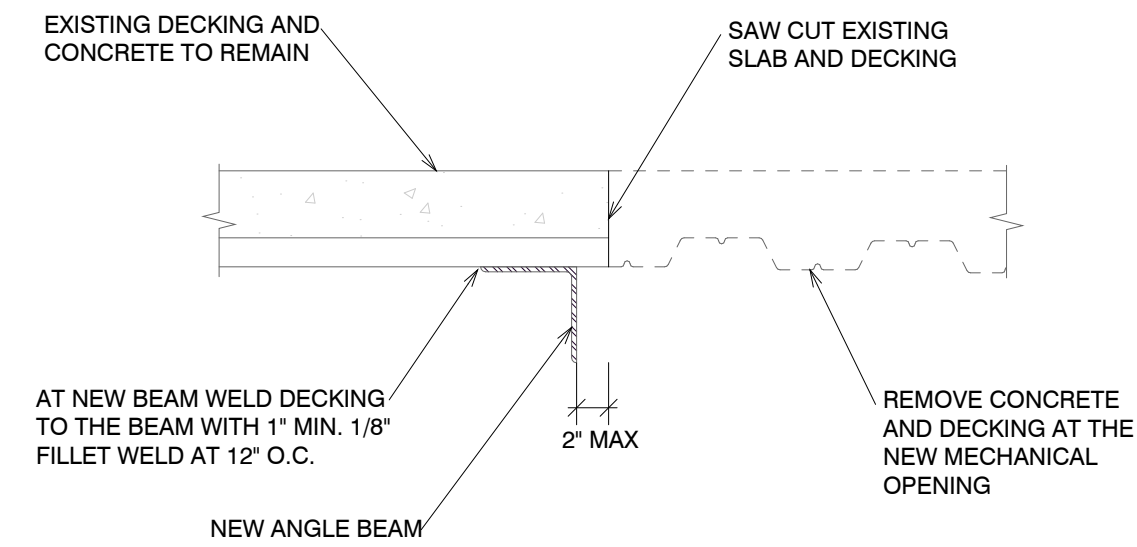
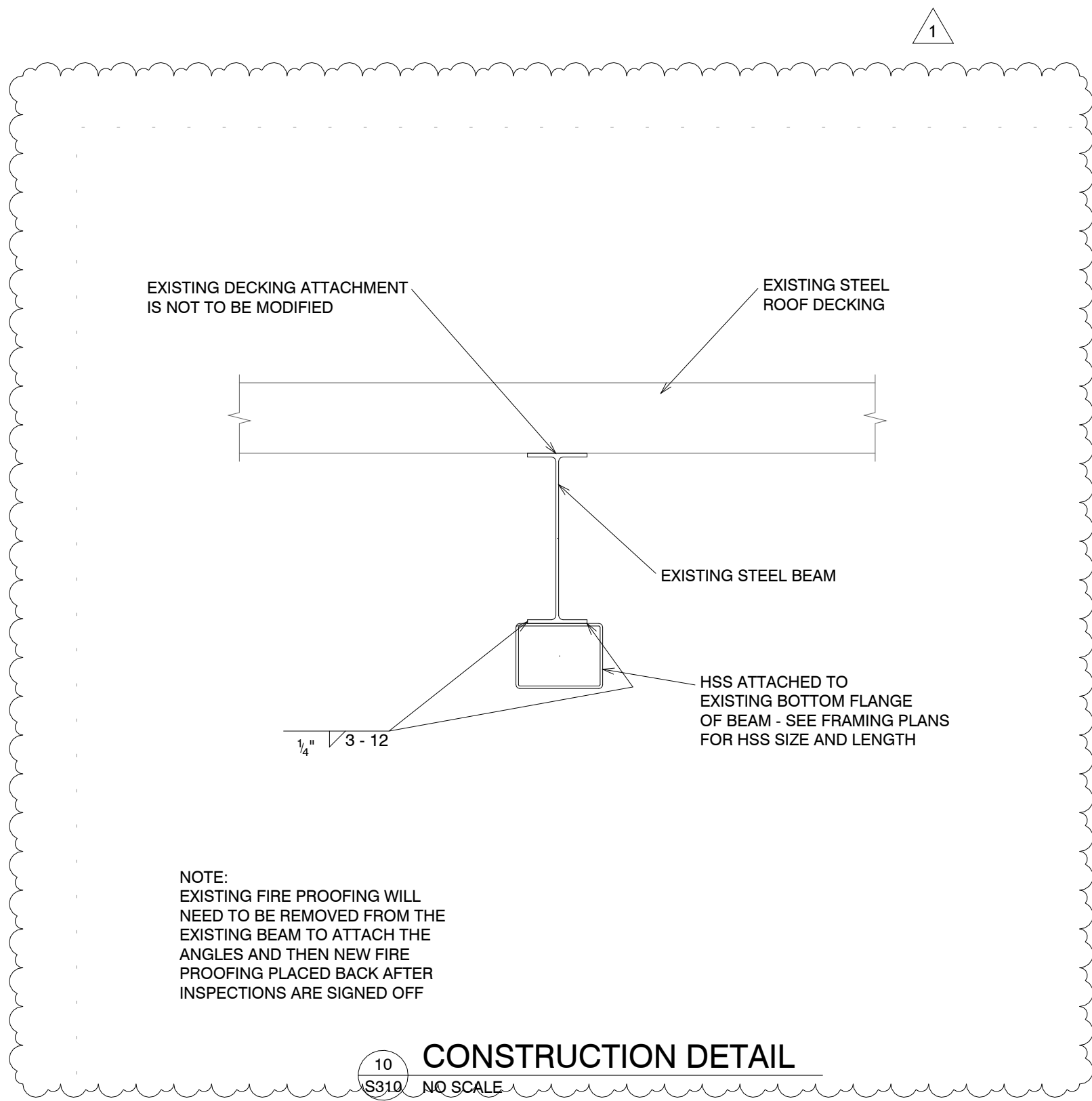
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ROOF  
FRAMING PLAN

Scale: 1/4" = 1'-0"

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NOTE:  
IF THESE PARAMETERS CAN NOT BE MET PLEASE CONTACT THE STRUCTURAL ENGINEER

