BOUNTIFUL 7 & 15 RE-ROOF & SEISMIC UPGRADE

1250 SOUTH MAIN STREET BOUNTIFUL, UTAH 5115876-19010101

MARCH 2019

Economic and Sustainable Designs, Professionals You Know a Economy Suite 200 Sandy, Utah 84070 801.255.7700 mcneil Civil Engineering • Consulting & Landscape Archit

The church of Jesus Christ of Latter-Day Saints

BOUNTIFUL UTAH SOUTH STAKE
1250 SOUTH MAIN STREET
BOUNTIFUL, UTAH

PROJECT NO. 17675

DRAWN BY DTS

CHECKED BY CEG

COVER SHEET

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REVISIONS DATE DESCRIPTION PROJECT NO. **DRAWN BY CHECKED BY**

CEG **MARCH 2019** DATE 511-5876 PROP. NO.

GENERAL NOTES

G1.01

GENERAL NOTES:

- 1. CONTRACTOR MUST VISIT THE SITE SO AS TO BE FAMILIAR WITH ALL EXISTING CONDITIONS BEFORE SUBMITTING BID, BRING ANY QUESTIONS OR CONCERNS TO THE ATTENTION OF THE ENGINEER FOR CLARIFICATION, FOR ADDENDUM PRIOR TO BID OPENING. NO ALLOWANCES WILL BE MADE FOR CONDITIONS THAT ARE CLEARLY VISIBLE.
- 2. CONTRACTOR SHALL WORK WITH LOCAL REPRESENTATIVE ON SCHEDULING TO INSURE CONTINUED USE OF THE BUILDING. NEITHER THE CONTRACTOR NOR ANY OF HIS PEOPLE SHALL HAVE ACCESS TO THE BUILDING WITHOUT PRIOR AUTHORIZATION.
- 3. ALL SAFETY STANDARDS AND REQUIREMENTS ARE THE RESPONSIBILITY OF THE CONTRACTOR.
- 4. EXISTING ROOF PENETRATIONS WILL BE FLASHED AND PAINTED.
- 5. ALL EXISTING ROOF VENTS, MECHANICAL UNITS, ROOF HATCHES, ETC. WILL BE A MINIMUM OF 10" ABOVE THE FINISHED ROOF.
- 6. ALL NEW METAL WILL BE GALVANIZED OR PRE-FINISHED. CAULKING WILL BE SAME COLOR AS METAL.
- 7. BEFORE FABRICATION OF ANY SHEET METAL WORK, SUBMIT SHOP DRAWINGS TO ENGINEER FOR REVIEW AND APPROVAL. ALL WORK TO CONFORM TO NRCA OR SMACNA DETAILS AND REQUIREMENTS WHERE NOT SPECIFICALLY DETAILED OTHERWISE.
- COMPLY WITH ALL MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS.
- CONTRACTOR RESPONSIBLE TO KEEP BUILDING WATERTIGHT AT ALL TIMES. STARTING FROM NOTICE TO PROCEED TO SUBSTANTIAL COMPLETION ANY DAMAGE TO THE BUILDING OR ITS CONTENTS WILL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- 10. CONTRACTOR TO CHECK ALL MECHANICAL EQUIPMENT BEFORE DISCONNECTING TO MAKE SURE THEY ARE OPERATING PROPERLY, CONTRACTOR IS ALSO RESPONSIBLE FOR UNITS TO BE IN COMPLETE OPERATING CONDITION AT THE COMPLETION OF THE PROJECT. COORDINATE SHUTDOWN WITH USERS.
- 11. RAIN GUTTERS & DOWNSPOUTS HEADS:
- a. RIVETS & SCREWS TO BE PAINTED SAME COLOR (NO SPRAY PAINT)
- b. USE POP RIVETS AT ALL CONNECTIONS FROM GUTTERS TO DOWNSPOUTS. c. EVERY DOWNSPOUT - TO CONNECT TO EXISTING SUB-GRADE DRAINAGE SYSTEM.
- WHERE THERE IS EXISTING DRAINAGE SYSTEM. PROVIDE NEW CONNECTION FITTING IF NONE EXISTING, OR IF DAMAGED. d. ALL JOINTS TO BE SEALED WATER TIGHT.
- 12. BEFORE ORDERING ANY MATERIALS, VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS. **DO NOT SCALE DRAWINGS FOR QUANTITIES.**
- 13. BEFORE INSTALLATION OF ALL NEW PIPE JACKS AND PIPE FLASHINGS VERIFY THAT ALL MECHANICAL FLUES AND VENTS ARE STRAPPED TO PREVENT ANY SETTLEMENT OR SHIFTING INTO ROOF. PRIOR TO COMPLETION OF WORK, CONTRACTOR TO VERIFY THAT MECHANICAL EQUIPMENT VENTING TO HAVE POSITIVE RELEASE FLOW TO ROOF VENT AND FLUE IS SECURED TO ORIGINAL HEIGHT AND ALL CONNECTIONS ARE TIGHT AND SECURE.
- PART OF THE CONTRACT. CARE MUST BE TAKEN TO ENSURE THAT ALL ITEMS TO BE REINSTALLED ARE NOT DAMAGED DURING REMOVAL AND/OR INSTALLATION. ALL PIECES THAT ARE DAMAGED WILL BE REPLACED BY CONTRACTOR.
- 15. AT THE END OF CONSTRUCTION, CONTRACTOR IS TO CLEAN OUT AND FLUSH ALL RAIN GUTTERS & DOWNSPOUTS TO MAKE SURE THEY ARE NOT PLUGGED AND ARE IN WORKING CONDITION.
- 16. CONTRACTOR TO SUPPLY AN ON SITE PORTABLE RESTROOM. FACILITY RESTROOMS ARE NOT TO BE USED BY CONTRACTOR OR CONTRACTOR'S EMPLOYEES. LOCATION OF PORTABLE RESTROOM TO BE DETERMINED DURING PRE-CONSTRUCTION MEETING.

SHEET TITLE

SHEET INDEX

COVER SHEET

ROOF PLAN

GENERAL NOTES

HIGH SLOPE ROOF DETAILS HIGH SLOPE ROOF DETAILS

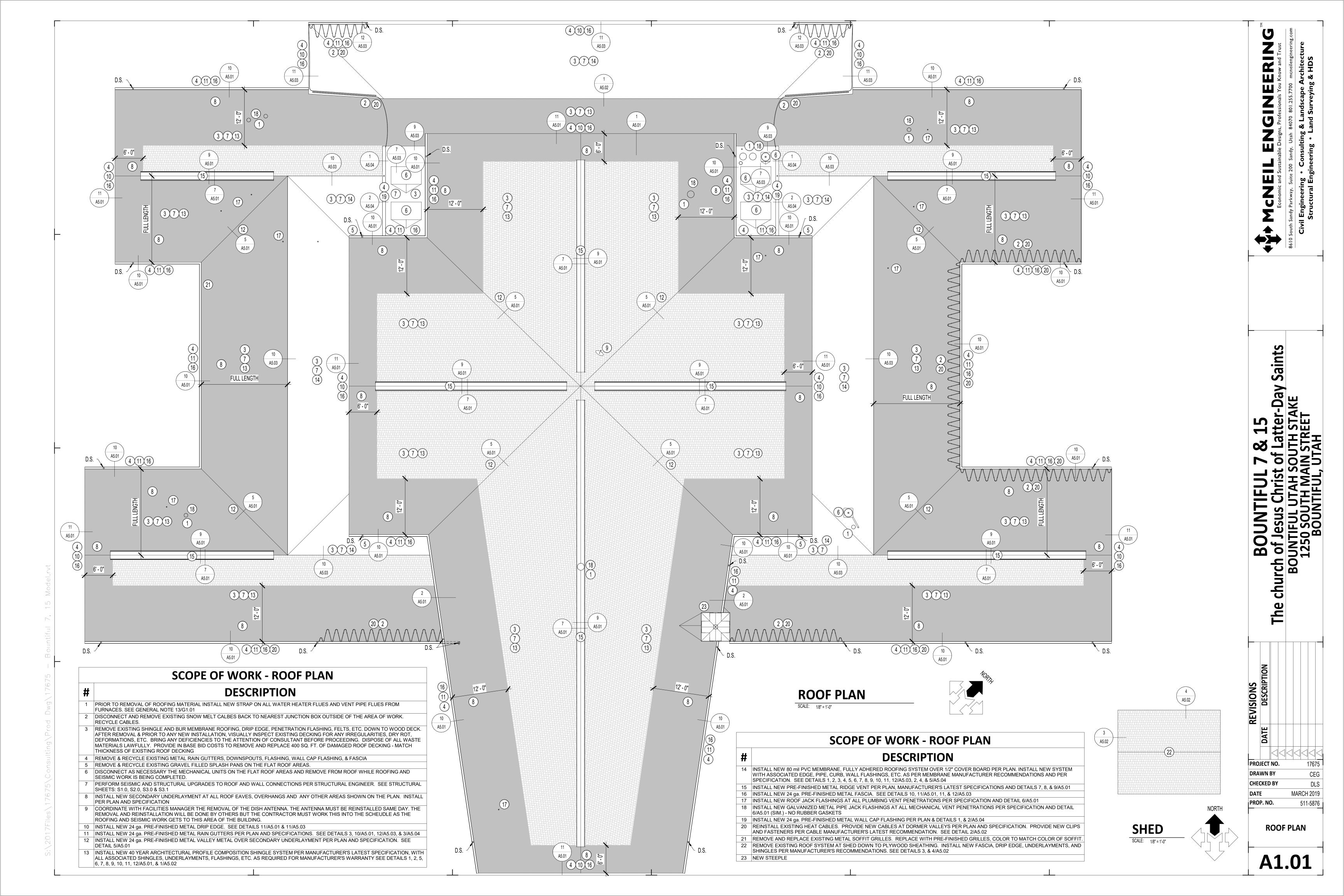
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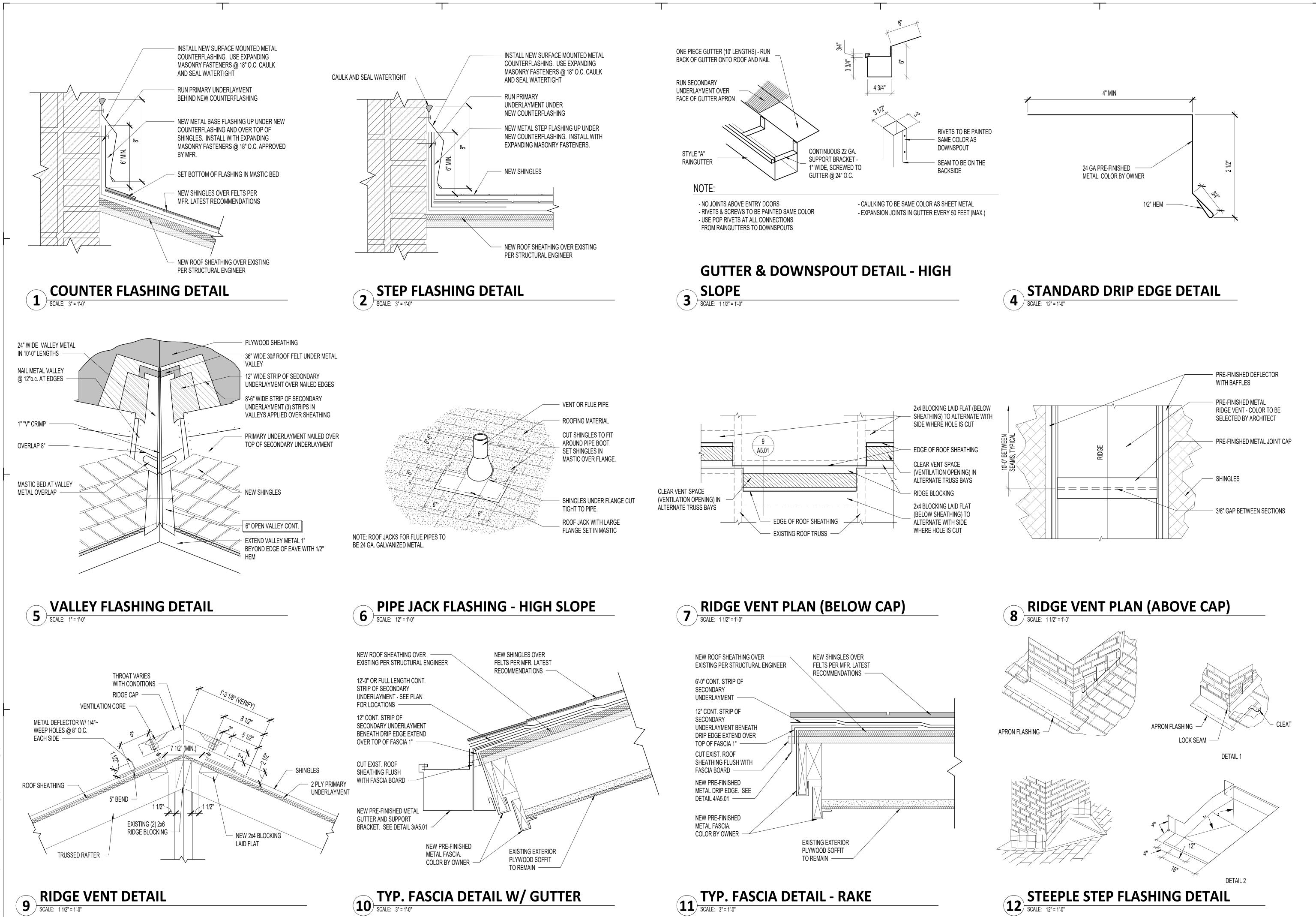


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		LOPE ROOF DETAILS		
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	COMM	ION SYMBOLS	S & ABBREVI	ATIONS
S	YMBOLS	AB	BREVIATIONS	
<u>#</u>	ROOM FINISH KEYNO	A F F - ADOVE FINICHED FLOOD		SEC. = SECTION
		ALUM. = ALUMINUM	LT. = LIGHT	SECT. = SECTION
<u>#</u>)	GENERAL KEYNOTE	BD. = BOARD	MAX. = MAXIMUM	SIM. = SIMILAR
#	WALL CALLOUT	CONC. = CONCRETE	MECH. = MECHANICAL	T&G = TUNG & GROOVE
<u>#</u>		DIA. = DIAMETER	MFR. = MANUFACTURER	T.O. = TOP OF
<u>#</u>]	WINDOW CALLOUT	EA. = EACH	MH = MANHOLE	TYP. = TYPICAL
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<u> </u>	5001107122001	FURN. = FURNACE	NO. = NUMBER	WT. = WEIGHT
#	SECTION CALLOUT	GA. = GAUGE	N.T.S. = NOT TO SCALE	
	ELEVATION CALLOUT	GALV. = GALVANIZED	O.C. = ON CENTER	
<u>#/} </u>		GPF = GALLONS PER FLUSH	O.H. = OVERHANG	
#	DETAIL CALLOUT	GYP. = GYPSUM	PR. = PAIR	
<u>#</u>	ELEVATION MAS: (22)	HB = HOSE BIB	RCP = REFLECTED CEILING PLAN	

RE. = REFERENCE





RIDGE VENT DETAIL

TYP. FASCIA DETAIL - RAKE

SCALE: 3"=1'-0"

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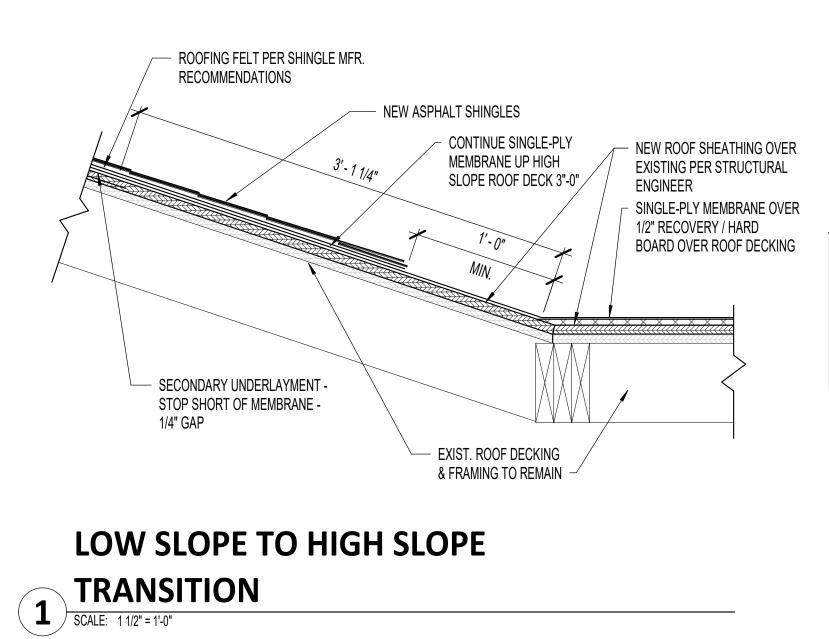
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17675 DTS CEG **MARCH 2019** 511-5876 PROP. NO.

HIGH SLOPE ROOF DETAILS

A5.01

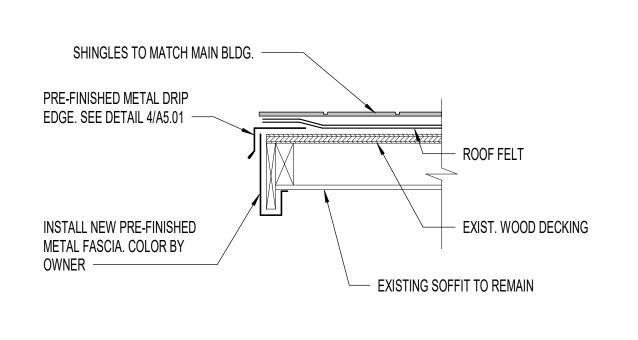


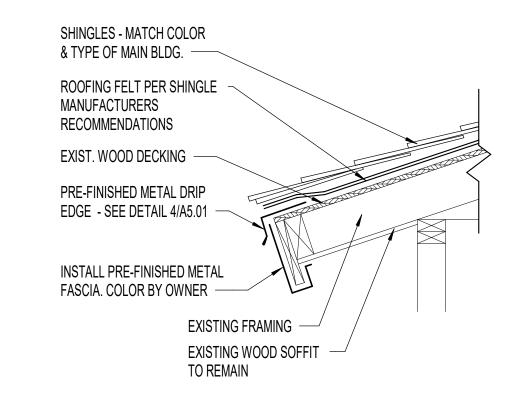
METAL CABLE CLIP PER SPEC. INSTALL INTEGRAL WITH SHINGLES DO NOT INSTALL ABOVE SHINGLES!

ELEC. SNOW MELT CABLE
PER SPEC.

METAL CABLE CLIP PER SPEC. INSTALL INTEGRAL WITH SHINGLES DO NOT INSTALL ABOVE SHINGLES!

ASPHALT SHINGLES PER PLAN





SNOW MELT CABLES

SCALE: 1"=1'-0"

SCALE: 1"1-1'-0"

3 SHED - RAKE END
SCALE: 1 1/2" = 1'-0"

SHED - EAVE DETAIL

SCALE: 1 1/2" = 1'-0"

BOUNTIFUL UTAH SOUTH STAKE
BOUNTIFUL UTAH SOUTH STAKE
BOUNTIFUL UTAH SOUTH STREET
BOUNTIFUL UTAH STREET
BOUNTIFUL, UTAH

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REVISIONS

DATE
DESCRIPTION

17675

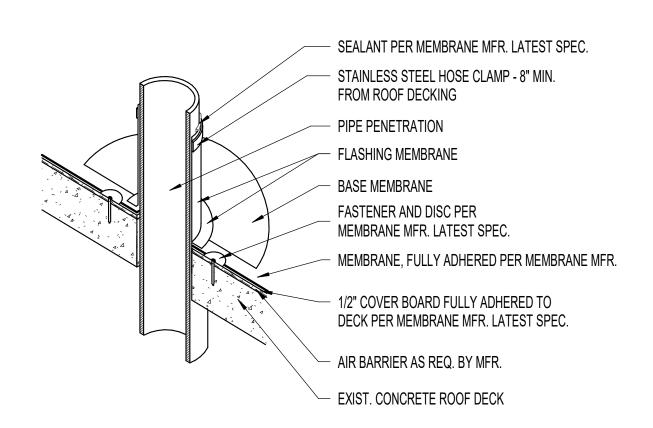
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PROP. NO. 511

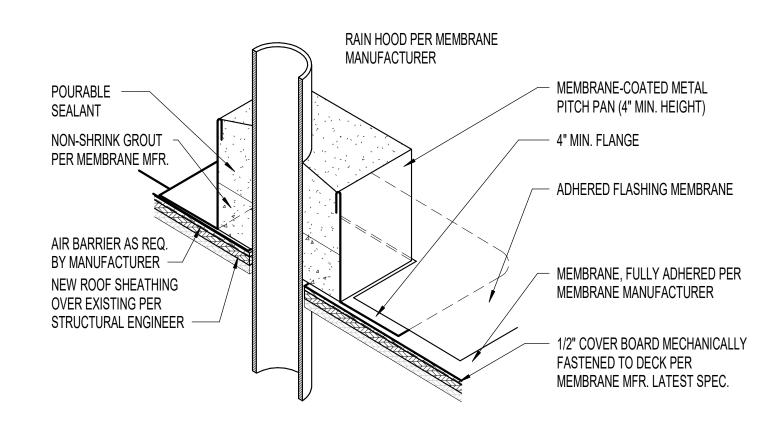
HIGH SLOPE ROOF DETAILS

A5.02

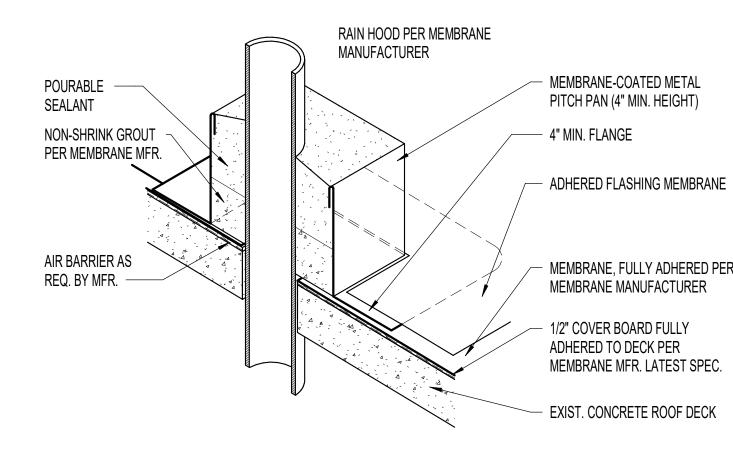
PIPE PENETRATION FLASHING - LOW SLOPE - PLYWOOD DECK SCALE: 1"= 1'-0"



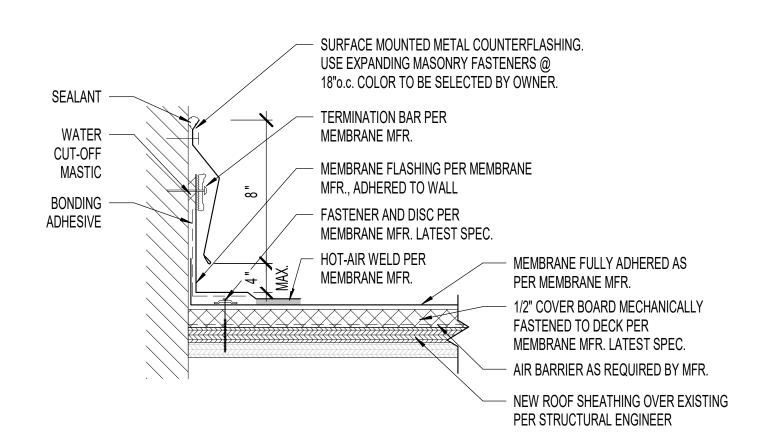
PIPE PENETRATION FLASHING - LOW SLOPE - CONCRETE DECK



SEALANT POCKET - PLYWOOD DECK SCALE: 1"= 1'-0"

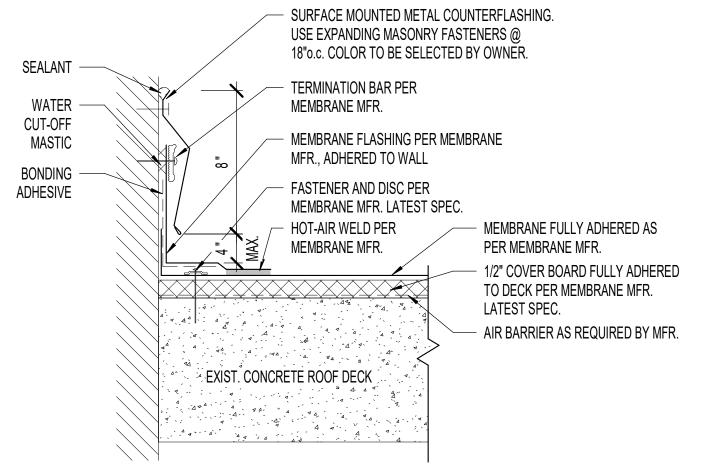


SEALANT POCKET - CONCRETE DECK



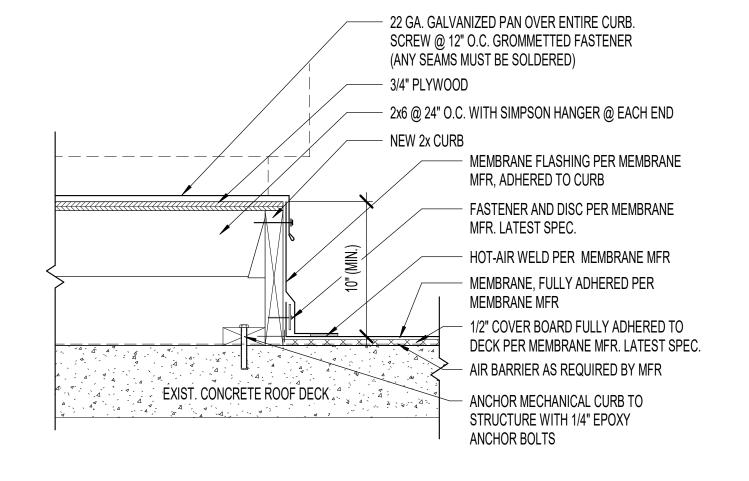
COUNTER FLASHING DETAIL PLYWOOD DECK

SCALE: 3" = 1'-0"



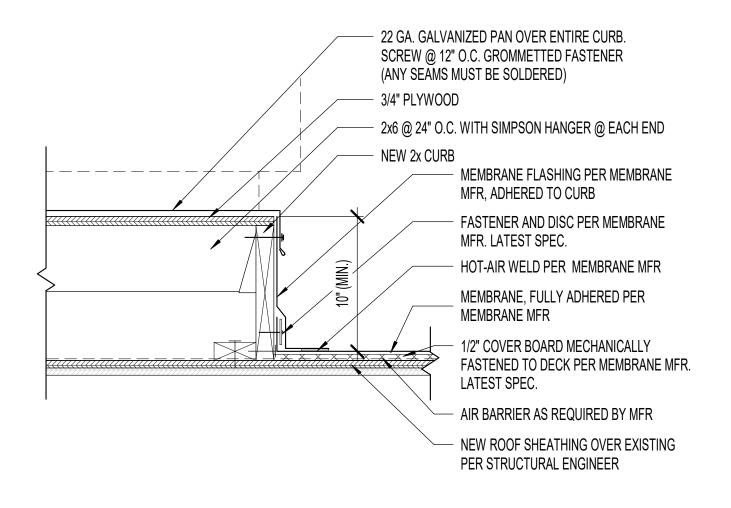
COUNTER FLASHING DETAIL PLYWOOD DECK

SCALE: 3"=1'-0"

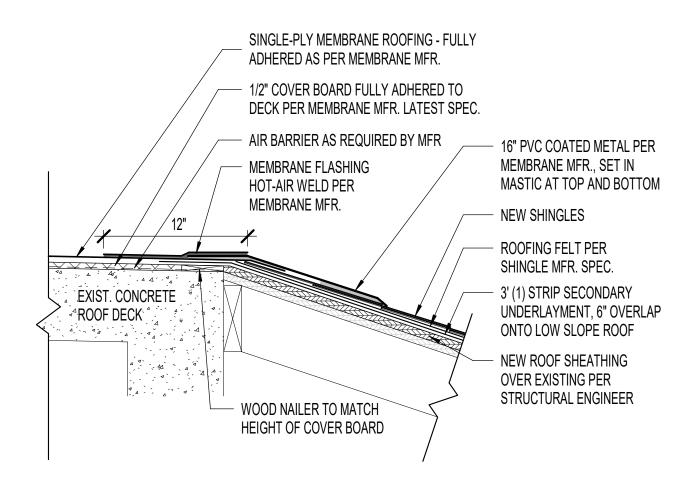


CURB DETAIL @ MECHANICAL UNIT
CONCRETE DECK

SCALE: 1 1/2" = 1'-0"



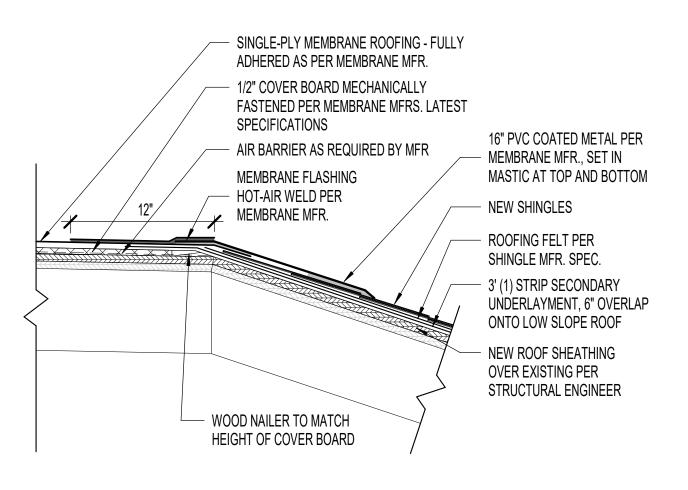
CURB DETAIL @ MECHANICAL UNIT - PLYWOOD DECK



HIGH SLOPE TO LOW SLOPE

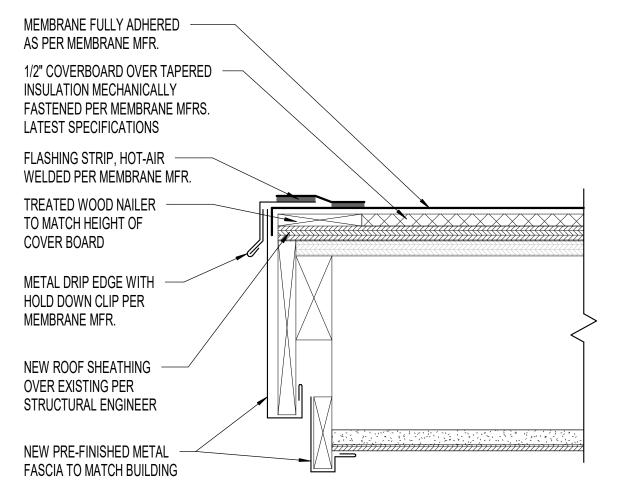
TRANSITION - CONCRETE DECK

SCALE: 1 1/2" = 1'-0"

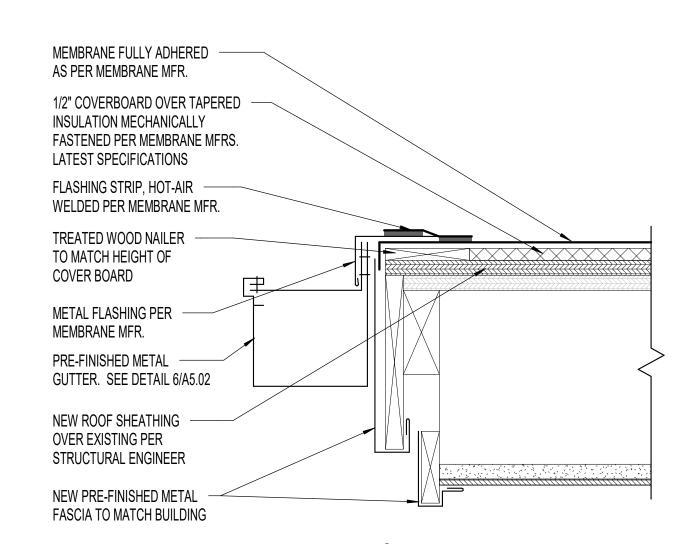


HIGH SLOPE TO LOW SLOPE

TRANSITION - PLYWOOD DECK



TYP. FASCIA DETAIL W/ DRIP EDGE LOW SLOPE



TYP. FASCIA DETAIL W/ GUTTER - LOW

SLOPE

SCALE: 3"=1'-0"

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LOW SLOPE ROOF

DETAILS

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RANSITION - CONCRETE DECK

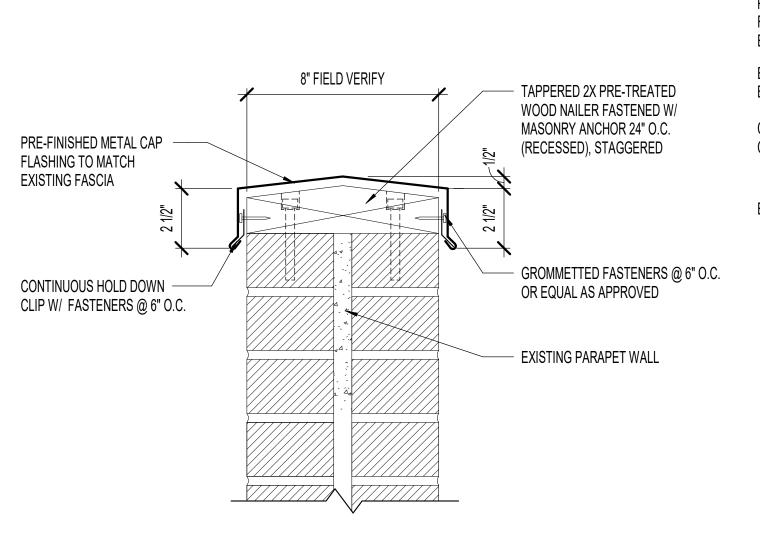
TRANSITION - PLYWOOD DECK

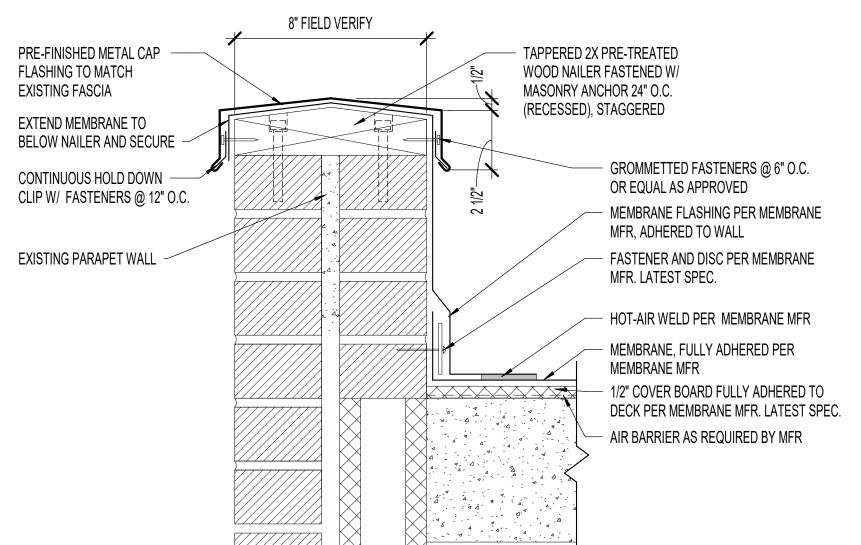
SCALE: 11/2" = 1'-0"

TRANSITION - PLYWOOD DECK

SCALE: 3" = 1'-0"

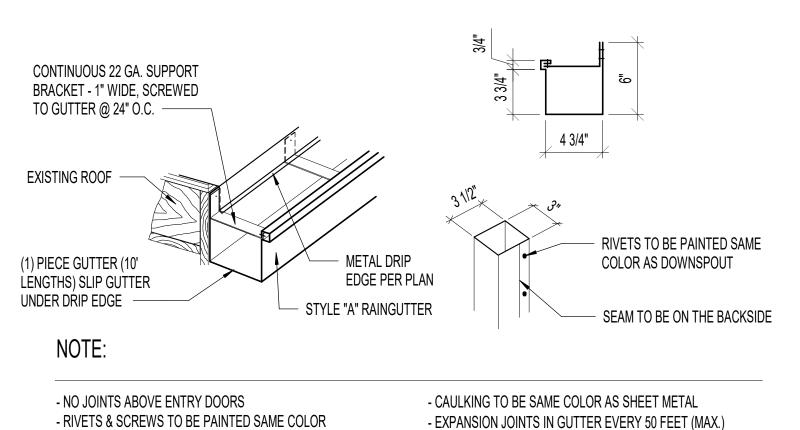
SCALE: 3" = 1'-0"

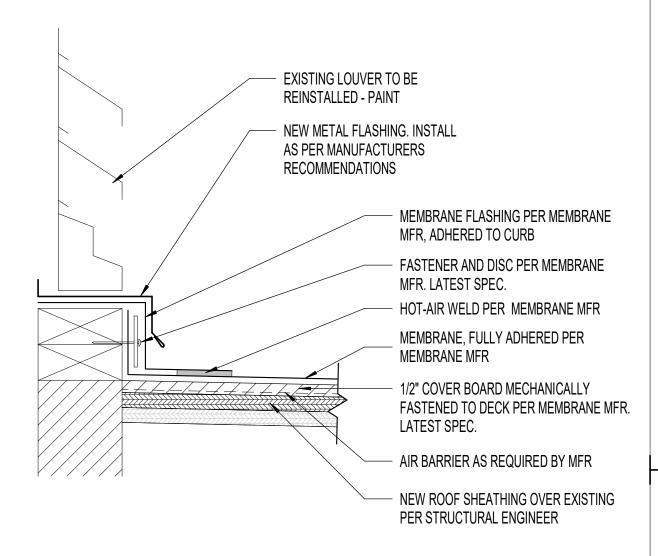




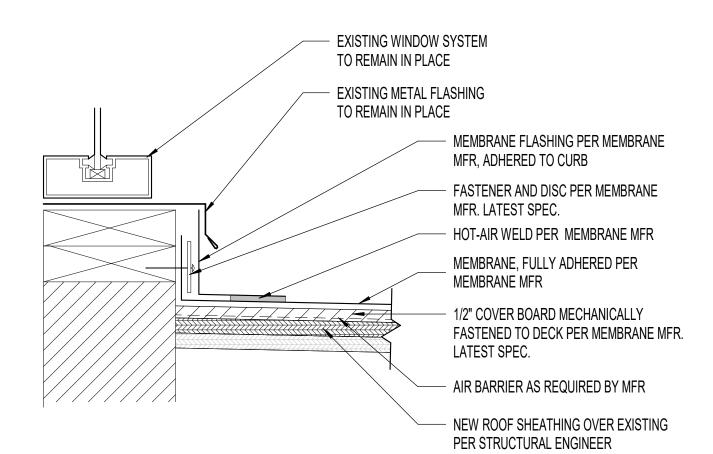
PARAPET WALL CAP @ LOW SLOPE

8CALE: 3" = 1'-0"





PARAPET WALL CAP DETAIL SCALE: 3" = 1'-0"



WINDOW FLASHING DETAIL

GUTTER & DOWNSPOUT DETAIL - LOW

3 SLOPE SCALE: 11/2" = 1'-0"

- USE POP RIVETS AT ALL CONNECTIONS

FROM RAINGUTTERS TO DOWNSPOUTS

LOUVER FLASHING DETAIL

BOUNTIFUL 7 & 15
The church of Jesus Christ of Latter-Day Saints
BOUNTIFUL UTAH SOUTH STAKE
1250 SOUTH MAIN STREET
BOUNTIFUL, UTAH

ENGRED PROGRESS Von Valley Brodering Professional Valley Brodering

McNEIL

PROJECT NO. 17675

DRAWN BY DTS

CHECKED BY CEG

DATE MARCH 2019

LOW SLOPE ROOF
DETAILS

PROP. NO.

A5.04

Snow load on sloped roof

4. SEISMIC DESIGN: Importance factor, Mapped Spectral response accelerations: 1.465 & 0.558 Spectral response coefficients: Seismic Design Category Basic Seismic-Force-Resisting System. Bearing wall systems Reinforced masonry shear walls

Design Base Shear. 1.35W kips Seismic response coefficient, 1.35 Analysis procedure Pseudo Lateral Force 5. SOILS: Soil bearing pressure Minimum frost cover

EOR = Engineer of record. See professional stamp this page. UNO = Unless noted otherwise

(E) = Existing condition (N) = New construction

GENERAL

6. ABBREVIATIONS:

1. THE GENERAL CONTRACTOR SHALL:

A. Be familiar with the contract documents and insure that subcontractors are familiar with their portion of the work. Submit a written request to the Arch/EOR for approval before proceeding with any changes. B. Verifies site conditions and dimensions at the site. If they differ from the contract documents, notify the Arch/EOR prior to fabrication/construction of affected elements. Existing condition information on the drawings is based on best

knowledge acquired during the design phase and may differ from actual conditions. Affected details may require C. Report to the Arch/EOR modifications made to the structure. D. Be responsible for safety and protection on and around the job site and adjacent properties.

2. THE GENERAL CONTRACTOR SHALL COORDINATE:

A. And verify locations, weights and sizes of mechanical units, equipment, etc. prior to the fabrication and erecting of structural supporting elements. Report sizes and locations that differ from those shown on the drawings to the Arch/EOR for review. Additional framing maybe required.

B. Roof, floor, and wall openings required for mechanical, etc. which are not shown on the structural drawings with the

C. Any structural situation not covered by the drawings with the Arch/EOR.

D. Doors, windows, walls, elevations, slopes, stairs, curbs, drains, recesses, depressions, railings, waterproofing, finishes, chamfers, kerfs, pads, landscape walls, trenches in slabs, etc. with the structural work. E. Inspections, testing, and structural observations as work proceeds. Notify the EOR 48 hours prior to any required

structural observations

3. CONTRACT DOCUMENTS & DRAWINGS: A. These structural notes complement the specifications and the drawings.

B. Specific details, sections and notes shown on the drawings govern over these general notes and typical details. C. Contract documents take precedence over shop drawings, UNO.

D. Apply typical or similar details, sections and notes to similar situations on the drawings where specific details are not

E. Drawings and details have been prepared to visually represent information provided in scaled form. However, DO NOT scale plans or details for dimensional information.

F. Refer to architectural drawings for dimensions.

4. BUILDING CODE COMPLIANCE: Construction, inspection, materials, testing, and workmanship shall conform to the requirements of the governing building code.

5. CONSTRUCTION SEQUENCE, SHORING, AND BRACING REQUIREMENTS: The general contractor is responsible for the method, means, and sequence of structural erection, UNO. He shall provide adequate temporary shoring or bracing for all structural elements until the entire structural system is completed. Design of shoring and bracing is by others at no additional cost to the owner.

6. OMISSIONS, CONFLICTS & DISCREPANCIES:

A. Bring omissions, conflicts or discrepancies between the elements of the contract documents to the attention of the

Arch/EOR before proceeding with work involved. B. In case of conflicts or discrepancies, follow the most stringent requirements as directed by the Arch/EOR.

A. During and after construction, builder and/owner shall keep loads on the structure within the limits of this design. See Basis of Design.

B. Site observations by WCA's field representative shall neither be construed as inspection nor approval of construction.

A. Make submittals in a timely manner. WCA's review is for general compliance only and is not intended as approval. Contractor is responsible for verifying sizes, dimensions and elevations on submittals as related to the contract

C. Have EOR approved shop drawings & materials on site before construction of those components begins.

B. Allow two weeks for the review of submittals by the EOR. D. Substitutions are not allowed unless approved by the EOR. Submit requests for structural substitutions to the Arch/EOR

EMBE	EMBEDMENT OF ADHESIVE ANCHORS				
BASE MATERIAL	REBAR DOWELS	THREADED ROD Ø	EMBEDMENT LENGTH	SCREEN LENGTH	
	#3	3/8"	5"	_	
	#4	1/2"	6"	_	
CONCRETE	#5	5/8"	8"	_	
	#6	3/4"	10"	_	
	#7	7/8"	12"	_	
	#3	3/8"	4"	_	
CMU	#4	1/2"	5"	-	
(GROUTED)	#5	5/8"	6"	_	
	#6	3/4"	7"	_	
	#3	3/8"	_	4"	
CMU	#4	1/2"	_	5"	
(HOLLOW)	#5	5/8"	_	6"	

INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS.

EMBEDMENT LENGTH IS INTO STRUCTURE AND NOT VENEER, UNO. REBAR SHALL BE DEFORMED. MINIMUM WALL THICKNESS TO BE EMBEDMENT LENGTH PLUS 1-1/2"

OTHERWISE SEE STRUCTURAL ENGINEER. URM WALL TO USE SCREENS AND TO BE EMBEDED IN MORTAR JOINT UNO. USE HILTI HY20 EPOXY FOR BOLTS INTO UNREINFORCED MASONRY FACE

USE HILTI HY150 EPOXY FOR BOLTS INTO CONCRETE AND GROUT FILLED

ONLY USE CMU HOLLOW IF CALLED OUT SPECIFIALLY IN DETAIL

POST-INSTALLED ANCHORS

1. PRODUCT: Adhesive Anchors

A. Adhesive for Concrete connections shall be: 1. HIT HY 200 (ESR-3187) by Hilti Corporation

2. Pure110+ (ESR-3298) by DeWalt 3. AC200+ (ESR-4027) by DeWalt

4. SET-XP (ESR-2508) by Simpson Strong Tie.

5. AT-XP (IAPMO UES ER-263) by Simpson Strong Tie. 6. Alternative epoxies may be used if an ESR approval for use in cracked concrete is submitted to the structural

B. Adhesive for solid grouted concrete masonry & hollow block connections shall be: 1. HIT HY 70 (IESR-3342) by Hilti Corporation

2. AC100+ Gold (ESR-3200) by DeWalt

3. AT-XP (IAPMO 0281) by Simpson Strong

C. Follow all of the manufacturer's recommendations and ESR for epoxy installation. 2. PRODUCT: Mechanical Anchors A. Mechanical Anchors for Concrete connections shall be:

1. Kwik Bolt TZ (ESR-1917) by Hilti Corporation 2. Power-Stud+ SD2 (ESR-2502) by DeWalt

Strong-Bolt (ESR-1771) by Simpson Strong Tie Inc. 4. Alternative mechanical anchors may be used if an ESR approval for use in cracked concrete is submitted to the

structural engineer prior to use. B. Mechanical Anchors for Masonry Connections shall be

1. Kwik Bolt 3 (ESR-1385) by Hilti corporation (grout filled masonry applications) 2. Power-Stud+ SD1 (ESR-2966) by DeWalt, Engineered by Powers (grout filled masonry applications) 3. Wedge-All (ESR-1396) by Simpson Strong Tie Inc. (grout filled masonry applications)

C. Follow all of the manufacturer's recommendations and ESR for mechanical anchor installation. 3. PRODUCT: Screw Anchors

A. Screw Anchors for Concrete and grout filled Masonry connections shall be: 1. Kwik HUS-eZ (ESR-3027) by Hilti Corporation

2. Screw-Bolt (ESR-3889) by DeWalt, Engineered by DeWalt 3. Titen HD (ESR-2713) by Simpson Strong Tie Inc.

4. Alternative screw anchors may be used if an ESR approval for use in cracked concrete is submitted to the structural engineer prior to use.

B. Follow all of the manufacturer's recommendations and ESR for screw anchor installation. 4. Installation requirements for Adhesive anchors:

A. Adhesive anchors installed in horizontal to vertically overhead orientation to support sustained tension loads shall be done by a certified adhesive anchor installer (aai) as certified through aci/crsi (ACI 318 17.1.2). proof of current certification shall be submitted to the engineer for approval prior to commencement of installation.

B. Adhesive anchors must be installed in concrete aged a minimum of 21 days (ACI 318 17.1.2). for installations sooner than 21 days consult adhesive manufacturer.

C. If temperature of base material at time of adhesive installation is at 45 degrees (fahrenheit) or less, an Acrylic adhesive (Dewalt AC200+, Hilti HIT-HY200, Simpson AT-XP) is required.

1. CODES AND STANDARDS. Comply with:

A. The ANSI/AF&PA "National Design Specification", (NDS).

B. The grading requirements of the WWPA. C. Preservative Treatment requirements of the AWPA.

2. MATERIALS: (All materials shall be clearly marked)

A. Structural lumber species and grade shall be as follows: a. Joists, beams or headers: 'DF/L #2' or better. 'DF/L #1' or better. b. Posts and columns:

'DF/L #2' or better. c. Studs: d. Sill plates: 'DF/L #2' or better, treated.

B. Manufactured joists: Trus-Joist or approved equal. C. Structural Glued-Laminated Timber: 24F-V4 for simple spans and 24F-V8 for continuous or cantilevered beams.

D. Engineered Lumber: a. Structural Laminated-Veneer-Lumber (LVL): conform to the following minimum design values:

Fb = 2,600 psi. (Joist/Beam orientation) Fv = 285 psi.E = 1,900,000 psi

F. Wood connectors shall be Simpson Strong-Tie.

STRUCTURAL OBSERVATIONS

ITEM FOR OBSERVATION

Roof to wall connections Structural Observer's Shall:

Wood Diaphragms

Sub-diaphragms

STRUCTURAL OBSERVATIONS

knowledge, have not been resolved (see IBC 1704.5)

E. Wood structural panels shall be Exposure 1 Grade or better APA rated sheathing with exterior glue and conform to Standard PS 1-83, or PS 2-92.

A. See plans for roof and floor joists sizes. Joists shall be laterally supported at bearing points by solid blocking or with

B. Erect manufactured joists in accordance with the fabricator's commendations. Joists shall be able support the loads published in their design catalogs. C. Provide bridging at 8'-0" o.c. maximum spacing for dimensional lumber and LVL joists. Provide bridging in all other

manufactured joists as per the manufacturer's recommendations D. Fill all nail holes, round and triangular, in wood connectors (framing anchors, joist hangers, purlin anchors, etc.) with

nails as specified by the manufacturer, UNO. E. Bolts and all-threads in wood connectors shall be machine rated bolts, A307 or better. F. Install washers under all bolt nuts. Make bolt holes only 1/32 to 1/16 inch larger than bolts. Tighten nuts snugly, but DO

NOT crush the wood. DO NOT countersink bolts, UNO. G. Specified nails are common and shall correspond to the following diameters and lengths: (16d -0.162" & 3-1/2" long; 10d-0.148"Ø & 3" long; 8d-0.131"Ø & 2-1/2" long)

H. Minimum nailing of members: Conform to IBC, Table 2304.9.1, UNO. I. Nail built-up beams of 2x_ members 12" deep or less together with 16d nails at 12" o.c., staggered. Add (2) 16d common nails at supports. Bolt 2x_ members deeper than 12" together with 1/2" bolts at 16" o.c. staggered. Add (2)

J. All drilled holes and cut ends of preservative-treated and fire-retardant-treated wood shall be field treated with a

PROPOSED FREQUENCY

1. Provide proof of licensure as a licensed professional/structural engineer in currently registered in the state of the project

After sheathing has been secured to framing

After blocking and strapping are secured to framing After blocking and strapping are secured to framing

After blocking and connectors are secured and before being covered up

2. If structural observations are performed by individuals other than the design professional in responsible charge, they should first be approved by the Building Official 3. At the conclusion of work a final structural observation report must be submitted to the Building Official noting any deficiencies which, to the best of the structural observer's

K. Fasteners (including nuts and washers) in preservative-treated and fire-retardant-treated wood shall conform to IBC Section 2304.10.5, UNO.

STRUCTURAL STEEL

1. CODES AND STANDARDS: Comply with: A. AISC "Specification for Structural Steel Buildings & Commentary". B. AISC "Code of Standard Practice" excluding sections 7.5.4, and 7.11.5. C. AWS "Structural Welding Code", exclude items conflicting with AISC.

2. MATERIALS SHALL CONFORM AS FOLLOWS: ASTM A992, Fy = 50 ksi.A. Wide Flange beams & columns: B. Rect. Hollow Structural Sections (HSS): ASTM A500, Fy = 46 ksi, Gr. B. ASTM A500, Fy = 42 ksi, Gr. B. B. Round Hollow Structural Sections (HSS): ASTM A53, Fy = 35 ksi, Gr. B. C. Pipe: D. Misc. shapes & plates: ASTM A36, Fy = 36 ksi.E. High strength bolts: ASTM A325 F. Anchor rods: ASTM F1554, Fy = 36 ksi.

ASTM A307 or better. G. Other bolts: H. Welded anchors studs (WAS, HAS): ASTM A108, Fu = 65 ksi.I. Deformed bar anchors (DBA's): ASTM A496, Fy = 70 ksi, DO NOT substitute reinforcing for DBA's

3. CONSTRUCTION:

A. Fabricate in an approved fabricator's shop. B. Fabricate beams with incidental camber up, UNO

C. Use 6000 psi (minimum at 28-day) non-shrink liquid grout beneath bearing plates. Place grout per manufacturer's recommendations prior to loading member.

D. Add deformed bar anchors to structural sections embedded in concrete or masonry, UNO. Use the same size and spacing as the adjacent reinforcing bars. Minimum length of bars shall be 48 bar diameters but not less than 24 inches.

4. BOLTED CONNECTIONS:

A. Use 3/4" diameter bolts in Std. holes (bolt diameter + 1/16"), UNO.

B. Steel-to-steel connections: Use ASTM A325 type "N" connections, UNO. C. Other connections: Use ASTM A307 bolts or better except for anchor rods, UNO.

D. Use hardened washers beneath the turned element of the bolt or nut. Use beveled hardened washers where the outer face of bolted parts has a slope greater than one in twenty with respect to the plane normal to the bolts axis. At oversized holes, use hardened washers or plates at least 5/16" thick conforming to ASTM F436.

E. Tighten bolts until all plies of the joint are in firm contact. Snug tight condition, UNO.

F. Pretensioned bolts with Class A faying surfaces are required at all steel to steel connections for Moment Frames (SMF, IMF and OMF), Braced Frames (SCBR, OCBF and BRBF) and Eccentrically Braced Frames (EBF). G. Enlarge bolt holes by reaming. DO NOT torch cut.

5. WELDED CONNECTIONS:

A. Perform welding and cutting by AWS certified welders in accordance with ANSI/AWS D1.1 (latest edition). B. For typical shop & field welds, use filler metals with nominal 70 ksi tensile strength having:

 a. Matching material for multiple pass welds. b. A diffusible hydrogen limit of H16 or less.

c. A CVN toughness of 20 ft-lbs at 0 deg. F

C. For shop & field weld connections of lateral load resisting elements (all braced frames and all moment frames (demand critical welds)), use filler metals with nominal 70 ksi tensile strength having:

a. Matching material for multiple pass welds. b. A diffusible hydrogen limit of H16 or less.

c. A CVN toughness of 40 ft-lbs at 70 deg. F

D. Use pre-qualified procedures. E. Weld intersecting steel shapes together, which are not connected with bolts, with all-around fillet welds, UNO.

F. Weld studs and DBA's according to Manufacturer's specs.

G. Wherever possible use shop welds. The contractor shall coordinate field and shop welds between shop fabrication and the steel erector.

1/4"

5/16"

H. Remove slag from welds.

12 1/4" - 16 1/2"

16 1/2" - 20 1/4"

6. Provide full depth web stiffeners at each side of all beams at all bearing points. A. Stiffener plates shall be the thickness called out below unless noted otherwise, and shall be welded both side with fillet

welds all around: Weld size Flange width Less than 8 1/4" 3/16" 8 1/4" - 12 1/4"

VERIFICATION AND INSPECTIONS FOR THE STRUCTURAL DOCUMENTS

STATEMENT OF SPECIAL INSPECTIONS

1. The inspection requirements as noted on this sheet are required for the items that are specifically noted, designed and detailed in the structural documents. Refer to the current IBC, Chapter 17, the architectural drawings, and the geotechnical report for additional information and additional inspection requirements for non-structural items.

2. The project owner shall employ one or more special inspectors to provide inspections during construction on the types of work listed below. The special inspector shall be a qualified person who shall demonstrate competence, to the satisfaction of the building official and/or EOR, for inspection of the particular type of construction or operation requiring special inspection. These inspections are in addition to the inspections required by the building department of the local jurisdiction.

3. Special inspectors shall keep records of inspections. The special inspector shall furnish inspection reports to the building official and to the EOR in responsible charge. Reports shall indicate that work inspected was done in conformance with approved construction documents. Discrepancies shall be brought to the immediate attention of the contractor for correction. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the building official and to the EOR in responsible charge prior to the completion of that phase of the work. A final report documenting required special inspections and correction of any discrepancies noted in the inspections shall be submitted at a point in time agreed upon by the permit applicant and the building

4. Special inspections for each task shall be carried out in compliance with requirements per the current IBC and other material standards.

A. Where fabrication of structural load bearing members and assemblies are being performed on the premises of a fabricators shop, special inspections required shall be provided in the shop during the fabrication process. This requirement may be excepted if the work is done on the premises of a fabricator registered and approved to perform such work without special inspection. A certificate shall be required to verify such approval. At completion of the fabrication, the approved fabricator shall submit a certificate of compliance to the building official stating that the work was performed in accordance with the approved construction drawings

6. TESTING: The owner will provide testing by qualified testing personnel for the following types of construction:

Welding: type, size, length, and quality of shop and all field welds by approved methods. Ultrasonically test complete penetration welds Drill and epoxy anchors: load test 10% of anchors, with a minimum of (2) anchors tested

7. THE CONTRACTOR SHALL

A. Coordinate testing. DO NOT proceed with subsequent work until inspections and testing has been approved.

B. Copy inspection reports/testing results to the Arch/EOR and owner before work proceeds. C. Correct deficient work at no additional cost to the owner.

TABLE OF SER AND RTER ITEMS

(all past, present and future SER and RTER Structual Upgrade Measures are included on this table)

STRUCTURAL UPGRADE MEASURES FOR 511-5876

STRUCTURAL UPGRADE MEASURES ADDRESSED PREVIOUSLY IN STRUCTURAL DRAWINGS:

L				
	ITEM	DESCRIPTION	COMMENT	
		MEASURES FROM STRUCTURAL EVALUATION REPORT (SER):		
nd	S-13	NO RECORDED ITEMS	NA	
		MEASURES FROM ROOF TRUSS EVALUATION REPORT (RTER):		
let	NA	NO RECORDED ITEMS	NA	

STRUCTURAL UPGRADE MEASURES ADDRESSED IN THESE STRUCTURAL DRAWINGS:

ITEM	DESCRIPTION	COMMENT
	MEASURES FROM STRUCTURAL EVALUATION REPORT (SER):	
S-1	STRENGTHEN DIAPHRAGM BY ADDING BLOCKING AND INCREASED NAILING PATTERNS AT INADEQUATE LOCATIONS	ADDRESSED IN THESE DRAWINGS
S-2	ADD CLIPS AND BOLTING TO PROVIDE IN-PLANE SHEAR TRANSFER FROM WOOD DIAPHRAGM INTO MASONRY WALLS	ADDRESSED IN THESE DRAWINGS
S-3	ADD OUT-OF-PLANE WALL ANCHORS AND STEEL STRAPPING TO CONNECT TRUSSES, JOISTS, AND WOOD DIAPHRAGM TO MASONRY WALLS	ADDRESSED IN THESE DRAWINGS
S-4	PROVIDE ADEQUATE DIAPHRAGM CROSSTIES AND CHORDS FOR OUT-OF-PLANE WALL ATTACHMENT	ADDRESSED IN THESE DRAWINGS
S-6	PROVIDE STEEL STRAPPING AND CONNECTION TO CREATE ROOF DRAG AND COLLECTOR TO DELIVER DIAPHRAGM SHEAR LOAD TO RESISTING WALLS	ADDRESSED IN THESE DRAWINGS
S-7	PROVIDE 2X RIBBON BRACING	ADDRESSED IN THESE DRAWINGS
S-8	PROVIDE BLOCKING AND LS CLIPS TO PROVIDE LOAD PATH FOR SHEAR TRANSFER AT OVERBUILDS. PROVIDE 2X PONY WALLS TO ADEQUATELY TRANSFER ROOF LOADS.	ADDRESSED IN THESE DRAWINGS
S-9	PROVIDE METAL CLIPS SCREWED TO THE SIDE OF THE BOTTOM CHORD AND THE FURRING STRIP @ 24" O.C.	ADDRESSED IN THESE DRAWINGS
NS-1	PROVIDE STEEL ANGLE ATTACHED TO THE TOP OF WALL AND TO THE TRUSS BOTTOM CHORD	ADDRESSED IN THESE DRAWINGS
NS-2	CUT AND LOWER THE EXISTING SPIRE. PROVIDE LIGHTWEIGHT ALUMINUM STEEPLE CAP.	ADDRESSED IN THESE DRAWINGS
	MEASURES FROM ROOF TRUSS EVALUATION REPORT (RTER):	
NA	NO RTER ITEMS	NONE

STRUCTURAL UPGRADE MEASURES NOT ADDRESSED PREVIOUSLY OR IN THESE STRUCTURAL DRAWINGS:

	HEIVI	DESCRIPTION	COMINENT
		MEASURES FROM STRUCTURAL EVALUATION REPORT (SER):	
		REMOVE PORTIONS OF REINFORCED MASONRY WALLS IN THE CULTURAL HALL. REPLACE WITH NEW REINFORCED WALLS WITH NO WINDOW OPENINGS; ATTACH WALLS TO FOUNDATION WALL BELOW AND CONCRETE BOND BEAM ABOVE.	UPGRADE DURING NEXT R&I
		MEASURES FROM ROOF TRUSS EVALUATION REPORT (RTER):	
ſ	None	NONE. STRUCTURAL UPGRADE MEASURES FOR ROOF TRUSSES ARE COMPLETE.	NONE

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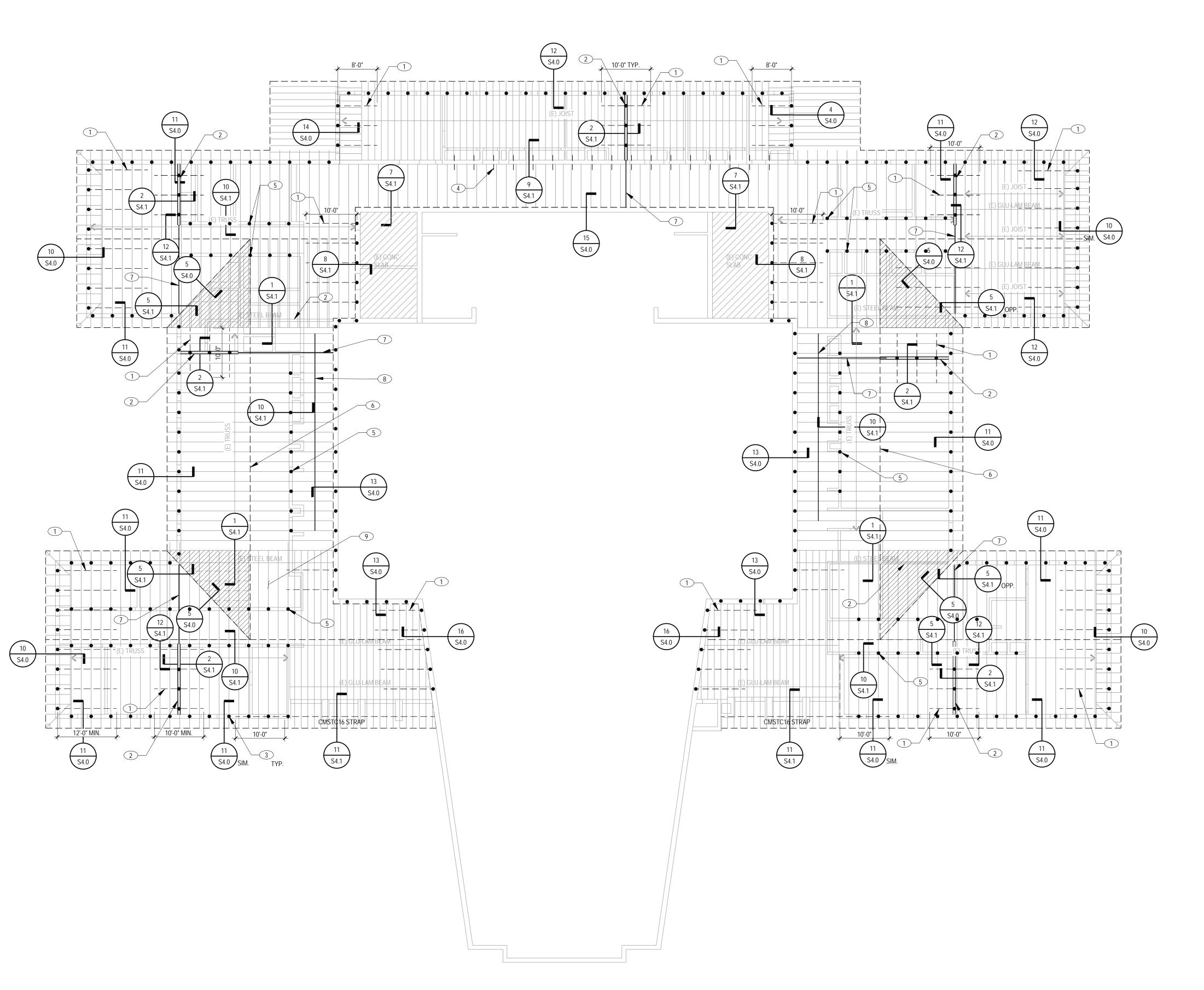
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PROP. NO.



LOWER ROOF UPGRADE PLAN SCALE: 3/32"=1'-0"

- A. VERIFY NAILING OF ENTIRE ROOF DIAPHRAGM. DO NOT MIX STAPLES EQUIVALENT FASTENERS OR RE-NAIL DIAPHRAGM WITH NAILS CALLED OUT IN SCHEDULE ON SHEET S1.2
- OUT-OF-PLANE CONNECTION, CONTRACTOR TO LOCATE EXACT LOCATIONS BASED ON FRAMING LAYOUT. SPACING IN SPECIFIC
- E. CONTRACTOR TO ANTICIPATE REMOVING A MINIMUM OF 4 FT SECTION OF ROOF SHEATHING ABOVE ALL MASONRY WALLS WHERE ADDITIONAL SEISMIC CONNECTIONS ARE BEING MADE. REPLACE SHEETING WITH MATCHING SHEETING, PER DETAIL 3/S3.0
- G. ALL REFERENCES TO CONSTRUCTION ADHESIVE REFER TO THE FOLLOWING. 3M5200 MARINE ADHESIVE MANUFACTURED BY 3M

COMPLY WITH THE INSTRUCTIONS ABOVE WILL REQUIRE CONTRACTOR TO DEMO THE DEFICIENT MEMBER AT THEIR

PLAN KEYNOTES:

- 1. CMSTC16 SUB-DIAPHRAGM STRAP @ 48"O.C. TYP. END LENGTH= 20". NAIL EVER HOLE IN END LENGTH AND EVERY OTHER HOLE THERE AFTER. LENGTH SHOWN ON PLANS
- 2. (E) MASONRY SHEAR WALL
- 4. SIMPSON CMSTC16x48" STRAP @ 48"O.C. OVER BEAM. PROVIDE BLOCKING BELOW STRAPS WIT 3/8" BEAD OF CONSTRUCTION ADHESIVE AND (6) 10d NAILS MIN.
- SPLICE WITH 10d NAILS EACH HOLE.

PLAN NOTES:

- AND NAILS. IF (E) SHEATHING IS ATTACHED WITH STAPLES, EITHER VERIFY STAPLES COMPLY WITH REQUIRED SPACING IN TABLE OF
- B. FOR BIDDING PURPOSES ITEMS IN CONSTRUCTION DOCUMENTS THAT ARE PROCEEDED WITH A (E) ARE ASSUMED TO BE EXISTING AND ALL OTHERS ARE TO BE ASSUMED TO BE NEW
- C. WD-# REPRESENTS THE WOOD DIAPHRAGM CALL OUT. SEE WOOD DIAPHRAGM KEY PLAN
- D. THIS MARK INDICATES APPROXIMATE LOCATION OF DETAILS
- HATCHED AREA OUTLINES OVER-BUILD LOCATIONS, SEE DETAIL 5/S4.0

WHERE DETAILS INDICATE A 3/8" DIAMETER BEAD OF CONSTRUCTION ADHESIVE. THE 3/8" IS BEFORE THE TWO MEMBERS ARE PRESSED EXPENSE, AND REPLACE USING THE PROPER GLUING PROCEDURES.



3. • OUT-OF-PLANE ROOF TO WALL CONN. @ 48"O.C. TYP. SEE DETAILS FOR ADDITIONAL INFORMATION

5. OUT-OF-PLANE CORRIDOR WALL CONNECTION, SEE **DETAIL 10/S4.1**

6. PROVIDE BOUNDARY NAILING @ EXISTING BLOCKING

- 2x6 FLAT PLATE DRAG WITH 2x4 SUPPORTS @ 48" O.C., SEE DETAIL 12/S4.1 FOR TYPICAL DRAG CONNECTION. FRAMING AT EACH CONDITION MAY DIFFER SLIGHTLY ADJUST ACCORDINGLY. PROVIDE CMSTC16 STRAP AT SPLICE IN FLAT PLATE. LAP LENGTH 20" EACH SIDE OF
- 8. PROVIDE 2x4 RIBBON BRACING WITH (2) 16d NAILS EACH WEB MEMBERS. LAP RIBBON BRACING ON TRUSS BAY WEB MEMBER NEAREST THE GYMNASIUM WALL.
- ROOF. SEE DETAILS 13/S4.1 AND 14/S4.1 FOR TYPICAL CONNECTION AND SPLICE.

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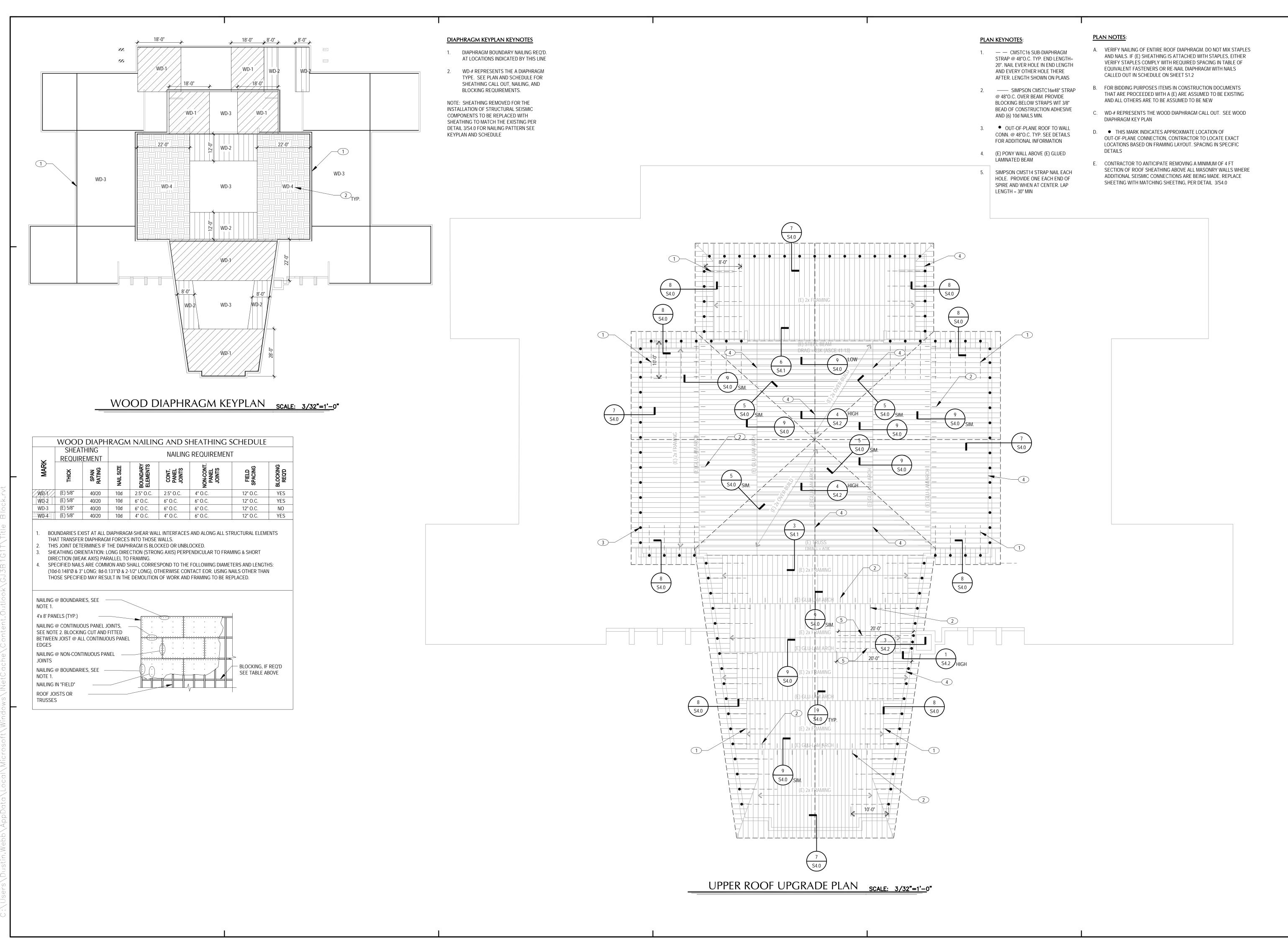
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> **LOWER ROOF UPGRADE PLAN**

S2.0



No.189660

CLIFF R. COLE

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BOUNTIFUL 7, 15, &

Schurch of Jesus Christ of Latter-Day Saints

BOUNTIFUL UTAH SOUTH STAKE
1250 SOUTH MAIN STREET
BOUNTIFUL LITAH

REVISIONS

DATE

DESCRIPTION

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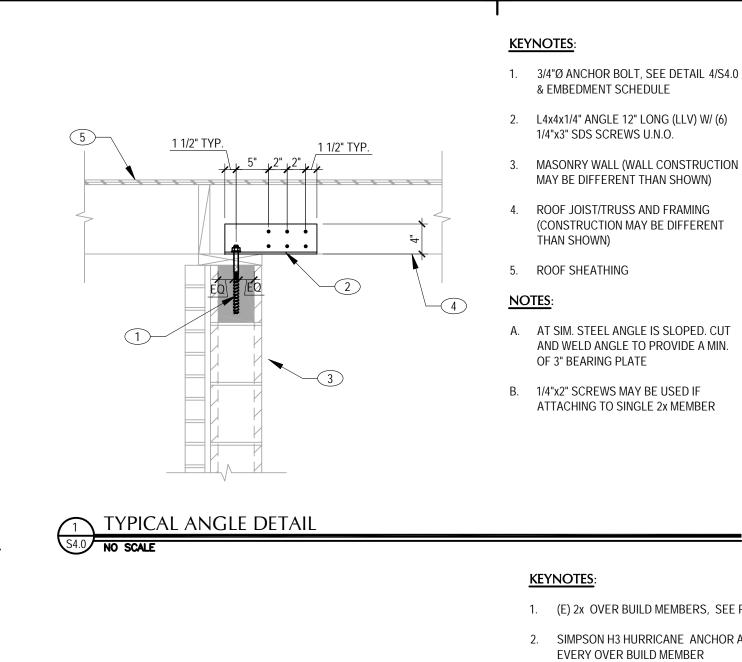
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UPPER ROOF UPGRADE PLAN

PROP. NO.

S3.0



1. 3/4"Ø ANCHOR BOLT, SEE DETAIL 4/S4.0 & EMBEDMENT SCHEDULE

- 2. L4x4x1/4" ANGLE 12" LONG (LLV) W/ (6)
- 1/4"x3" SDS SCREWS U.N.O.
- ROOF JOIST/TRUSS AND FRAMING (CONSTRUCTION MAY BE DIFFERENT THAN SHOWN)
- ROOF SHEATHING

- A. AT SIM. STEEL ANGLE IS SLOPED. CUT AND WELD ANGLE TO PROVIDE A MIN. OF 3" BEARING PLATE
- B. 1/4"x2" SCREWS MAY BE USED IF ATTACHING TO SINGLE 2x MEMBER

KEYNOTES:

- 1. SIMPSON STRAP, SEE PLANS FOR LENGTH
- 2. SHEATHING, SEE PLAN
- 3. 3x BLOCKING, UNO (SEE SPECIFIC DETAILS) W/ SIMPSON Z38 @ BOTH ENDS

- A. ALL BLOCKING: USE HEM-FIR #1 OR BTR
- B. CONTRACTOR MAY USE 2X BLOCKING, BUT IF THE 2X SPLITS THEY WILL NEED TO REMOVE THE BLOCK AND REPLACE WITH USE 3x BLOCKING, OR PRE-DRILL A 2X MEMBER AS FOLLOWS: PRE-DRILL HOLES W/ 7/64"Ø BIT FOR 10d COMMON NAILS & 3/32"Ø BIT FOR 8d COMMON

PARALLEL TO FRAMING

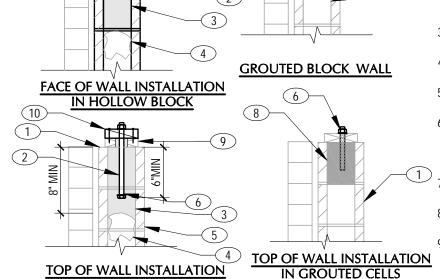
PERPENDICULAR TO FRAMING

KEYNOTES:

- 1. MATCH REPLACEMENT SHEATHING 7)— (2'-0" MIN. WIDTH)
- 2. REQ'D PANEL EDGE NAILING
- 3. SISTERED 2x6 CONT. BLOCKING W/
- 16d NAILS AT 6" O.C.
- 4. (E) FRAMING.
- 2x4 BLOCKING FLAT

NOTES: A. ALL BLOCKING: USE HEM-FIR #1 OR

BTR GRADE.



- IN HOLLOW BLOCK A. SEE EMBEDMENT SCHEDULE ON GSN FOR REQ'D EMBEDMENT UNO.
- B. FOR HOLLOW BLOCK CORE DRILL 2 -1/2"Ø HOLE. USE OF IMPACT EQUIPMENT IS NOT REQUIRED WHEN ATTACHING STEEL ANGLE IN DETAIL NOT ALLOWED. DRY PACK CELL BELOW AND CELL WHERE BOLT WILL BE INSTALLED

END LAP LENGTH SEE

- 1. (E) MASONRY BLOCK. IF EMBED CANNOT BE OBTAINED OR THINNER THAN 8" BLOCK , CONTACT EOR
- 2. BOLT LOCATED IN CENTER OF BLOCK USE DOUBLE NUT AT END OF BOLT

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- 3. GROUT CELL BENEATH BOLT
- PAPER DAM
- HOLLOW BLOCK WALL
- 6. THREADED ANCHOR BOLT EPOXIED IN GROUTED CELLS, SEE GSN FOR EPOXY TYPES. FOLLOW MFR. INSTALLATION INSTRUCTIONS
- 7. 1/2" BETWEEN INTERIOR FACE SHELL AND DOUBLE NUT.
- 8. GROUTED MASONRY WALL
- 9. DRILL 2 1/2" Ø HOLE IN EXISTING PLATE TO GROUT TOP COURSE OF BLOCK. PLACE GROUT AS SHOWN FILLING TO TOP OF WALL PLATE.
- 10. 2X BLOCK MATCHING SIZE OF PLATE WITH (8) 10d COMMON NAILS (4) EACH SIDE OF NEW ANCHOR BOLT. EXTRA PLATE

KEYNOTES:

- 1. (E) 2x OVER BUILD MEMBERS, SEE PLAN
- 2. SIMPSON H3 HURRICANE ANCHOR AT EVERY OVER BUILD MEMBER
- 2x10 BLOCKING BETWEEN FRAMING SHAPE AS REQ'D. PROVIDE (2) #8 x 3" **SCREWS**
- 4. EXISTING JOIST OR TRUSSES TOP CHORD
- EXISTING ROOF SHEATHING
- 2x NAILERS W/(2) #8 x3" WOOD SCREWS INTO EACH FRAMING MEMBER BELOW
- SIMPSON A35 EACH JOIST 2x4 PONY WALL WITH STUDS SPACED AT 24"O.C. POSITION WALLS AT 24" O.C. OR AT PANEL POINTS OF TRUSSES BELOW.
- 9. 2x FLAT BLOCK W/ (2) ROWS OF EDGE NAILING

A. AT SIM PONY WALLS ARE EXISTING.

3. NEW FULL HEIGHT 2x BLOCKING

COORDINATE WITH ARCH.

4. (E) ROOF SHEATHING, SEE PLANS

5. BOUNDARY NAILING INTO BLOCKING

6. SIMPSON COIL STRAP @ SIM. ONLY SEE

7. 2x BLOCK BELOW STRAP, GLUE AND

(E) JOIST @ SIM. ONLY

NAIL WITH 10d NAILS FROM BLOCK TO

BETWEEN EACH JOIST WITH SIMPSON

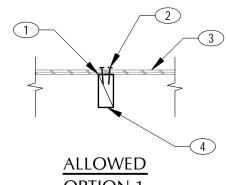
HOLES FOR VENTILATION EACH BLOCK

L50 EACH BLOCK. PROVIDE (2) 3"Ø

KEYNOTES:

1. (E) GLB

2. (E) ROOF JOISTS

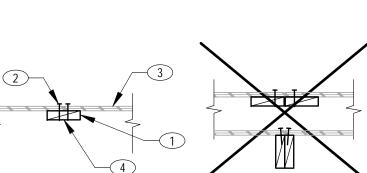




ALLOWED

OPTION 2

TYP. DIAPHRAGM BLOCKING DETAIL



NOT ALLOWED

KEYNOTES:

- BUTT BLOCKING TIGHT TO SHEATHING.
- 2. PANEL EDGE NAILING, SEE PLAN AND SCHEDULE. AT STEEL STRAP USE 10d TICO NAILS
- 3. SHEATHING, SEE KEYPLAN &
- 4. BLOCKING ON EDGE OR FLAT WISE, SEE SCHEDULE OR DETAIL FOR SIZE.

- ALL BLOCKING: USE HEM-FIR #1 OR BTR GRADE.
- PROVIDE SIMPSON Z4 AT ENDS.

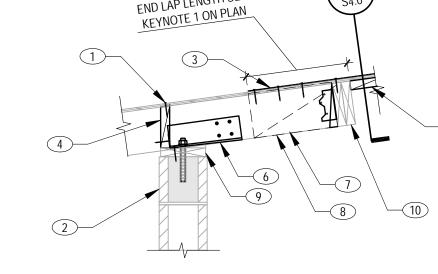
TYPICAL DIAPHRAGM REPLACEMENT DETAIL

KEYNOTES:

(E) MASONRY WALL

2. (E) ROOF JOISTS

- 3. STEEL ANGLE @ 48" O.C. SEE DETAIL
- 4. FULL HEIGHT 3x SHAPED BLOCKING WITH
- SIMPSON LS50 EACH BLOCK
- 5. (E) PLYWOOD ROOF SHEATHING, SEE PLANS FOR ADDITIONAL NAILING AND BLOCKING
- 6. REQ'D. PANEL BOUNDARY NAILING 7. (E) 2x SILL PLATE WITH NEW 5/8" BOLTS
- @ 32"O.C. 8. DIAPHRAGM BLOCKING SEE DETAIL 6/S4.0



/8 JOISTS TO WALL CONNECTION AT HIGH ROOF

ANCHOR BOLTS IN CMU BLOCK WALLS

KEYNOTES:

- 1. REQ'D PANEL BOUNDARY NAILING, SEE SCHED.
- 2. (E) MASONRY WALL
- 3. SIMPSON COIL STRAP, ALIGNED W/ STEEL ANGLE. SEE PLAN
- 4. 2x FULL HEIGHT BLOCKING WITH SIMPSON LS50
- 5. FLAT BLOCKING SEE DETAIL 2/S4.0
- (3) STEEL ANGLES EVENLY SPACED
- AT 48" O.C. SEE DETAIL 1/S4.0 7. 2x BLOCK BELOW STEEL STRAP. GLUE

BETWEEN GLUED LAMINATED BEAMS, OR

- AND NAIL WITH (12) 10d NAILS
- 8. (E) JOIST PROVIDE SIMPSON LS50 IF NO HANGER PRESENT
- 9. (E) SILL PLATE W/ (3/4" BOLTS AT 32" O.C., SEE DETAIL 4/S4.0
- 10. (E) DBL 2x ROOF JOIST

1. (E) ROOF SHEATHING, SEE PLANS AND

2. REQ'D. PANEL BOUNDARY NAILING

ANGLES ONLY

SCHEDULE FOR VERIFICATION OF NAILING

3. 2x6 BLOCK 36" LONG GLUE & NAIL W/ (12) 10d

NAILS. BLOCKS OCCUR AT NEW STEEL

4. STEEL ANGLE @ 4'-0"O.C. SEE DETAIL 1/S4.0

5. SIMPSON LS50 IF NO JOIST HANGER PRESENT

6. SIMPSON STRAP, SEE PLAN FOR LENGTH

7. 2x4 FLAT BLOCKING BELOW STRAP, SEE

8. 2x BLOCKING WITH (N) SIMPSON LS50 EACH

9. (E) 2x PLATE. VERIFY PLATE IS ATTACHED

WITH 3/4"Ø BOLTS @ 32"O.C., ADD

KEYNOTES:

TYP. HIGH ROOF TO WALL CONNECITON

KEYNOTES:

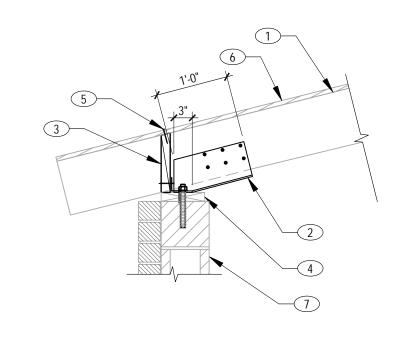
WHERE OCCURS

- (E) MASONRY WALL
- 2. (E) WOOD TRUSS
- 3. STEEL ANGLE @ 48"O.C. SEE DETAIL
- 1/S4.0
- 4. FULL HEIGHT 2x SHAPED BLOCKING WITH SIMPSON LS50 EACH BLOCK
- BLOCKING WHERE OCCURS

5. (E) PLYWOOD ROOF SHEATHING, SEE

KEYNOTE:

1. (E) MASONRY WALL



- (E) ROOF JOIST
- 2. L6x4x1/4 AT 48" O.C. w/ (6) 1/4"x2" SDS
- SCREWS, SEE DETAIL 4/S4.0, SIM. 3. FULL HEIGHT BLOCKING WITH
- 4. (E) SILL PLATE W/ NEW 3/4" SHEAR

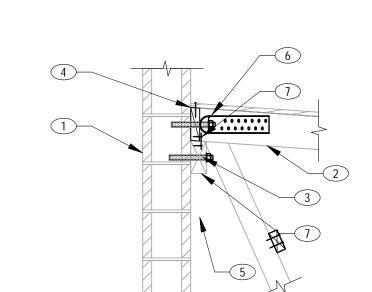
SIMPSON LS50 EACH BLOCK

- BOLTS AT 48" O.C. (SEE 4/S4.0)
- 5. REQ'D PANEL BOUNDARY NAILING 6. ROOF SHEATHING, SEE PLAN & SCHED.
- 7. (E) MASONRY WALL WITH BOND BEAM

S4.0 END LAP LENGTH SEE PLAN KEYNOTE

7 TYPICAL BLOCKING ABOVE GLB

TYPICAL OVERBUILD DETAIL



KEYNOTE:

- 2. (E) ROOF TRUSS

- 6. SIMPSON HDU2 W/ 5/8"Ø DRILL & EPOXY ANCHOR @ 4'-0" O.C. (SEE
- 7. SIMPSON A35 @ 24" O.C.
- 8. RIBBON BRACING AT MIDSPAN OF WEB

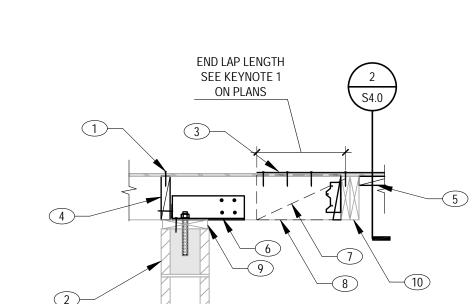
TRUSS TO WALL CONNECTION

* INSTALL HDU ANCHORS ON SAME

- 1. (E) MASONRY WALL
- (E) LEDGER
- 4. FULL-HEIGHT BLOCKING WITH BOUNDARY NAILING, SEE SCHED.
- 5. 2x8 LEDGER W/ 3/4"Ø BOLTS AT 16" O.C., STAGGER TOP & BOTTOM
- ANCHOR DETAIL 4/S4.0) *
- WHERE OCCURS, SEE PLAN KEYNOTE

TRUSSES AS ANGLE AT EAVE (SEE DETAIL

JOIST TO MASONRY WALL CONNECTION S4.0 NO SCALE



KEYNOTES:

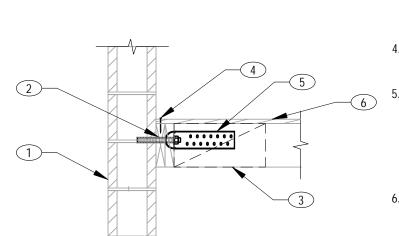
- 1. REQ'D PANEL BOUNDARY NAILING, SEE

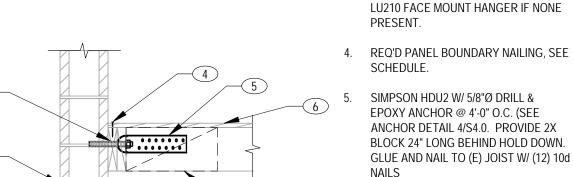
ACCORDINGLY

10. (E) TRUSS OR JOIST

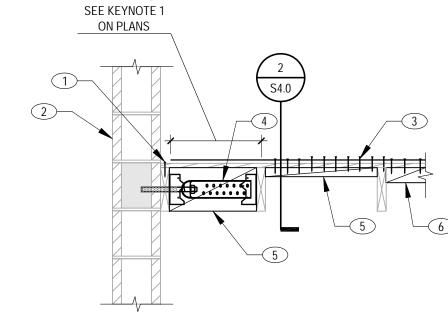
- 2. (E) MASONRY WALL 3. SIMPSON COIL STRAP, ALIGNED W/ STEEL
- ANGLE. SEE PLAN 4. 2x FULL HEIGHT BLOCKING WITH
- SIMPSON LS50
- STEEL ANGLES @ 48" O.C. SEE DETAIL
- 7. (2) 2x BLOCK BELOW STEEL STRAP. GLUE
- HANGER PRESENT
- 10. (E) DBL 2x ROOF JOIST

TYPICAL TRUSS TO WALL CONNECTION

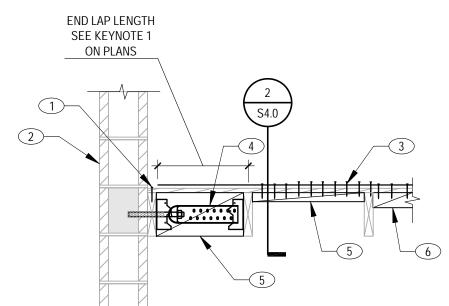




6. (E) ROOF SHEATHING, SEE PLANS FOR ADDITIONAL NAILING REQUIREMENTS.



JOIST TO WALL CONNECTION



1. REQ'D PANEL BOUNDARY NAILING, SEE

KEYNOTES:

3. SIMPSON STRAP, ALIGNED W/ HDU

4. SIMPSON HDU2 WITH 5/8"Ø ANCHOR BOLTS @ 4'-0" O.C.,

S4.0

WCA

02/12/19

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5. 2x FLAT BLOCKING SEE DETAIL 2/S4.0

- AND NAIL WITH (12) 10d NAILS 8. (E) JOIST PROVIDE SIMPSON LS50 IF NO
- 9. (E) SILL PLATE W/ (3/4" BOLTS AT 32" O.C. SEE DETAIL 4/S4.0

PLANS FOR ADDITIONAL NAILING AND

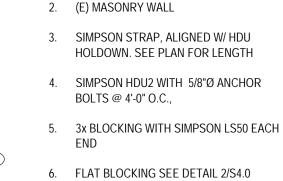
6. REQ'D. PANEL BOUNDARY NAILING 7. (E) 2x SILL PLATE WITH NEW 5/8" BOLTS

@ 32"O.C. SEE DETAIL 4/S4.0

A. AT SIM. PROVIDE LS90 CLIP EACH BLOCK WITH SIMPSON COIL STRAP ABOVE

2. (E) LEDGER PROVIDE 3/4" BOLTS AT 24"

3. (E) ROOF JOIST PROVIDE SIMPSON



6. FLAT BLOCKING SEE DETAIL 2/S4.0

A. SEE DETAIL 4/S4.1 WHERE JOIST SPACING IS SMALLER THEN LAP LENGTH

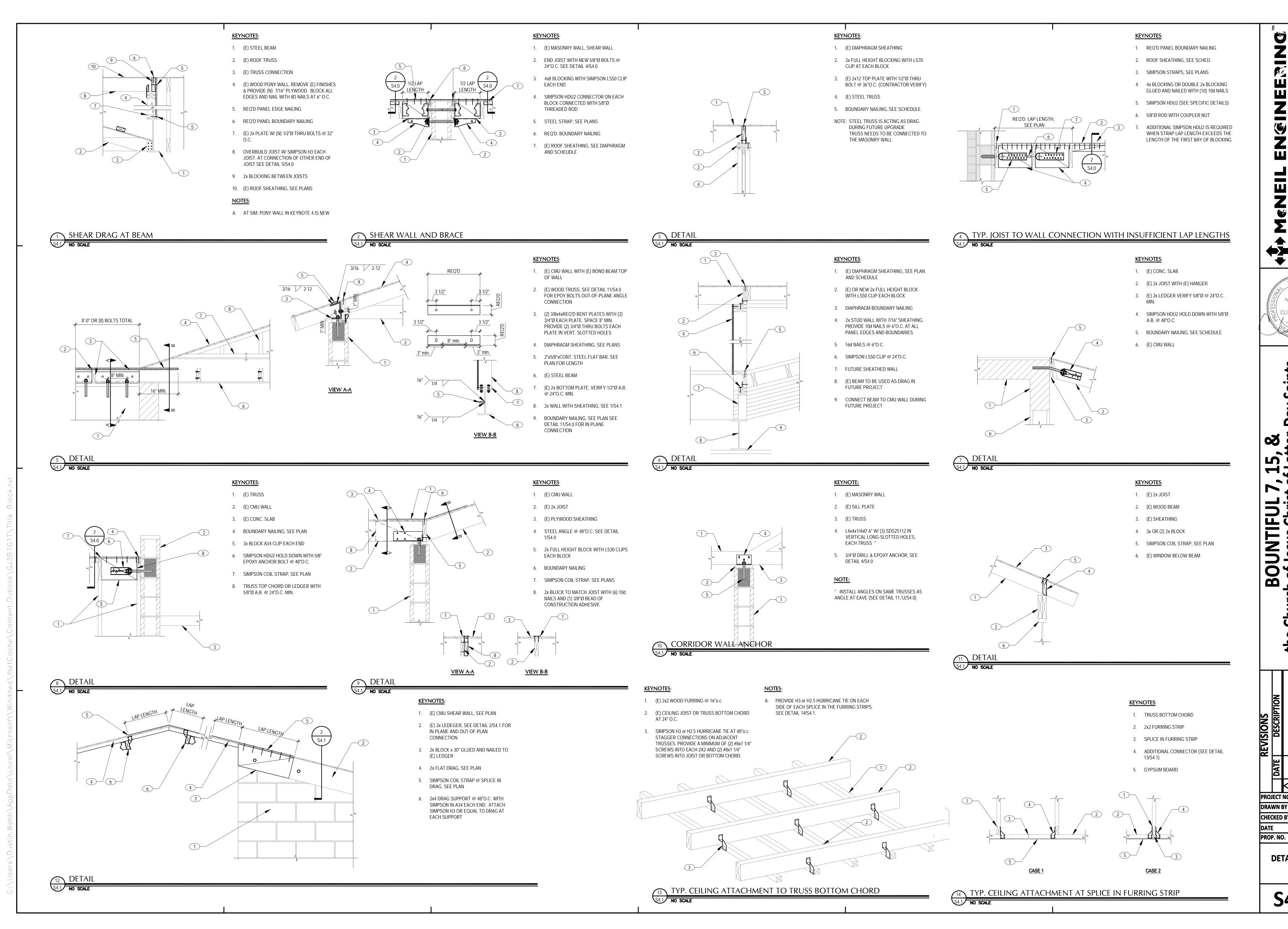
DETAILS

PROJECT NO.

DRAWN BY

CHECKED BY

PROP. NO.



Saints

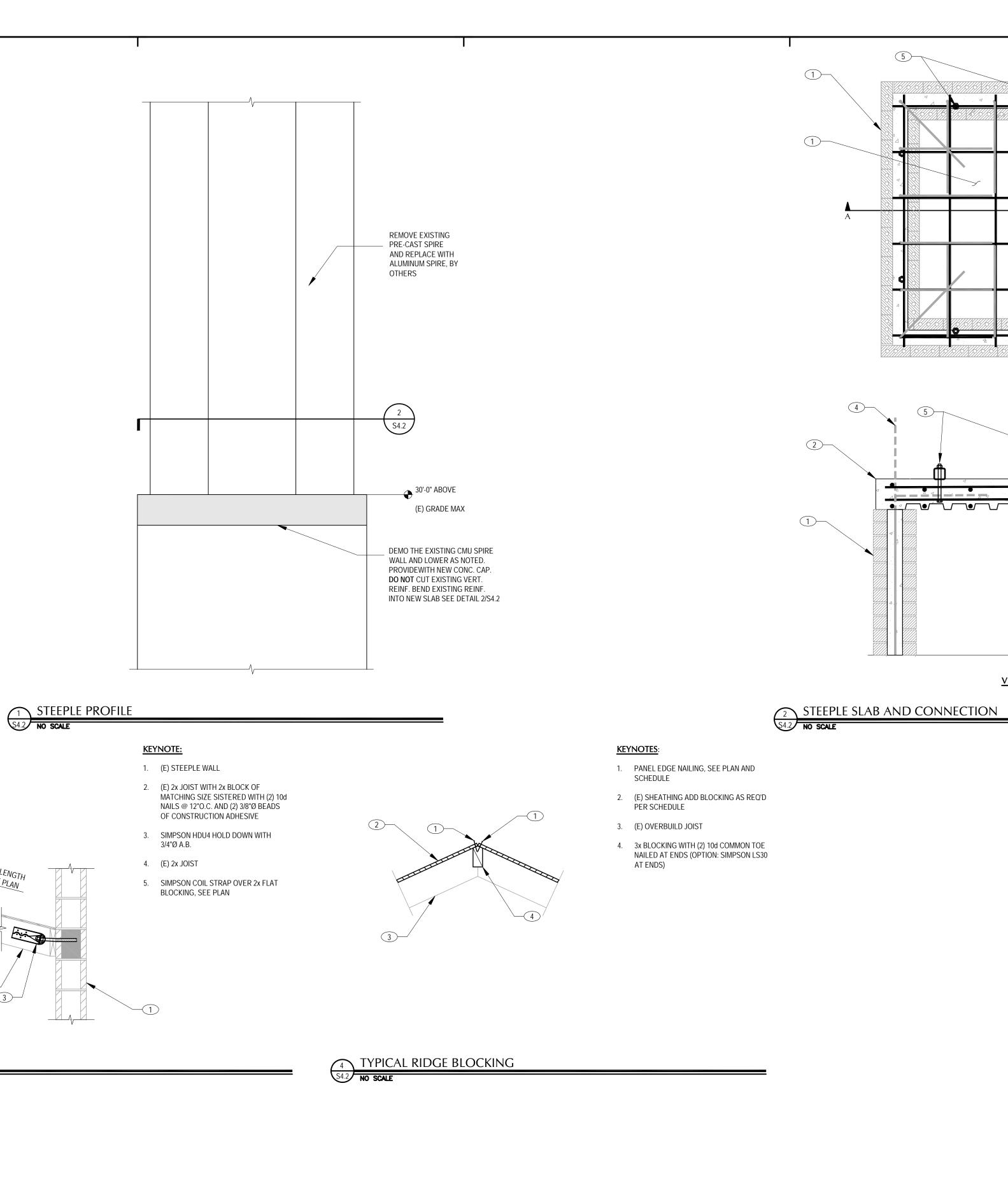
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PROJECT NO. DRAWN BY

WCA CHECKED BY CC 02/12/19 5115876

DETAIL

S4.1



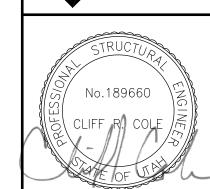
3 DETAIL
S4.2 NO SCALE

KEYNOTES:

- 1. (E) REINFORCED MASONRY WALL
- 2. PROVIDE 8" REINFORCED CONCRETE SLAB ON 3" 20 GA. B FORMLOK STEEL DECKING.
- 3. (6) #4 BARS EQUALLY SPACED TOP AND BOTTOM EACH DIRECTION
- 4. (E) REINFORCED MASONRY TOWER WALL, REMOVE THE EXISTING PRE-CAST STONE SPIRE AND BRICK AS REQ'D. LEAVE MIN OF 24" OF EXISTING REINFORCING FROM OLD TOWER EXPOSED AND BEND INTO NEW SLAB.
- 5. BASE OF NEW ALUMINUM STEEPLE BY OTHERS, PROVIDE 3/4" Ø ANCHOR BOLTS. COORDINATE BOLT QUANTITY AND PLACEMENT WITH STEEPLE FRAME MANUFACTURER

VIEW A-A

- A. CONTRACTOR **NOT** TO POUR CONCRETE CAP UNTIL STEEPLE SHOP DRAWINGS HAVE BEEN APPROVED
- B. DO NOT CUT THE EXISTING VERTICAL REBAR, EXISTING VERTICAL BARS MUST BE BENT INTO THE NEW CONCRETE SLAB.



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DETAIL

S4.2