

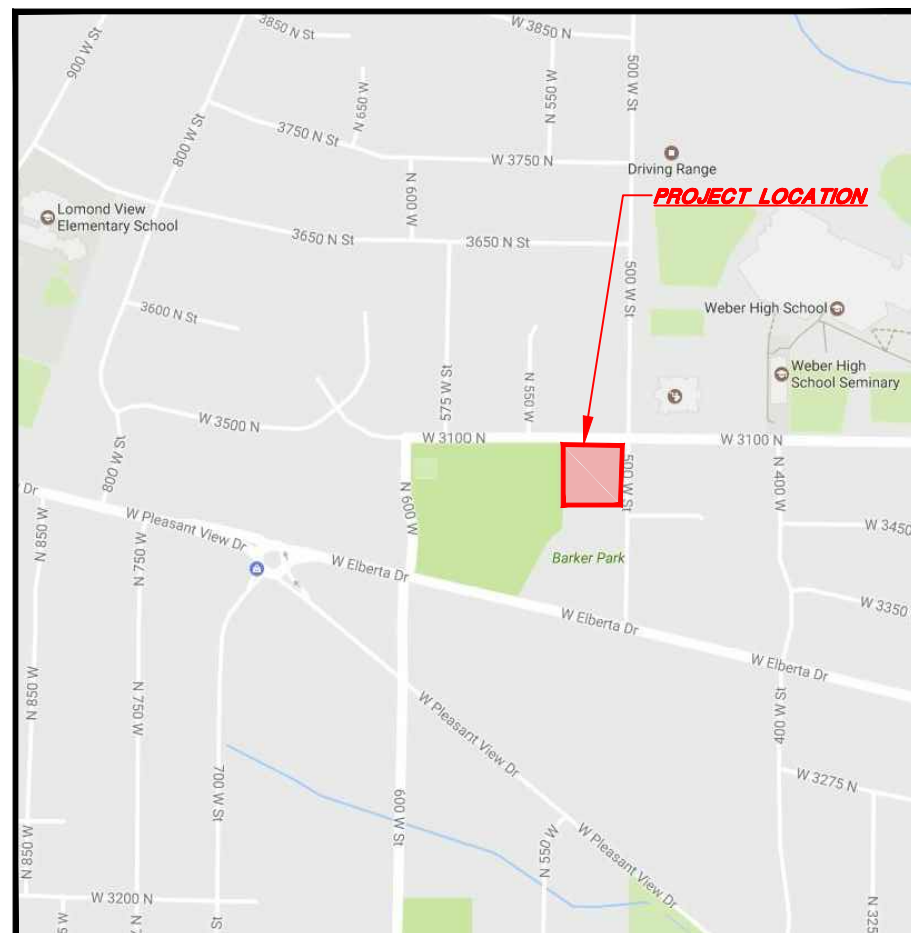
Pleasant View City Corporation

CITY SHOPS PARKING BAY ROOF PROJECT

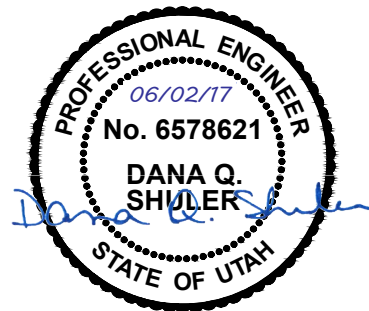
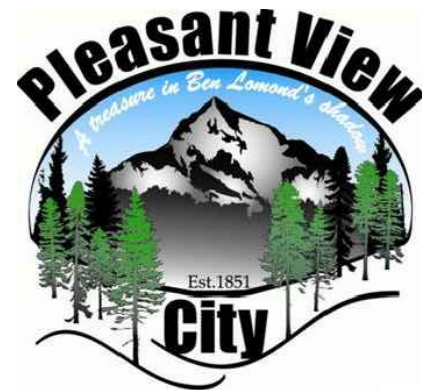
Index

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- C4.....PARKING BAY & RETAINING WALL PLAN (RECORD)
- C5.....RETAINING WALL DETAILS (RECORD)
- S2.....FLOOR PLAN
- S3.....SECTION LINE 1 & 2
- S4.....SECTION LINE 3 & 4
- S5.....SECTION LINE 5
- SP1.....SITE PLAN AND DETAILS

ADDENDUM #1



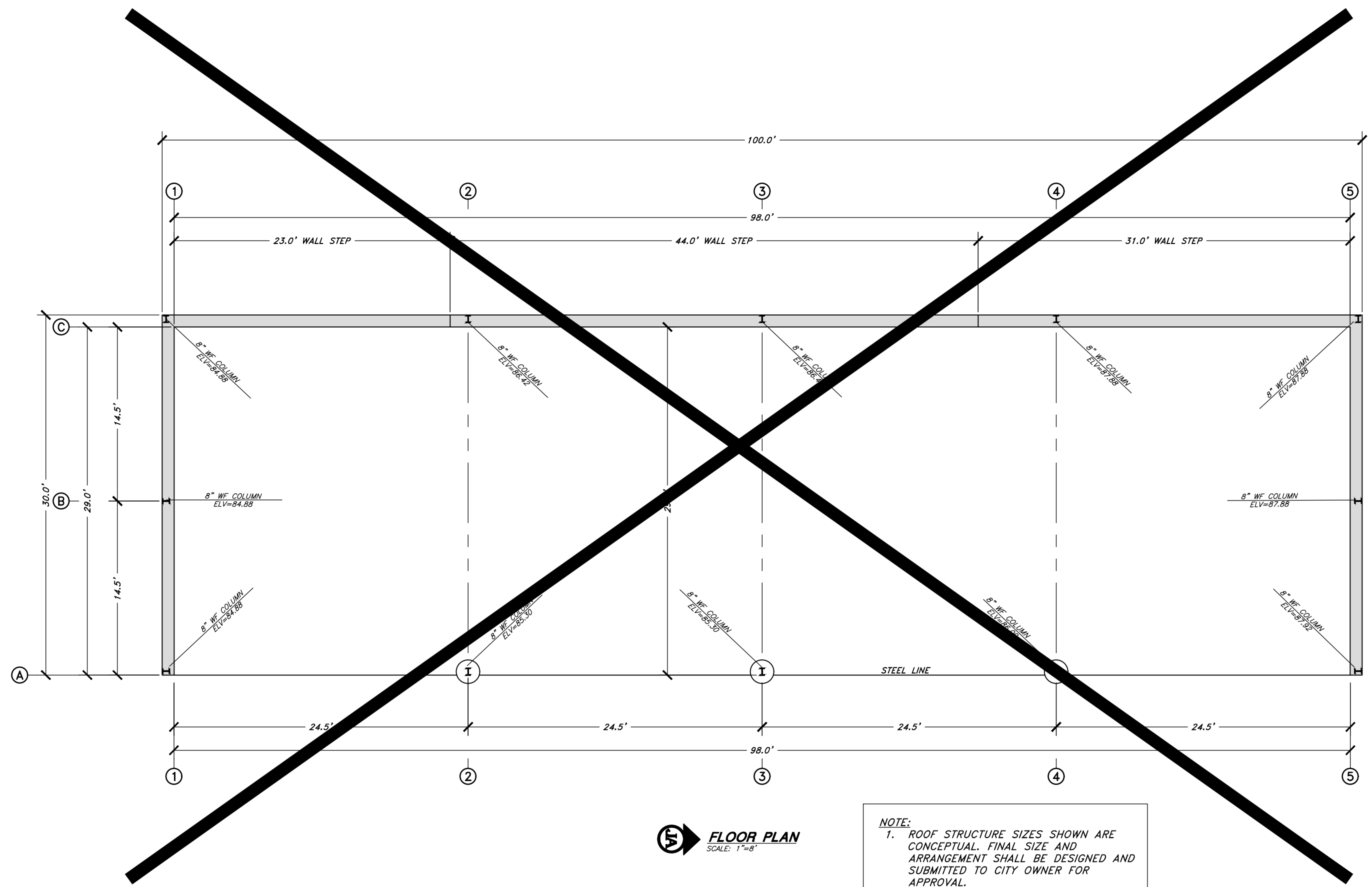
Location Map



MAY 2017

**PLEASANT VIEW CITY CORPORATION
 CITY SHOPS BAY ROOF PROJECT**

FLOOR PLAN



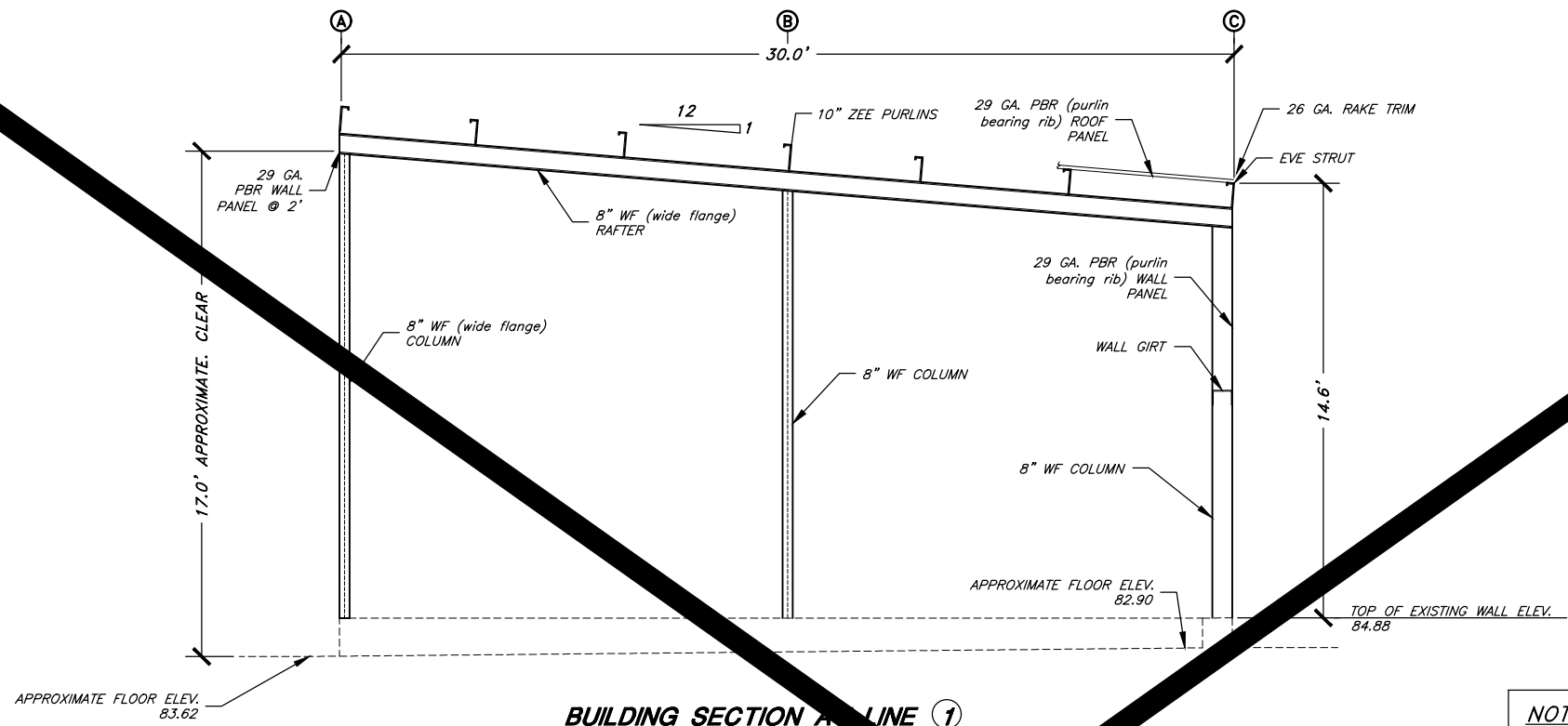
FLOOR PLAN
 SCALE: 1"=8'

NOTE:
 1. ROOF STRUCTURE SIZES SHOWN ARE CONCEPTUAL. FINAL SIZE AND ARRANGEMENT SHALL BE DESIGNED AND SUBMITTED TO CITY OWNER FOR APPROVAL.
 2. ANCHORING PLATE DETAIL SHALL BE DESIGNED AND SUBMITTED TO OWNER FOR APPROVAL.

NO.	DATE	BY	CHKD.	APP.
1	9/1/17	DQS	ADDENDUM #1	

SLS DESIGNED	SLS DRAWN	SLS CHECKED

SCALE: 24" x 36" / 11" x 17"
 SHEET: **S2**
 OF 1 SHEETS

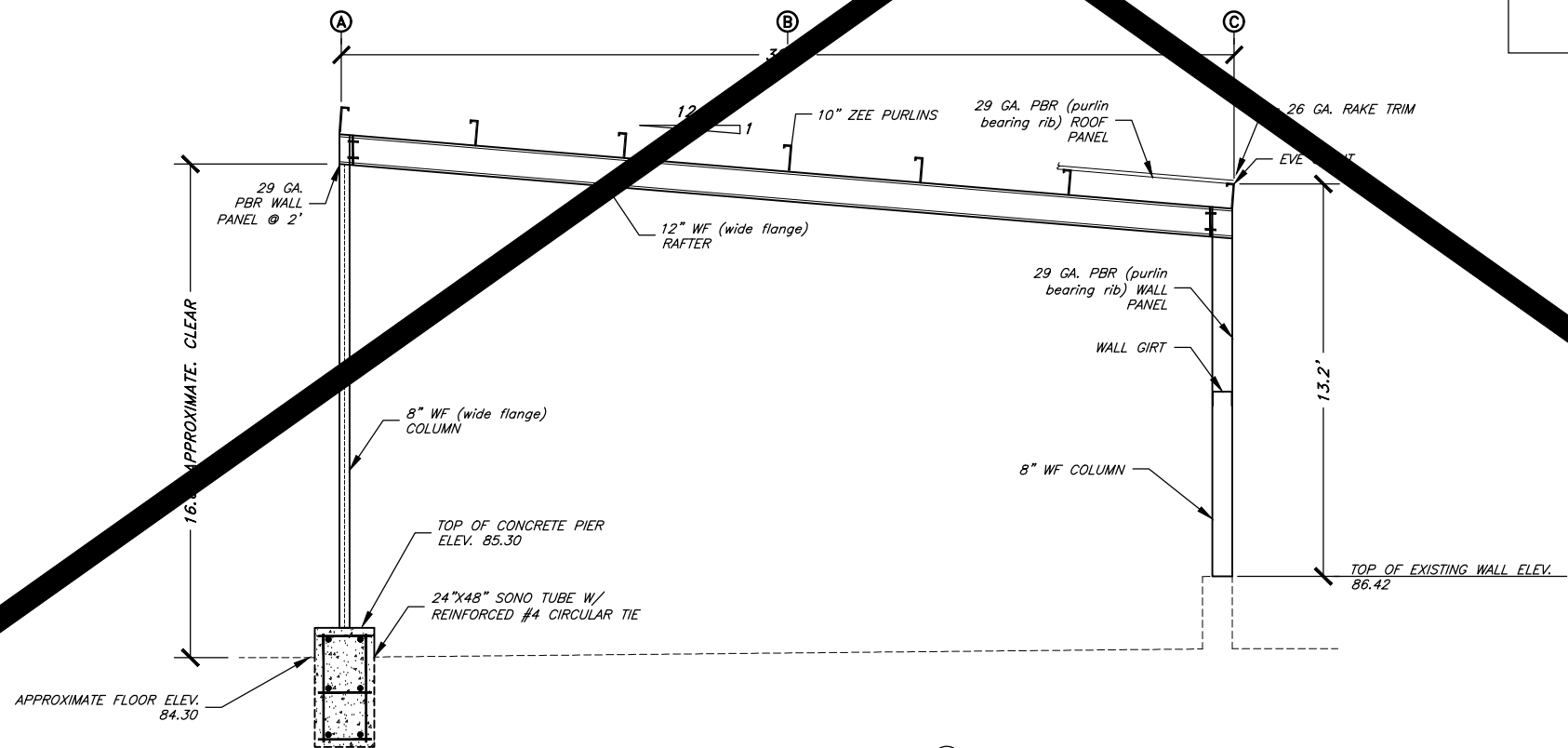


BUILDING SECTION AT LINE ①

SCALE: 1"=6'

NOTE:

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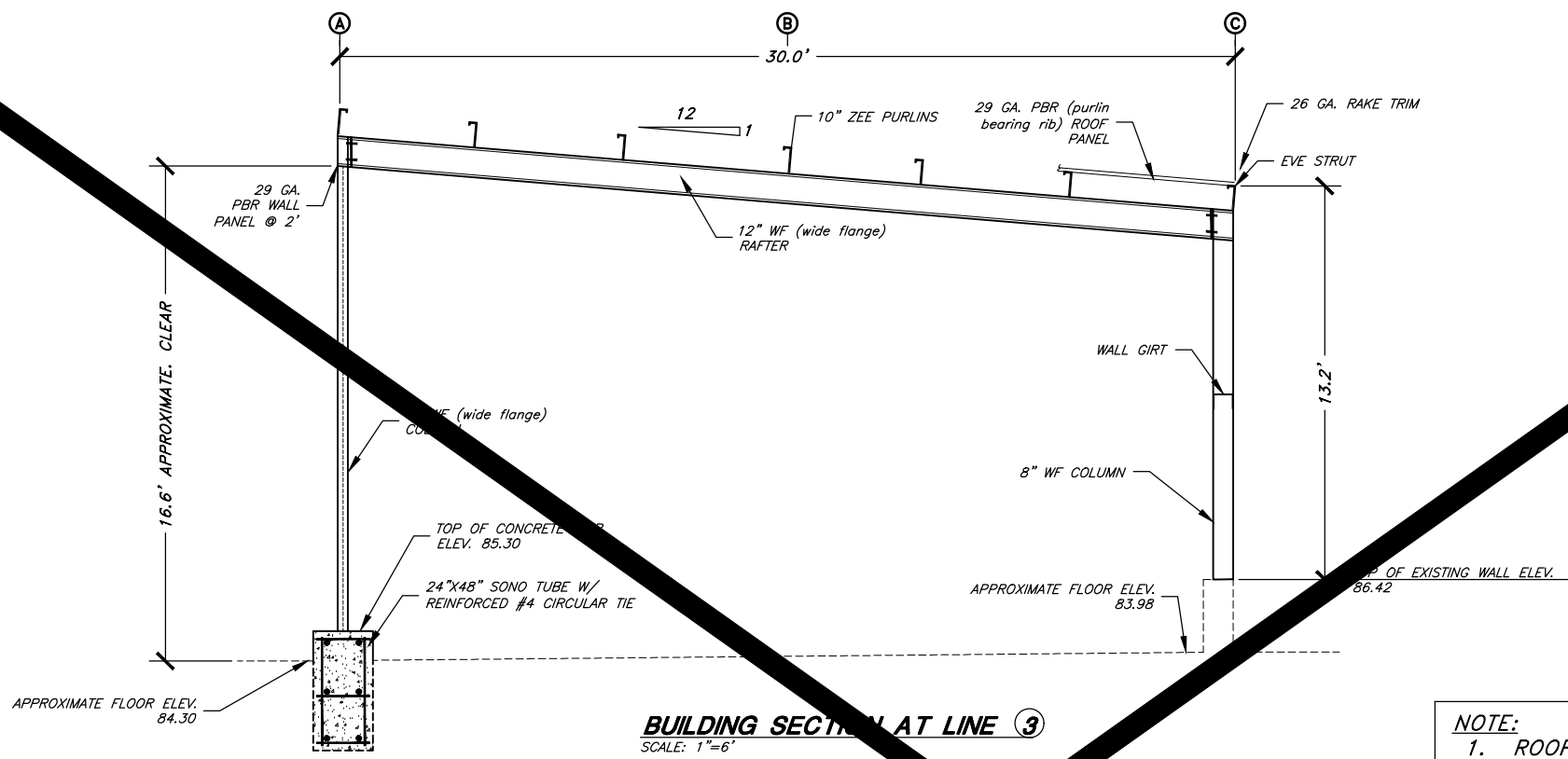


BUILDING SECTION AT LINE ②

SCALE: 1"=6'

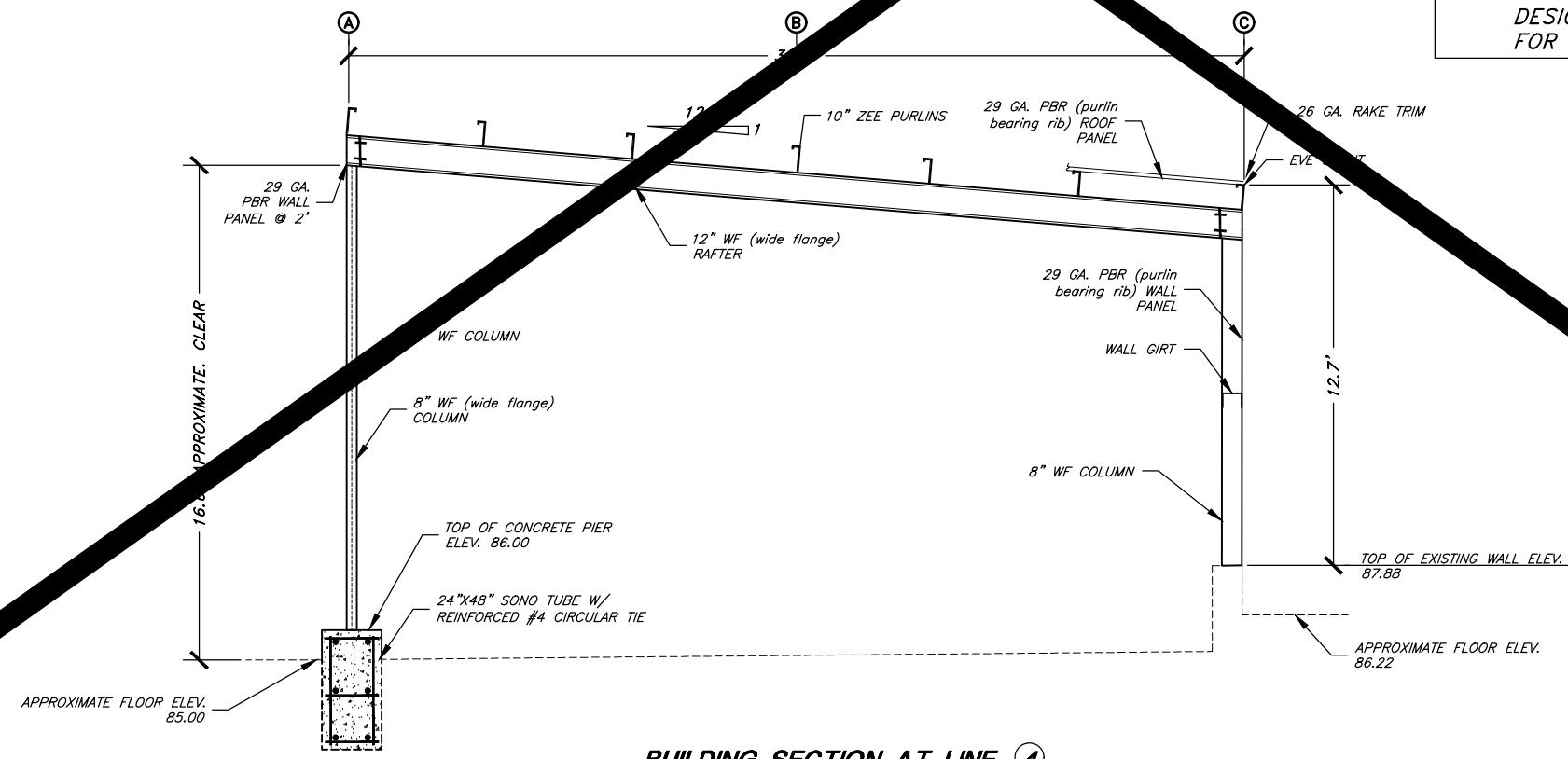
NO.	DATE	REV.	DATE	APPR.
1	9/1/17	DQS		

DESIGNED	SLS
DRAWN	SLS
CHECKED	SLS



BUILDING SECTION AT LINE 3
SCALE: 1"=6'

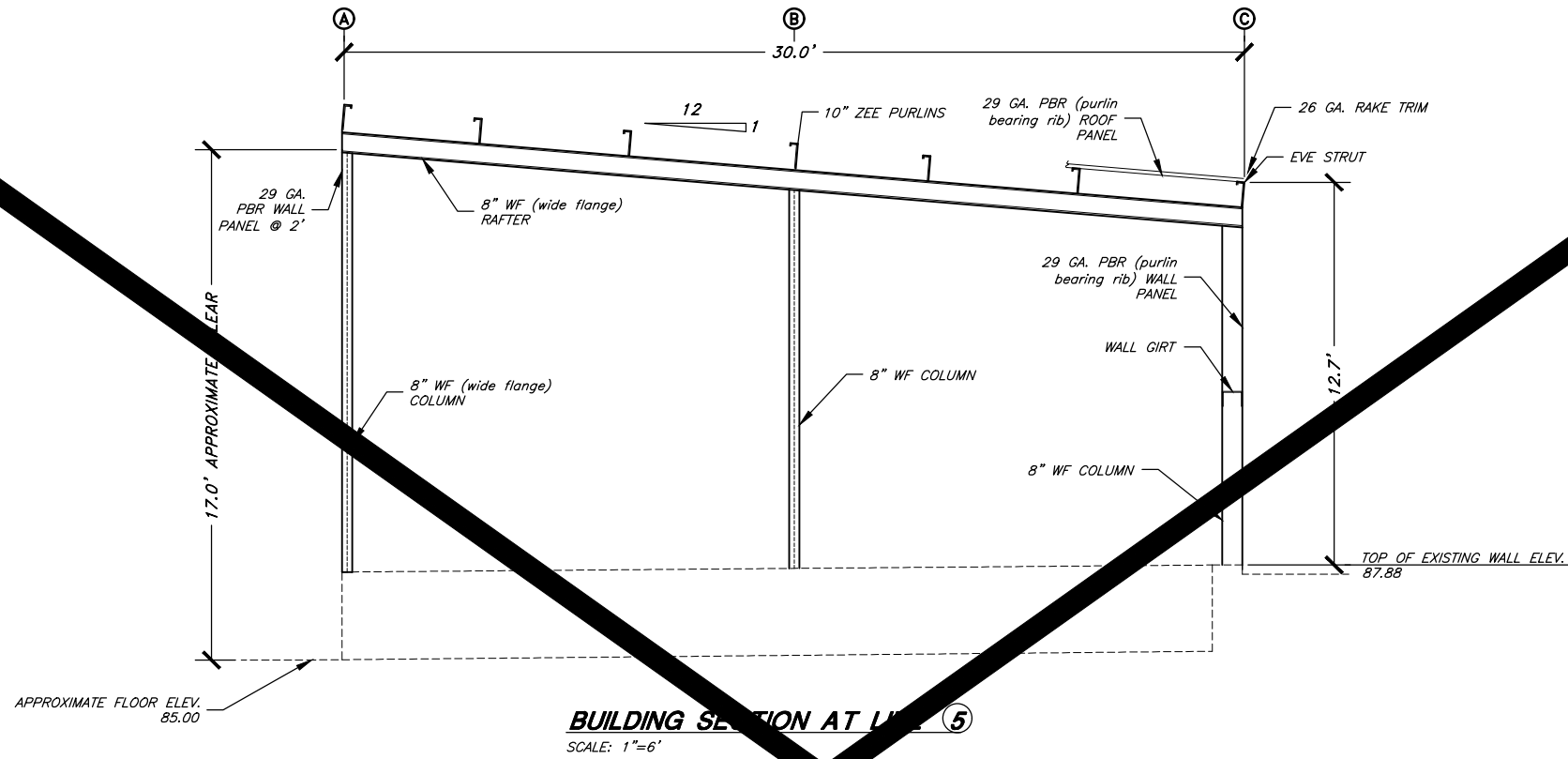
NOTE:
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BUILDING SECTION AT LINE 4
SCALE: 1"=6'

REV.	DATE	APPR.
1	9/1/17	DQS

SCALE: 24"X36"	SLS DESIGNED
11"X17"	SLS DRAWN SLS CHECKED

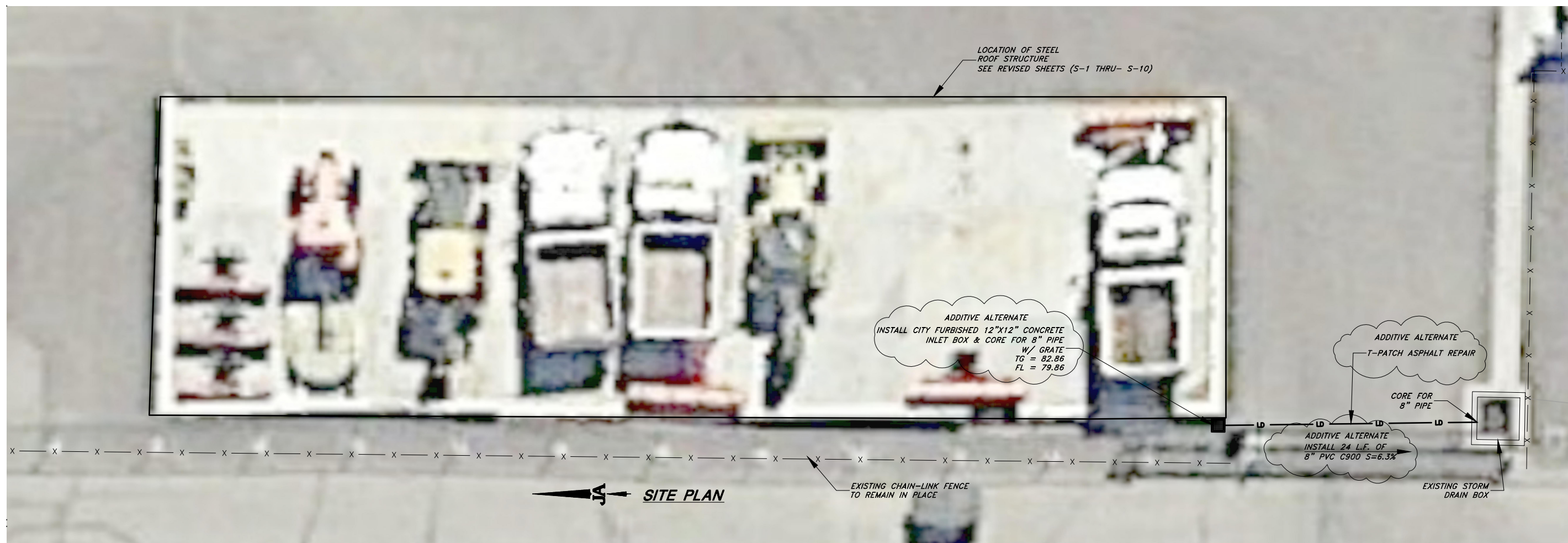


NOTE:

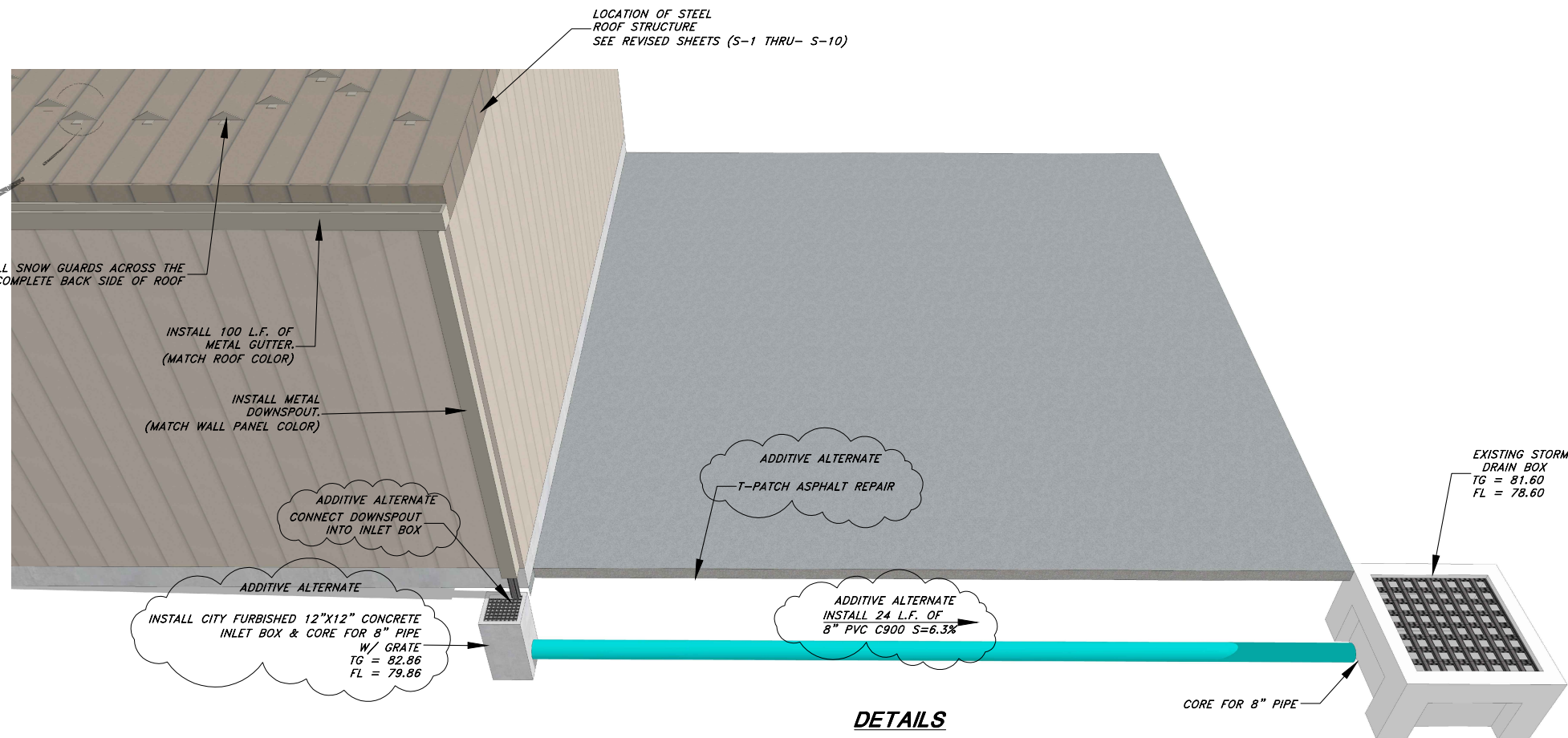
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2. ANCHORING PLATE DETAIL SHALL BE DESIGNED AND SUBMITTED TO OWNER FOR APPROVAL.

REV.	DATE	APPR.
1	9/1/17	DQS

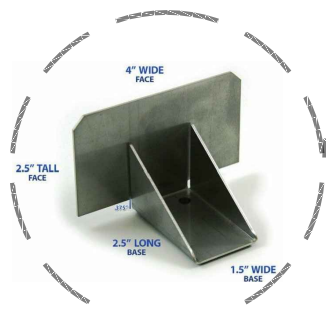
DESIGNED	SLS
DRAWN	SLS
CHECKED	SLS



SITE PLAN



DETAILS



SNOW GUARD DETAIL

REV.	DATE	APPR.
1	6/1/17	DOCS

SCALE:	24" x 36"	11" x 17"
SLS DESIGNED	SLS DRAWN	SLS CHECKED

PLEASANT VIEW CITY SHED

PLEASANT VIEW, UTAH

GENERAL STRUCTURAL NOTES

1. 2015 International Building Code Design Criteria (ASCE 7-10)

- A. Floor Live Load N.A.
- B. Roof Live Load 20 psf
- C. Roof Snow Load Data
 - Ground Snow Load P_g 43 psf
 - Flat-roof Snow Load P_f 36 psf
 - Exposure Factor C_e 1.0
 - Importance Factor I_s 1.0
 - Thermal Factor C_t 1.2
- D. Wind Design Data
 - Ultimate Design Wind Speed V_{ult} 115 mph
 - Nominal Design Wind Speed V_{asd} 90 mph
 - Risk Category II
 - Exposure C
 - Component & Cladding See ASCE 7-10 Chapter 30
- E. Earthquake Design Data
 - Risk Category II
 - Importance Factor I_e 1.0
 - Mapped Spectral Parameters
 - S_s 1.537g
 - S_1 0.559g
 - Site Class D
 - Design Spectral Parameters
 - S_{DS} 1.025g
 - S_{D1} 0.559g
 - Seismic Design Category C
 - Seismic Force Resisting System OSMF, OSCBF
 - Seismic Response Coefficient C_s 0.293W, 0.315W
 - Response Modification Factor R 3.5, 3.25
 - Analysis Procedure Equivalent Lateral Force Procedure
- F. Frost Depth 2'-6"

2. Earthwork

- A. Foundation Design Values (assumed)
 - i. Allowable Soil Bearing Pressure - 1500 psf
 - ii. Coefficient of Friction - 0.25
 - iii. Passive Earth Pressure - 150 psf/ft of depth
- B. The building pad area shall be stripped of all frozen soil, debris, vegetation, and topsoil. All fill soils and any remaining loose natural soils shall be excavated to expose suitable natural soils.
- C. Proof roll the entire building pad area to locate and remove all soft spots. Replace with compacted structural fill.
- D. Place all footings and slabs on undisturbed natural soil or on properly compacted structural fill. Contractor shall verify that soil under footings is suitable to support footings.
- E. Structural Fill: Structural fill should consist of well-graded sandy gravels with a maximum particle size of 3 inches and 5 to 15 percent fines (materials passing the No. 200 sieve). The liquid limit of fines should not exceed 35 and the plasticity index should be below 15. All fill soils should be free from topsoils, highly organic material, frozen soil, and other deleterious materials. Structural fill should be placed in maximum 8-inch thick loose lifts at a moisture content within 2 percent of optimum and compacted to at least 95 percent of modified proctor density (ASTM D1557) under the building and 90 percent under concrete flatwork.
- F. It is the responsibility of the contractor to ensure that the depth of the bottom of the foundation is far enough below the adjacent grade to ensure adequate frost protection.

3. Concrete and Reinforcement

- A. Material Standards
 - i. Concrete
 - a. Footings and foundation walls - $f'c = 3000$ ** p.s.i.
 - b. Slabs on grade - $f'c = 3500$ p.s.i.
 - ** Concrete has been designed using $f'c = 2500$ p.s.i. Special Inspection not required unless noted otherwise, see Special Inspection Notes.
 - c. Normal weight aggregates - ASTM C33
- ii. Cement
 - a. Use Type I/II cement as per ASTM C150
 - b. Air-entraining admixtures (where required) - ASTM C260
 - c. Calcium chloride shall not be used.
- iii. Reinforcing
 - a. Rebar - ASTM A615 Grade 60 ($F_y = 60$ ksi)
 - b. Welded wire - ASTM A1064
 - c. Epoxy - Simpson SET-XP (ICC-ES ESR-2508)
- iv. Anchor Rods/Bolts
 - a. Steel column anchor rods/bolts - ASTM F1554 Grade 36 with ASTM A563 heavy hex nuts and hardened washers
 - b. Adhesive (epoxy) anchors - Simpson AT-XP (APMO UES ER-263)
 - c. Expansion anchors - Simpson Strong-Bolt (ICC-ES ESR-1771) or Hilti Kwik Bolt TZ (ICC-ES ESR-1917)
 - d. Screw Anchors - Simpson Titen HD (ICC-ES ESR-2713), Hilti Kwik HUS-TZ (ICC-ES ESR-3027), or Powers Fasteners Wedge-Bolt+ (ICC-ES ESR-2526)

B. Detail reinforcing to comply with ACI 315 "Manual of Standard Practice for Detailing Reinforcing Concrete Structures" and the Concrete Reinforcing Steel Institute (CRSI) recommendations.

- i. Minimum clear concrete cover for reinforcement shall be as follows unless noted otherwise:
 - a. Concrete cast directly against and permanently exposed to earth - 3"
 - b. Concrete exposed to weather or earth:
 - 1. #5 bars or smaller - 1 1/2"
 - 2. #6 bars or larger - 2"
 - c. Concrete not exposed to weather or in contact with the ground - 3/4"
 - d. Slabs on grade - as shown in details, 3/4" min. from top of slabs not exposed to weather
- ii. Lap Splice Lengths (unless noted otherwise)
 - a. $f'c = 2500-3500$ p.s.i.
 - 1. #6 and smaller - 36 bar diameters
 - 2. #7 and larger - 45 bar diameters
 - b. $f'c = 4000$ p.s.i. or greater
 - 1. #6 and smaller - 29 bar diameters
 - 2. #7 and larger - 36 bar diameters
 - c. Lap splice lengths may be decreased by 25% for slabs on grade and horizontal wall reinforcing.
- iii. Increase lap splice lengths by 50% where epoxy coated bars are used.
- iv. Stagger splices in walls so that no two adjacent bars are spliced in the same location, unless shown otherwise.
- v. Make all bars continuous around corners or provide corner bars of equal size and spacing.
- vi. Vertical bars in walls, grade beams, and piers to terminate in footings with ACI standard hooks (12 bar diameters) to within 4" of the bottom of the footing unless noted otherwise.
- vii. Horizontal wall reinforcing shall terminate at the ends of walls with a 90 degree hook plus a 6 bar diameter extension, unless shown otherwise.
- viii. Horizontal wall reinforcing shall be continuous through construction and control joints.
- ix. Splices in horizontal reinforcement shall be staggered. Splices in two curtains (where used) shall not occur in the same location.
- x. Use chairs or other support devices as required for proper clearance.
- xi. Rebar hairpins shall be centered in slabs and shall be wire tied to the slab reinforcing (if any). Rebar hairpins shall be continuous through walls and piers; lap splices in hairpins may only occur in the floor slab unless noted otherwise.
- xii. Unless noted otherwise, openings in walls shall be reinforced with #5 bar on all sides of the opening. Reinforcing shall extend 24" min. past the edge of the opening. For one layer of wall reinforcing provide (1) #5 bar around openings, for two layers provide (2) #5 bars.
- C. Control joints in slabs on grade are recommended to control cracking. See plans for control joint spacing and details.
- D. Slabs and grade beams shall not have joints in a horizontal plane. All reinforcement shall be continuous through all construction joints.
- E. Floor slab thickness and reinforcing shown in these drawings are adequate to support typical uniform loads only. Mountain View Engineering has not designed the slab for any specific concentrated forces such as those from vehicles, storage racks, or heavy equipment (unless noted otherwise).
- F. Welding of rebar is not allowed unless specifically indicated in the drawings. All embedments, reinforcing, and dowels shall be securely tied to framework or to adjacent reinforcing prior to placement of the concrete. Tack welding of rebar joints in grade beams, walls, or cages is not allowed. Where welding of rebar is shown in the drawings, all rebar to be welded shall be ASTM A706 Grade 60.

4. Structural Steel

- A. Material:
 - i. Angles, Plates, and Channels: ASTM A36 ($F_y = 36$ k.s.i.)
 - ii. Wide Flanges: ASTM A992 ($F_y = 50$ k.s.i.)
 - iii. Tubes (HSS): ASTM A500, Grade B ($F_y = 46$ k.s.i.)
 - iv. Pipes: ASTM A53, Grade B ($F_y = 35$ k.s.i.)
- B. Fabrication and construction shall comply with the latest edition of the following Codes and Standards:
 - i. American Institute of Steel Construction (AISC) "Specification for Structural Steel Buildings." (360-10)
 - ii. AISC "Seismic Provisions for Structural Steel Buildings," including supplement No. 1 (341-10)
 - iii. AISC "Code of Standard Practice."
 - iv. RCSC "Specification for Structural Joints Using ASTM A325 or A490 Bolts."
 - v. Steel Joist Institute (SJI), "Standard Specifications and Code of Standard Practice."
 - vi. American Welding Society (AWS), Structural Welding Code (specific items do not apply when they conflict with the AISC requirements).
 - vii. American Iron and Steel Institute (AISI), "Specification for the Design of Cold-Formed Steel Structural Members".

C. Welding:

- i. Certification of Welders: All shop and field welding shall be executed by AWS certified welders. Certification shall be considered current if dated within the past 12 months. Welders will be considered certified if they have been certified by AWS and their work records are current within every six month period thereafter as required by AWS. Certification and records must comply with AWS Standards. Certification and appropriate records must be provided to the inspector prior to beginning work.
- ii. Electrodes: E-70XX or as noted otherwise. E-60XX may be used for welding steel floor and roof decks.
- iii. Minimum Welds: All intersecting steel shapes which are not bolted shall be connected by a fillet weld all around, unless noted otherwise.
- iv. Welded reinforcing bars shall be ASTM A706 Grade 60.
- D. Bolted Connections:
 - i. Use ASTM A325N bolts for hot-rolled steel to steel (e.g. girder to column, rafter to column/cap plate, beam to beam, etc.) connections unless noted otherwise in the drawings. All connections shall conform to the RCSC "Specification for Structural Joints Using ASTM A325 or A490 Bolts".
 - ii. A325 bolts shall be pretensioned using an acceptable method such as "Turn of the Nut" as per the RCSC Specification Section 8.2 at all 8-bolt wide flange to wide flange ridge and haunch connections only.
 - iii. All other A325 bolts shall be fully tightened to the "Snug Tight" condition in accordance with the RCSC Specification Section 8.1.
- E. Provide full height web stiffener plates to each side of all beams bearing on top of columns. Plate thickness shall match the thickness of the beam web except that the thickness need not exceed 1/4" unless noted otherwise on drawings.
- F. All structural steel, except plates embedded in concrete or masonry, to have one coat (min.) of gray shop primer, 1.5 mil minimum thickness.
- G. Open Web Steel Joists and Girders:
 - i. The steel joist supplier shall be responsible for the design of all parallel chord and double pitched top chord steel joists and girders.
 - ii. Field Modifications: Do not modify any joist or girder, including holes through the top and bottom chords, without the written consent and direction from the manufacturer.
 - iii. At completion of fabrication, the steel joist manufacturer shall submit a certificate of compliance as an approved fabricator and that the work was performed in accordance with approved construction documents and with SJI standard specifications (IBC 2206.5).
- H. Cold Formed Girts & Purlins:
 - i. All cold formed cees, zeeks, and eave struts shall be MBCI standard or equal.
 - ii. All girt and purlin connection bolts shall be GR-5 or equal.
- J. Use 26 gage "PBR" roof and wall panels with trims as required.
- K. Flange braces as shown by marks UB and FB to be 2"x2"x3/8" angle.
- L. Steel Stairs, Handrails, and Guardrails
 - i. Design of steel stairs, handrails, and guardrails is not by Mountain View Engineering.
 - ii. All stairs, handrails, and guardrails shall comply with the requirements of the 2015 IBC unless noted otherwise in the project specifications.
 - iii. The fabricator shall be responsible for the design and certification of all steel stairs, handrails, and guardrails, including member sizes and connection details.
 - iv. See the architectural plans for all stair information including, but not limited to, stair layout, dimensions, and style.

5. Special Inspections

- A. Special inspections, as required by Section 1705 of the IBC, shall be provided by an independent agency employed by the owner unless waived by the building official. The contractor shall coordinate and cooperate with the required inspections. Items requiring special inspection are:
 - i. Steel Construction (IBC 1705.2)
 - a. Field welding (if any is used).
 - b. High-strength bolts.
 - c. Structural steel shall be fabricated by a fabricator that has been approved by the local building department or shall have special inspection as per IBC 1705.2.1 performed by an approved inspection agency.
 - ii. Concrete Construction (IBC 1705.3)
 - a. Special inspections of concrete footings, grade beams, walls, and slabs are not required as per Exceptions 1, 2.3, 3, 4, & 5 to IBC Section 1705.3. Third party special inspection of reinforcing placement need only be performed where specifically required by the building official.
 - b. Special inspection of anchor rods/bolts is required per IBC Table 1705.3. Continuous special inspections required for epoxy anchors.
 - c. Special inspection of rebar welding is required (if any is used).
- B. Special inspector must be qualified and approved by local building department.

6. Miscellaneous

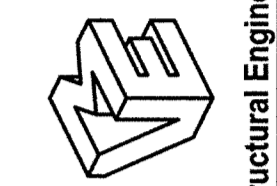
- A. The project specifications are not superseded by the General Structural Notes. Notes and details on the drawings shall take precedence over General Structural Notes and typical details. Should any of the detailed instructions shown on the plans conflict with the General Structural Notes, or with each other, the strictest provisions shall govern.
- B. It is solely the responsibility of each contractor to follow all applicable safety codes and regulations during all phases of construction. The engineer is not engaged in, and does not supervise, construction.

C. Erection, Shoring, and Bracing

- i. It is the contractor's responsibility to determine erection procedures and sequence, and to ensure the stability of the building and its component parts during erection.
 - ii. It is solely the contractor's responsibility to provide any temporary shoring, bracing, guys, and tie downs that may be necessary to provide adequate vertical and lateral support. Such material is not shown on the drawings. Shoring and bracing shall remain in place until all permanent members are in place and all final connections are completed, including all roof and floor attachments.
 - iii. The building shall not be considered stable until all connections are complete.
 - iv. The engineer has no expertise in, and takes no responsibility for, construction means and methods or job site safety during construction. Approval of submittals made by the contractor which may contain information related to construction methods or safety issues, or participation in meetings where such issues might be discussed, shall not be construed as voluntary assumption by the engineer of any responsibility for safety procedures.
 - D. Equipment framing loads, openings, and structure in any way related to mechanical, plumbing, or electrical requirements are shown for bidding purposes only. The contractor shall coordinate this information with the involved trades before proceeding with such portion of the work. Excess cost related to variation in these requirements shall be borne by the appropriate contractor.
 - E. The contractor shall notify engineer of any variations in dimensions.
 - F. The engineer is not responsible for any deviations from these plans unless such changes are authorized in writing by the engineer.
- #### 7. Deferred Submittals
- A. Deferred submittal items are those portions of the design that are not submitted at the time of application, and have received prior approval from the building official to be deferred. The following items are deferred submittals. Documents shall be submitted, reviewed, & approved by Mountain View Engineering & the Building Official prior to installation.
 - i. Open Web Steel Joists and Girders

IMPORTANT NOTE: DUE TO THE USE OF MINIMALLY REINFORCED EXISTING CONCRETE WALLS, THERE ARE LIMITS TO THE METAL BUILDING COLUMN REACTIONS WHICH CAN BE IMPOSED ON THE WALLS AND MEET BUILDING CODE REQUIREMENTS. THE STRUCTURAL FRAMING SHOWN IN THESE PLANS PRODUCES REACTIONS WHICH THE EXISTING CONCRETE CAN SAFELY RESIST AND WHICH REQUIRES THE SPECIAL ANCHOR BOLTS DETAILS SHOWN. THE INTERIOR FRAME COLUMNS PARTICULARLY REQUIRE AN UNUSUAL NUMBER OF BOLTS AND PRECISE LOCATIONS OF THOSE BOLTS IN THE EXISTING CONCRETE TO ADEQUATELY TRANSFER THE COLUMN BASE FORCES TO THE FOUNDATIONS. OTHER FRAMING MAY BE USED PROVIDED THAT COLUMN BASE REACTIONS FOR ALL APPLICABLE LOAD CASES ARE LESS THAN THE EXISTING WALLS ARE CAPABLE OF CARRYING AND WILL REQUIRE SPECIAL DETAILING OF THE ANCHOR BOLTS USED. REACTIONS AND ANCHORAGE DETAILS OF ALTERNATE SYSTEMS NEED TO BE REVIEWED WITH AND APPROVED BY MOUNTAIN VIEW ENGINEERING, BEFORE THEY MAY BE USED.

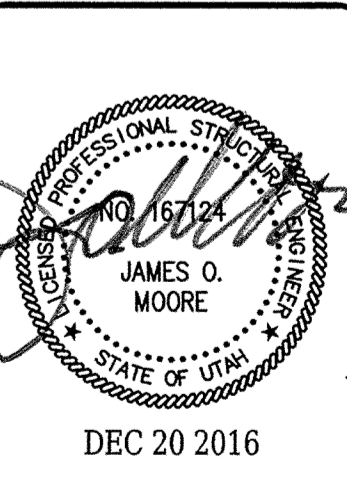
DRAWING	INDEX OF DRAWINGS
GN	GENERAL STRUCTURAL NOTES
F-1	FOUNDATION & ANCHOR ROD PLAN
F-2	FOUNDATION DETAILS
S-1	ROOF FRAMING PLAN
S-2	FRAMING / SHEETING ELEVATIONS
S-3	FRAMING / SHEETING ELEVATIONS
S-4	FRAMING / SHEETING ELEVATIONS
S-5	FRAMING / SHEETING ELEVATIONS
S-6	RIGID FRAME ELEVATION
S-7	RIGID FRAME ELEVATION
S-8	RIGID FRAME ELEVATION
S-9	ERECTION DETAILS
S-10	ERECTION DETAILS



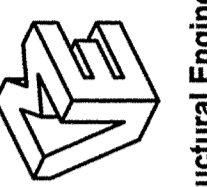
MOUNTAIN VIEW ENGINEERING, INC.
 Consulting
 Structural Engineering
 Design
 345 North Main Street, Ste. A, Brigham City, Utah 84302 (435) 734-9700 Fax: (435) 734-9919

SHEET TITLE: GENERAL STRUCTURAL NOTES
 JOB NAME: PLEASANT VIEW CITY SHED
 LOCATION: PLEASANT VIEW, UTAH
 CONTRACTOR: -

PLAN	ISSUE	DATE
BY:	DESCRIPTION:	
T.W.	FOR CONSTRUCTION	
DATE:		
12-20-16		



SHEET NUMBER:
GN
 DRAWN BY: T.W.
 ENGINEER: J.O. MOORE
 FILE JOB NUMBER: **160633**



MOUNTAIN VIEW ENGINEERING, INC.
 Consulting
 Structural Engineering
 345 North Main Street Ste. A, Brigham City, Utah 84302 (435) 734-9700 Fax (435) 734-9519
 Design

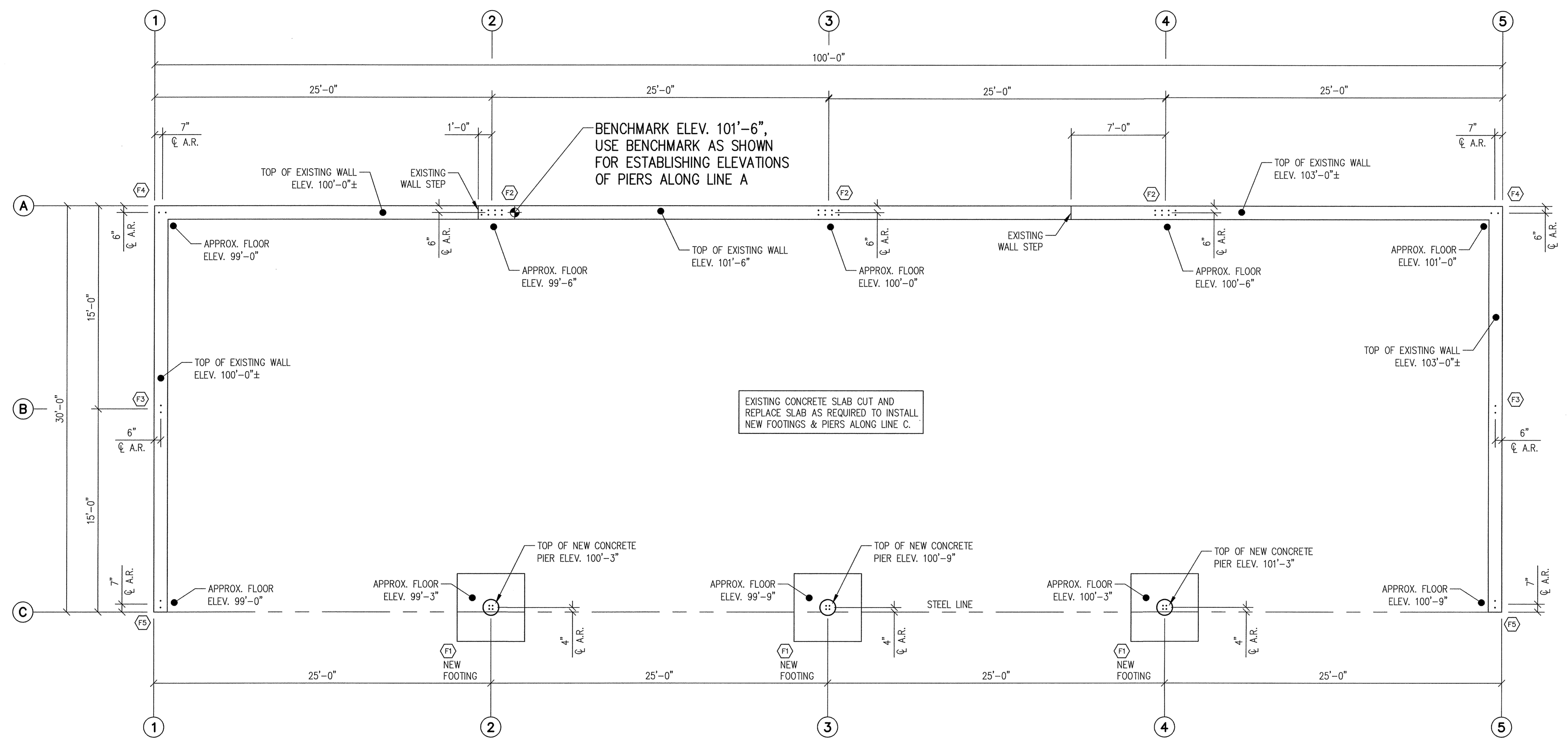
SHEET TITLE: **FOUNDATION & ANCHOR ROD PLAN**
 JOB NAME: **PLEASANT VIEW CITY SHED**
 LOCATION: **PLEASANT VIEW, UTAH**
 CONTRACTOR: -

PLAN ISSUE DATES	
DATE	DESCRIPTION
12-20-16	T.W. FOR CONSTRUCTION

PROFESSIONAL SEAL
 NO. 15712
 JAMES O. MOORE
 STATE OF UTAH
 REGISTERED PROFESSIONAL ENGINEER
 DEC 20 2016

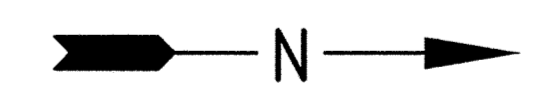
SHEET NUMBER:
F-1

DRAWN BY: T.W.
 ENGINEER: J.O. MOORE
 MVE JOB NUMBER: **160633**



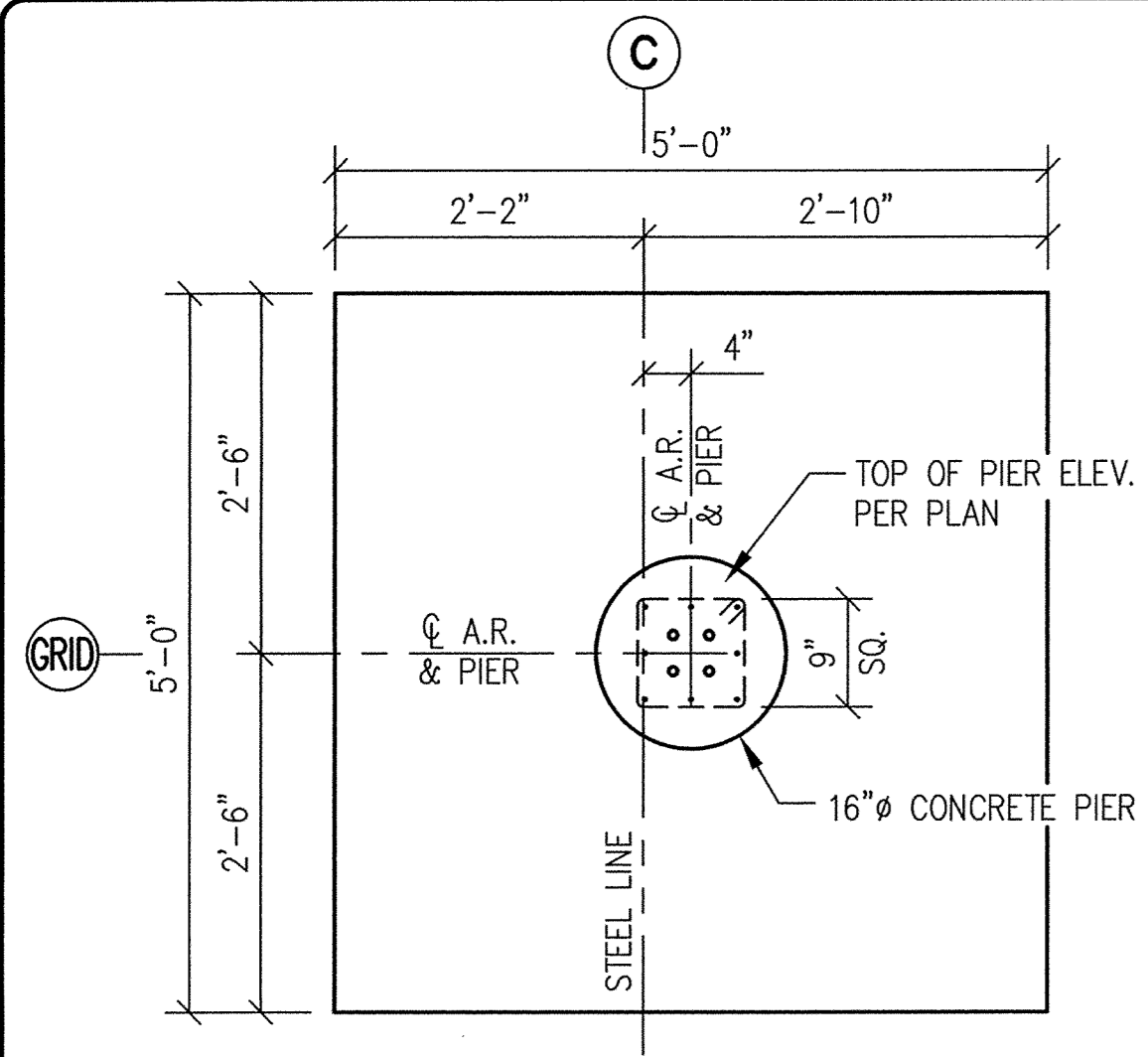
(F1) INDICATES APPLICABLE FOOTING DETAIL.

FOUNDATION & ANCHOR ROD PLAN
 SCALE: 1/4"=1'-0"

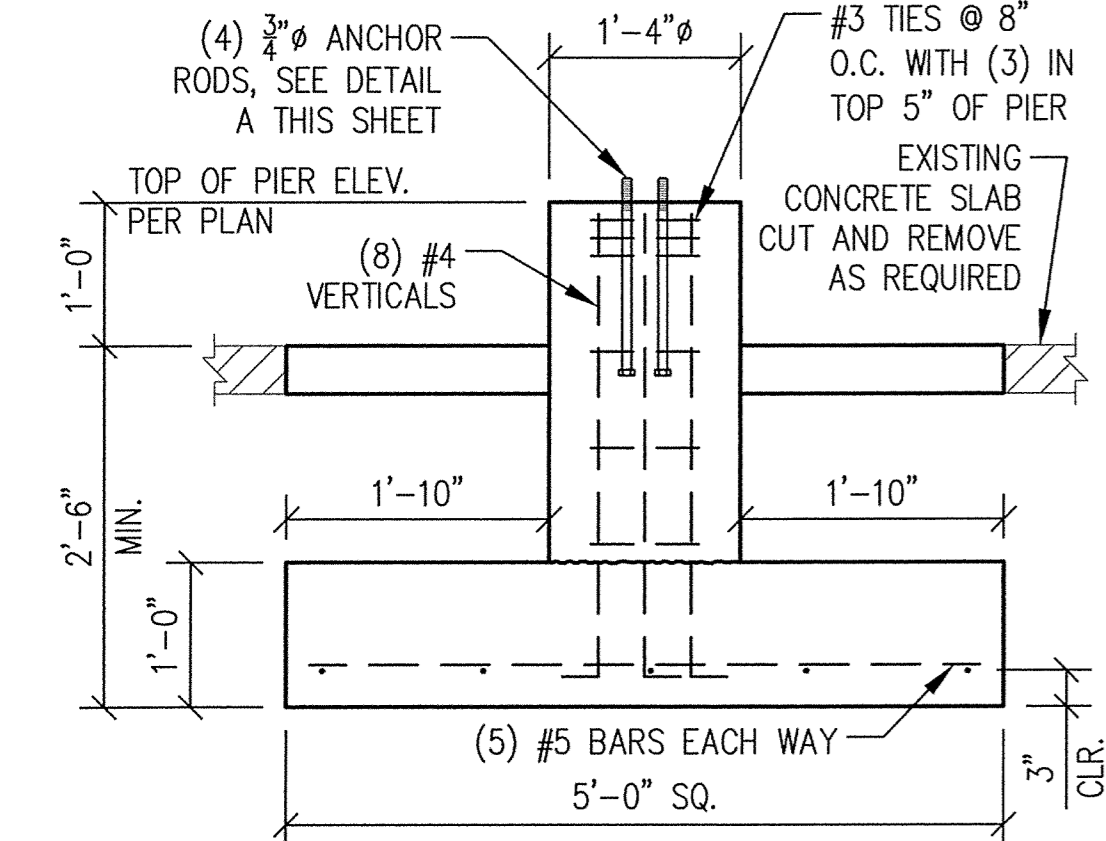


IMPORTANT NOTE: DUE TO THE USE OF MINIMALLY REINFORCED EXISTING CONCRETE WALLS TO SUPPORT THE NEW METAL BUILDING STRUCTURE, IT IS NECESSARY TO INSTALL THE ANCHOR BOLTS EXACTLY AS SHOWN, MAINTAINING ALL EMBEDMENTS & EDGE DISTANCES, TO MEET BUILDING CODE REQUIREMENTS. SEE IMPORTANT NOTE ON SHEET GN.

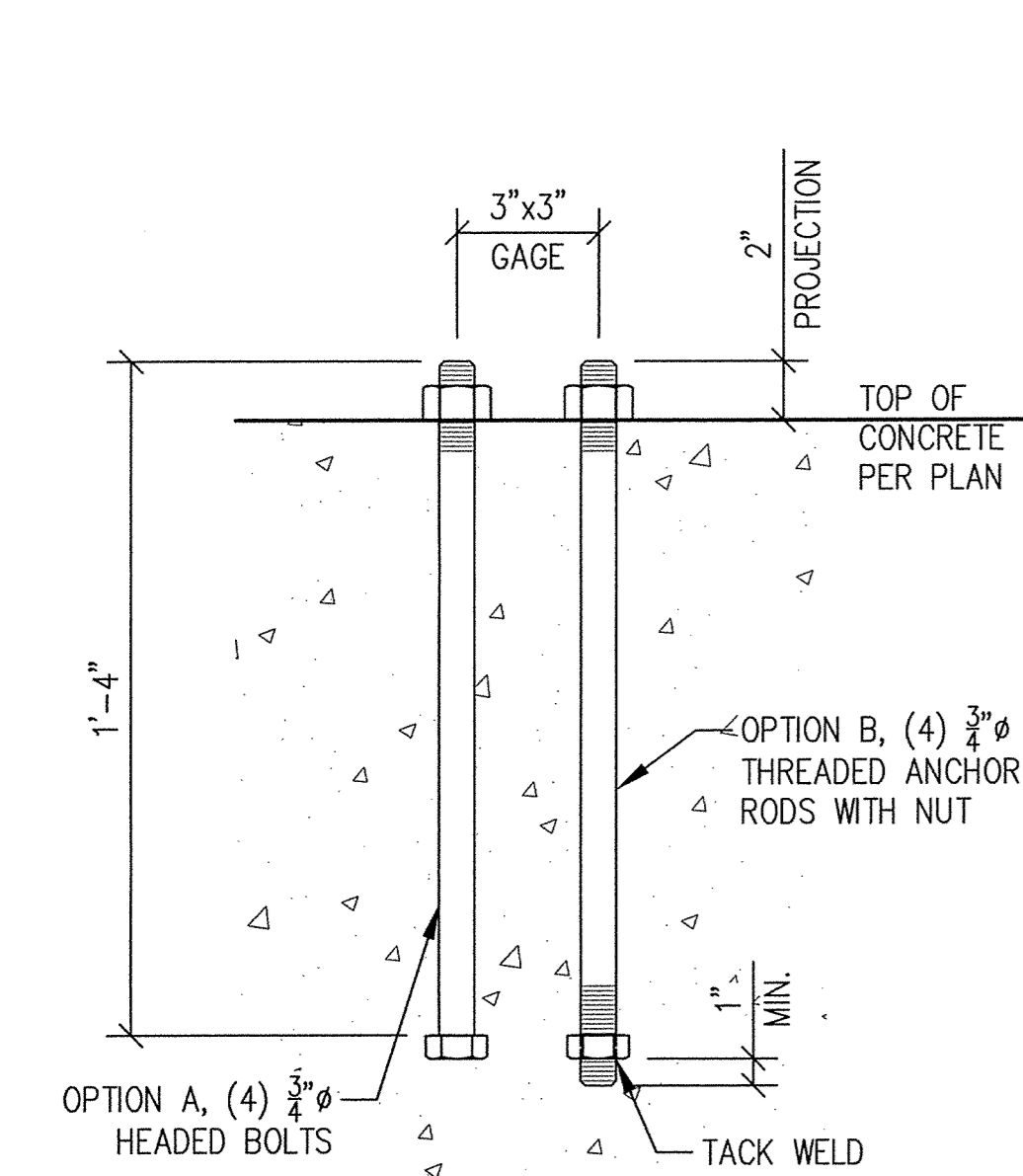
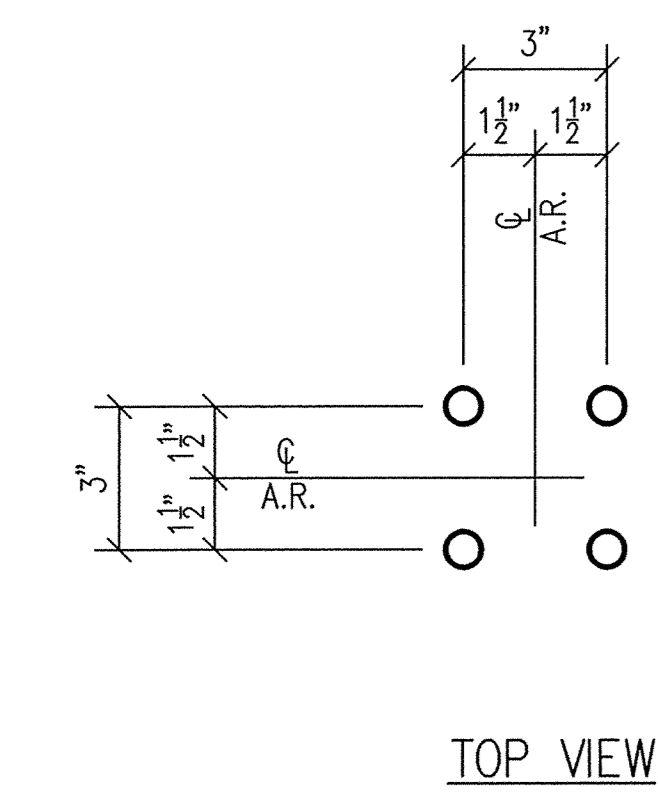
NOTE: STEEL LINE OF BUILDING IS 1" INSIDE OF OUTSIDE FACE OF EXISTING CONCRETE WALLS ALONG LINES 1, 5, & A.



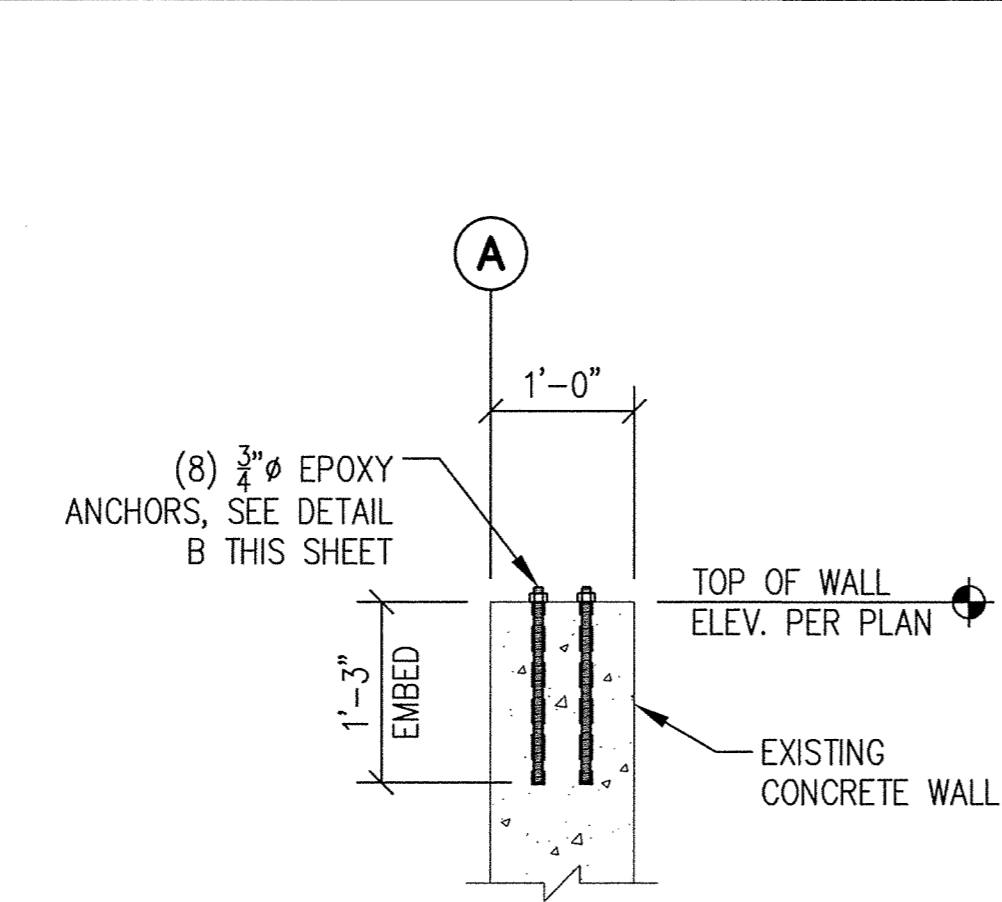
NOTE: COLUMN AND BASE PLATE NOT SHOWN FOR CLARITY SEE DETAIL A THIS SHEET



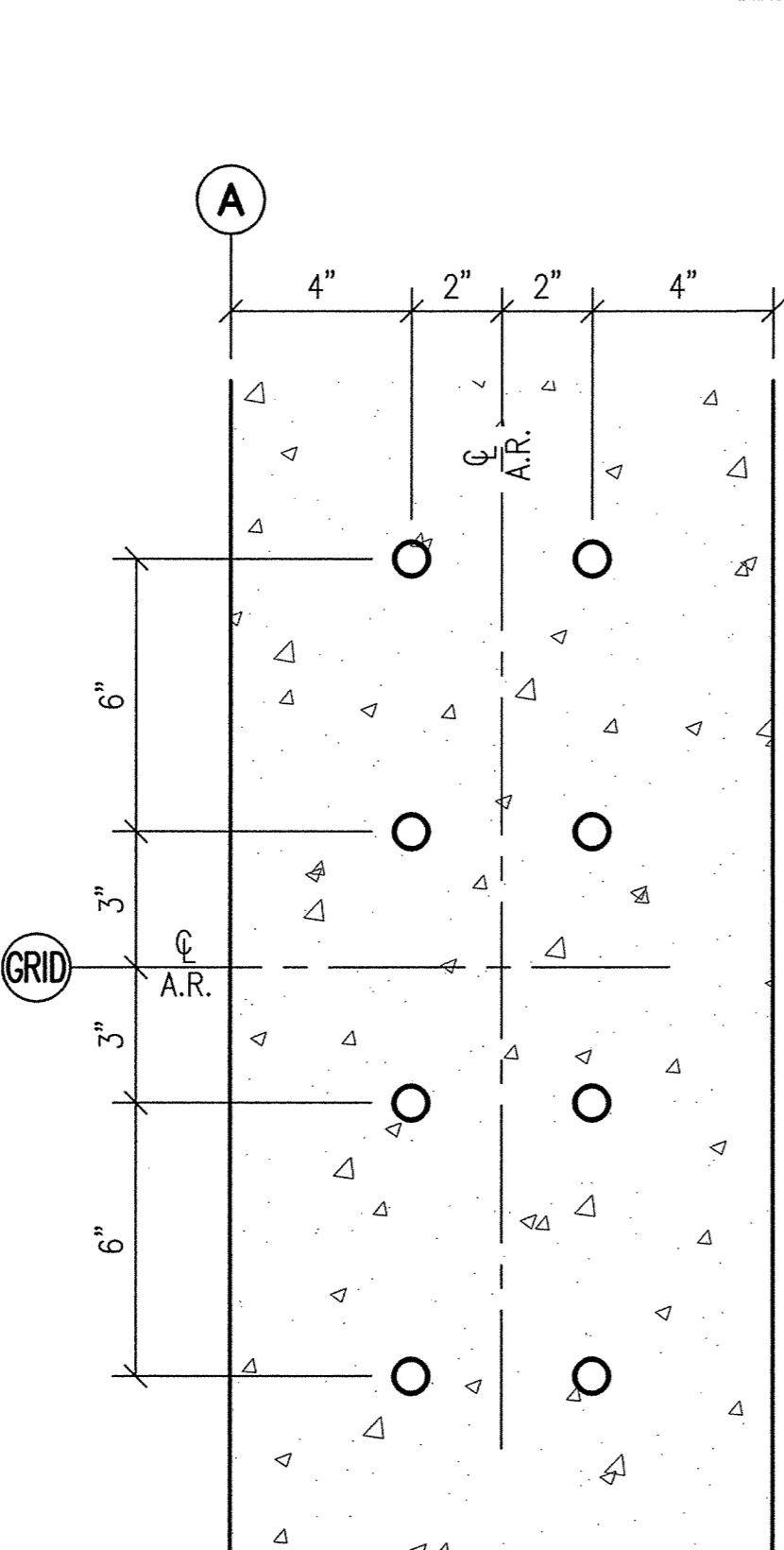
F1 SIDEWALL FOOTING DETAIL
3/4"=1'-0"



A ANCHOR ROD DETAIL
N.T.S.

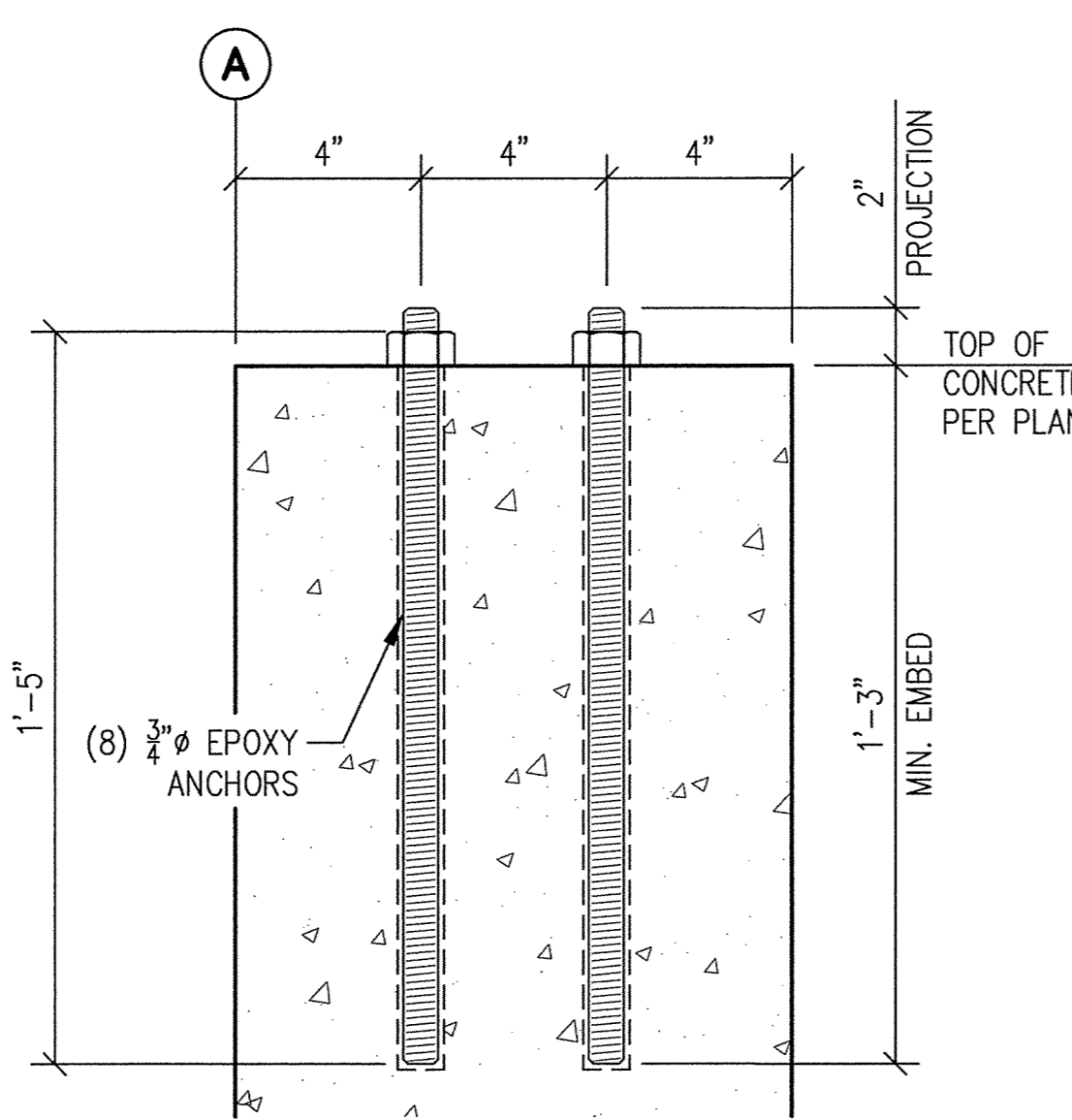


F2 SIDEWALL FOOTING DETAIL
3/4"=1'-0"

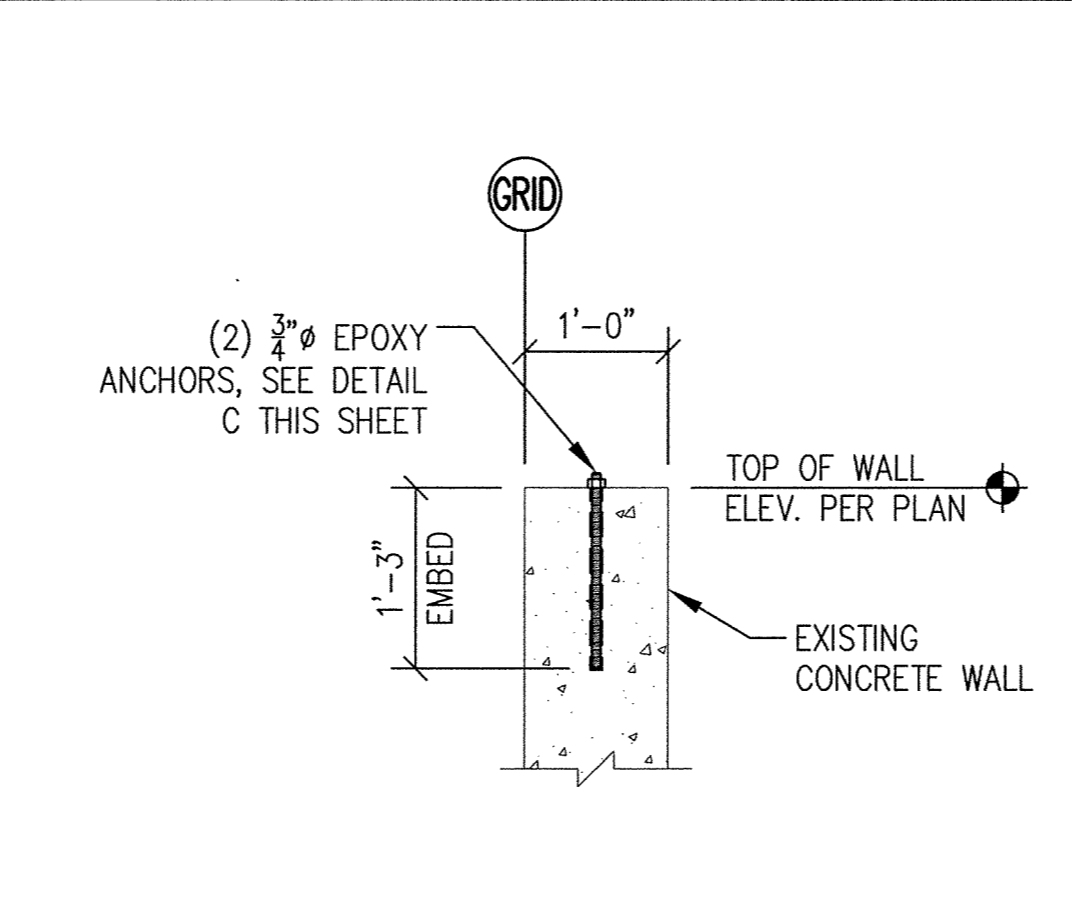


TOP VIEW

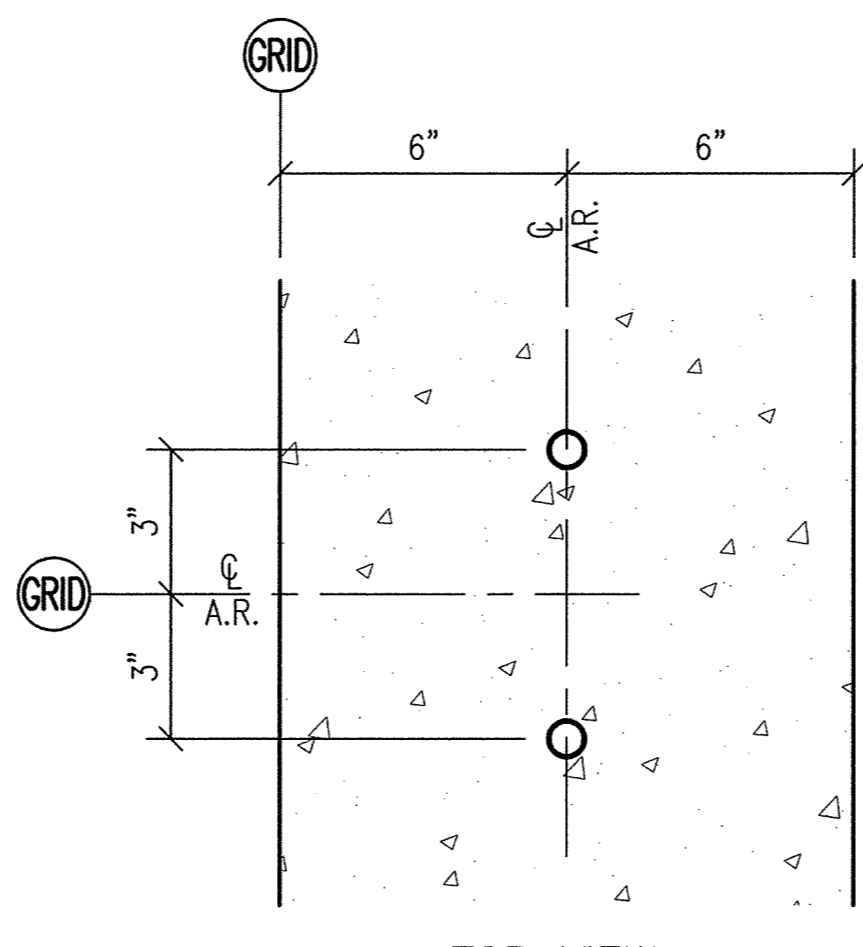
NOTE: ORIENT ANCHOR ROD PATTERN AS SHOWN ON FOUNDATION AND ANCHOR PLAN.



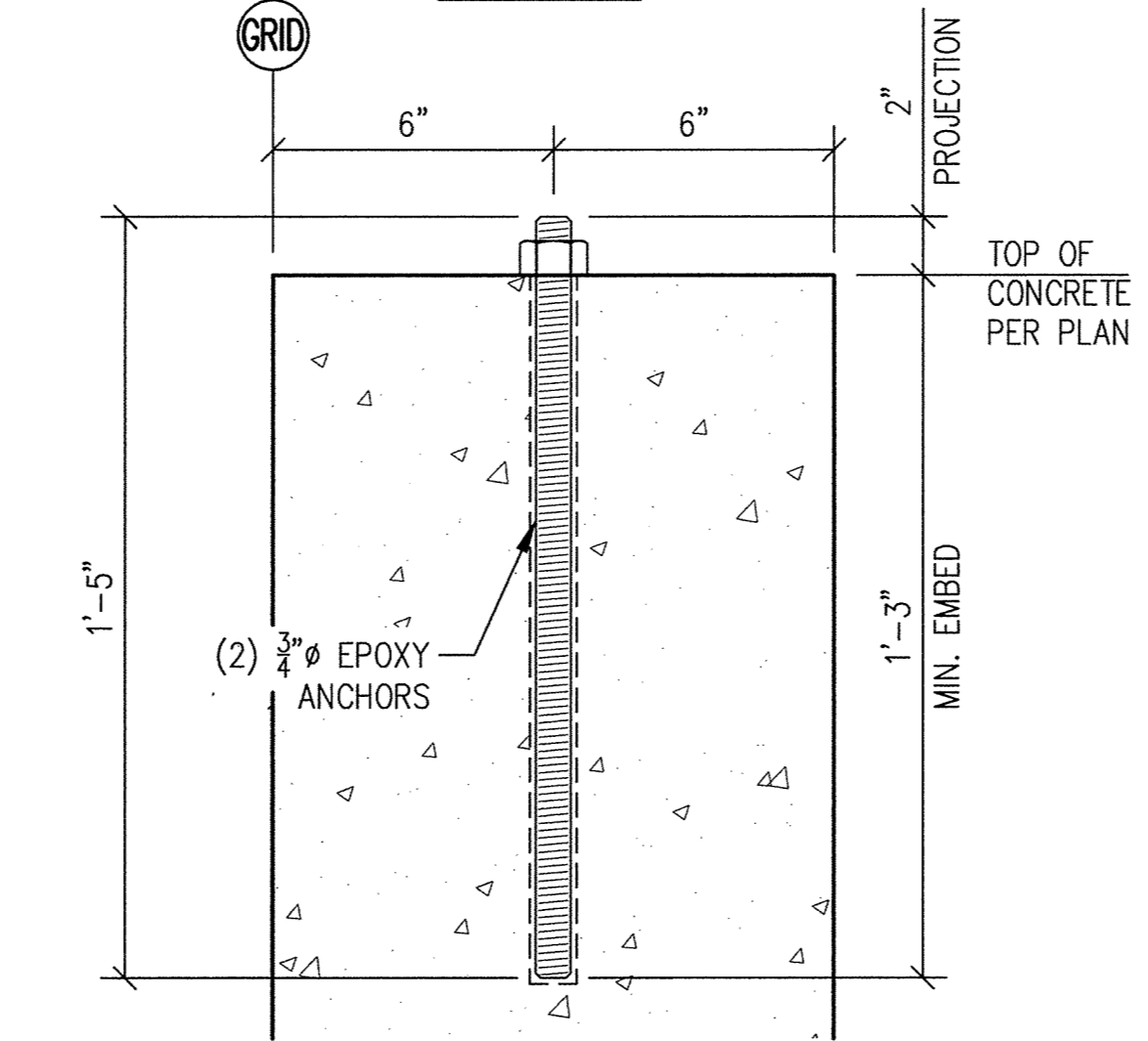
B ANCHOR ROD DETAIL
N.T.S.



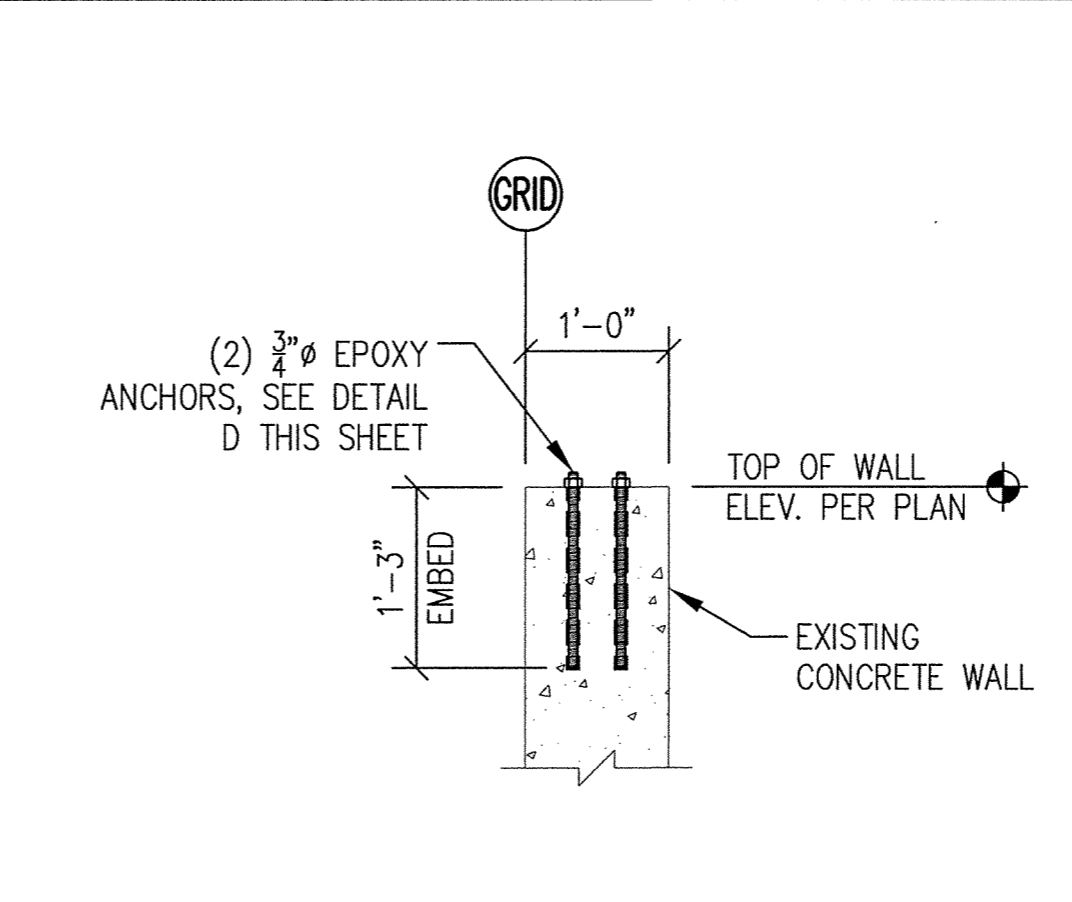
F3 ENDWALL FOOTING DETAIL
3/4"=1'-0"



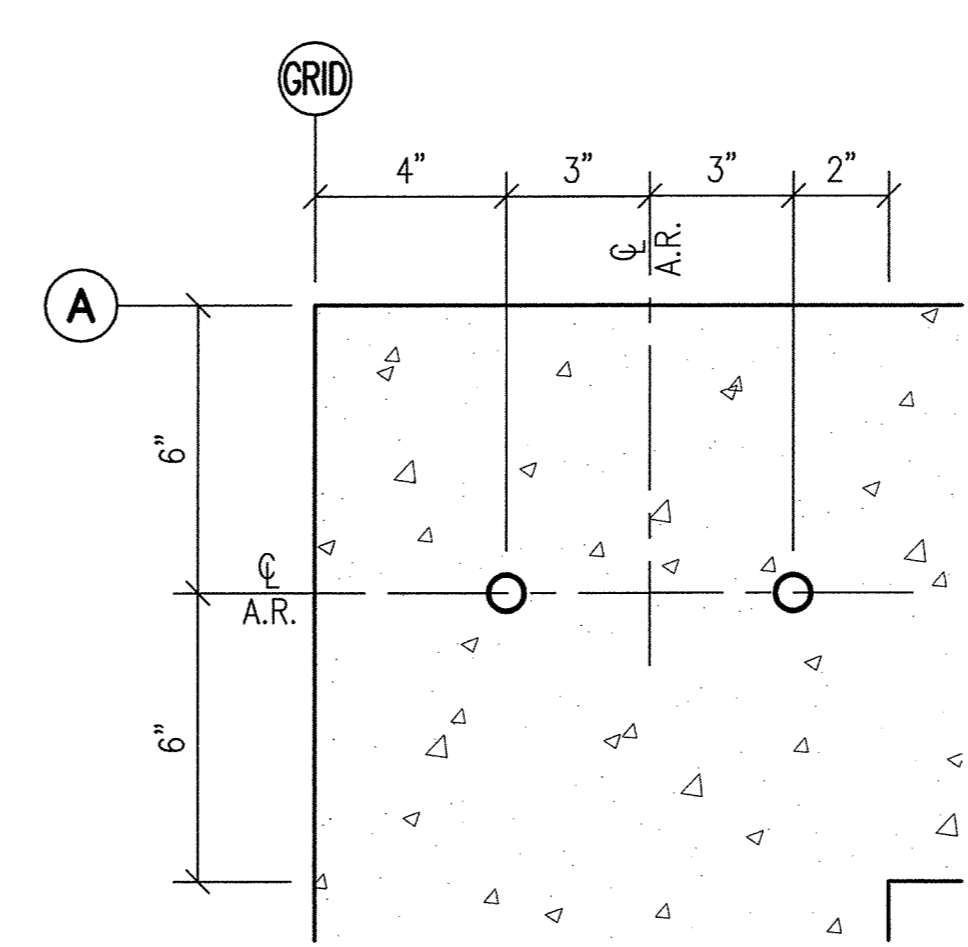
TOP VIEW



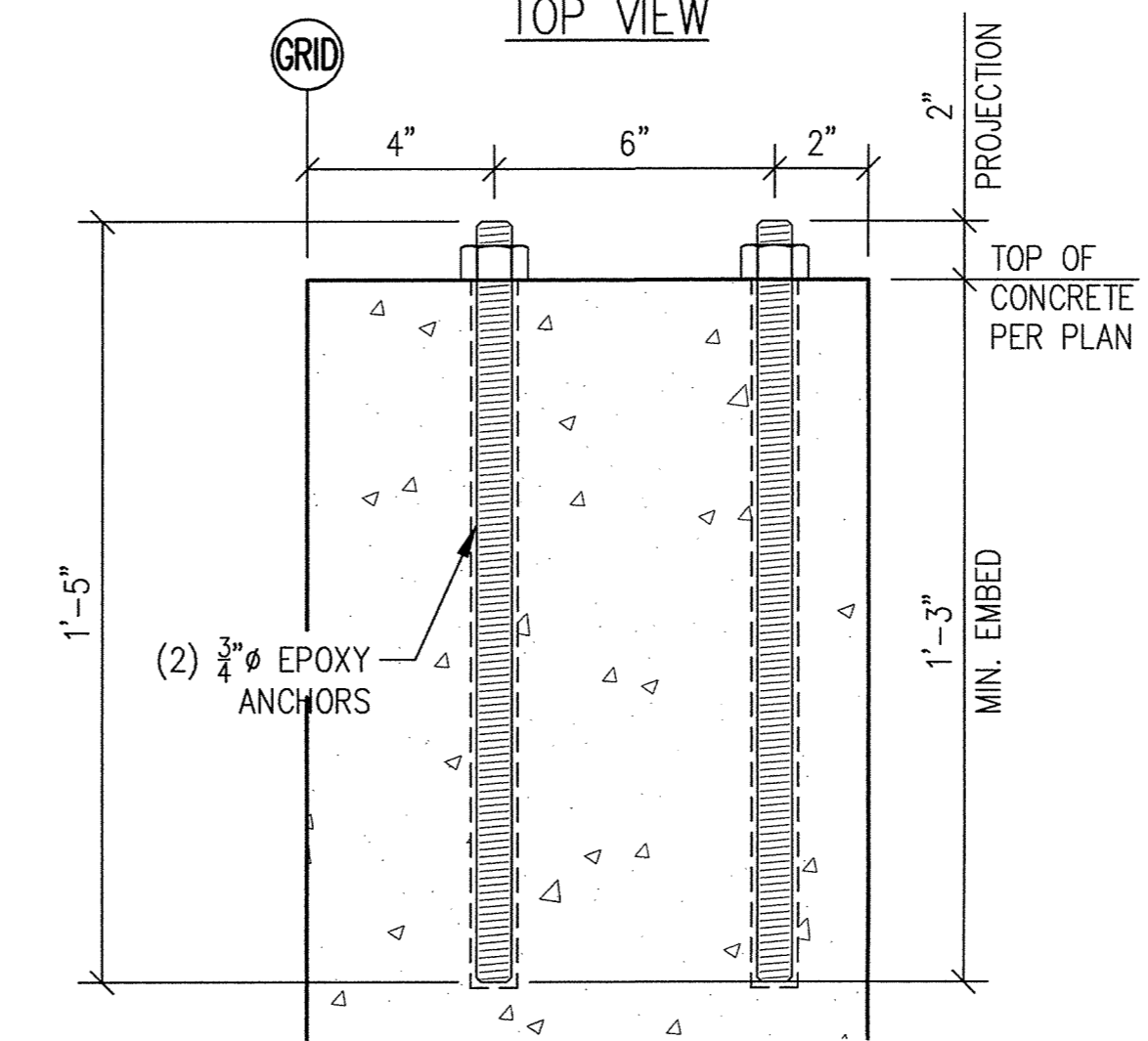
C ANCHOR ROD DETAIL
N.T.S.



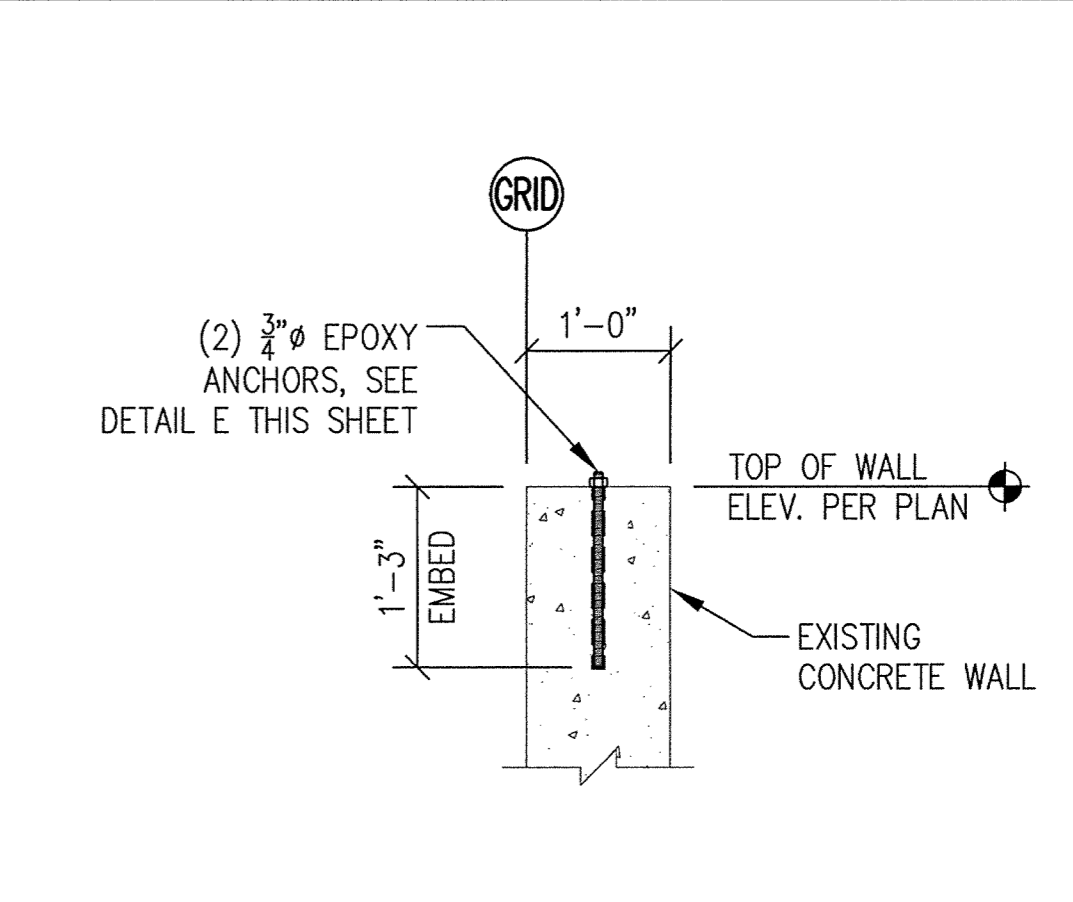
F4 CORNER FOOTING DETAIL
3/4"=1'-0"



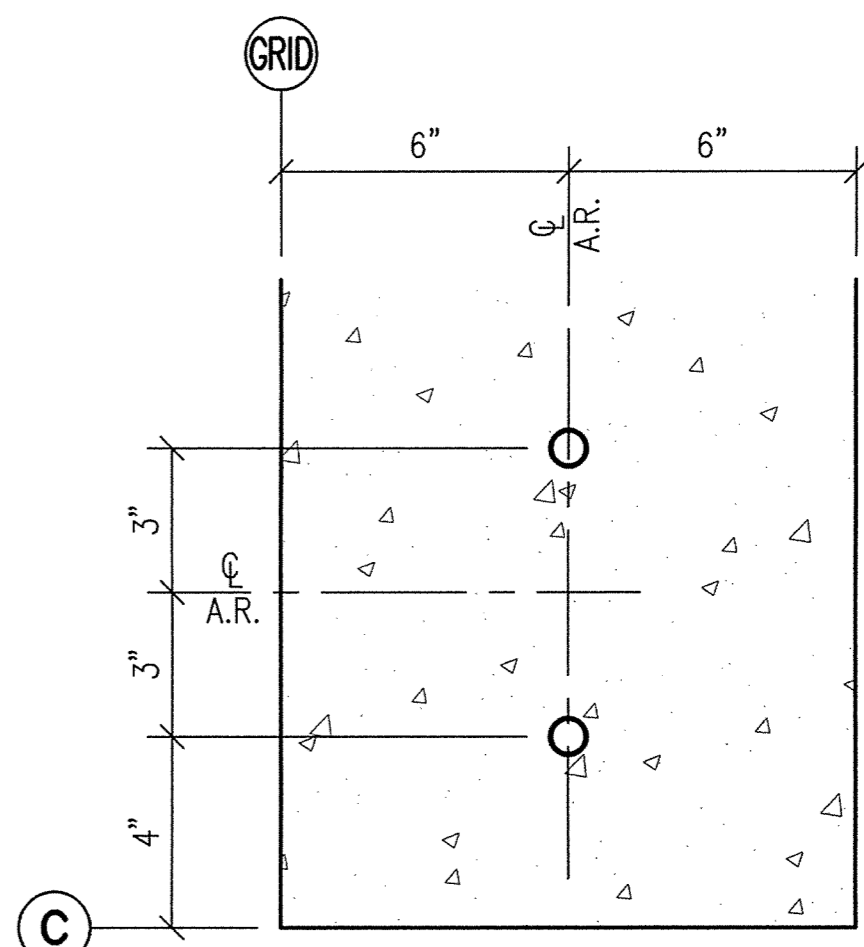
TOP VIEW



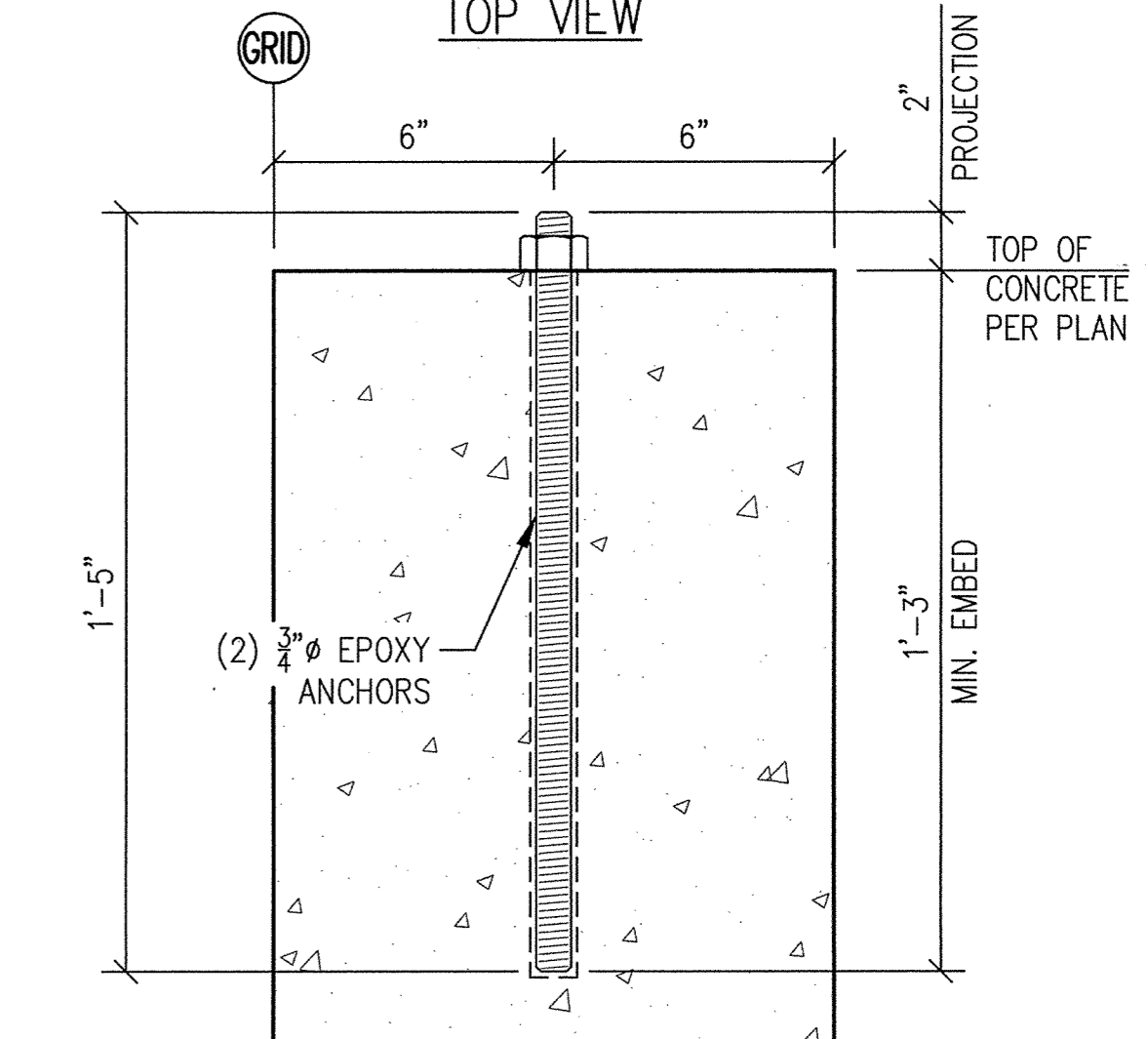
D ANCHOR ROD DETAIL
N.T.S.



F5 CORNER FOOTING DETAIL
3/4"=1'-0"



TOP VIEW



E ANCHOR ROD DETAIL
N.T.S.

MOUNTAIN VIEW ENGINEERING, INC.
Structural Engineering Consulting
345 North Main Street Ste. A, Brigham City, Utah 84302 (435) 734-9700 Fax (435) 734-9519

SHEET TITLE: **FOUNDATION DETAILS**
JOB NAME: **PLEASANT VIEW CITY SHED**
LOCATION: **PLEASANT VIEW, UTAH**
CONTRACTOR: **-**

PLAN ISSUE DATES	
DATE	DESCRIPTION
12-20-16	T.W. FOR CONSTRUCTION

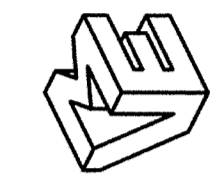
PROFESSIONAL SEAL
JAMES O. MOORE
STATE OF UTAH
DEC 20 2016

SHEET NUMBER:
F-2

DRAWN BY:	T.W.
ENGINEER:	J.O. MOORE
M/E JOB NUMBER:	160633

SPECIAL BOLTS ROOF PLAN				
○ ID	QUAN	TYPE	DIA	WASH
1	2	GR_5	1/2"	2

MEMBER TABLE ROOF PLAN	
MARK	PART
P-1	10X35Z12
P-2	10X35Z14
P-3	10X35Z14
P-4	10X35Z12
E-1	10ESI112
E-2	10ESI112
E-3	10ESI112
E-4	10ESIL14
E-5	10ESIL14
E-6	10ESIL14



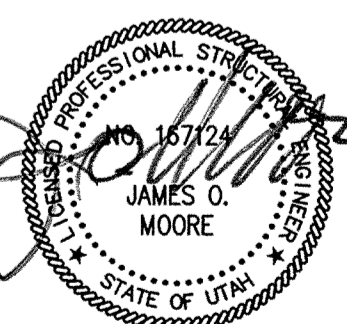
**MOUNTAIN VIEW
ENGINEERING, INC.**

Structural Engineering Consulting
345 North Main Street Ste. A, Brigham City, Utah 84302 (435) 734-9700 Fax (435) 734-9519

SHEET TITLE: **ROOF FRAMING PLAN**
JOB NAME: **PLEASANT VIEW CITY SHED**
LOCATION: **PLEASANT VIEW, UTAH**
CONTRACTOR: -

PLAN ISSUE DATES

DATE	BY	DESCRIPTION
12-20-16	J.J.	FOR CONSTRUCTION

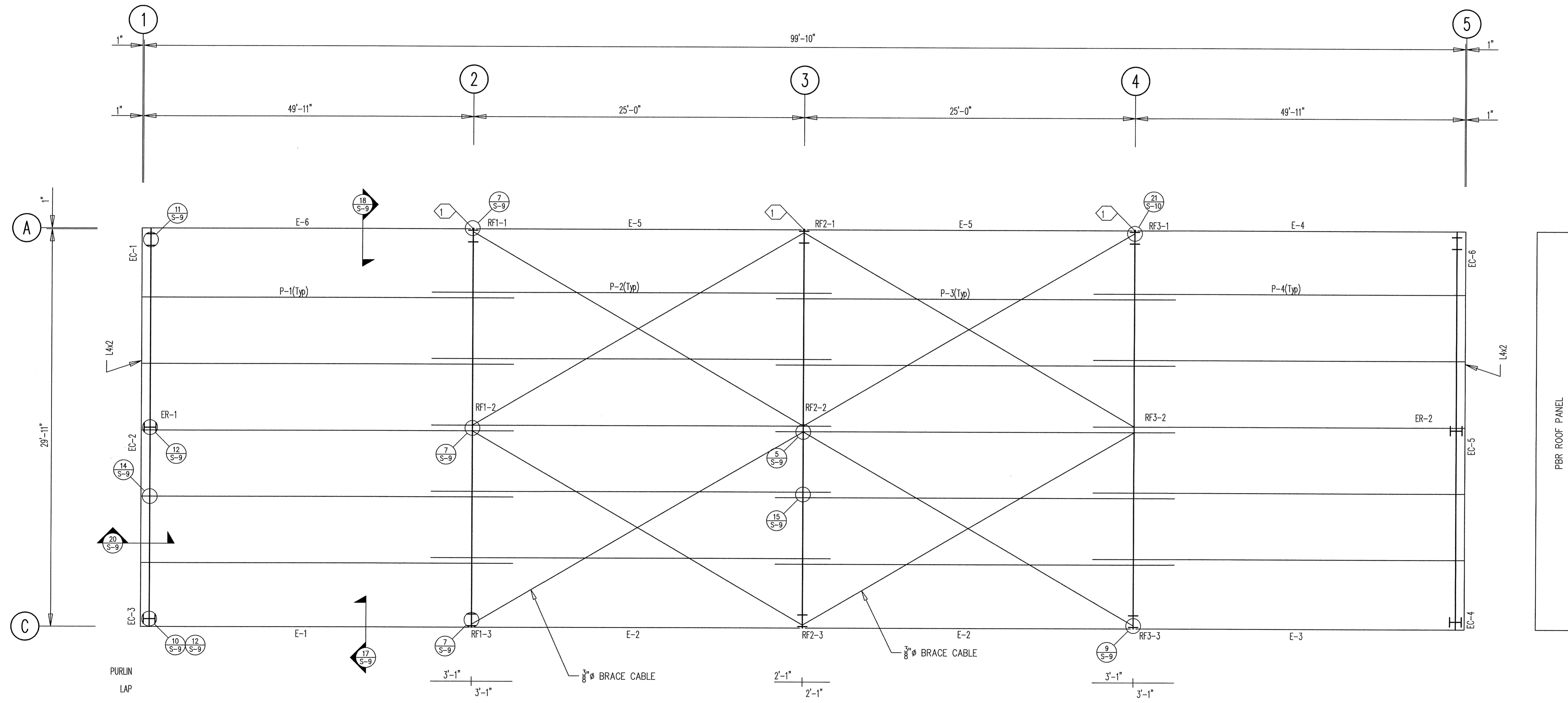


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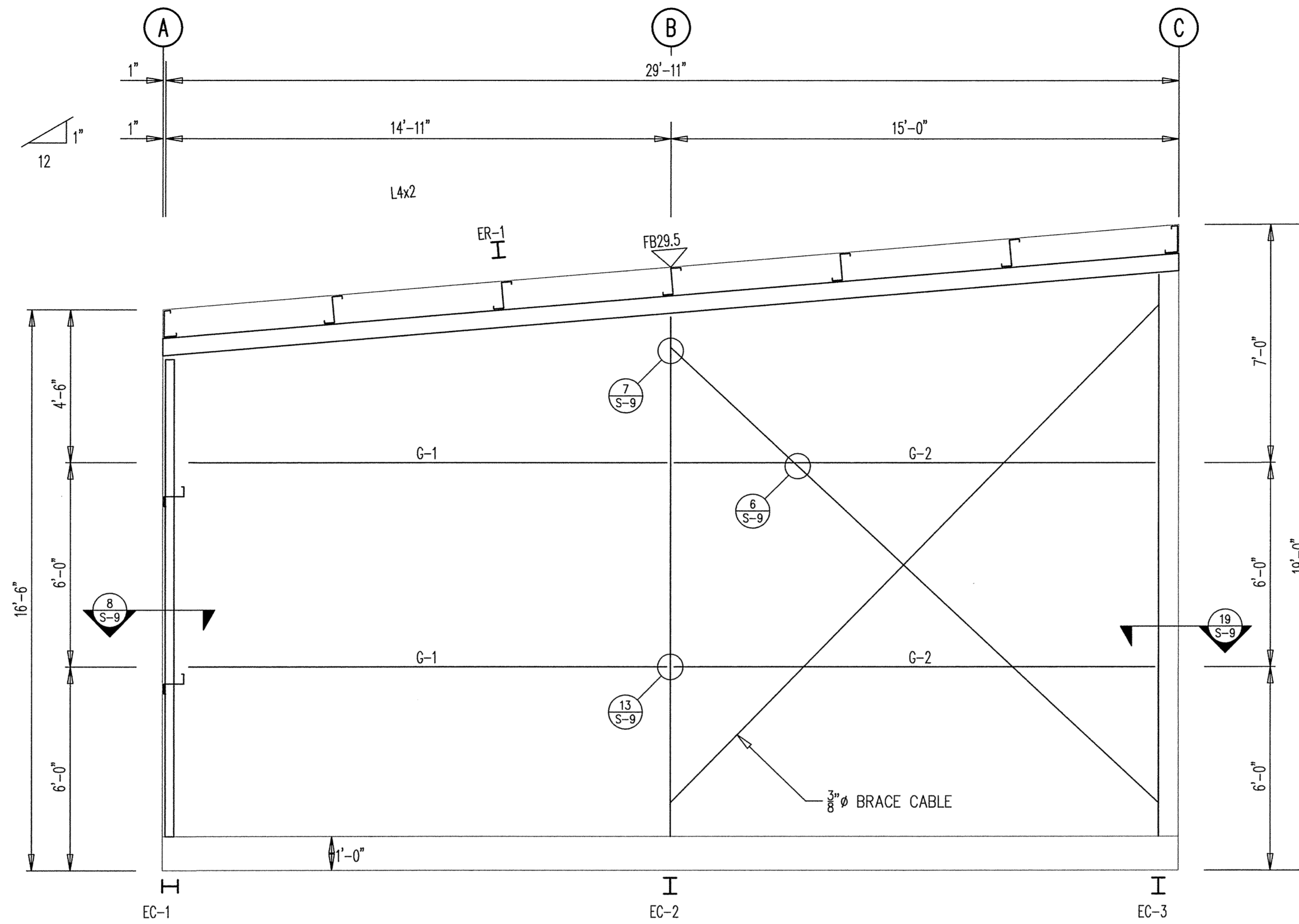
SHEET NUMBER:

S-1

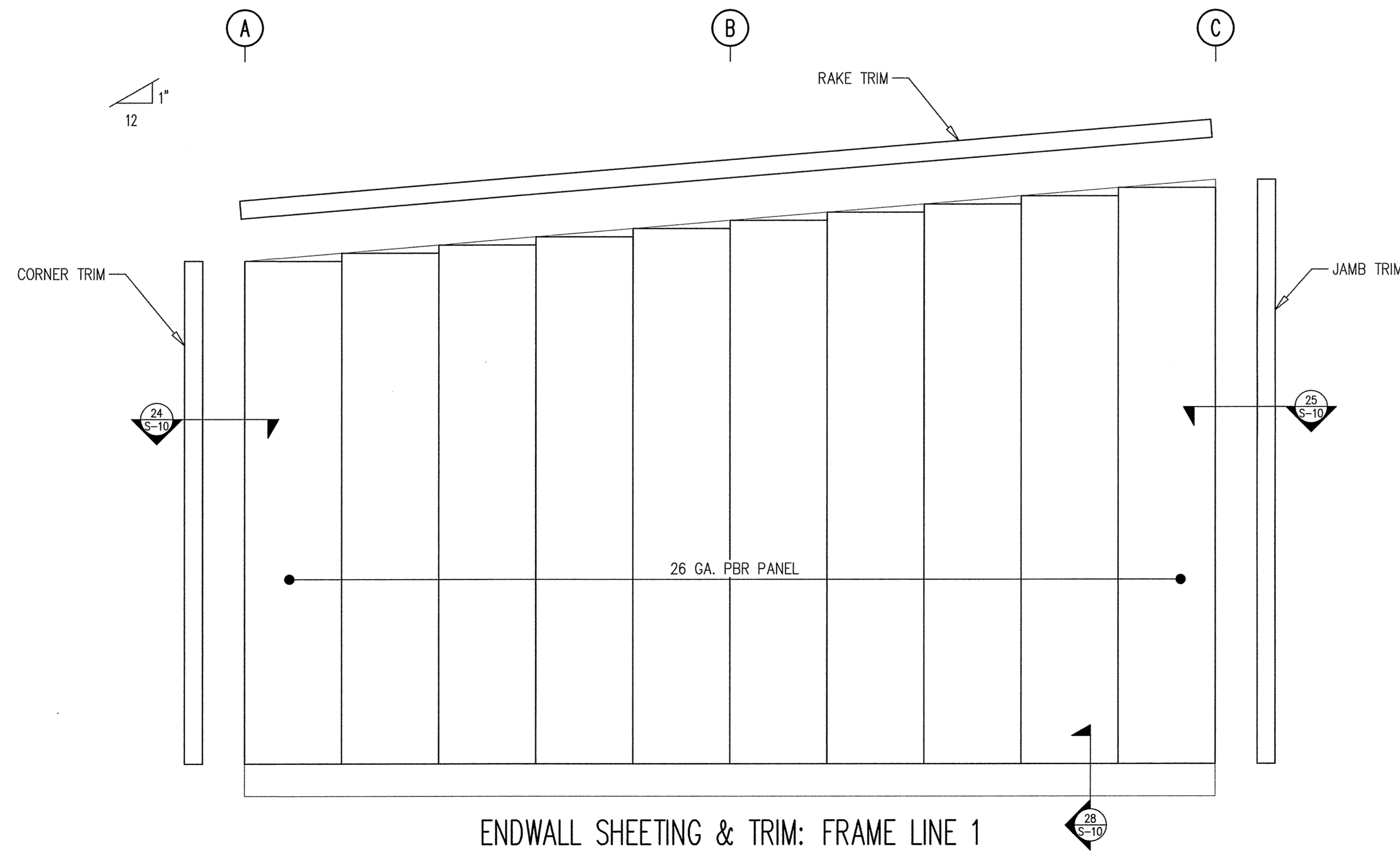
DRAWN BY:	J.J.
ENGINEER:	J.O. MOORE
MVE JOB NUMBER:	160633



ROOF FRAMING PLAN



ENDWALL FRAMING: FRAME LINE 1

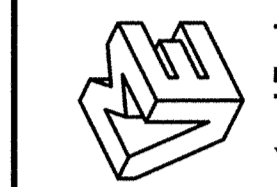


ENDWALL SHEETING & TRIM: FRAME LINE 1

BOLT TABLE			
FRAME LINE 1			
LOCATION	QUAN	TYPE	DIA
Columns/Raf	4	A325	5/8"

MEMBER TABLE	
FRAME LINE 1	
MARK	PART
EC-1	WBX10
EC-2	WBX10
EC-3	WBX10
ER-1	WBX10
G-1	8X25Z16
G-2	8X25Z16

FLANGE BRACE TABLE	
FRAME LINE 1	
∇ID	MARK
1	FB29.5



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Structural Engineering Consulting Design
345 North Main Street Ste. A, Brigham City, Utah 84302 (435) 734-9700 Fax (435) 734-9519

SHEET TITLE: FRAMING / SHEETING ELEVATIONS

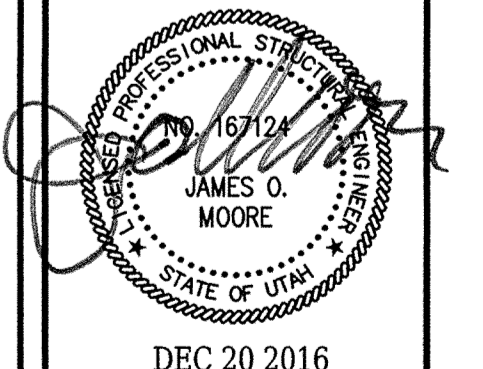
JOB NAME: PLEASANT VIEW CITY SHED

LOCATION: PLEASANT VIEW, UTAH

CONTRACTOR: *

PLAN ISSUE DATES

DATE	BY	DESCRIPTION
12-20-16	J.J.	FOR CONSTRUCTION

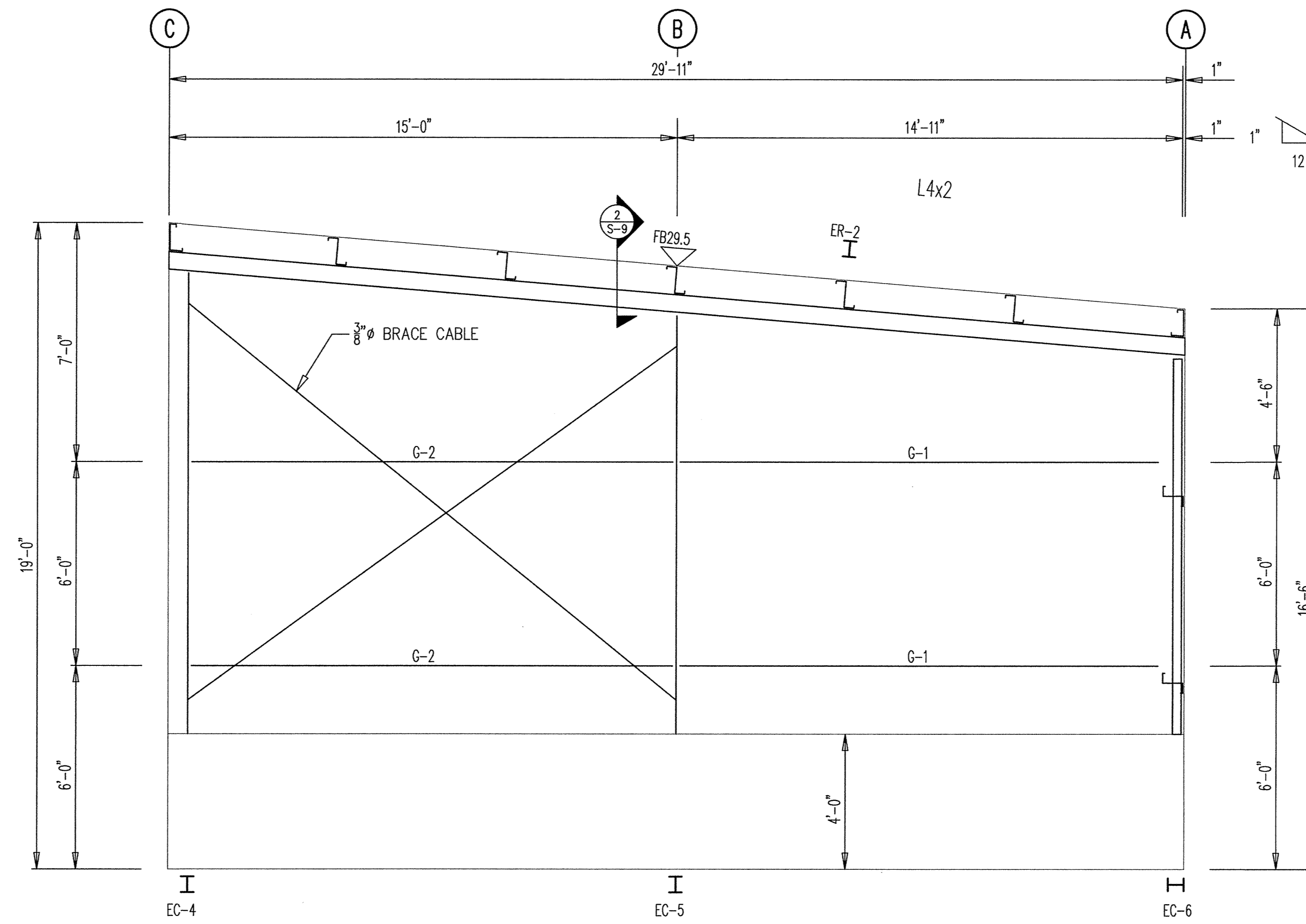


DEC 20 2016

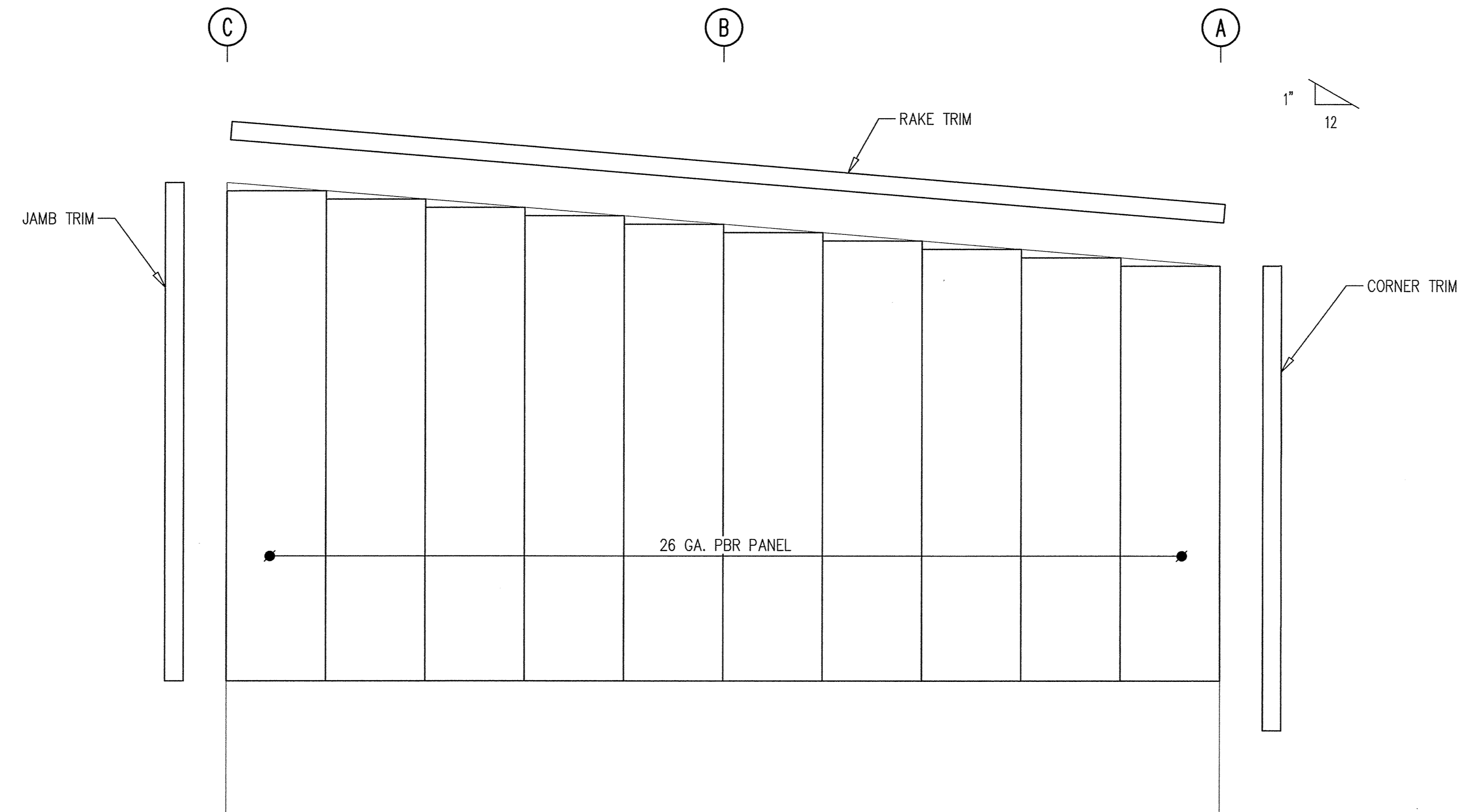
SHEET NUMBER:

S-2

DRAWN BY:	J.J.
ENGINEER:	J.O. MOORE
M/E JOB NUMBER:	160633



ENDWALL FRAMING: FRAME LINE 5

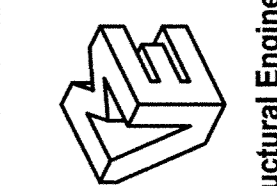


ENDWALL SHEETING & TRIM: FRAME LINE 5

BOLT TABLE			
FRAME LINE 5			
LOCATION	QUAN	TYPE	DIA
Columns/Raf	4	A325	5/8"

MEMBER TABLE	
FRAME LINE 5	
MARK	PART
EC-4	W8X10
EC-5	W8X10
EC-6	W8X10
ER-2	W8X10
G-1	8X25Z16
G-2	8X25Z16

FLANGE BRACE TABLE	
FRAME LINE 5	
ID	MARK
1	FB29.5

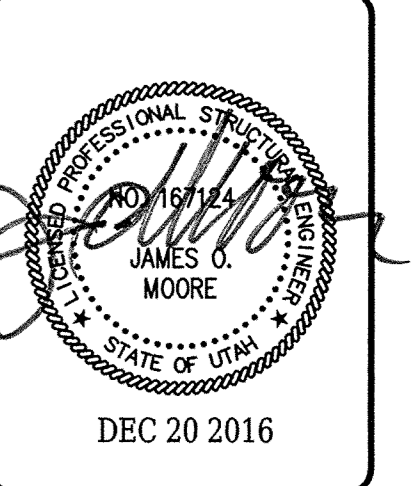


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Consulting
Structural Engineering
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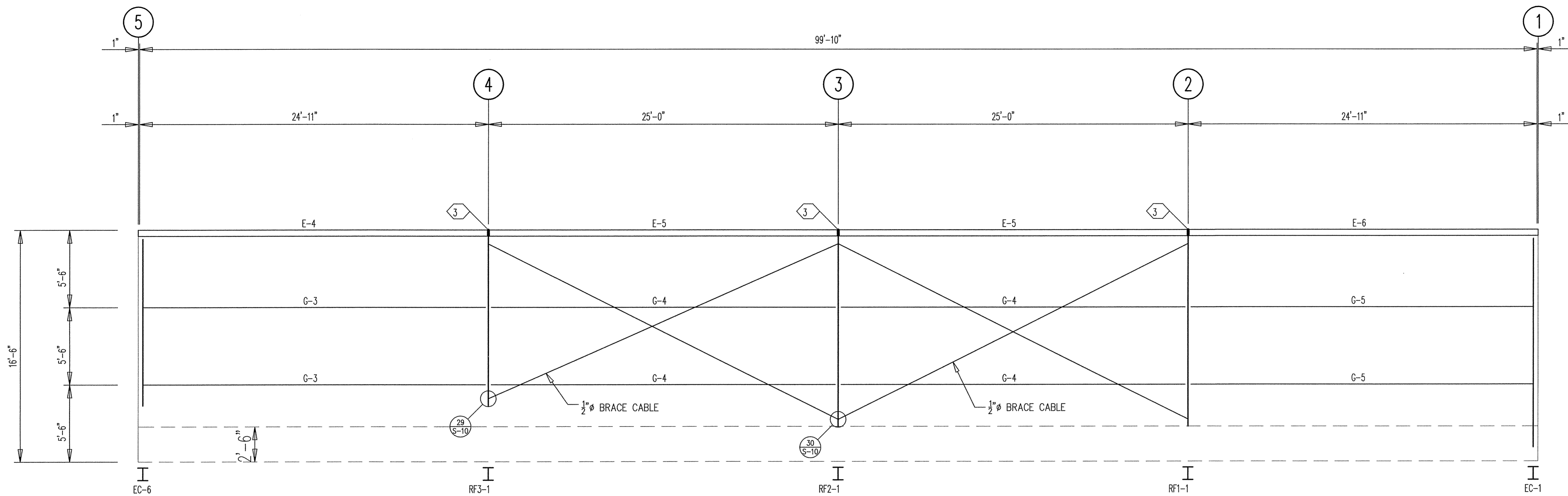
SHEET TITLE: **FRAMING / SHEETING ELEVATIONS**
 JOB NAME: **PLEASANT VIEW CITY SHED**
 LOCATION: **PLEASANT VIEW, UTAH**
 CONTRACTOR: -

PLAN / ISSUE DATES	
DATE	DESCRIPTION
12-20-16	J.J. FOR CONSTRUCTION



SHEET NUMBER:
S-3

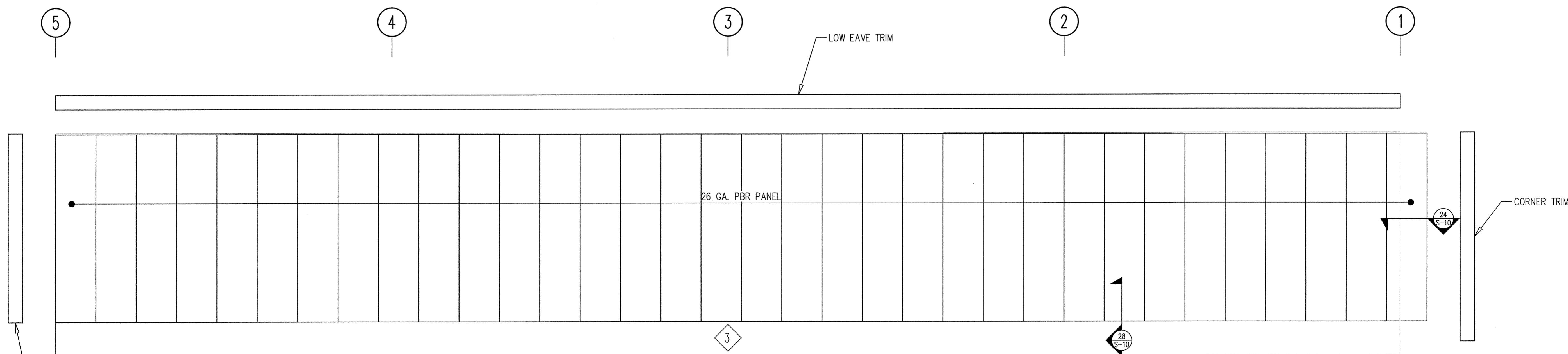
DRAWN BY:	J.J.
ENGINEER:	J.O. MOORE
AVE JOB NUMBER:	160633



SIDEWALL FRAMING: FRAME LINE A

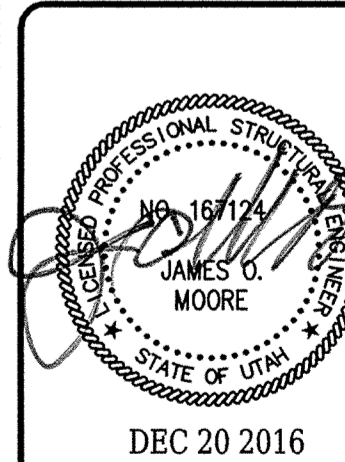
SPECIAL BOLTS				
◇ ID	QUAN	TYPE	DIA	WASH
3	2	GR_5	1/2"	2

MEMBER TABLE FRAME LINE A	
MARK	PART
E-4	10ES1L14
E-5	10ES1L14
E-6	10ES1L14
G-3	8X25Z12
G-4	8X25Z12
G-5	8X25Z12



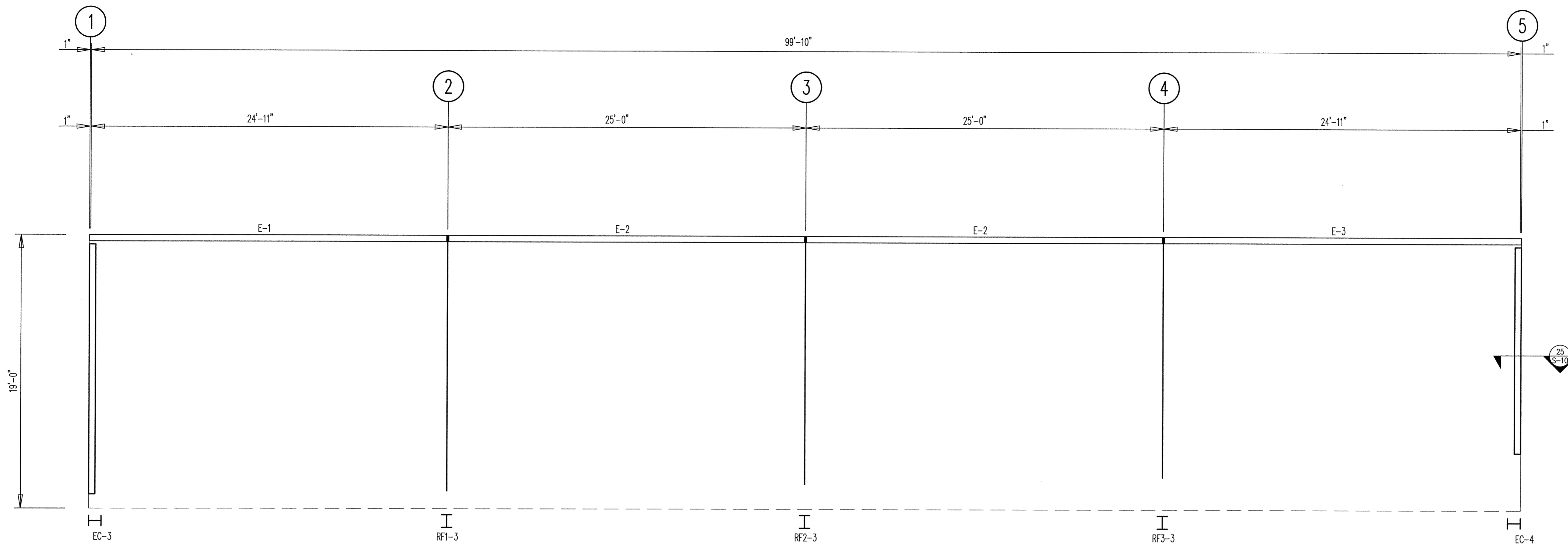
SIDEWALL SHEETING & TRIM: FRAME LINE A

PLAN ISSUE DATES	
DATE	DESCRIPTION
12-20-16	J.J. FOR CONSTRUCTION



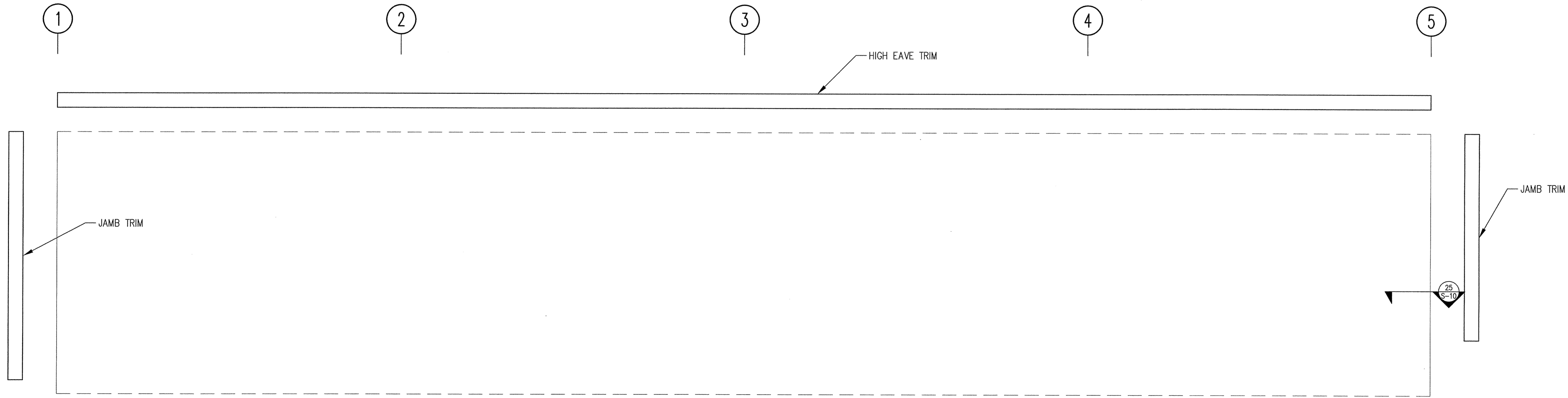
SHEET NUMBER:
S-4

DRAWN BY:	J.J.
ENGINEER:	J.C. MOORE
M/E JOB NUMBER:	160633

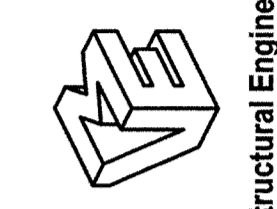


SIDEWALL FRAMING: FRAME LINE C

MEMBER TABLE FRAME LINE C	
MARK	PART
E-1	10ES1H12
E-2	10ES1H12
E-3	10ES1H12



SIDEWALL SHEETING & TRIM: FRAME LINE C



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345 North Main Street Ste. A, Brigham City, Utah 84302 (435) 734-9700 Fax (435) 734-9519

SHEET TITLE: **FRAMING / SHEETING ELEVATIONS**

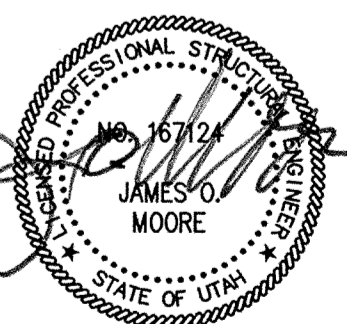
JOB NAME: **PLEASANT VIEW CITY SHED**

LOCATION: **PLEASANT VIEW, UTAH**

CONTRACTOR: -

PLAN ISSUE DATES

DATE	BY: DESCRIPTION
12-20-16	J.J. FOR CONSTRUCTION



DEC 20 2016

SHEET NUMBER:

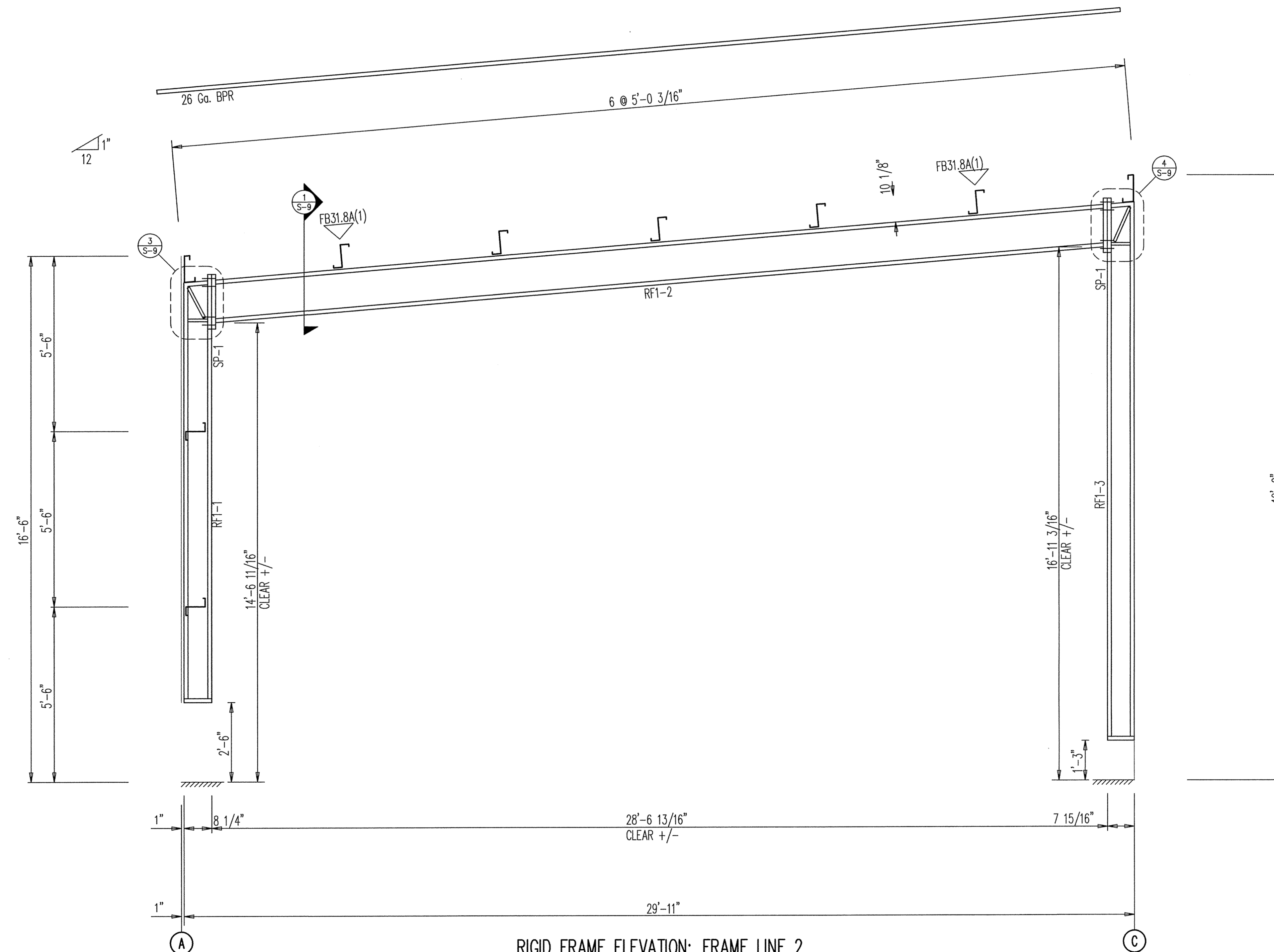
S-5

DRAWN BY:	J.J.
ENGINEER:	J.O. MOORE
MVE JOB NUMBER:	160633

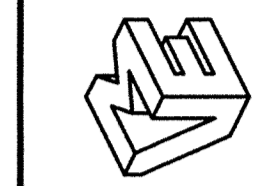
SPlice Bolt Table					
Mark	Qty Top	Qty Bot	Int	Type	Dia
SP-1	4	4	0	A325	0.625

MEMBER SIZE TABLE	
MARK	MEMBER
RF1-1	W8X21
RF1-2	W14X22
RF1-3	W8X24

FLANGE BRACES: Both Sides(U.N.)
 FBxxA(1); xx=length(in)
 A - FB2x14g



RIGID FRAME ELEVATION: FRAME LINE 2



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SHEET TITLE: RIGID FRAME ELEVATION

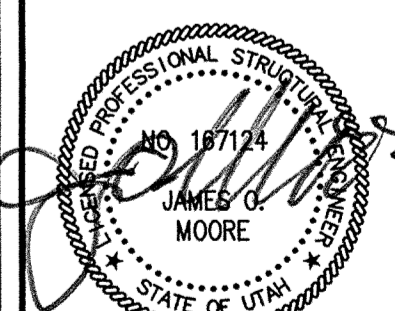
JOB NAME: PLEASANT VIEW CITY SHED

LOCATION: PLEASANT VIEW, UTAH

CONTRACTOR: -

PLAN ISSUE DATES

DATE	BY	DESCRIPTION
12-20-16	J.J.	FOR CONSTRUCTION



DEC 20 2016

SHEET NUMBER:

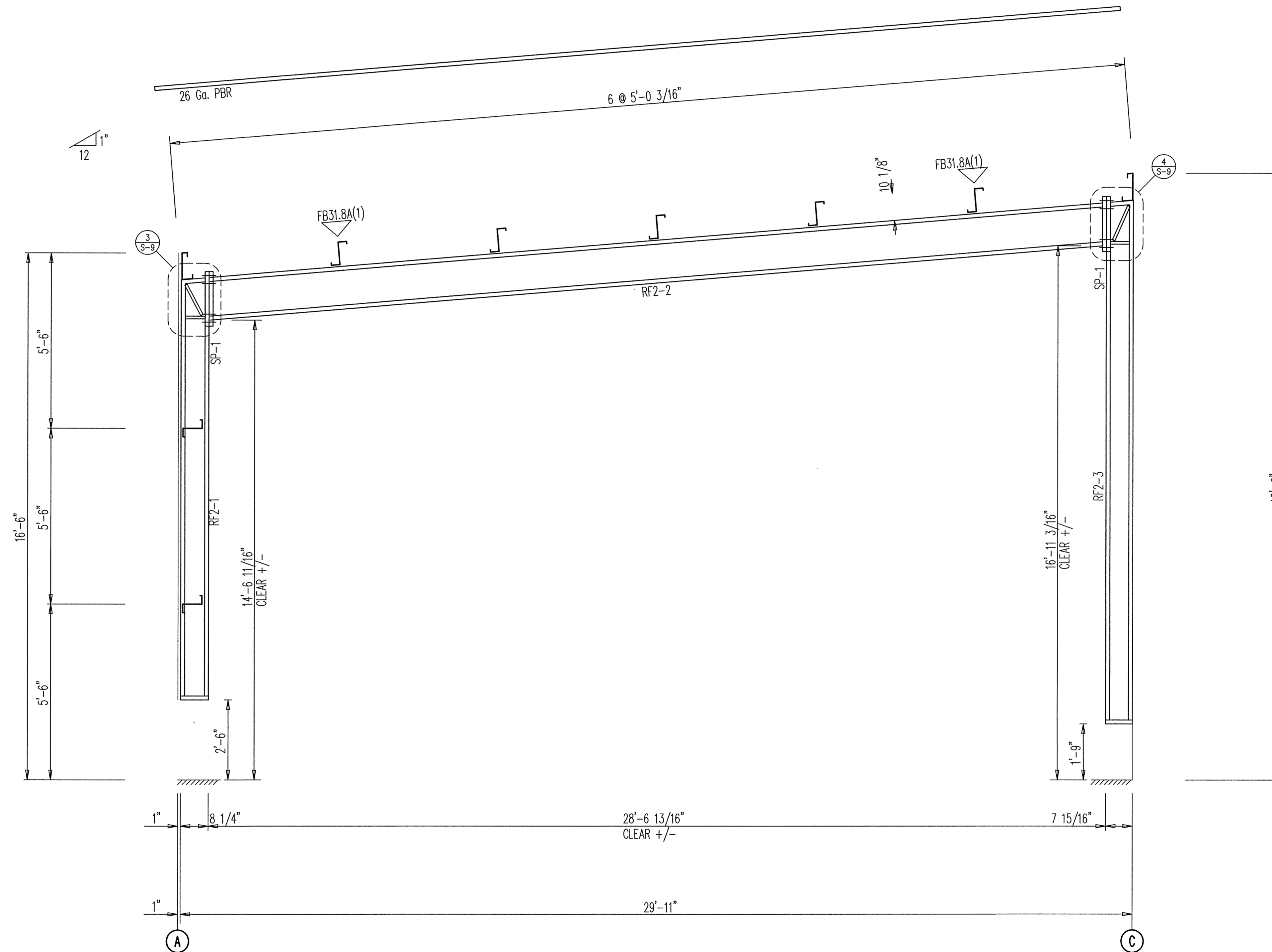
S-6

DRAWN BY: J.J.
 ENGINEER: J.C. MOORE
 M/E. JOB NUMBER: **160633**

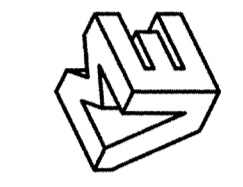
SPLICE BOLT TABLE					
Mark	Qty Top	Qty Bot	Int	Type	Dia
SP-1	4	4	0	A325	0.625

MEMBER SIZE TABLE	
MARK	MEMBER
RF2-1	W8X21
RF2-2	W14X22
RF2-3	W8X24

▽ FLANGE BRACES: Both Sides(U.N.)
 FBxxA(1): xx=length(m)
 A - FB2x14g



RIGID FRAME ELEVATION: FRAME LINE 3



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SHEET TITLE: RIGID FRAME ELEVATION

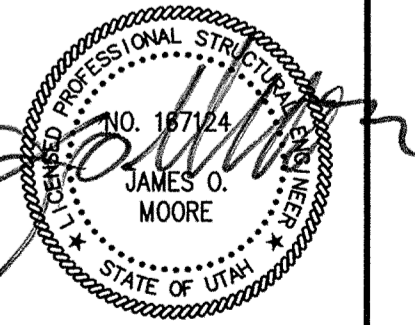
JOB NAME: PLEASANT VIEW CITY SHED

LOCATION: PLEASANT VIEW, UTAH

CONTRACTOR: -

PLAN ISSUE DATES

DATE	BY	DESCRIPTION
12-20-16	J.J.	FOR CONSTRUCTION



DEC 20 2016

SHEET NUMBER:

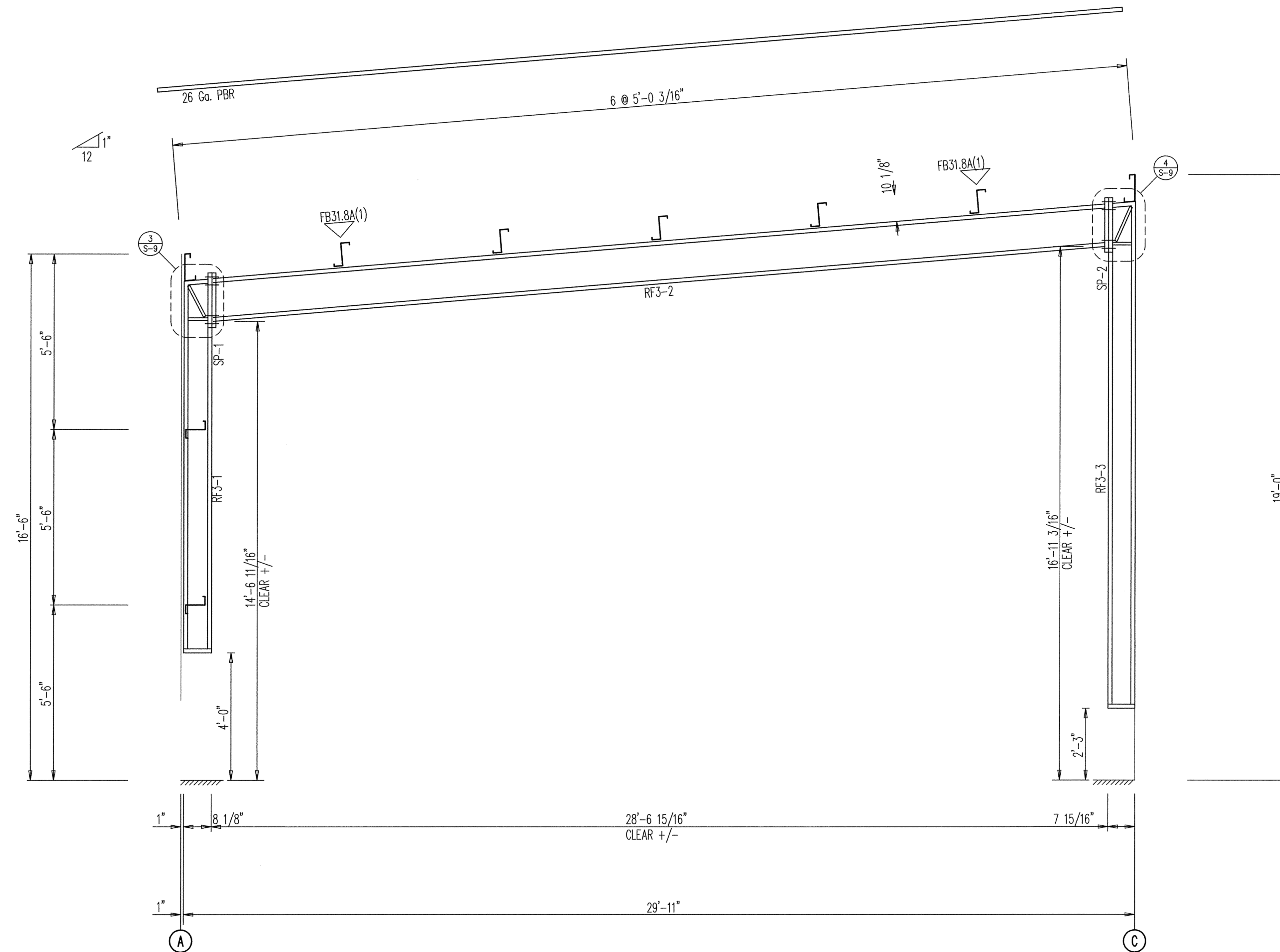
S-7

DRAWN BY:	J.J.
ENGINEER:	J.O. MOORE
W/E, JOB NUMBER:	160633

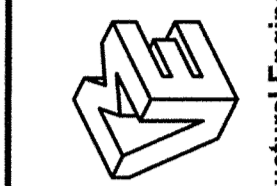
SPLICE BOLT TABLE					
Mark	Qty Top	Qty Bot	Qty Int	Type	Dia
SP-1	4	4	0	A325	0.625
SP-2	4	4	0	A325	0.625

MEMBER SIZE TABLE	
MARK	MEMBER
RF3-1	W8X18
RF3-2	W14X22
RF3-3	W8X24

FLANGE BRACES: Both Sides(U.N.)
 FBxxA(1): xx=length(in)
 A - FB2x14g



RIGID FRAME ELEVATION: FRAME LINE 4



MOUNTAIN VIEW ENGINEERING, INC.

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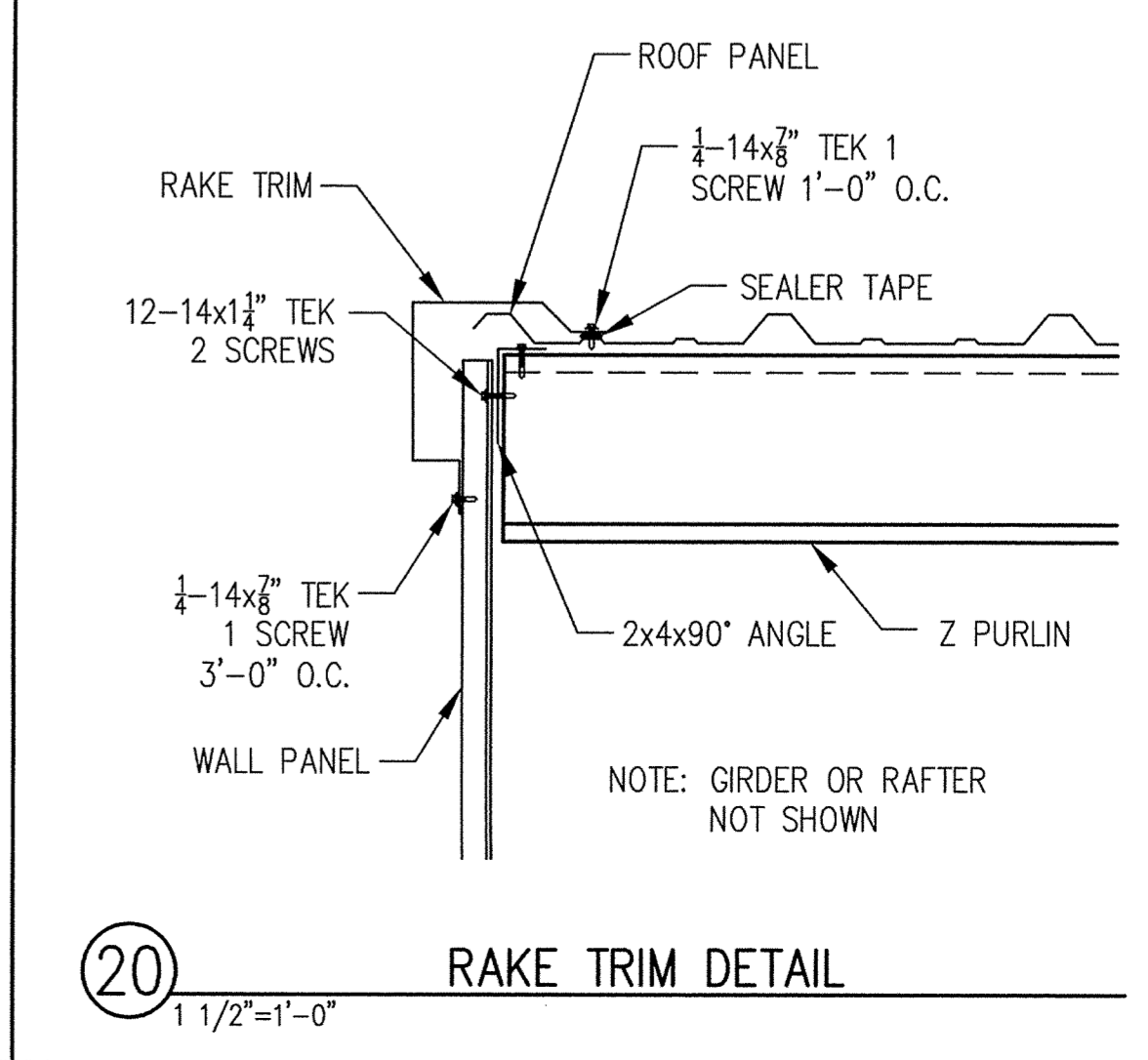
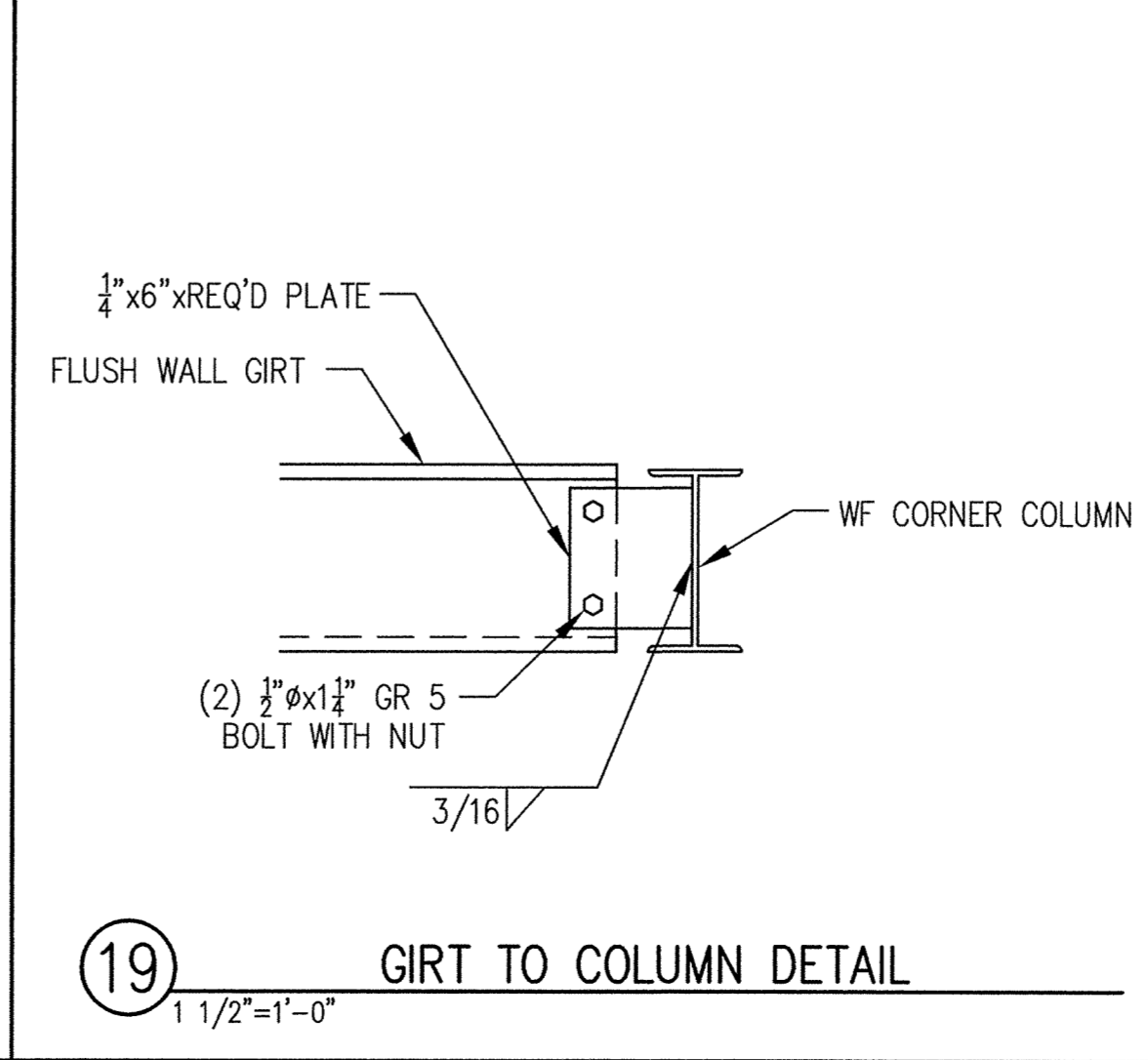
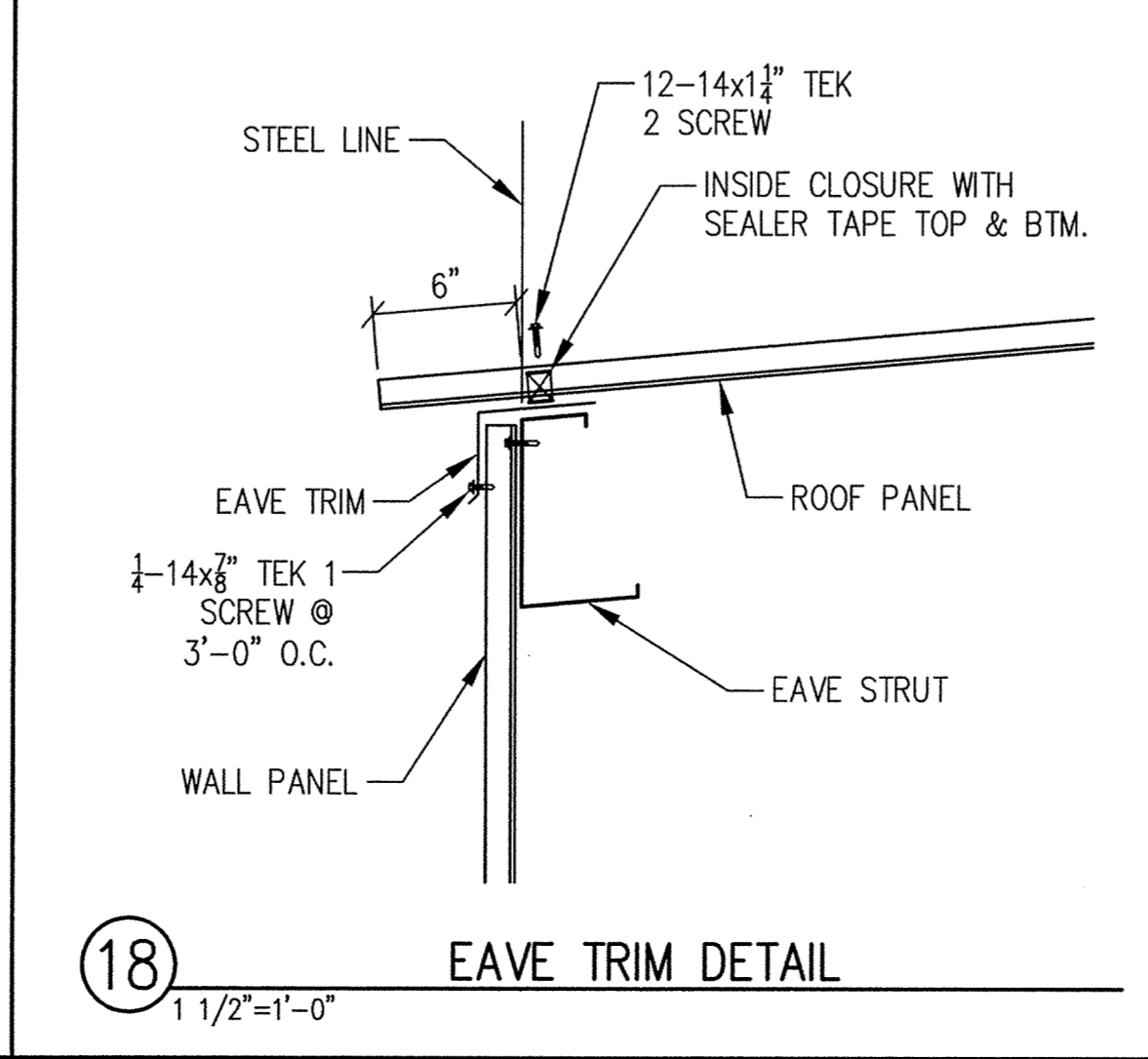
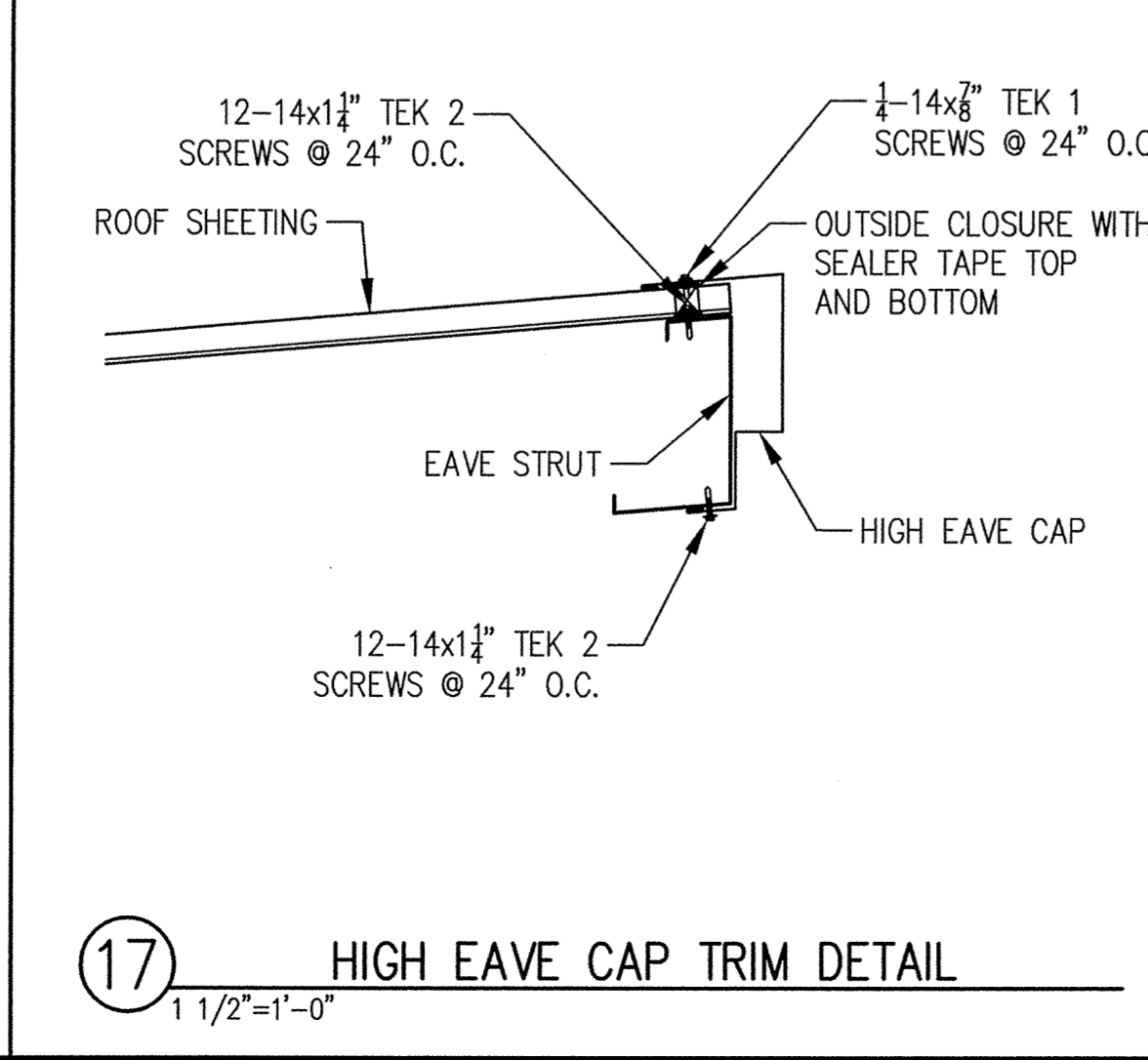
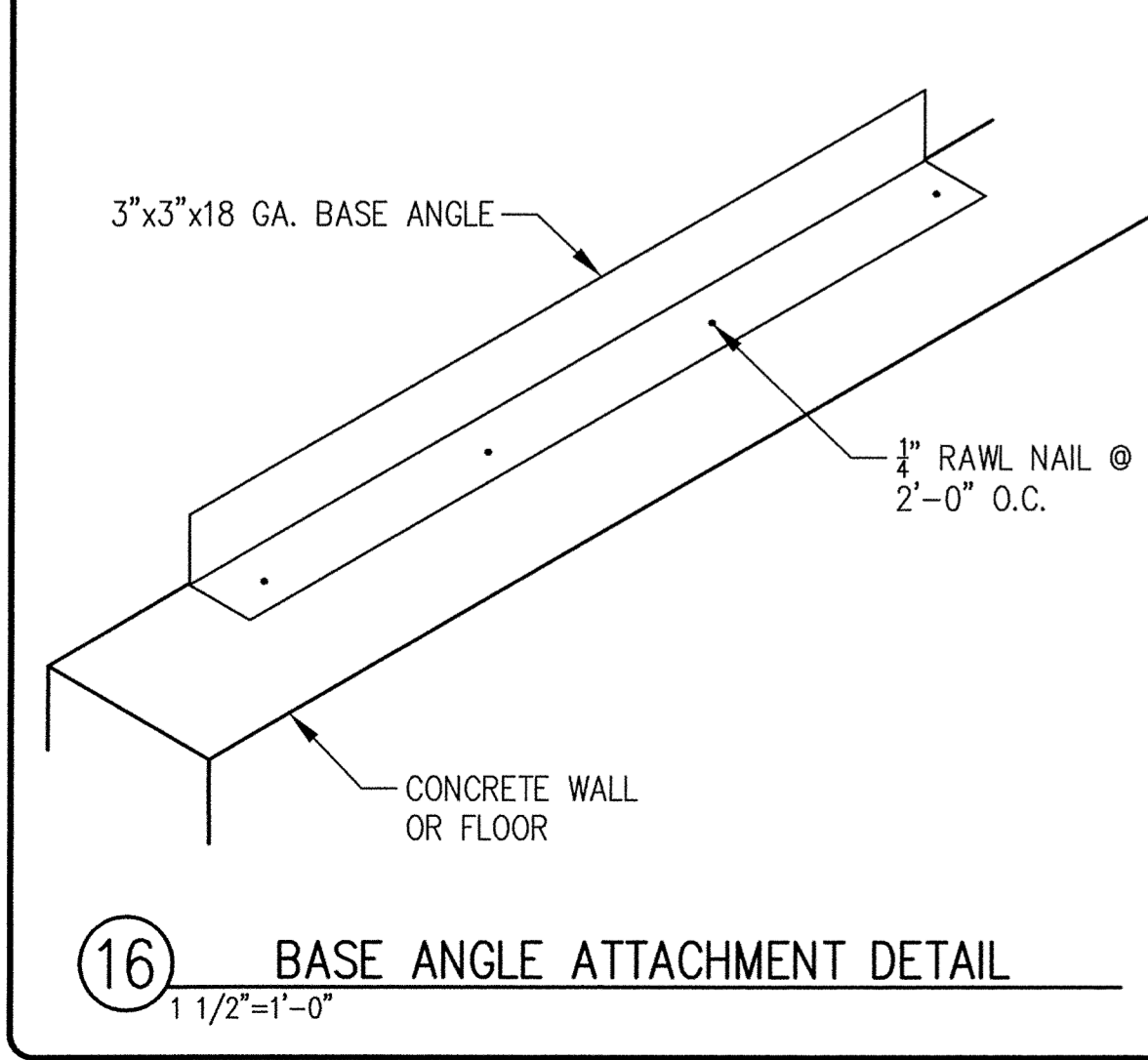
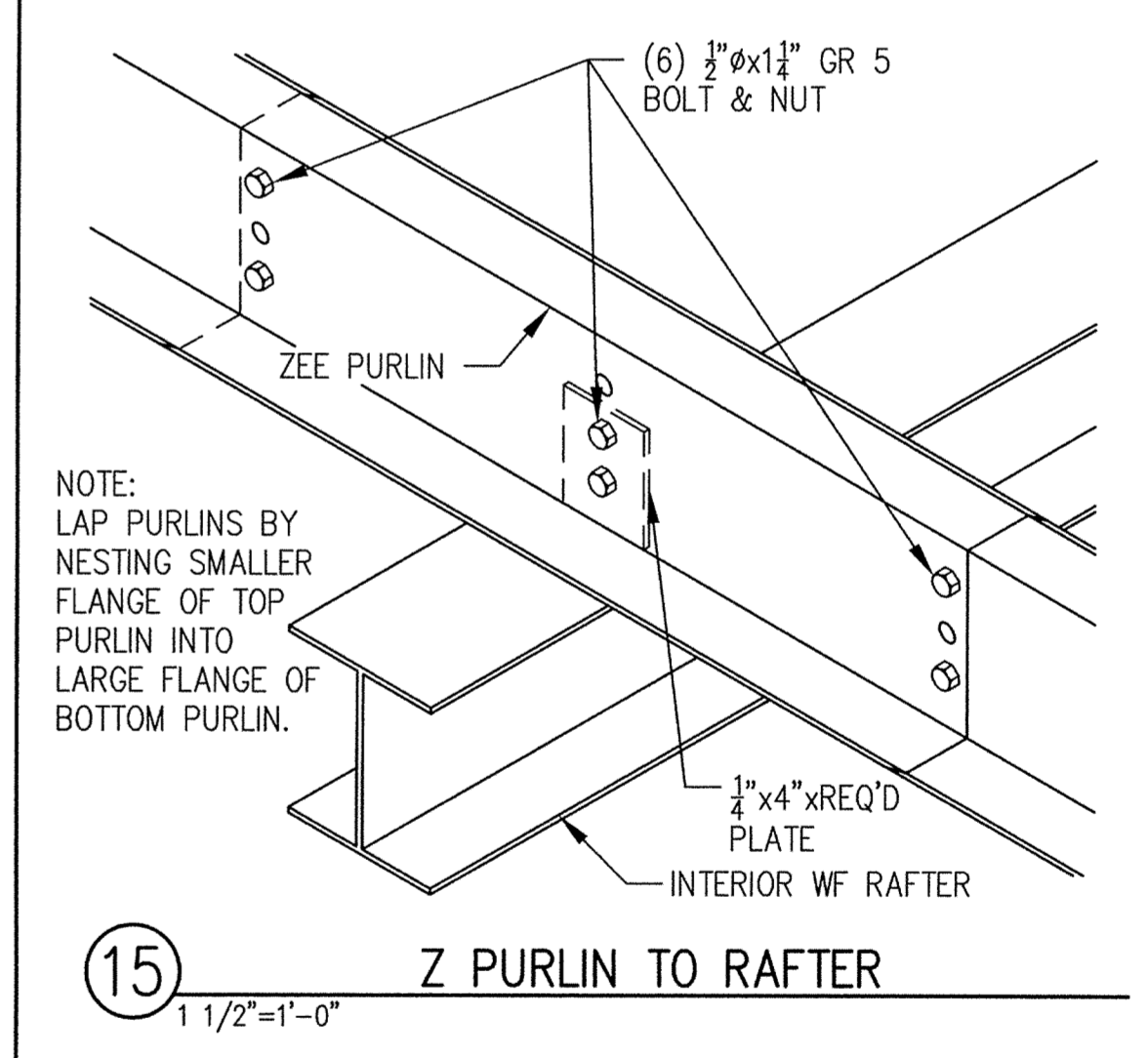
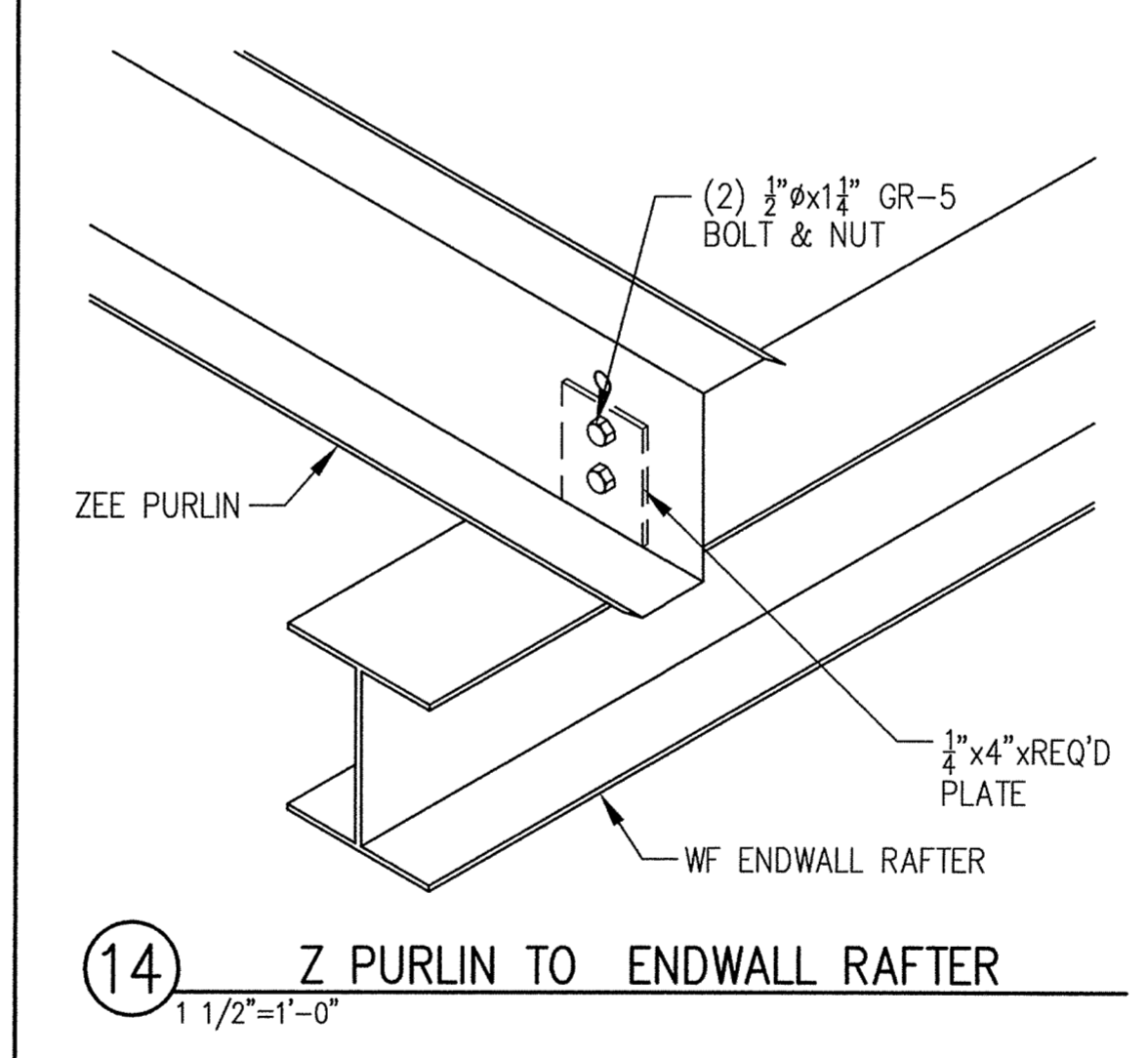
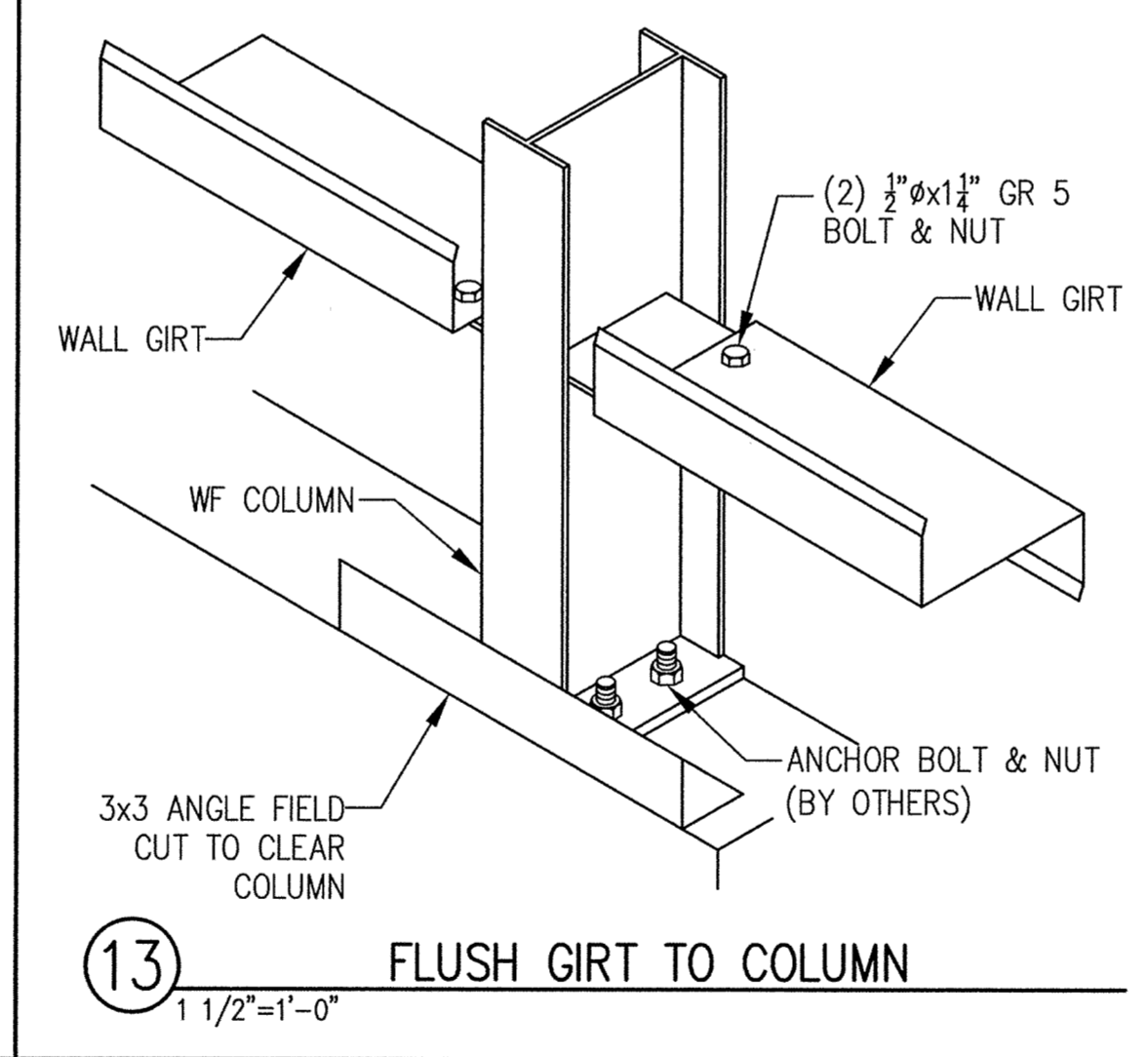
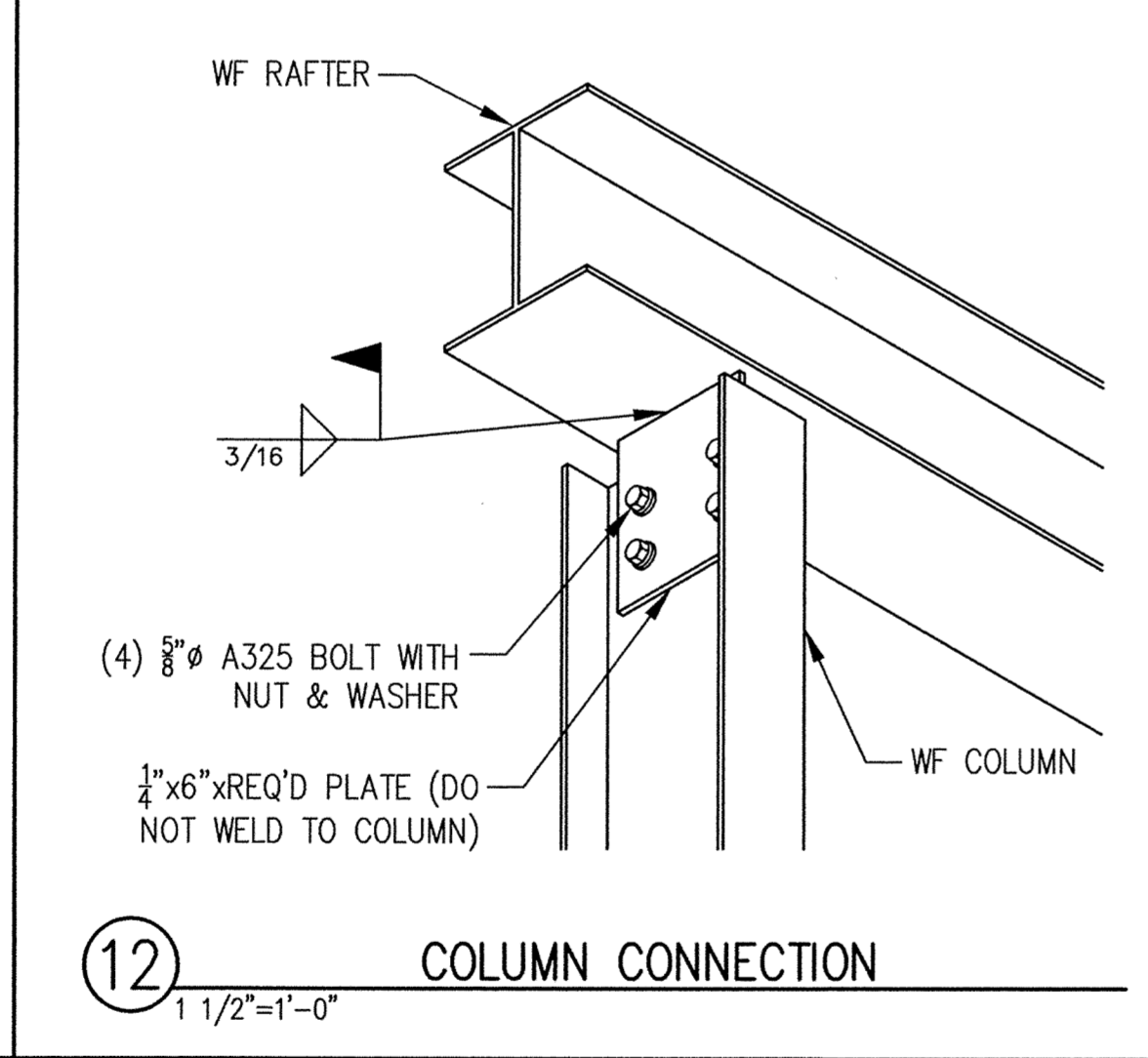
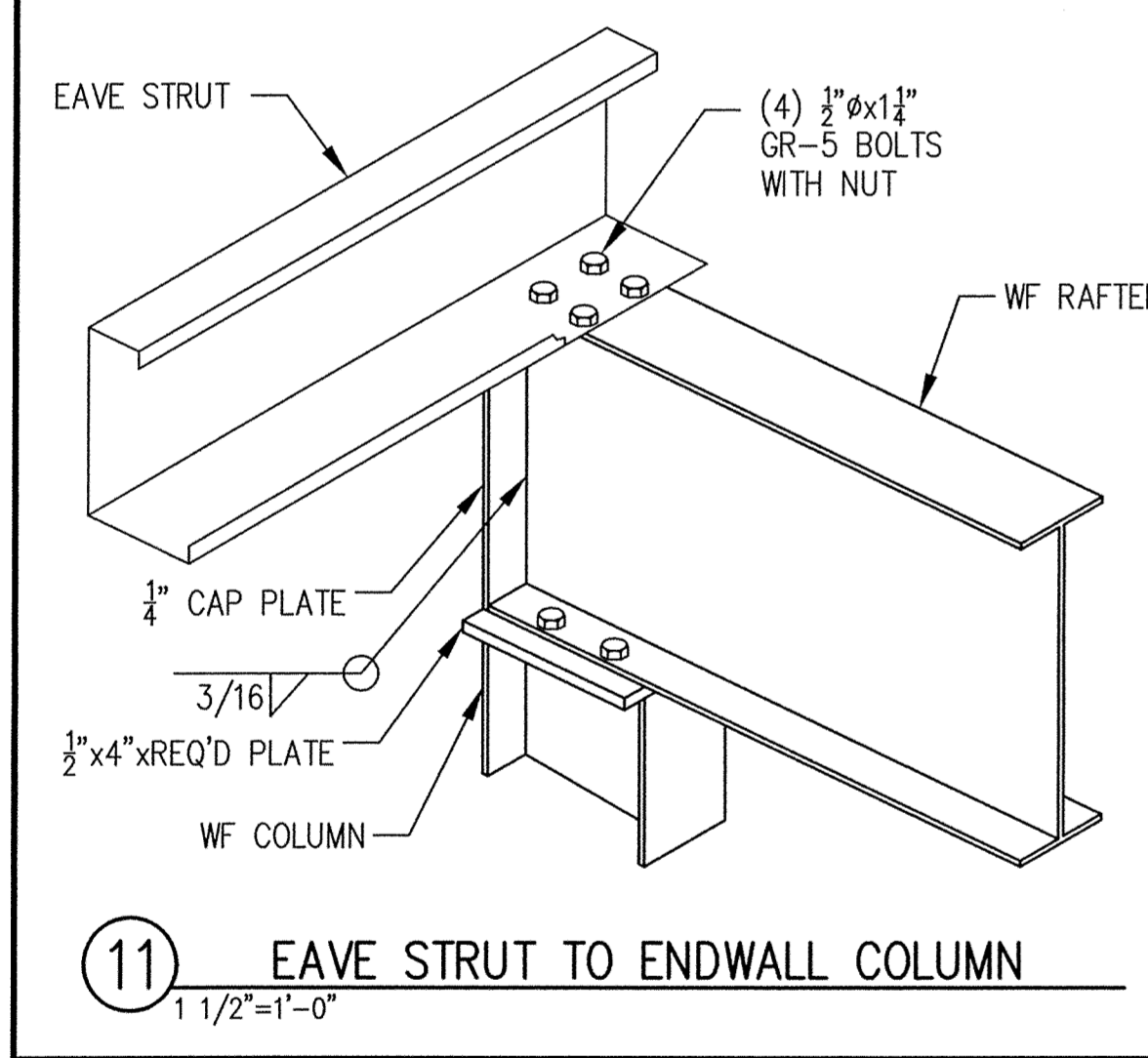
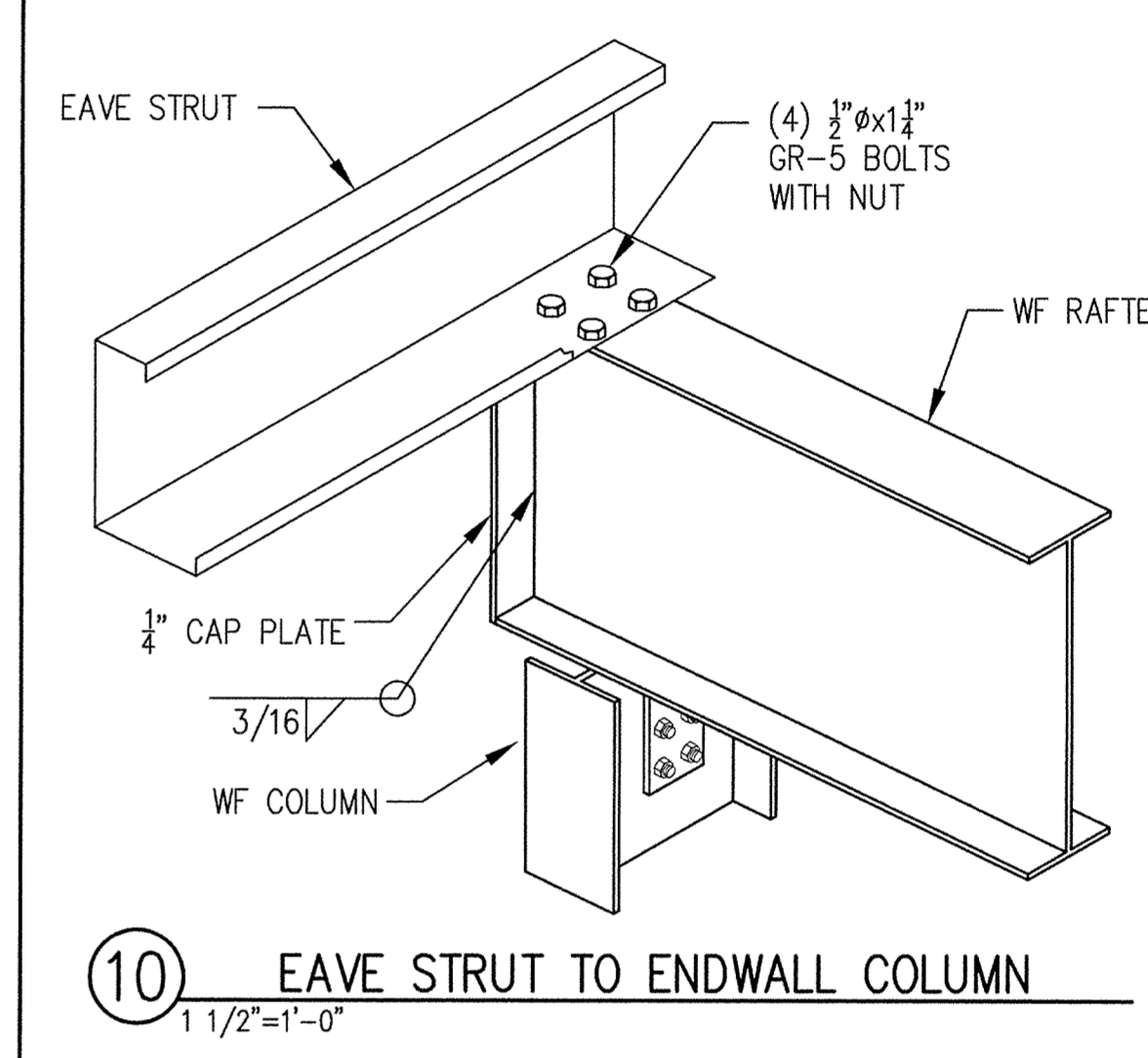
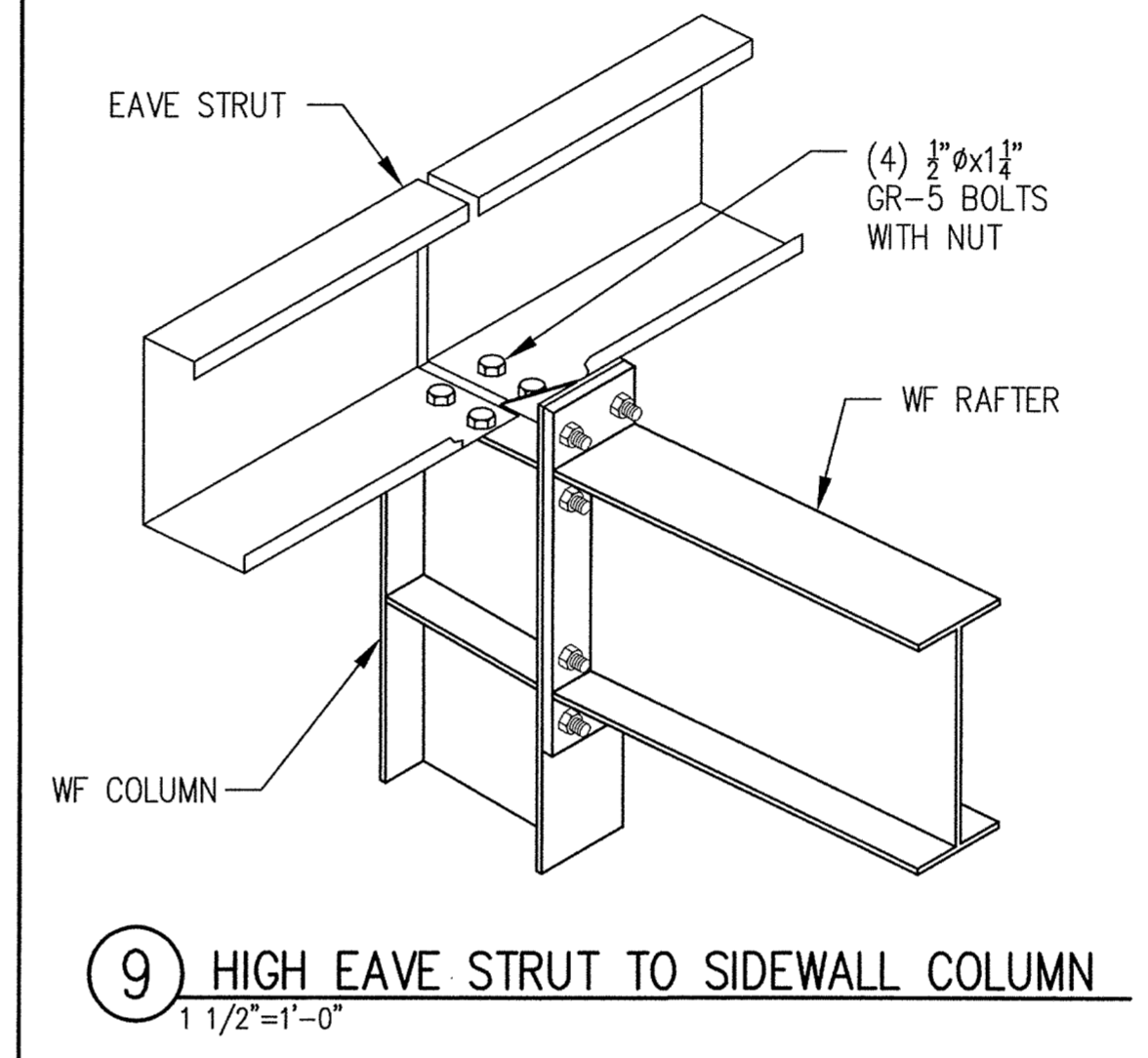
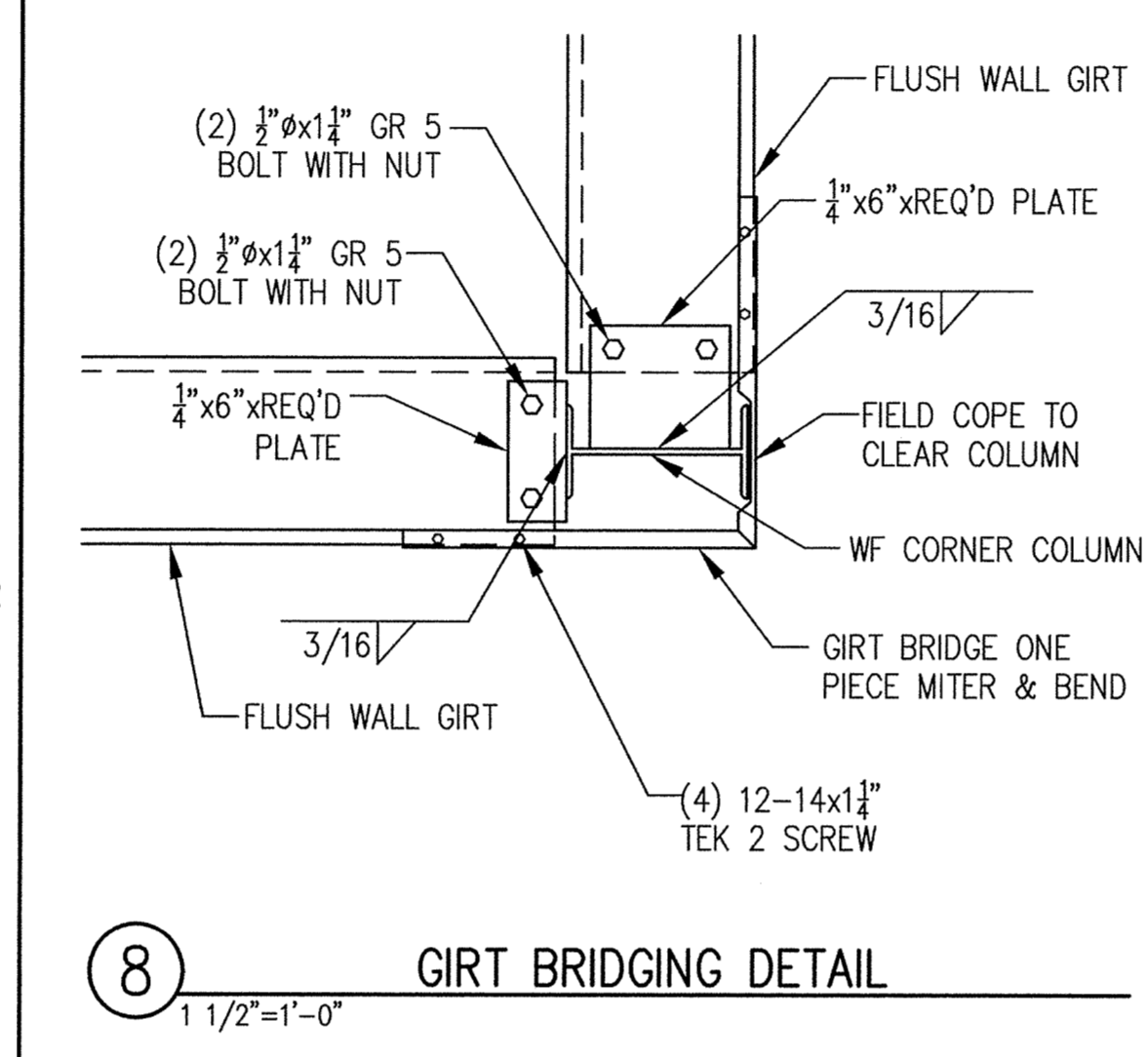
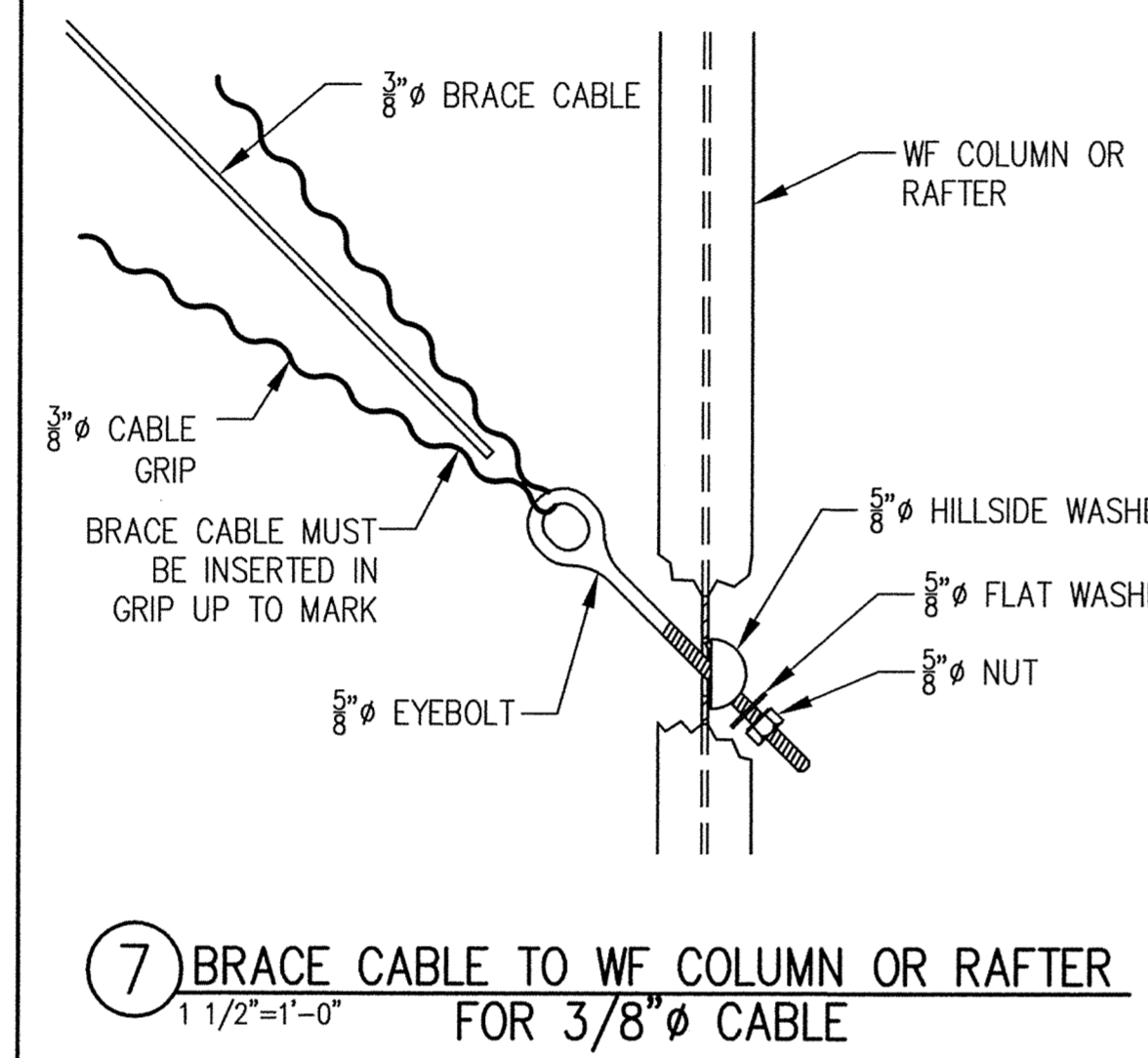
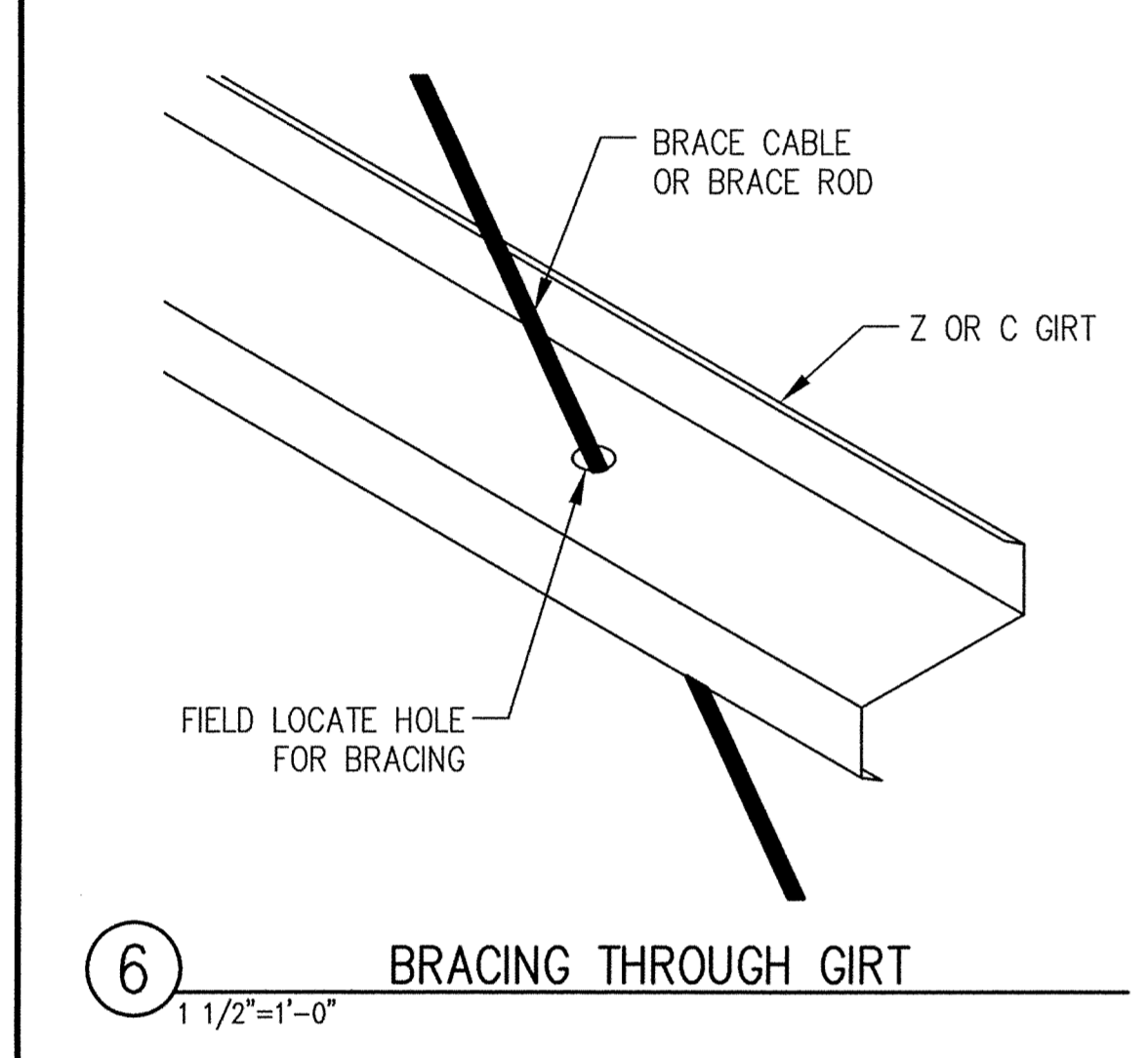
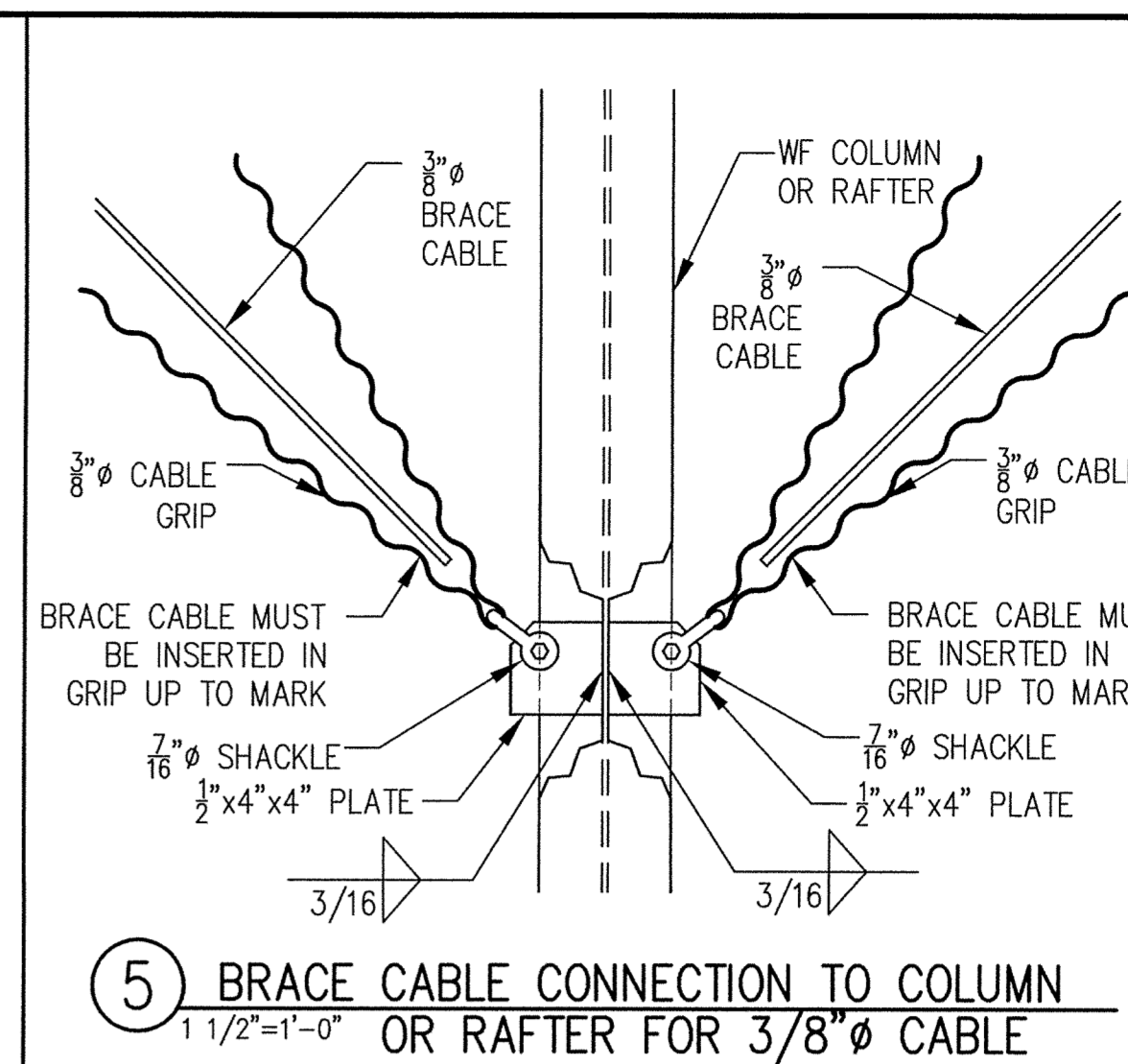
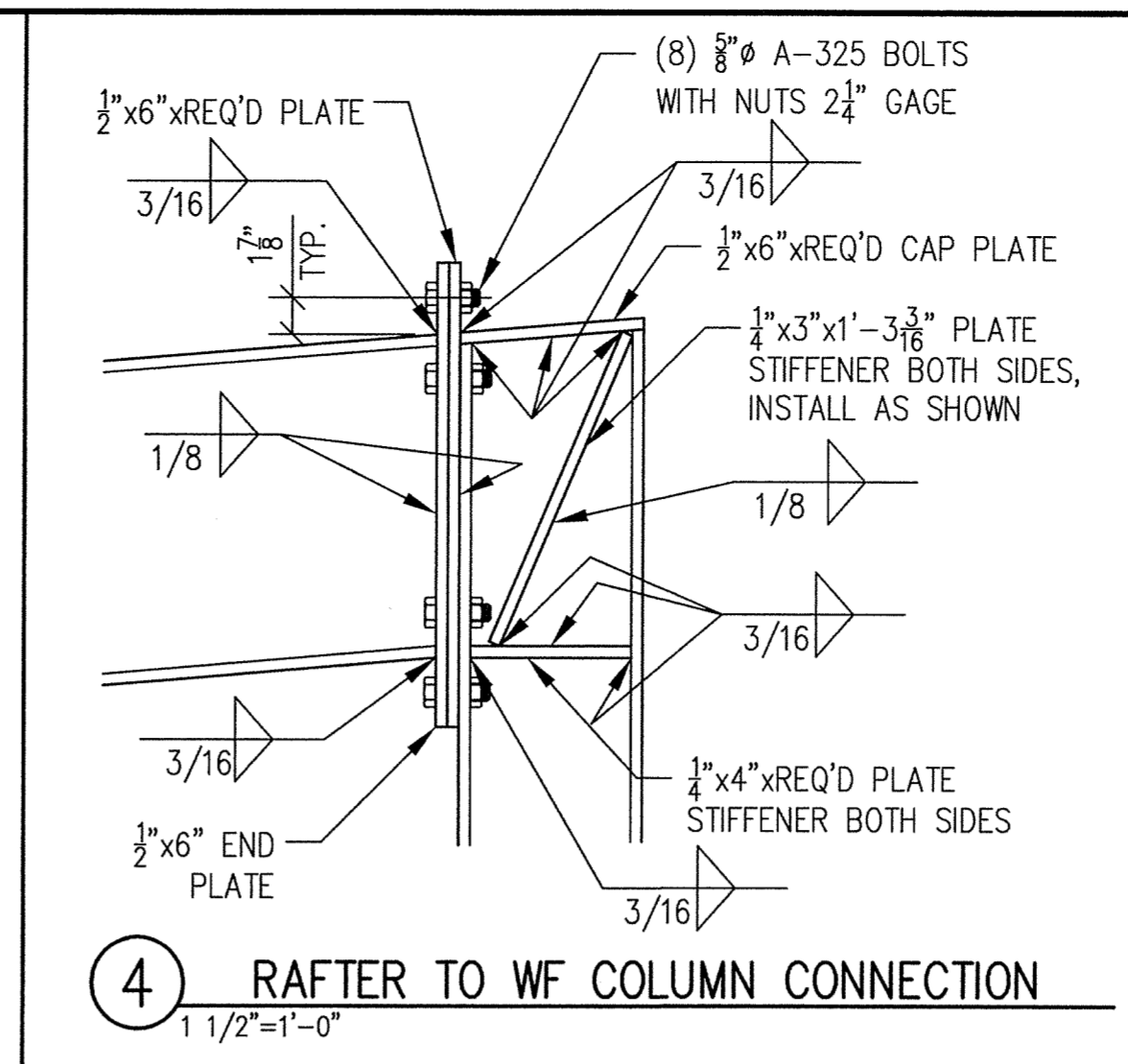
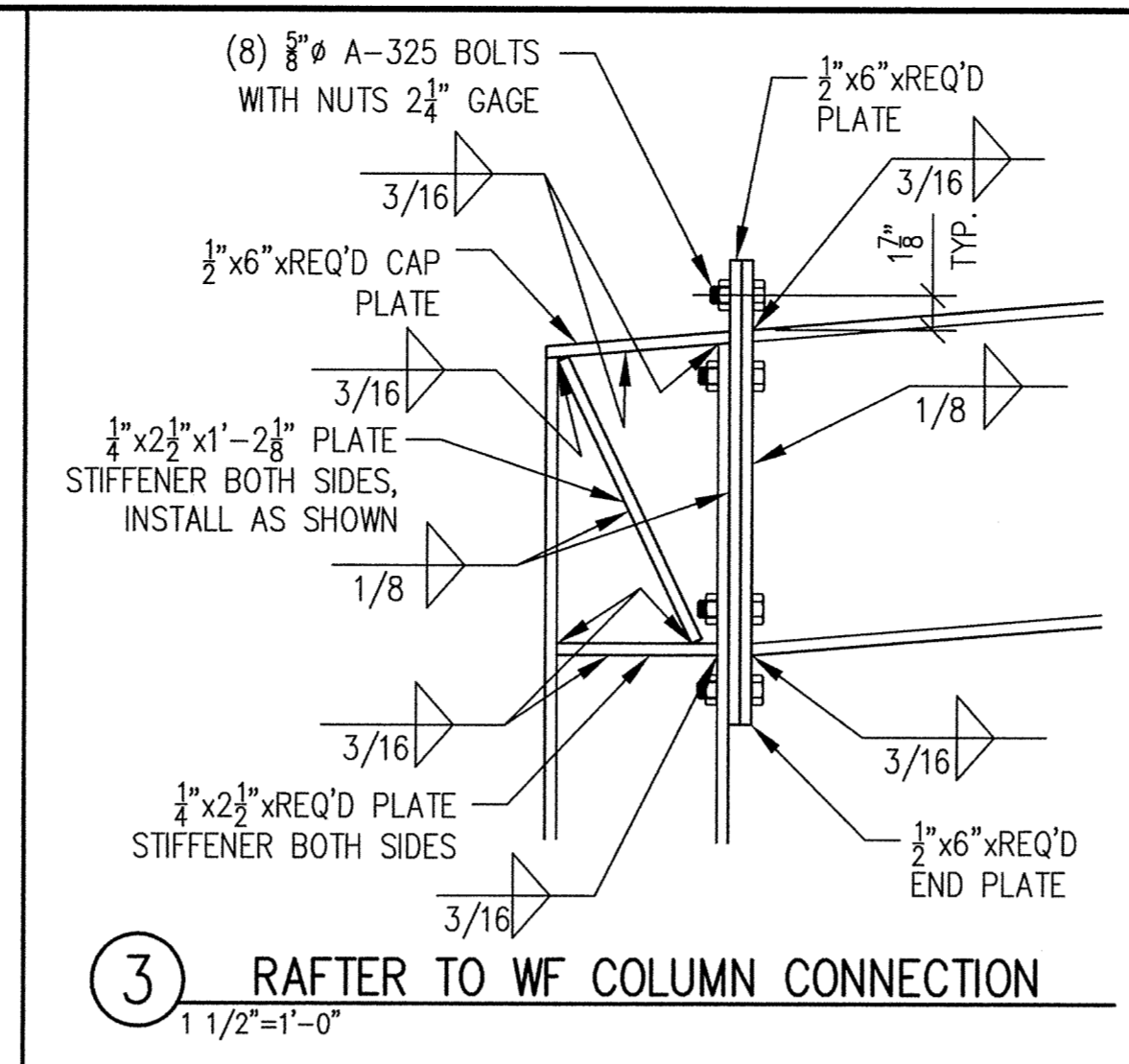
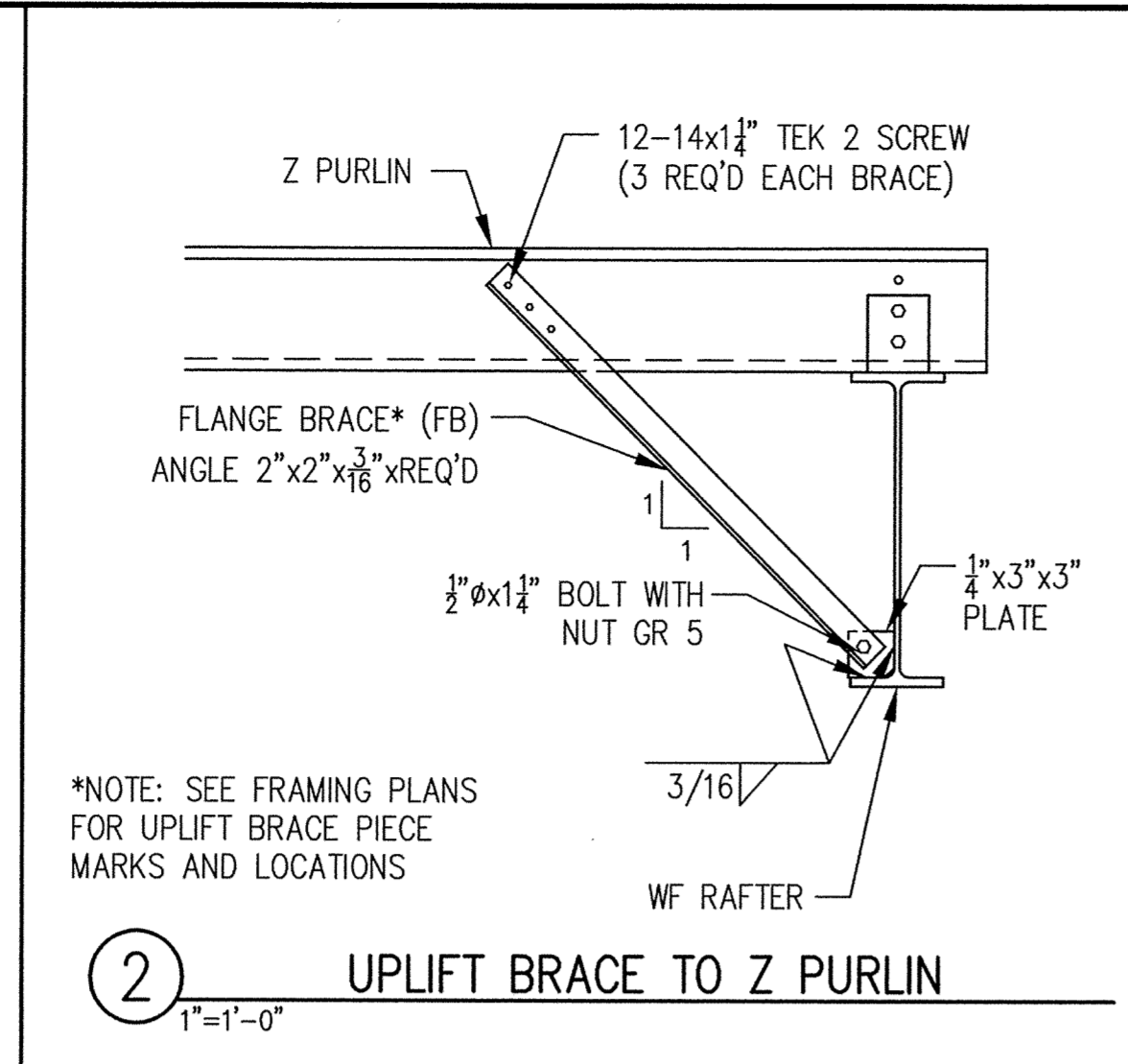
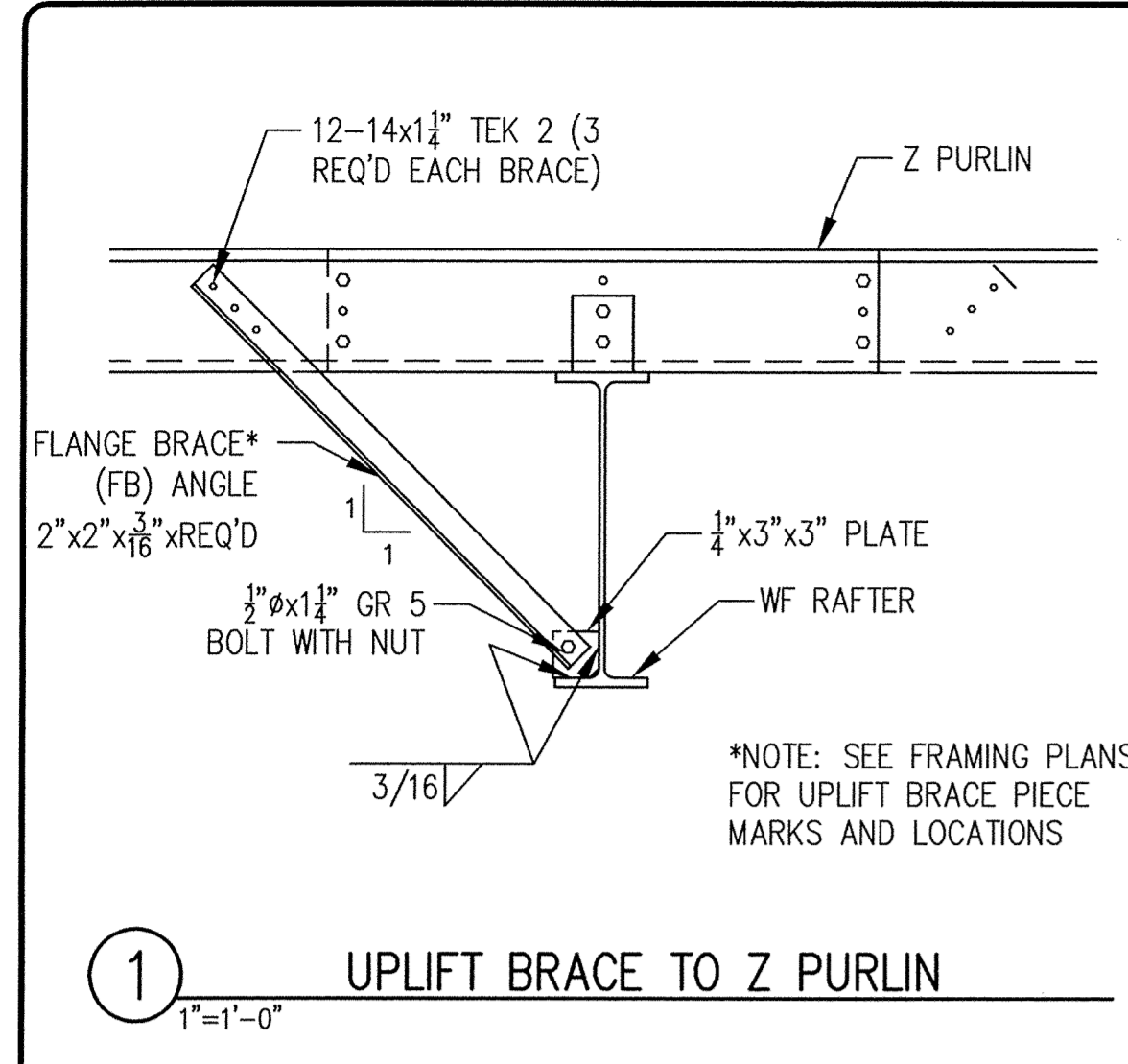
SHEET TITLE: RIGID FRAME ELEVATION
 JOB NAME: PLEASANT VIEW CITY SHED
 LOCATION: PLEASANT VIEW, UTAH
 CONTRACTOR: •

PLAN ISSUE DATES	
DATE	DESCRIPTION
12-20-16	J.J. FOR CONSTRUCTION

PROFESSIONAL STRUCTURAL ENGINEER
 NO. 167724
 JAMES O. MOORE
 STATE OF UTAH
 DEC 20 2016

SHEET NUMBER:
S-8

DRAWN BY:	J.J.
ENGINEER:	J.O. MOORE
AVE JOB NUMBER:	160633



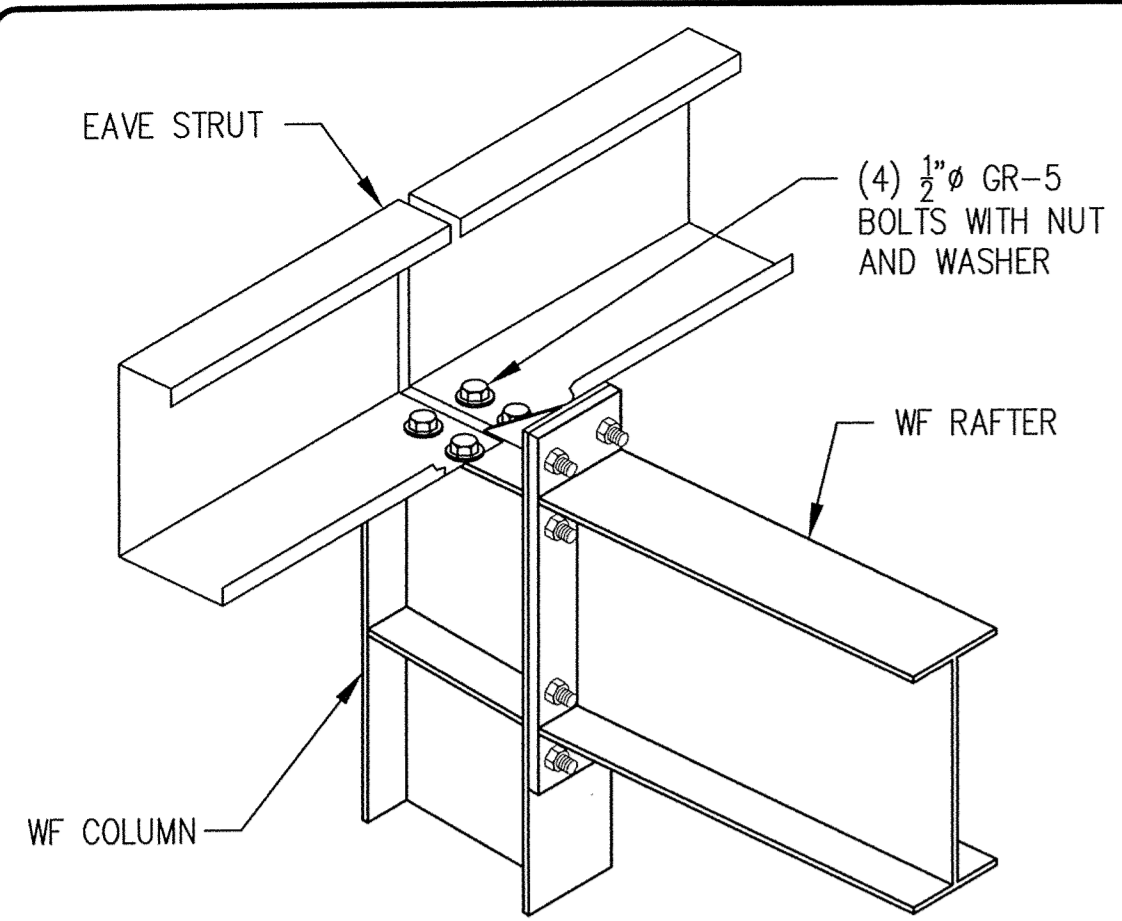
MOUNTAIN VIEW ENGINEERING, INC.
Structural Engineering Consulting
345 North Main Street Ste. A, Brigham City, Utah 84302 (435) 734-9700 Fax (435) 734-9519

SHEET TITLE: **ERECTION DETAILS**
JOB NAME: **PLEASANT VIEW CITY SHED**
LOCATION: **PLEASANT VIEW, UTAH**
CONTRACTOR: -

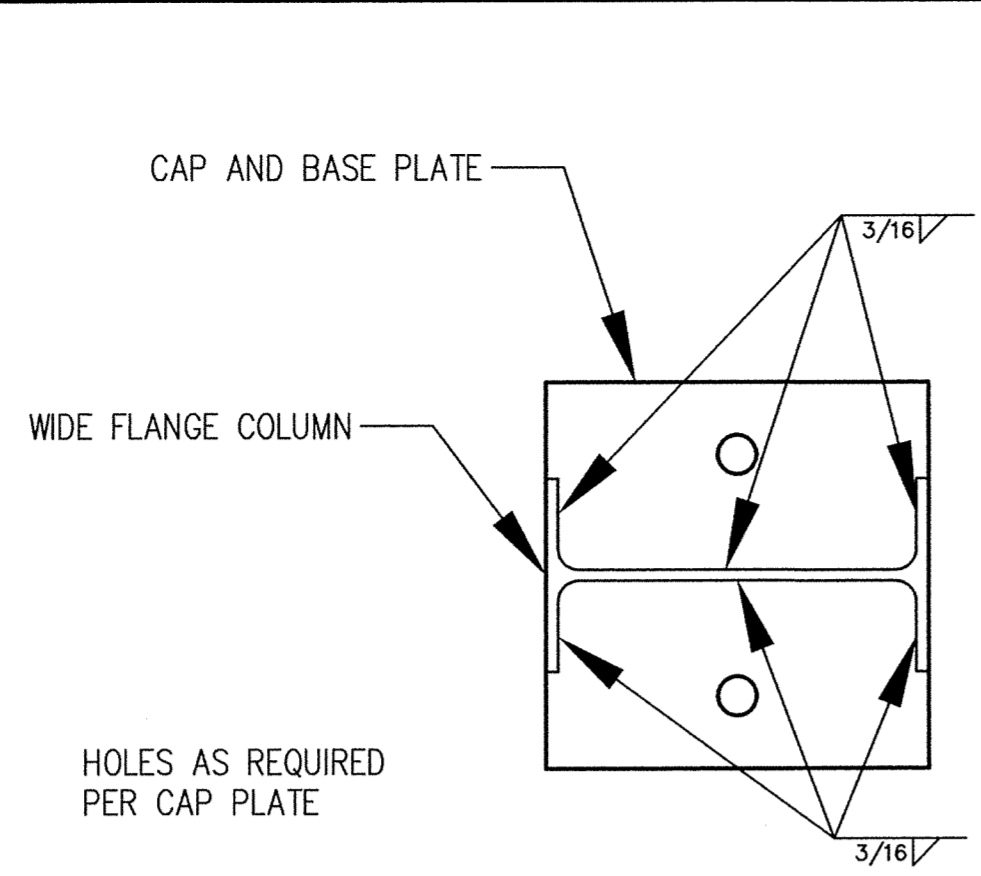
DATE	BY	DESCRIPTION
12-20-16	J.J.	FOR CONSTRUCTION

PROFESSIONAL SEAL
NO. 16714
JAMES O. MOORE
STATE OF UTAH
DEC 20 2016

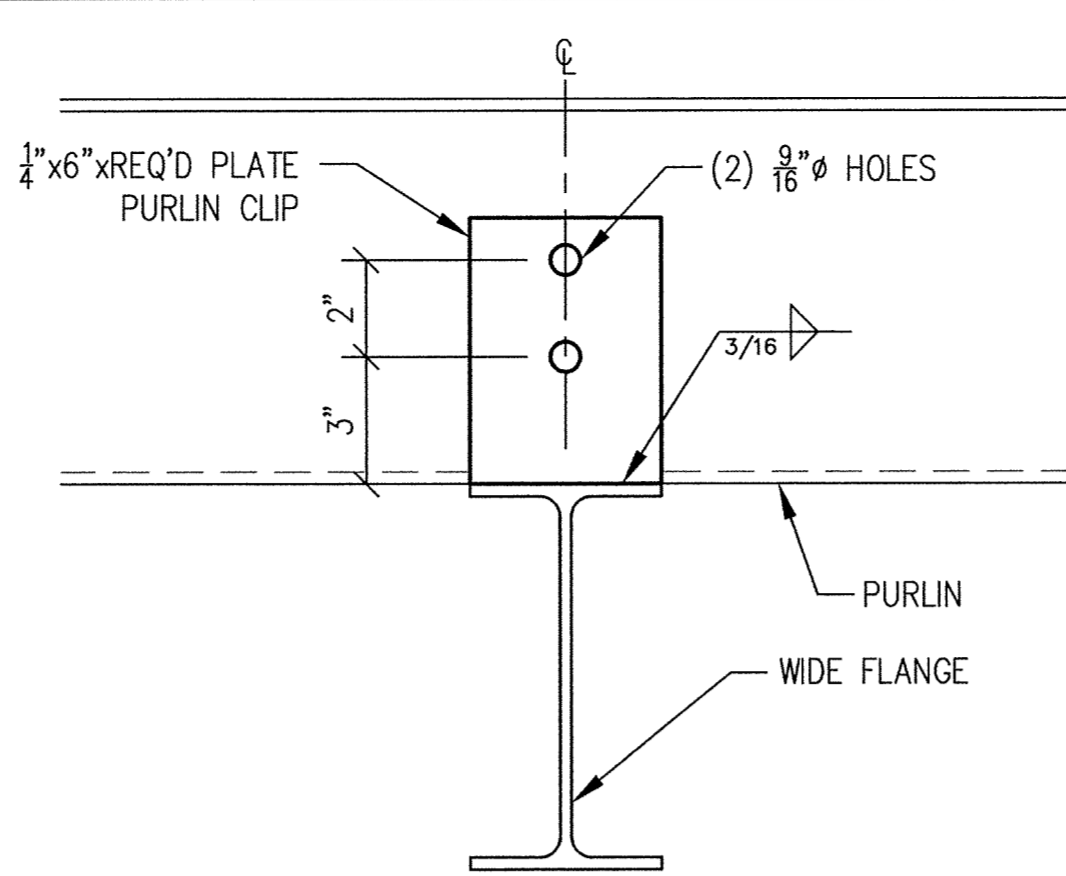
SHEET NUMBER:
S-9
DRAWN BY: J.J.
ENGINEER: J.O. MOORE
MVE JOB NUMBER: **160633**



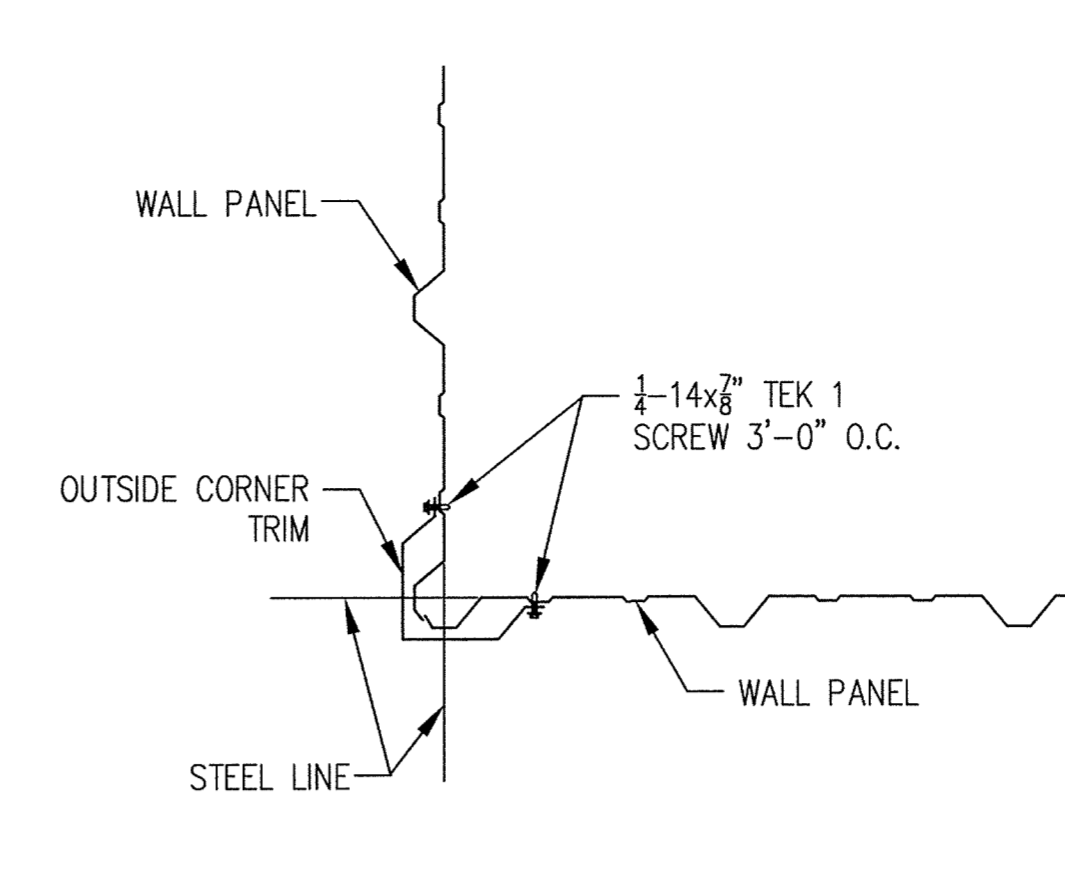
21 LOW EAVE STRUT TO SIDEWALL COLUMN
 1 1/2"=1'-0"
 @ GRID LINES 2, 3, & 4



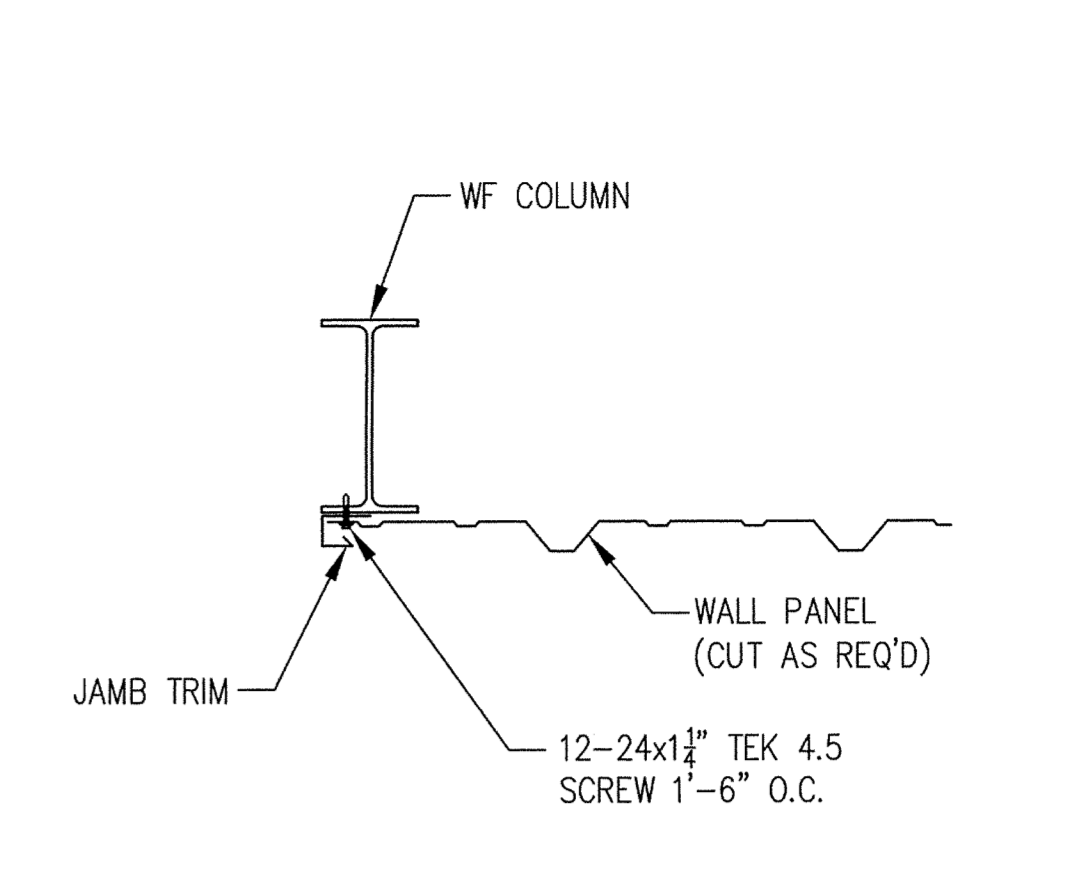
22 CAP AND BASE PLATE WELD DETAIL
 1 1/2"=1'-0"



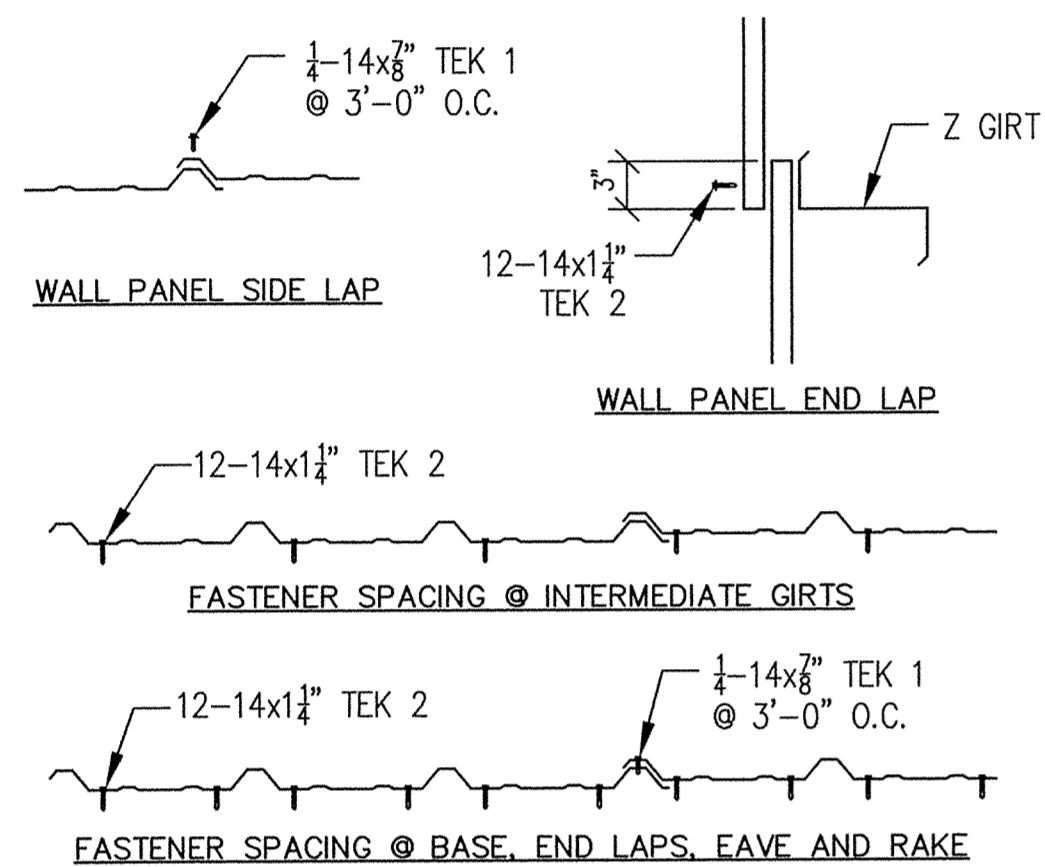
23 Z PURLIN TO RAFTER
 1 1/2"=1'-0"



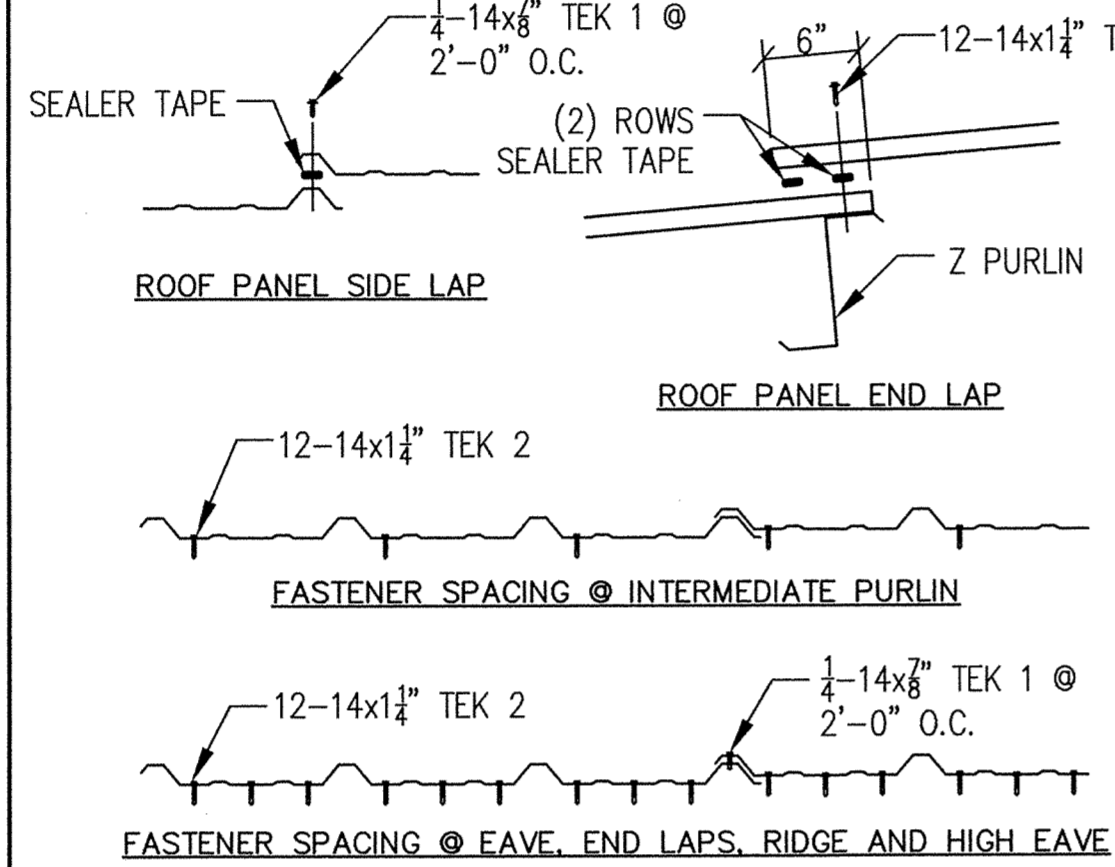
24 OUTSIDE CORNER TRIM DETAIL
 1 1/2"=1'-0"



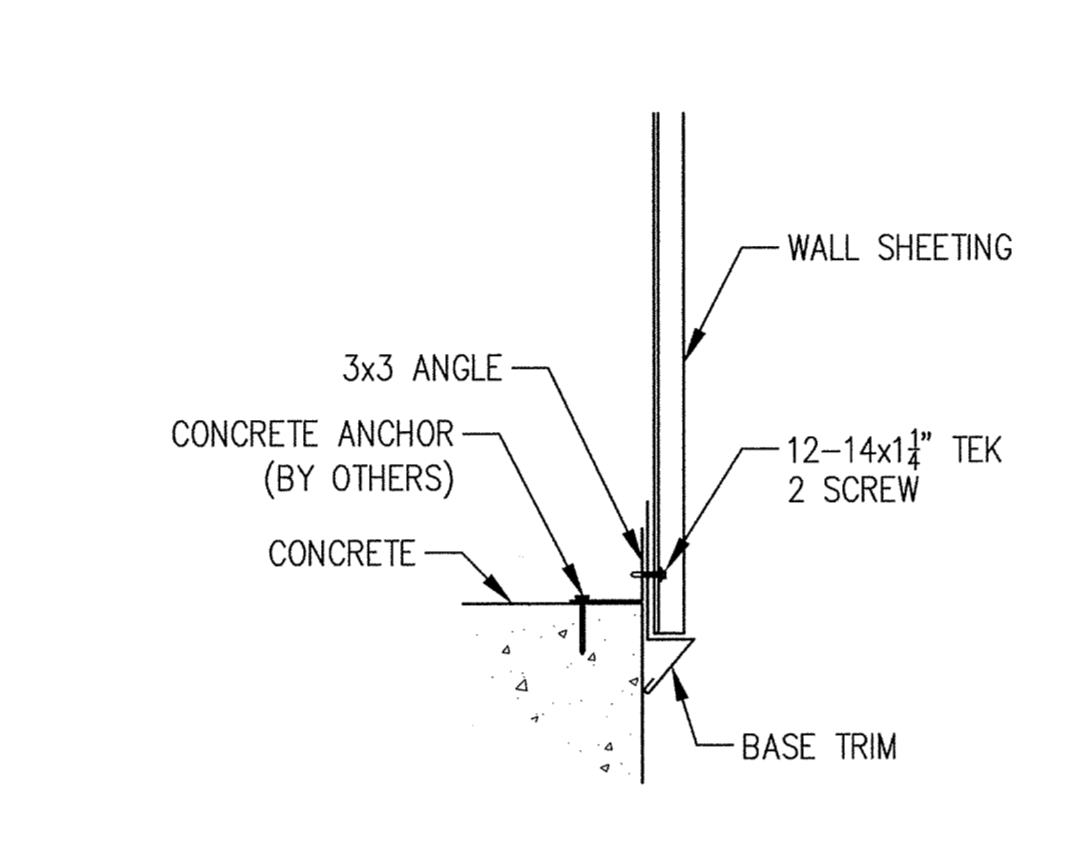
25 JAMB TRIM DETAIL
 1 1/2"=1'-0"



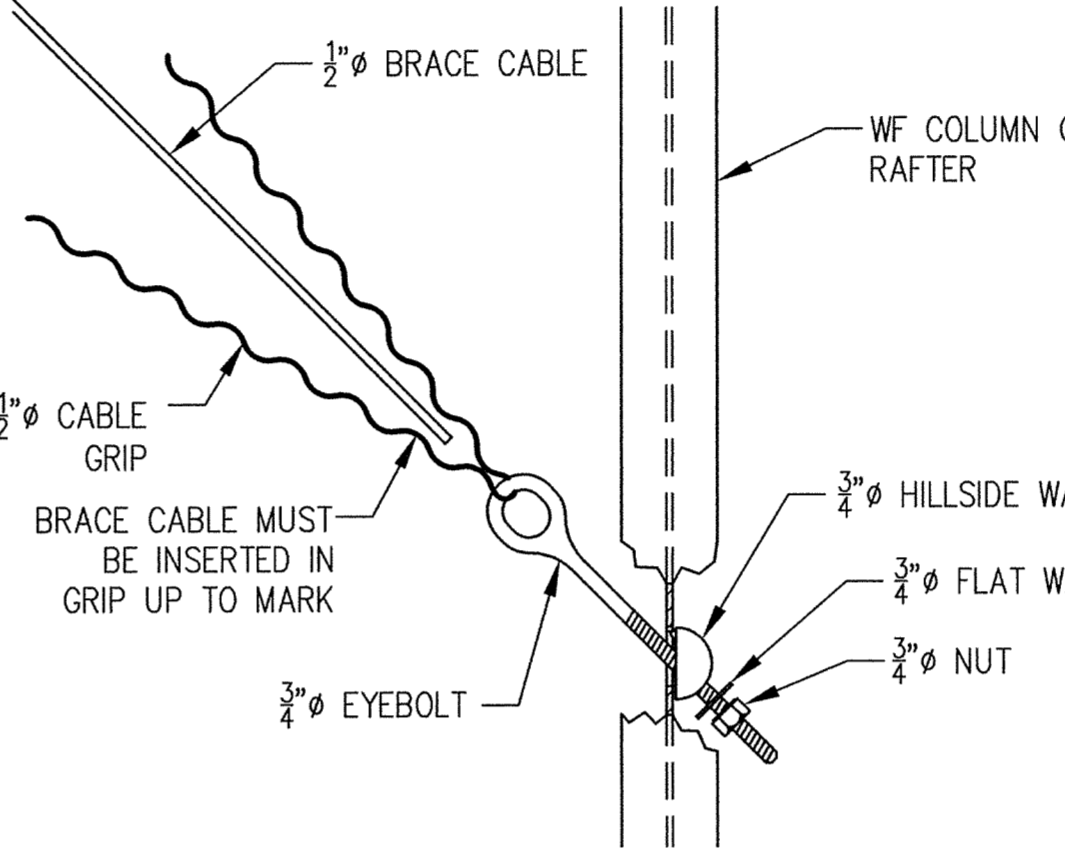
26 WALL PBR PANEL FASTENER SPACING
 N.T.S.



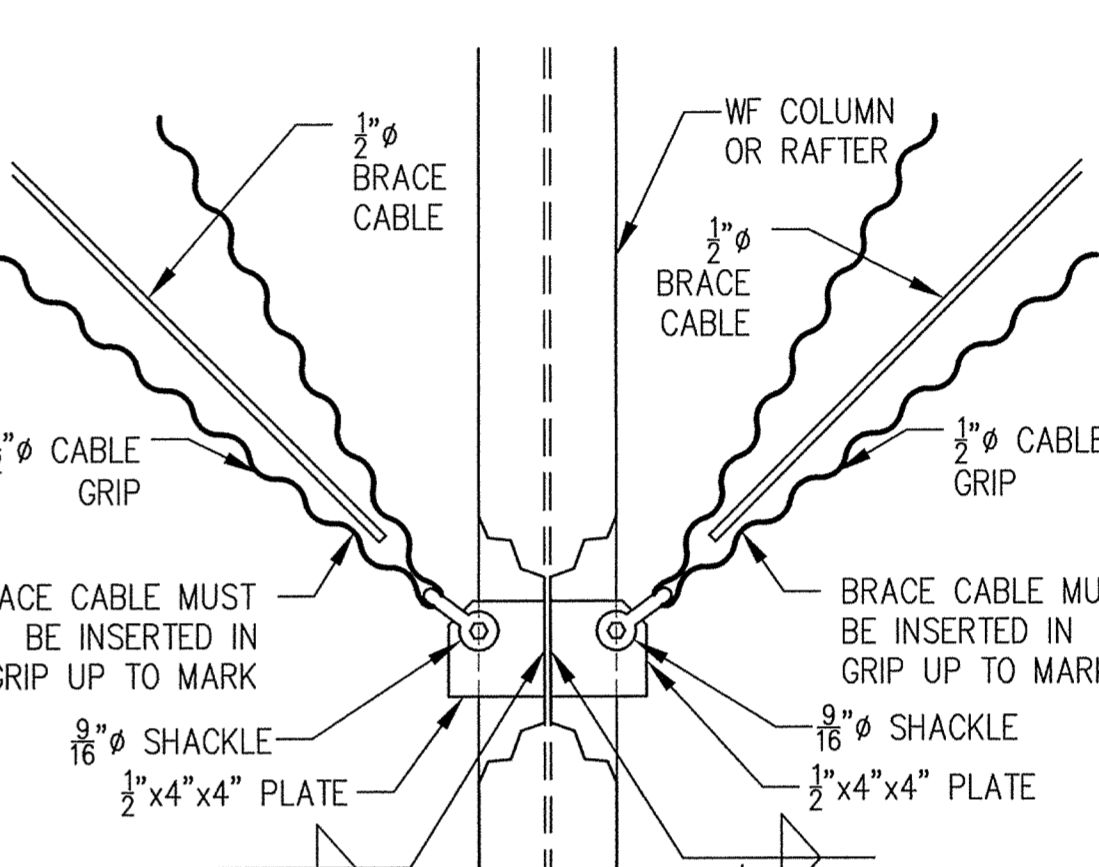
27 ROOF PBR PANEL FASTENER SPACING
 N.T.S.



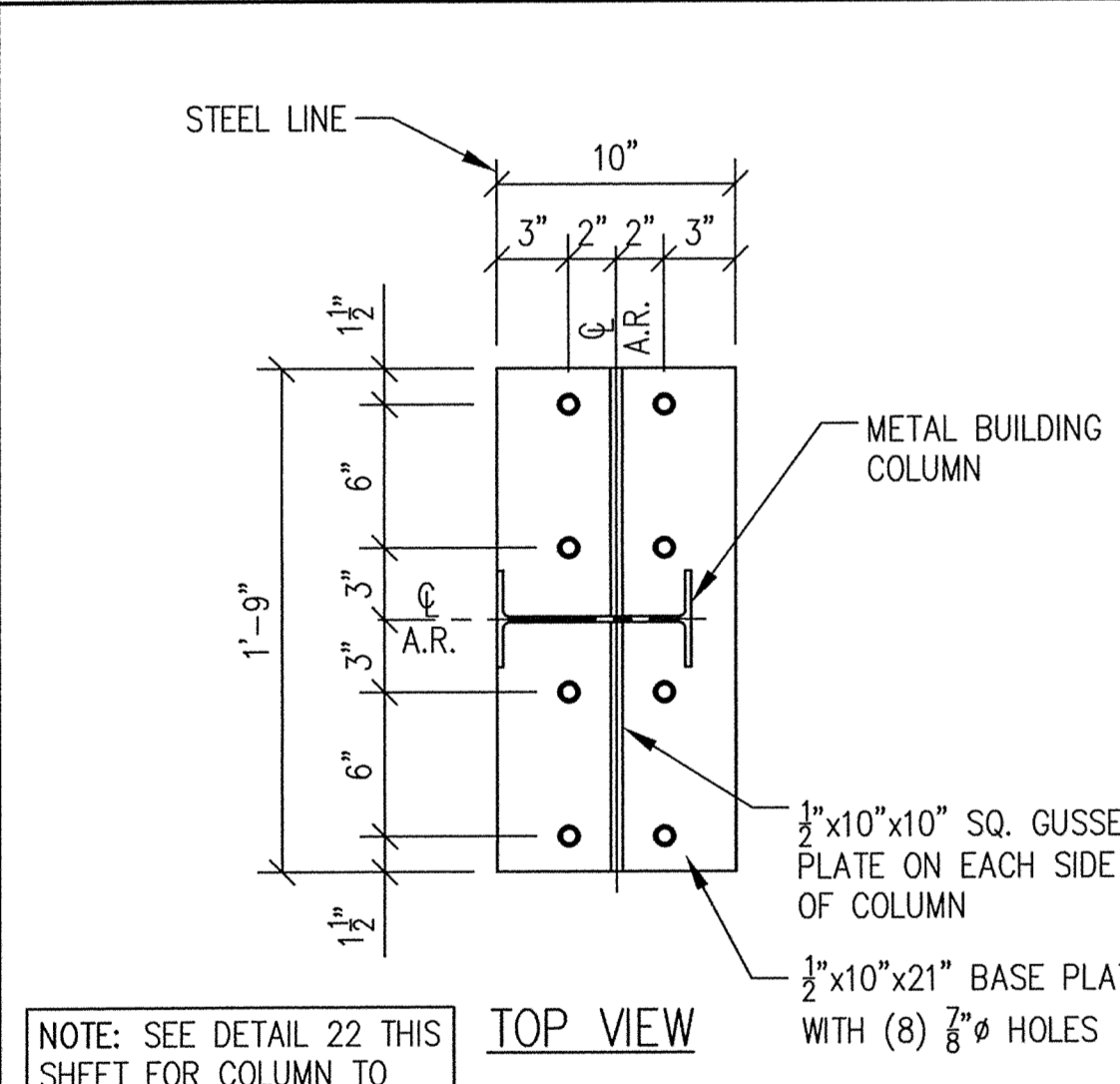
28 BASE TRIM DETAIL
 1 1/2"=1'-0"



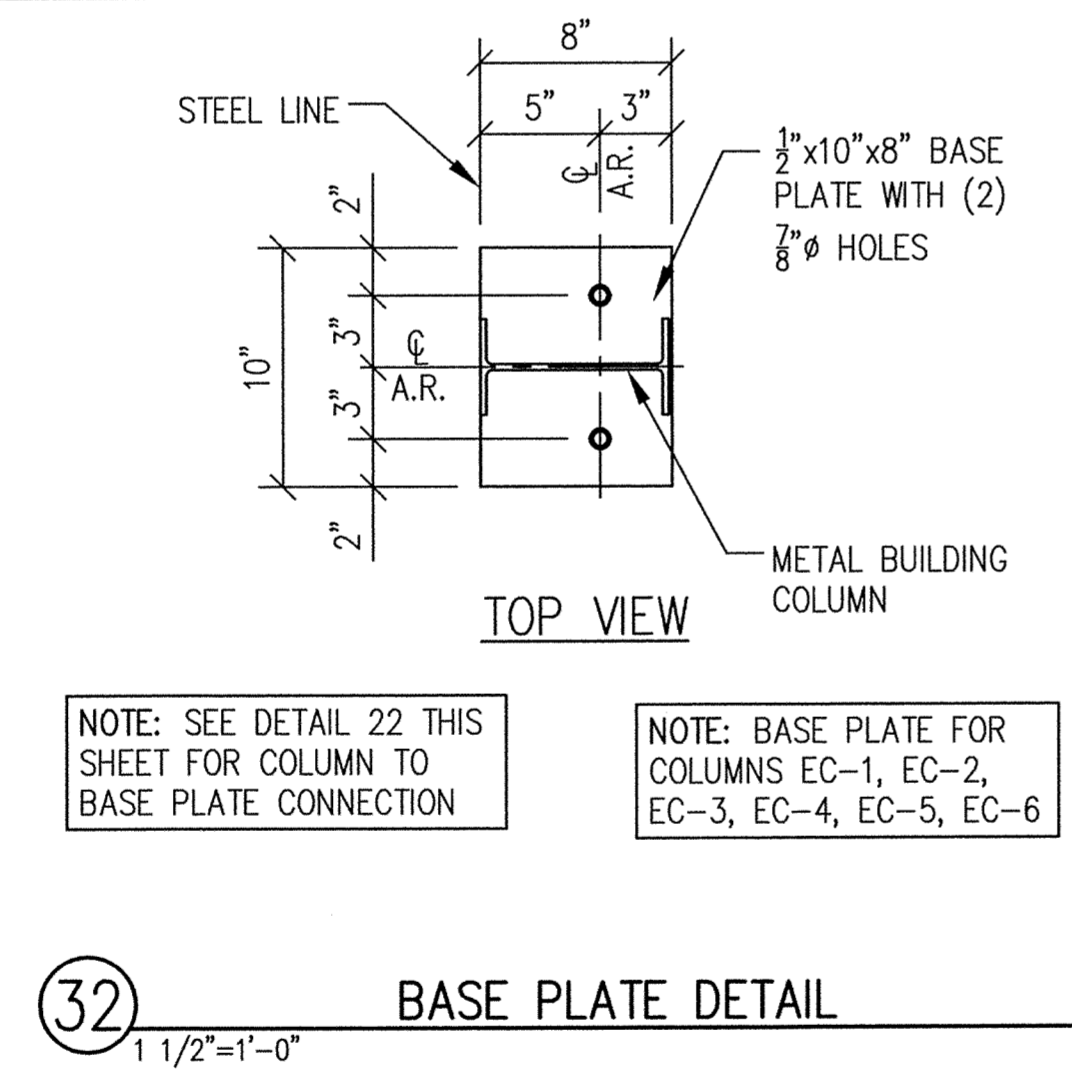
29 BRACE CABLE TO WF COLUMN
 FOR 1/2" Ø CABLE
 1 1/2"=1'-0"



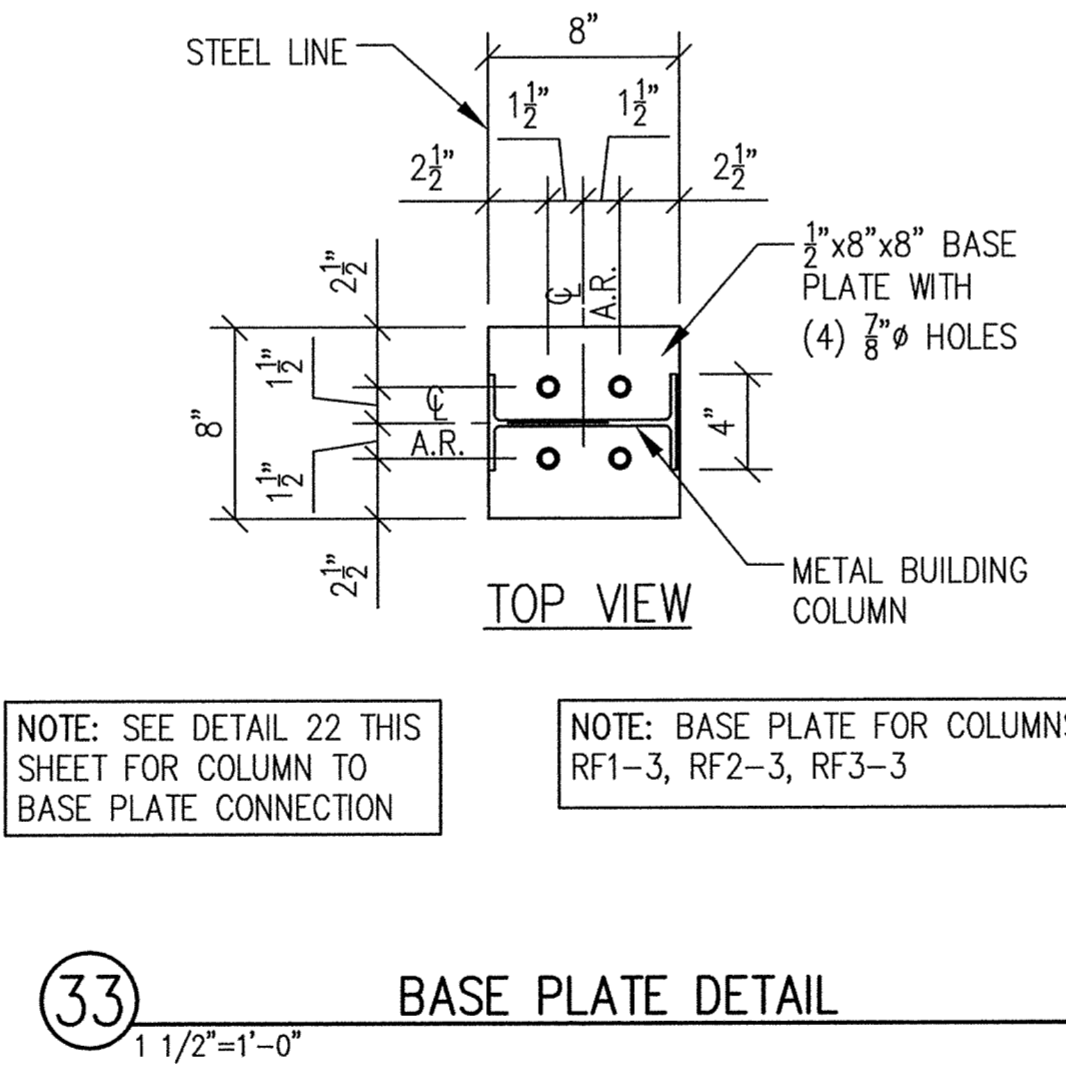
30 BRACE CABLE CONNECTION TO COLUMN
 OR RAFTER FOR 1/2" Ø CABLE
 1 1/2"=1'-0"



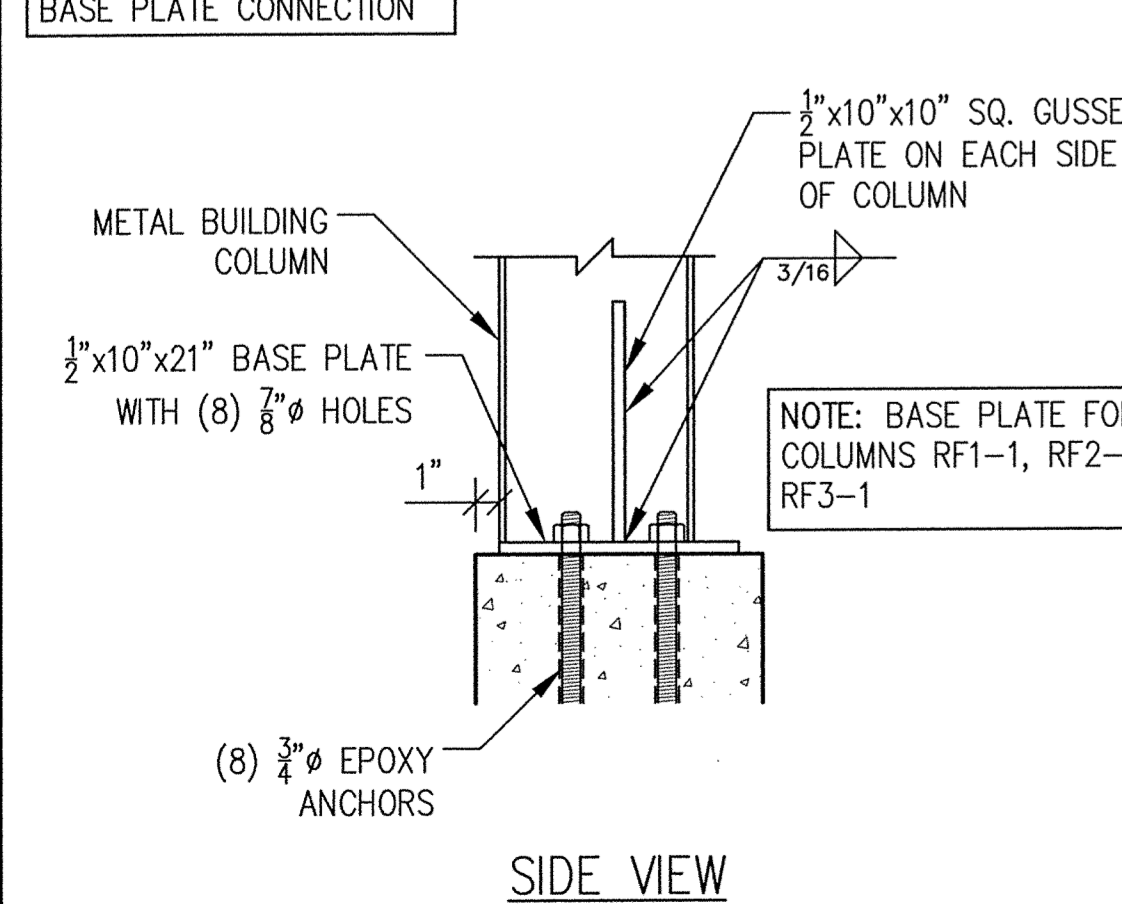
31 BASE PLATE DETAIL
 1 1/2"=1'-0"



32 BASE PLATE DETAIL
 1 1/2"=1'-0"

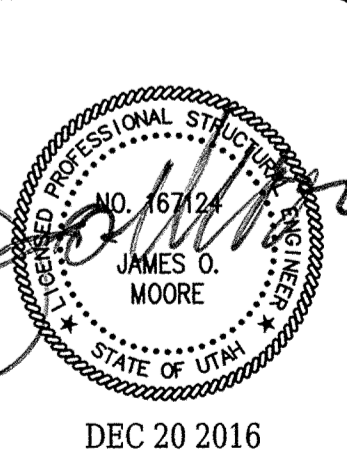


33 BASE PLATE DETAIL
 1 1/2"=1'-0"



31 BASE PLATE DETAIL
 1 1/2"=1'-0"

PLAN ISSUE DATES	
DATE	DESCRIPTION
12-20-16	J.J. FOR CONSTRUCTION



SHEET NUMBER:
S-10

DRAWN BY:	J.J.
ENGINEER:	J.O. MOORE
NVE JOB NUMBER:	160633