

1. Design Criteria

- 1.1. Governing Building Code 2021 International Building Code (IBC)
- A. Risk Category II
- 1.2. Floor Live Loading
- A. Exam and Procedure Rooms 80 psf Live Load+ 20 psf Partition Load
- B. Offices 80 psf Live Load + 20 psf Partition Load
- C. Exit facilities and corridors 100 psf Live Load
- D. Lobbies 100 psf Live Load
- 1.3. Earthquake
- A. Seismic Design Category D
- B. Spectral Response Accelerations
- $S_s = 1.324\text{ g}$ $S_{0.2} = 1.059\text{ g}$
- $S_1 = 0.475\text{ g}$ $S_{0.1} = 0.578\text{ g}$
- C. Soil Site Class D
- $F_a = 1.2$ $F_v = 1.83$
- 1.4. Wind
- A. Interior Partition Wall Pressure 5 psf

2. Structural Steel

- 2.1. Material:
- A. W-Shapes: ASTM A992, ($F_y = 50\text{ ksi}$), except as noted otherwise
- B. All Other Shapes and Plates: ASTM A36 ($F_y = 36\text{ ksi}$), except as noted otherwise
- C. Rectangular and Square Hollow Structural Sections (HSS): ASTM A500, Grade C ($F_y = 50\text{ ksi}$)
- D. High-Strength Bolts.
- E. 1. Group A: ASTM F3125 Grades A325 & F1952
2. Anchor Rods: ASTM F1554, Grade 36, unless noted otherwise, with ASTM A563 heavy hex nuts and ASTM F436 hardened washers
- 2.2. Fabrication and construction shall comply with the following Codes and Standards:
- A. American Institute of Steel Construction (AISC):
1. AISC 360-16, "Specification for Structural Steel Buildings"
2. AISC 341-16, "Seismic Provisions for Structural Steel Buildings"
3. AISC 303-16, "Code of Standard Practice for Steel Buildings and Bridges"
- a. The structural drawings shall be used in conjunction with the architectural drawings. Detailing and shop drawing production for structural elements will require information (including dimensions) contained in architectural, structural, and/or other consultants' drawings. Refer to the Special Instructions section of the general notes, below.
- B. Research Council on Structural Connections (RCSC), "Specification for Structural Joints Using High-Strength Bolts", August 1, 2014.
- C. American Welding Society (AWS):
1. AWS D1.1—2015, "Structural Welding Code – Steel" (specific items do not apply when they conflict with the AISC requirements)
2. AWS D1.8—2016, "Structural Welding Code – Seismic Supplement" (specific items do not apply when they conflict with the AISC requirements)
- 2.3. Structural shapes and plates shall be fabricated from newly rolled (milled) one-piece sections without splices, unless specifically noted otherwise on the structural drawings. Connections for structural steel shall comply with the structural drawings, unless written approval is given by the Structural Engineer.
- 2.4. Bolted Connections:
- A. Provide snug tightened joints with Group A (threads not excluded) bolts for steel to steel connections, unless noted otherwise. Snug tightened joints shall be used in connections for simple span framing and beam (or girder) to bearing plate connections. Snug tight is the condition that exists when all of the piles in a connection have been pulled into firm contact by the bolts in the joint and all of the bolts in the joint have been tightened sufficiently to prevent the removal of the nuts without the use of a wrench. The snug tightened condition is typically achieved with a few impacts of an impact wrench, application of an electric torque wrench until the wrench begins to slow, or the full effort of a worker on an ordinary spud wrench.

3. Miscellaneous

- 3.1. Post-Installed Anchors in Concrete
- A. Anchorage to hardened concrete shall include all mechanical and adhesive anchors and epoxy dowelled reinforcing bars of size, quantity, spacing, and embedment as shown on the drawings. Additional anchors shall not be used without approval from the Engineer prior to installation.
- B. Special inspection is required during the installation of all post-installed anchors. Refer to applicable code evaluation reports and the Quality Assurance and Statement of Special Inspections sections of the General Structural Notes.
- C. Anchorage to Concrete:
1. All post-installed anchors into hardened concrete shall be selected from the following pre-approved products, unless noted otherwise:
- | Steel Screw Anchor | Evaluation Report |
|--------------------|-------------------|
| Hilti Kwik-Hus EZ | ICC ESR-3027 |
| DeWalt Screw-Bolt+ | ICC ESR-3889 |
| Simpson Titen HD | ICC ESR-2713 |
- | Steel Expansion/Wedge Anchor | Evaluation Report |
|------------------------------|-------------------|
| Hilti Kwik Bolt TZ2 | ICC ESR-4266 |
| DeWalt Power-Stub+ SD2 | ICC ESR-2502 |
| Simpson Strong-Bolt 2 | ICC ESR-3037 |
- | Adhesive Anchor System | Evaluation Report |
|------------------------|-------------------|
| Hilti HIT-HY 200 | ICC ESR-3187 |
| Hilti HIT-RE 500 V3 | ICC ESR-3814 |
| DeWalt AC208+ | ICC ESR-4027 |
| DeWalt Pure 110+ | ICC ESR-3299 |
| Simpson SET-3G | ICC ESR-4057 |
2. Adhesive anchors shall be installed into concrete having a minimum age of 21 days. For installations sooner than 21 days, consult the adhesive manufacturer.
- D. Alternate anchors or adhesives are permitted with approval of the Engineer. The Contractor shall submit the proposed anchor product data and code evaluation report demonstrating the anchor is equivalent to or exceeds the capacity of the specified anchor.
- E. Installation of adhesive anchors horizontally or upwardly inclined to support sustained tension loads shall be performed by personnel certified by an applicable certification program. Certification shall include written and performance tests in accordance with the ACI/CRSI Adhesive Anchor Installer Certification program, or equivalent. Proof of current certification shall be submitted to the Engineer for approval prior to commencement of installation.
- F. Anchors shall be installed according to the Manufacturer's Printed Installation Instructions and applicable code evaluation reports including:
1. Hole diameter, depth, and cleaning procedure
2. Adhesive mixing, preparation, and placement
3. Installation torque
- G. Locate all existing reinforcement and embedded items prior to drilling into concrete or masonry elements. Do not damage rebar or embeds while drilling or installing anchors.
- H. Grout all defective or abandoned holes with non-shrink grout or an injectable epoxy adhesive matching the surrounding concrete compressive strength. Consult the Architect for additional requirements at architecturally exposed concrete.
- I. Drilled anchors are not allowed in post-tensioned concrete without approval of the Architect and Engineer.
- J. Carbon steel anchors are limited to use in dry, interior locations.
- K. Holes for post-installed anchors may not be core drilled unless specifically allowed by the manufacturer's installation instructions and the code evaluation report.

4. Special Instructions

- 4.1. The project specifications are not superseded by the General Structural Notes but are intended to be complementary to them. Consult the specifications for additional requirements in each section. Notes and specific details on the drawings shall take precedence over General Structural Notes and typical details.
- 4.2. The architectural drawings are the prime contract drawings. Consultant drawings by other disciplines are supplementary to the architectural drawings. All omissions or conflicts, including dimensions, between the various elements of the consultants' drawings and/or specifications shall be brought to the attention of the Architect before proceeding with any work involved. In case of conflict, follow the most stringent requirement as directed by the Architect without additional cost to the Owner. Any work done by the Contractor after discovery of such discrepancy shall be done at the Contractor's risk.
- 4.3. The structural drawings shall be used in conjunction with the architectural drawings. Primary structural elements and overall structural layout are indicated within the structural plans and details. Some secondary elements, architectural layouts, alcoves, elevations, slopes, depressions, curbs, mechanical equipment and electrical equipment, are not indicated within the structural drawings. Detailing and shop drawing production for structural elements will require information (including dimensions) contained in the architectural, structural and/or other consultants' drawings.
- 4.4. All expansion joints (E.J.) shown in the structural drawings shall be considered seismic separation joints, unless noted otherwise. The width dimensioned shall be provided with a tolerance of (+1'-0") regardless of the tolerances stated in material reference standards.
- 4.5. Submittals: A copy of all shop drawings that have been submitted for review must be kept at the construction site for reference. These drawings must bear the appropriate review stamps. The shop drawing review shall not relieve the Contractor of the responsibility of completing the project according to the contract documents. The General Contractor shall review and mark all shop drawings prior to submitting them to the Architect.
- 4.6. Project Coordination: It shall be the responsibility of the General Contractor to coordinate with all trades any and all items that are to be integrated into the structural system. Openings or penetrations through, or attachments to the structural system that are not indicated on these drawings shall be the responsibility of the General Contractor and shall be coordinated with the Architect/Engineers. The order of construction is the responsibility of the General Contractor. It is the Contractor's obligation to provide all items necessary for the chosen procedure.

- 4.7. Contractor shall field verify all dimensions, and conditions. If the contract drawings do not represent actual conditions, Contractor shall notify Architect/Engineer prior to fabrication or construction within that area.
- 4.8. Notice of Copyright: The structural drawings, plans, schedules, notes and details are hereby copyrighted by Reaveley Engineers. Submission or distribution of documents to meet official regulatory requirements or for similar purposes in connection with the project is not to be construed as publication in derogation of Reaveley Engineers' reserved rights. The documents defining the structure are instruments of service prepared by Reaveley Engineers for one use only. Furthermore, these documents shall not be reproduced, or copied, in whole or in part by the Contractor or subcontractors for preparation of shop drawings or other submittals.

5. Quality Assurance

- 5.1. Quality Assurance Agency Requirements:
- A. The Owner shall engage a qualified Quality Assurance Agency (QAA) to provide all special inspection and quality assurance testing for the project. The QAA shall provide all information necessary for the building official to determine that the agency meets the applicable requirements.
1. The QAA shall be objective, competent and independent from the Contractor responsible for the work being inspected. The agency shall disclose to the building official and the registered design professional in responsible charge possible conflicts of interest so that objectivity can be confirmed.
2. The QAA shall have adequate equipment to perform required tests. The equipment shall be periodically calibrated.
3. The QAA shall employ experienced personnel educated in conducting, supervising and evaluating tests and special inspections. Experience or training shall be considered relevant where the documented experience or training is related in complexity to the same type of special inspection or testing activities for projects of similar complexity and material qualities.
4. The QAA shall send copies of all inspection and testing reports to the building official, Owner, Architect, Engineer and Contractor. Reports shall indicate that the work inspected was or was not completed in conformance to the approved construction documents. Discrepancies shall be brought to the immediate attention of the Contractor for correction. If they are not corrected, the discrepancies shall be brought to the attention of the Architect and Engineer.
5. The QAA shall submit a final report documenting required special inspections and tests, and correction of any discrepancies noted in the inspections or tests. The final report shall be distributed to the building official, Owner, Architect and Engineer in a timely manner prior to the completion of the project.
- 5.2. Contractor Responsibilities:
- A. The Contractor shall submit a written statement of responsibility to the building official and the Owner or the owner's authorized agent prior to the commencement of work on the systems or components listed in the statement of special inspections. The Contractor's statement of responsibility shall contain acknowledgement or awareness of the special requirements contained in the statement of special inspections.
- B. Notification of QAA: The Contractor shall notify the QAA in a timely manner so that inspection and testing may be performed as outlined in the statement of special inspections.
- 5.3. Structural Observations by the Engineer of Record:
- A. The Engineer of Record will perform structural observations at critical phases of the project as listed below. Observations will be made on a periodic basis throughout the construction of the structural system. During this timeframe, one site visit will be provided. Copies of the Engineer's report will be distributed to the Architect, Contractor, Owner, and QAA.
- B. Observation visits to the site by the Engineer's field representatives shall not be construed as inspection or approval of construction.

6. Statement of Special Inspections

- 6.1. The following materials, systems and components require special inspection or testing per Chapter 17 of the International Building Code (IBC).
- 6.2. For items requiring continuous inspection, a special inspector must be present onsite during the performance of that task. In most cases, periodic inspections/tests shall be performed prior to commencing the task, intermittently during the task, and at the completion of the task. Frequency marked with (E) designates periodic inspections that must be performed prior to or upon completion of every task.

Structural Steel per IBC Section 1705.2.1, 1705.13.1 & 1705.14.1

Item	Frequency	Detailed Instructions
<i>Prior to Bolting (Table N5.6-1, AISC 360-16):</i>		
Certifications of fasteners	Continuous	Verify that manufacturer's certificates are available for fastener materials.
Fasteners marked	Periodic	Verify that fasteners have been marked in accordance with ASTM requirements.
Proper fasteners for joint	Periodic	Verify grade, type, and bolt length if threads are excluded from the shear plane.
Proper bolting procedure	Periodic	Verify proper procedure is used for the joint detail.
Connecting elements	Periodic	Verify appropriate faying surface condition and hole preparation, if specified, meet requirements.
Pre-installation verification testing	Periodic	Observe and document verification testing by installation personnel for fastener assemblies and methods used.
Proper storage	Periodic	Verify proper storage of bolts, nuts, washers, and other fastener components.
<i>During Bolting (Table N5.6-2, AISC 360-16):</i>		
Fastener assemblies	Periodic	Verify that fastener assemblies are of suitable condition, spaced in all holes, and washers and nuts are positioned as required.
Snug-tight prior to pretensioning	Periodic	Verify that joints are brought to snug-tight condition prior to pretensioning operation.
Fastener component	Periodic	Verify that fastener component not turned by wrench is prevented from rotating.
Pretensioned fasteners	Periodic	Verify that fasteners are Pretensioned in accordance with RCSC Specification, progressing systematically from the most rigid point toward the free ends.
<i>After Bolting (Table N5.6-3, AISC 360-16):</i>		
Documentation	Periodic (E)	Document the acceptance or rejection of bolted connections.
<i>Other Steel Inspections (Section N5.6, AISC 360-16; Table J8.1, J10.1, AISC 341-16):</i>		
Structural steel details	Periodic	All fabricated steel or steel frames shall be inspected to verify compliance with the details shown in the approved construction documents, such as braces, stiffeners, member locations, and proper application of joint details at each connection.
Anchor rods and other embedments supporting structural steel	Periodic	Shall be on the premises during the placement of anchor rods and other embedments supporting structural steel for compliance with construction documents. Verify the diameter, grade, type, and length of the anchor rod or embedded item, and the extent or depth of embedment prior to placement of concrete.

Concrete Construction per IBC Sections 1705.3 & 1705.12

Item	Frequency	Detailed Instructions
Post-installed adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads	Continuous	All post-installed anchors/dowels shall be specially inspected as required by the approved ICC-ES report. Horizontally or upwardly inclined anchors that resist sustained tension loads require continuous inspection and approved installers.
Post-installed mechanical anchors and adhesive anchors not defined above	Periodic	

PLAN LEGEND	
	STEEL BEAM OR GIRDER
	STEEL JOIST OR PURLIN
	STEEL ANGLE BRACE / KICKER, SEE A1/SF501.
	EXISTING STEEL COLUMN - TUBE (HSS)
	EXISTING STEEL COLUMN - WIDE FLANGE
	EXISTING STEEL BEAM OR GIRDER
	EXISTING STEEL JOIST OR PURLIN
	EXISTING CROSS BRIDGING
	EXISTING HORIZONTAL BRIDGING
	EXISTING OPENING

ABBREVIATIONS	
@	AT
AB	ANCHOR BOLT (S)
ABV	ABOVE
ALT	ALTERNATE
APPROX	APPROXIMATE
ARCH	ARCHITECT(URAL)
BLDG	BUILDING
BLW	BELOW
BM	BEAM
BOT	BOTTOM
BRG	BEARING
BTWN	BETWEEN
CJ	CONSTRUCTION JOINT OR CONTROL JOINT
CJP	COMPLETE JOINT PENETRATION
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONST	CONSTRUCTION
CONT	CONTINUOUS
CONTR	CONTRACTOR
CTR	CENTER
D.B.	DECK BEARING
db	DIAMETER OF REINFORCING BAR
DBA	DEFORMED BAR ANCHORS
DBL	DOUBLE
DET	DETAIL
DIA (OR Ø)	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DK	DECK
DN	DOWN
DWG	DRAWING
DWL	DOWEL
E.F.	EACH FACE
E.J.	EXPANSION JOINT (SEISMIC SEPARATION JOINT)
E.W.	EACH WAY
EA	EACH
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR
ENG	ENGINEER
EQ	EQUAL
EQUIP	EQUIPMENT
EXIST (E)	EXISTING
EXP	EXPANSION / EXPOSED
EXT	EXTERIOR
F.D.	FLOOR DRAIN
F.F.	FINISH FLOOR
F.V.	FIELD VERIFY
FDN	FOUNDATION
FIN	FINISH
FL	FLOOR
FT	FOOT
FTG	FOOTING
GA	GAUGE
GALV	GALVANIZED
GLB	GLU-LAMINATED BEAM
GR	GRADE
GSN	GENERAL STRUCTURAL NOTES
HB	HORIZONTAL BRIDGING
HORIZ	HORIZONTAL
HSA	HEADED STUD ANCHORS
HSS	HOLLOW STRUCTURAL STEEL
HT	HEIGHT
I.F.	INSIDE FACE
IBC	INTERNATIONAL BUILDING CODE
ICC	INTERNATIONAL CODE COUNCIL
IN	INCH
INSUL	INSULATION
INT	INTERIOR
JST	JOIST
JT	JOINT
K	KIPS - 1,000 POUNDS
KLF	KIPS PER LINEAL FOOT
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
LBS	POUNDS
Ld, Lt, Lsb, Lsb1, Ldc, Lsc	SEE CONCRETE REINFORCING BAR DEVELOPMENT AND LAP LENGTH SCHEDULE
LF	LINEAL FOOT
LFRS	LATERAL FORCE RESISTING SYSTEM (SFRS & WFRS)
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LSH	LONG SIDE HORIZONTAL
LSV	LONG SIDE VERTICAL
MAS	MASONRY
MAX	MAXIMUM
MCJ	MASONRY CONTROL JOINT
MECH	MECHANICAL
MFGR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
NIC	NOT IN CONTRACT
NORM	NORMAL
NTS	NOT TO SCALE
O.C.	ON CENTER
O.F.	OUTSIDE FACE
OPNG	OPENING
OPP	OPPOSITE
OWSJ	OPEN WEB STEEL JOIST
P.T.	POST-TENSIONED
PAF	POWDER ACTUATED FASTENER
PCF	POUNDS/CUBIC FOOT
PJP	PARTIAL JOINT PENETRATION
PL	PLATE
PLF	POUNDS/LINEAL FOOT
PNL	PANEL
PSF	POUNDS/SQ FOOT
PSI	POUNDS/SQ INCH
R.D.	ROOF DRAIN

ABBREVIATIONS	
REINF	REINFORCING
REQD	REQUIRED
SDS	SELF-DRILLING SCREW
SFRS	SEISMIC FORCE RESISTING SYSTEM
SHT	SHEET
SI	SPECIAL INSPECTION (SP. INSP.)
SIM	SIMILAR
SOG	SLAB ON GRADE
SQ	SQUARE
STAG	STAGGERED
STD	STANDARD
STIFF	STIFFENER
STL	STEEL
STRUCT	STRUCTURAL
T & B	TOP AND BOTTOM
T.O.	TOP OF
TEMP	TEMPERATURE
THDS	THREADS
TOC	TOP OF CONCRETE
TOCP	TOP OF CONCRETE PIER
TOF	TOP OF FOOTING
TOS	TOP OF SLAB
TOST	TOP OF STEEL
TOW	TOP OF WALL
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W.P.	WORK POINT
W/	WITH
WF	WIDE FLANGE
WFRS	WIND FORCE RESISTING SYSTEM
WT	WEIGHT
WWF	WELDED WIRE FABRIC
YD	YARD

PLAN MARKS	
BF-#	BRACED FRAME
CB-#	CONCRETE BEAM
CC-#	CONCRETE COLUMN
CCSS-#	CANTILEVERED CONCRETE SUSPENDED SLAB
CDP-#	CONCRETE DRILLED PIER
CFW-#	CONCRETE FOUNDATION WALL
CGB-#	CONCRETE GRADE BEAM
CJ-#	CONCRETE JOIST
CJC-#	CONCRETE JAMB COLUMN
CL-#	CONCRETE LINTEL
CP-#	CONCRETE PIER
CRW-#	CONCRETE RETAINING WALL
CSG-#	CONCRETE SLAB ON GRADE
CSH-#	CONCRETE SHEAR HEAD
CSS-#	CONCRETE SUSPENDED SLAB
CSW-#	CONCRETE SHEAR WALL
CW-#	CONCRETE WALL
FC#	CONTINUOUS FOOTING
FM#	MAT FOOTING
FR#	RECTANGULAR FOOTING
FS#	SQUARE FOOTING
FTS#	THICKENED SLAB FOOTING
HD-#	HOLD DOWN ANCHOR
MC-#	MASONRY COLUMN
MF-#	MOMENT FRAME
ML-#	MASONRY LINTEL
MP-#	MASONRY PIER
MW-#	MASONRY WALL
PTB-#	POST-TENSIONED CONCRETE BEAM
SBP-#	STEEL BASE PLATE
SC-#	STEEL COLUMN
SCP-#	STEEL CAP PLATE
SD-#	STEEL DECK
SDA-#	STEEL DECK ATTACHMENT
SG-#	STEEL GIRDER
SJ-#	STEEL JOIST
SND-#	SNOW DRIFT
WB-#	WOOD BEAM
WBW-#	WOOD BEARING WALL
WC-#	WOOD COLUMN
WD-#	WOOD DIAPHRAGM
WJ-#	WOOD JOIST
WSW-#	WOOD SHEAR WALL

STRUCTURAL DRAWING LIST	
SHT NO.	SHT NAME
SE001	GENERAL STRUCTURAL NOTES
SF101	PARTIAL MEDICAL EQUIPMENT SUPPORT FRAMING PLANS
SF501	MEDICAL EQUIPMENT SUPPORT DETAILS
SF502	MEDICAL EQUIPMENT SUPPORT DETAILS

HKS

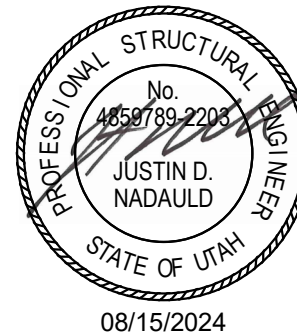
ARCHITECT
HKS ARCHITECTS, INC.
222 SOUTH MAIN, SUITE 230
SALT LAKE CITY, UT 84101

STRUCTURAL ENGINEER
REAVELEY ENGINEERS & ASSOCIATES
675 EAST 500 SOUTH, SUITE 400
SALT LAKE CITY, UTAH 84102

MECHANICAL ENGINEER
VAN BOERUM & FRANK ASSOCIATES, INC
181 EAST 5600 SOUTH, SUITE 130
MURRAY, UTAH 84107

ELECTRICAL ENGINEER
SPECTRUM ENGINEERS, INC
324 SOUTH STATE STREET, SUITE 400
SALT LAKE CITY, UTAH 84111

LAYTON HOSPITAL
MISC PROJECTS



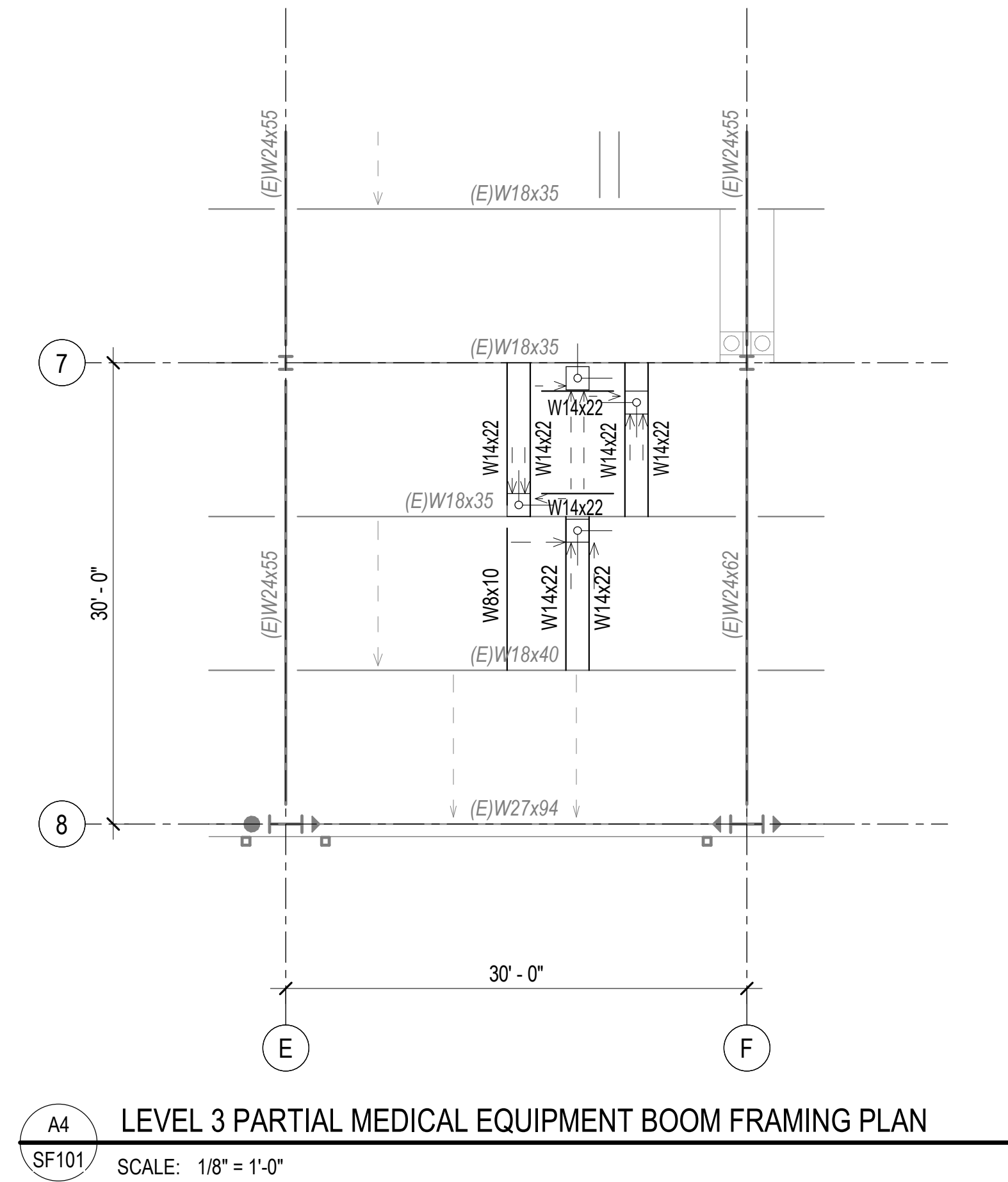
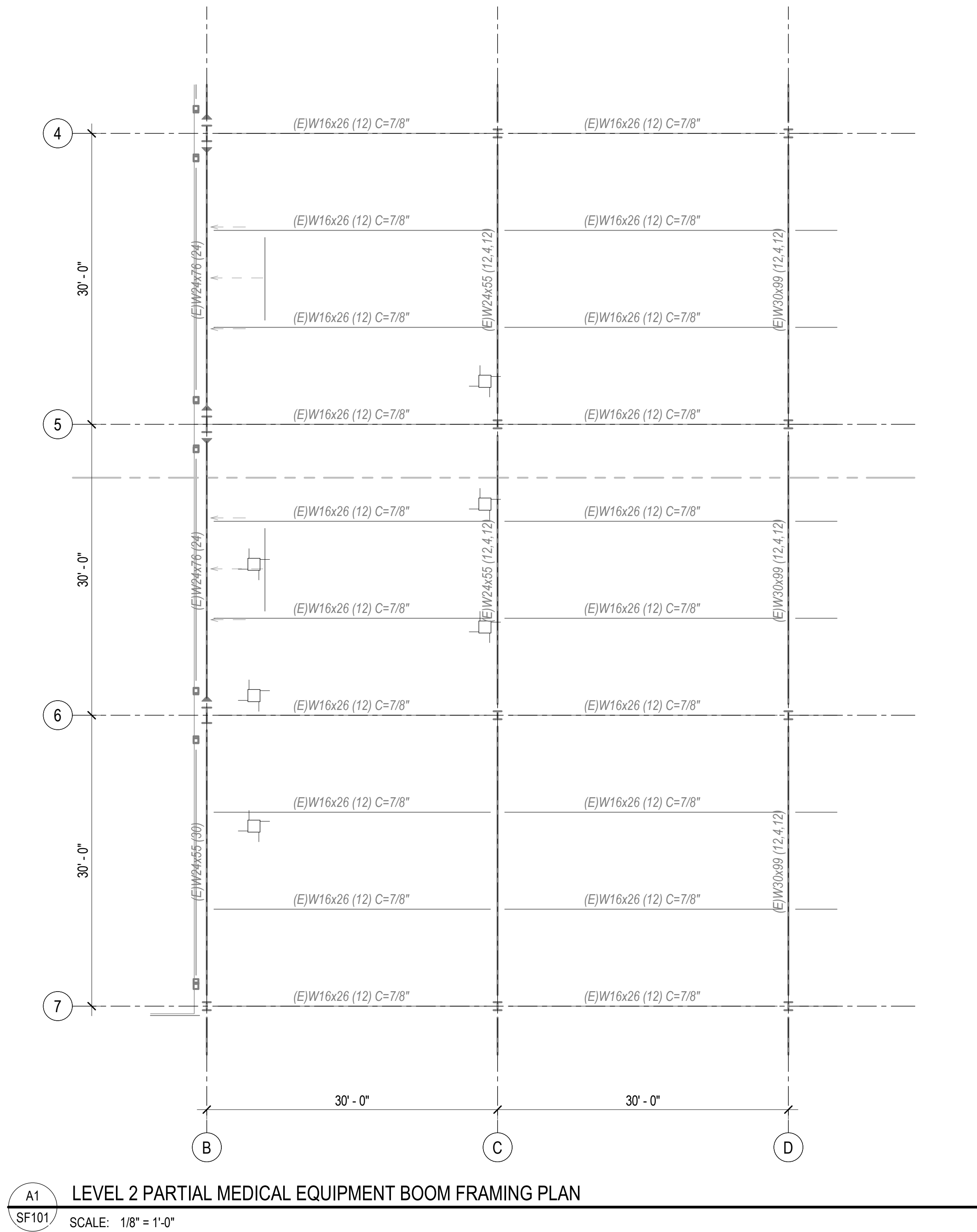
KEY PLAN

REVISION
NO. DESCRIPTION DATE

HKS PROJECT NUMBER
26404.000
DATE
08/15/2024
ISSUE
CONSTRUCTION DOCUMENTS
SHEET TITLE
GENERAL STRUCTURAL NOTES
SHEET NO.

SE001

PLOT DATE: 8/16/2024 1:17:22 PM
TEMPLATE VERSION: 218.20141012



MEDICAL EQUIPMENT LEGEND	
	SINGLE EXAM ROOM LIGHT, SEE B4/SF502. SEE ARCH FOR DIMENSIONAL LAYOUT.
	EQUIPMENT SUPPORT FOR STERIS EQUIPMENT SEE B5/SF501. SEE ARCH FOR DIMENSIONAL LAYOUT.

EXISTING BUILDING NOTES	
1. THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO DETAILING, FABRICATING, ERECTING OR INSTALLING ANY STRUCTURAL ELEMENT. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGN TEAM IN A TIMELY MANNER SUCH THAT WORK WILL NOT BE DELAYED.	
2. THE CONTRACTOR SHALL PROVIDE ADEQUATE SHORING OF EXISTING STRUCTURE DURING CONSTRUCTION.	

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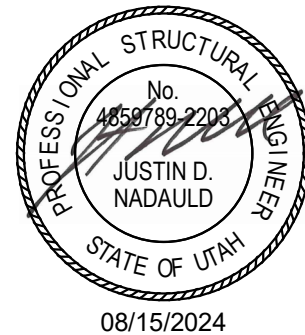
ARCHITECT
HKS ARCHITECTS, INC.
222 SOUTH MAIN, SUITE 230
SALT LAKE CITY, UT 84101

STRUCTURAL ENGINEER
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ELECTRICAL ENGINEER
SPECTRUM ENGINEERS, INC.
324 SOUTH STATE STREET, SUITE 400
SALT LAKE CITY, UTAH 84111

**LAYTON HOSPITAL
MISC PROJECTS**



KEY PLAN

REVISION:
NO. DESCRIPTION DATE

HKS PROJECT NUMBER
26404.000
DATE
08/15/2024
ISSUE
**CONSTRUCTION
DOCUMENTS
PARTIAL MEDICAL
EQUIPMENT
SUPPORT
FRAMING PLANS
SF101**

ARCHITECT

HKS ARCHITECTS, INC.
222 SOUTH MAIN, SUITE 230
SALT LAKE CITY, UT 84101

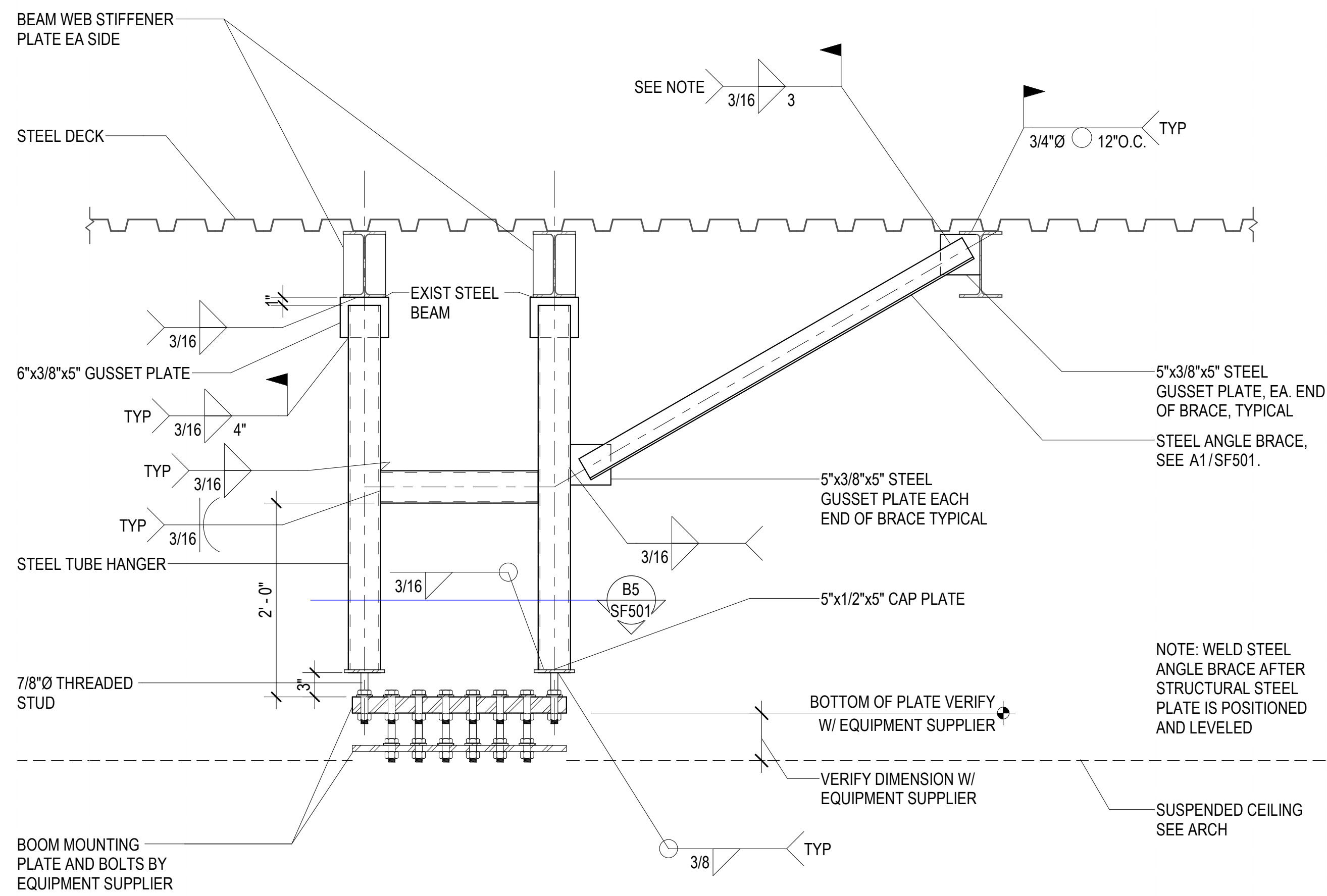
STRUCTURAL ENGINEER

REAVELEY ENGINEERS & ASSOCIATES
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SALT LAKE CITY, UTAH 84102

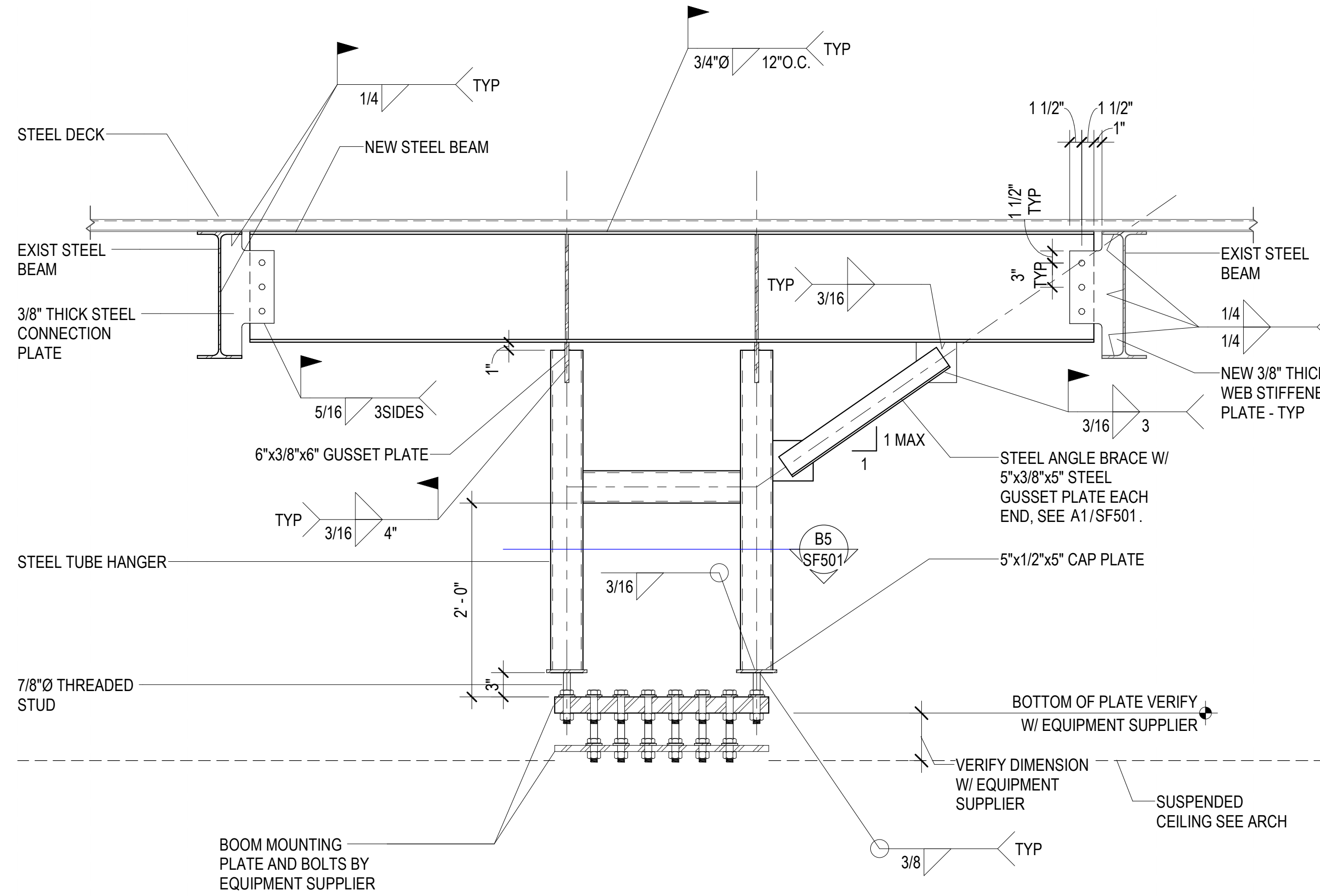
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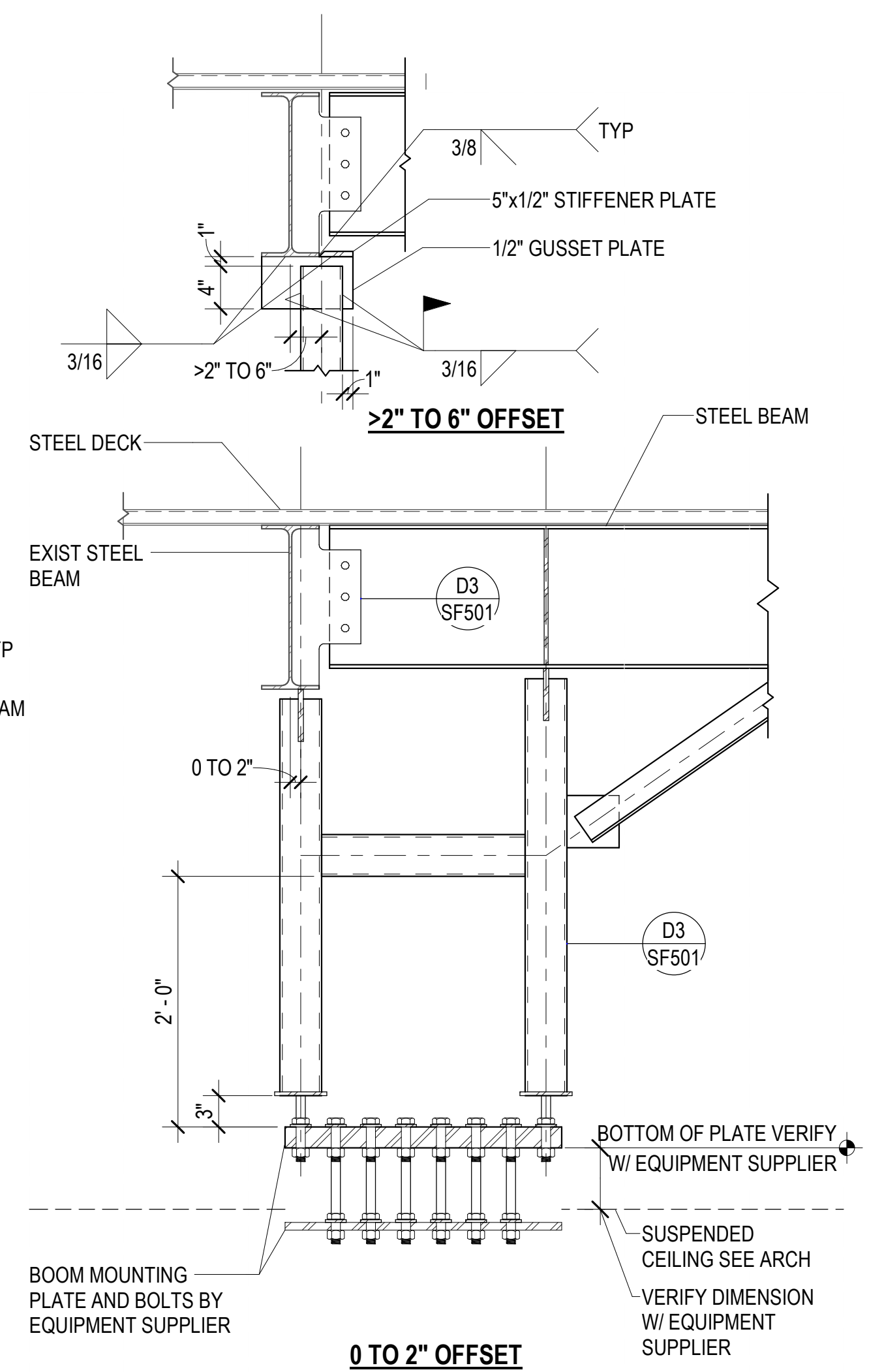
ELECTRICAL ENGINEER

SPECTRUM ENGINEERS, INC.
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SALT LAKE CITY, UTAH 84111

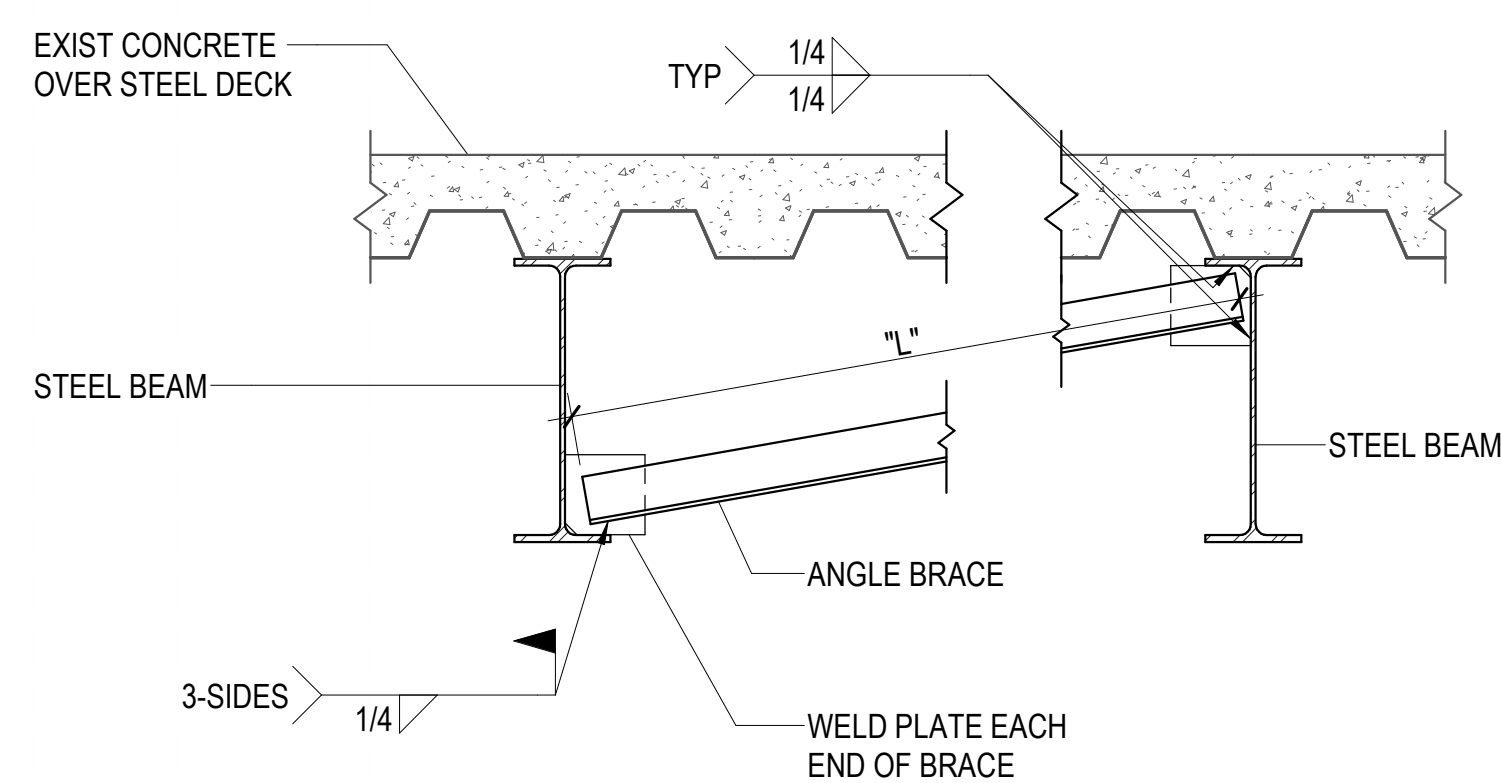
D1
SF501 SECTION AT STERIS TANDEN SUPPORT
NO SCALE



D3
SF501 SECTION AT STERIS TANDEN SUPPORT
NO SCALE



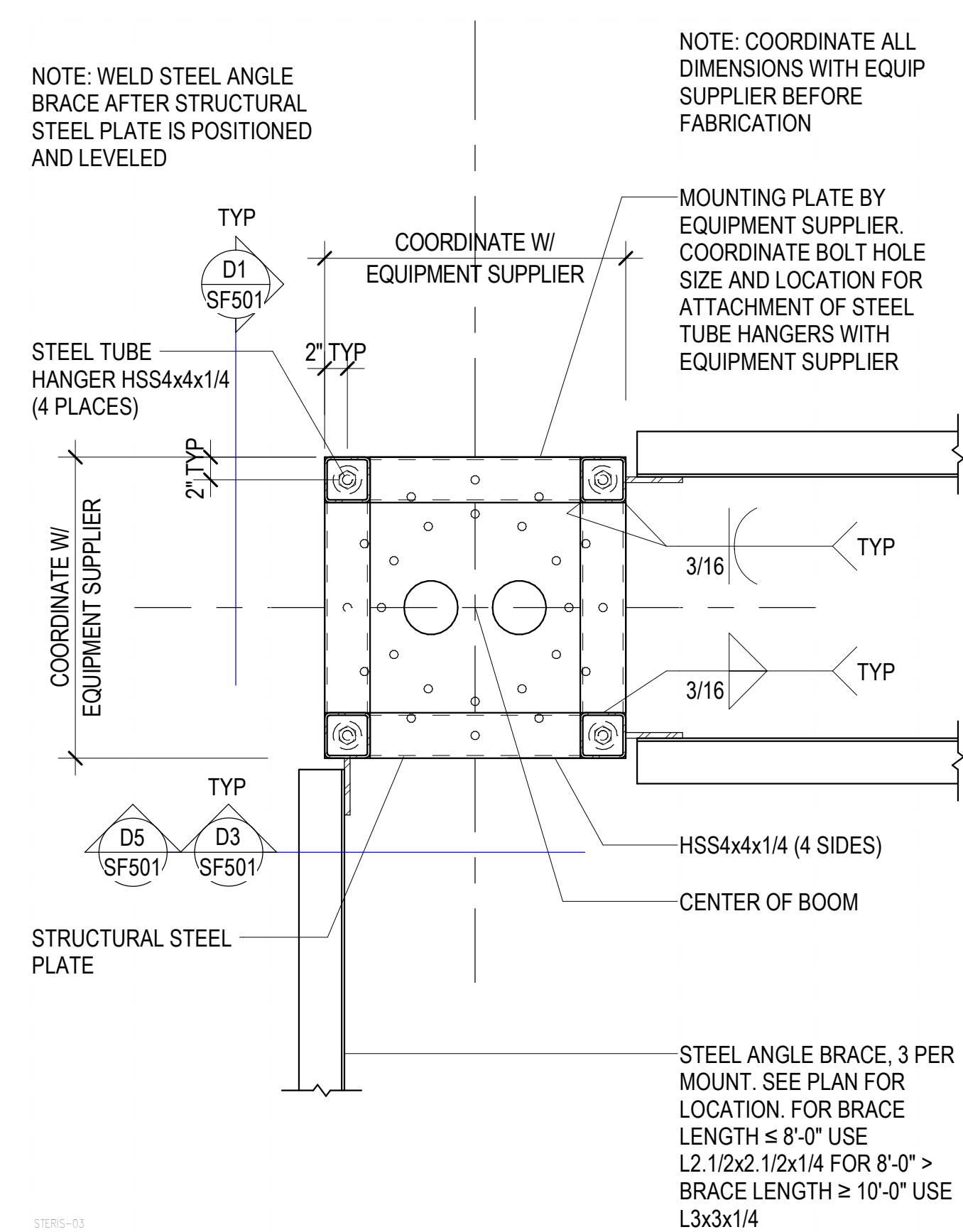
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SF501 SECTION AT STERIS TANDEN SUPPORT - CONDITION AT
HANGER BELOW GIRDER OR PURLIN
NO SCALE



A1
SF501 TYPICAL STEEL BEAM BOTTOM FLANGE BRACE DETAIL
NO SCALE

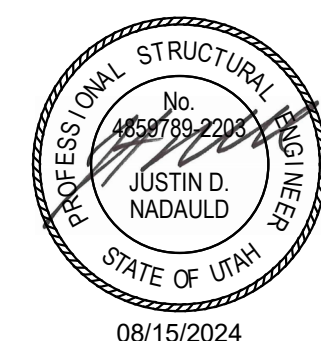
ANGLE BRACE SCHEDULE		
BRACE LENGTH "L"	ANGLE BRACE SIZE	WELD PLATE SIZE
UP TO 4'-0"	L2x2x1/4	4"x1/4"x4"
4'-0" TO 8'-0"	L3x3x1/4	4"x1/4"x4"
8'-0" TO 12'-0"	2-L2.1/2x2.1/2x1/4	4"x3/8"x4"
12'-0" TO 16'-0"	2-L3x3x1/4	5"x3/8"x5"

NOTE:
1. WHERE DOUBLE ANGLES ARE USED
PROVIDE 3"x3/8" SPACER PLATES AT
THIRD POINTS.



B5
SF501 STERIS TANDEN SUPPORT PLATE DETAIL - PLAN VIEW
NO SCALE

LAYTON HOSPITAL MISC PROJECTS



KEY PLAN

REVISION
NO. DESCRIPTION DATE

HKS PROJECT NUMBER

26404.000

DATE

08/15/2024

ISSUE

CONSTRUCTION
DOCUMENTS

SHEET TITLE

MEDICAL
EQUIPMENT
SUPPORT DETAILS

SHEET NO.

SF501

ARCHITECT

HKS ARCHITECTS, INC.
222 SOUTH MAIN, SUITE 230
SALT LAKE CITY, UT 84101

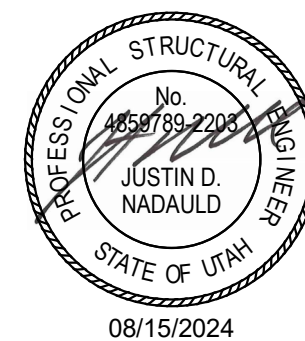
STRUCTURAL ENGINEER

REAVELEY ENGINEERS & ASSOCIATES
675 EAST 500 SOUTH, SUITE 400
SALT LAKE CITY, UTAH 84102

MECHANICAL ENGINEER

VAN BOERUM & FRANK ASSOCIATES, INC.
181 EAST 5600 SOUTH, SUITE 130
MURRAY, UTAH 84107

ELECTRICAL ENGINEER

SPECTRUM ENGINEERS, INC.
324 SOUTH STATE STREET, SUITE 400
SALT LAKE CITY, UTAH 84111LAYTON HOSPITAL
MISC PROJECTS

KEY PLAN

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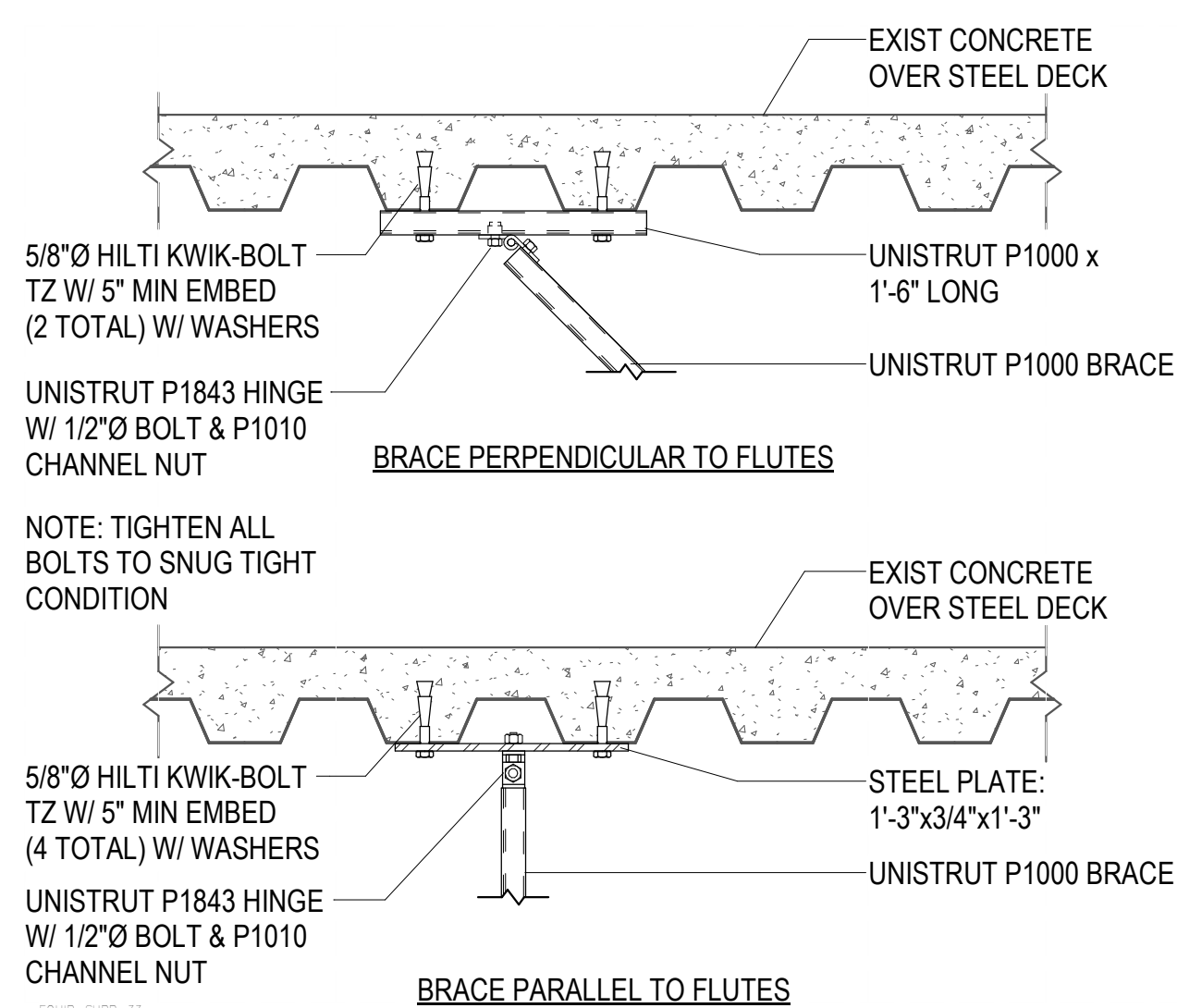
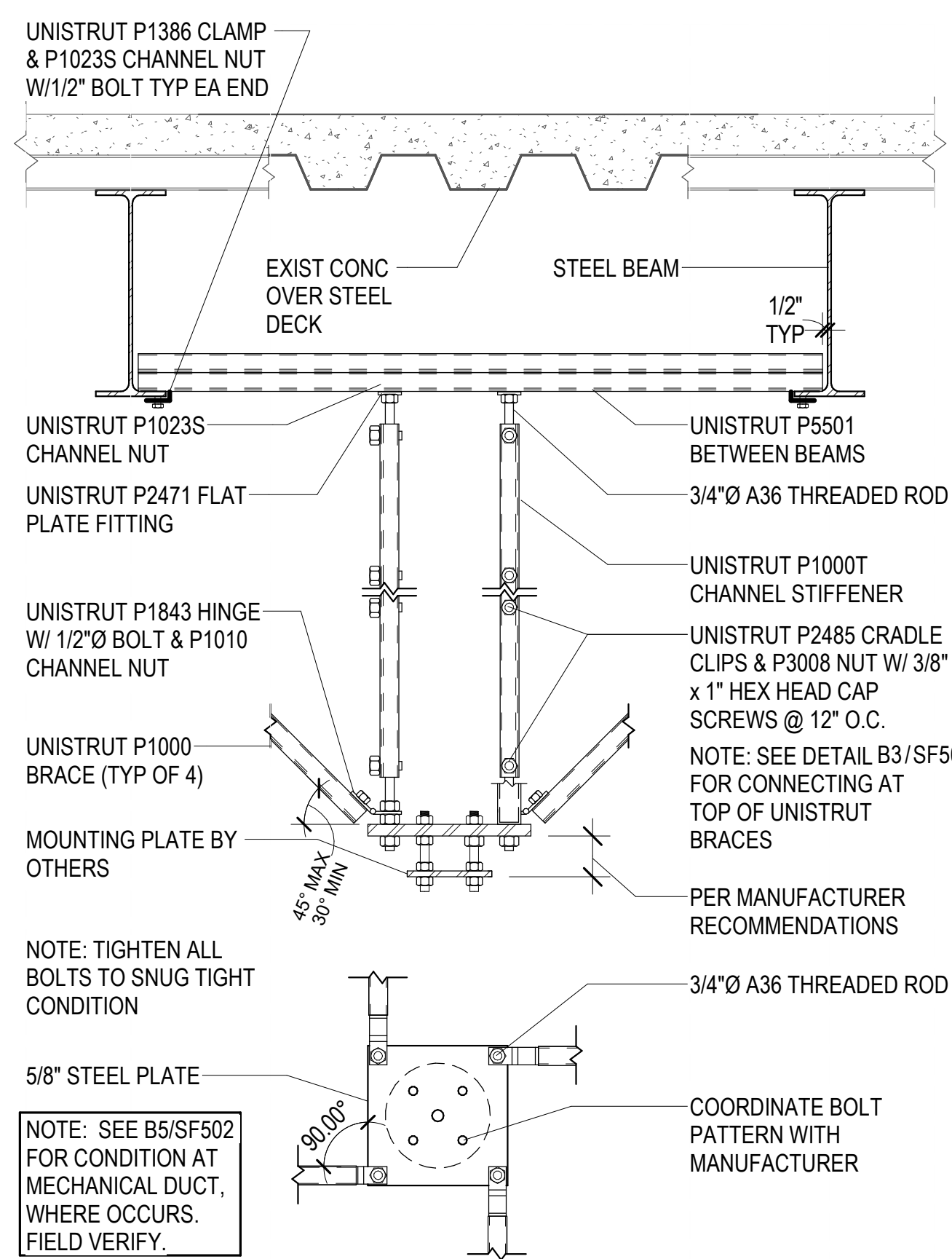
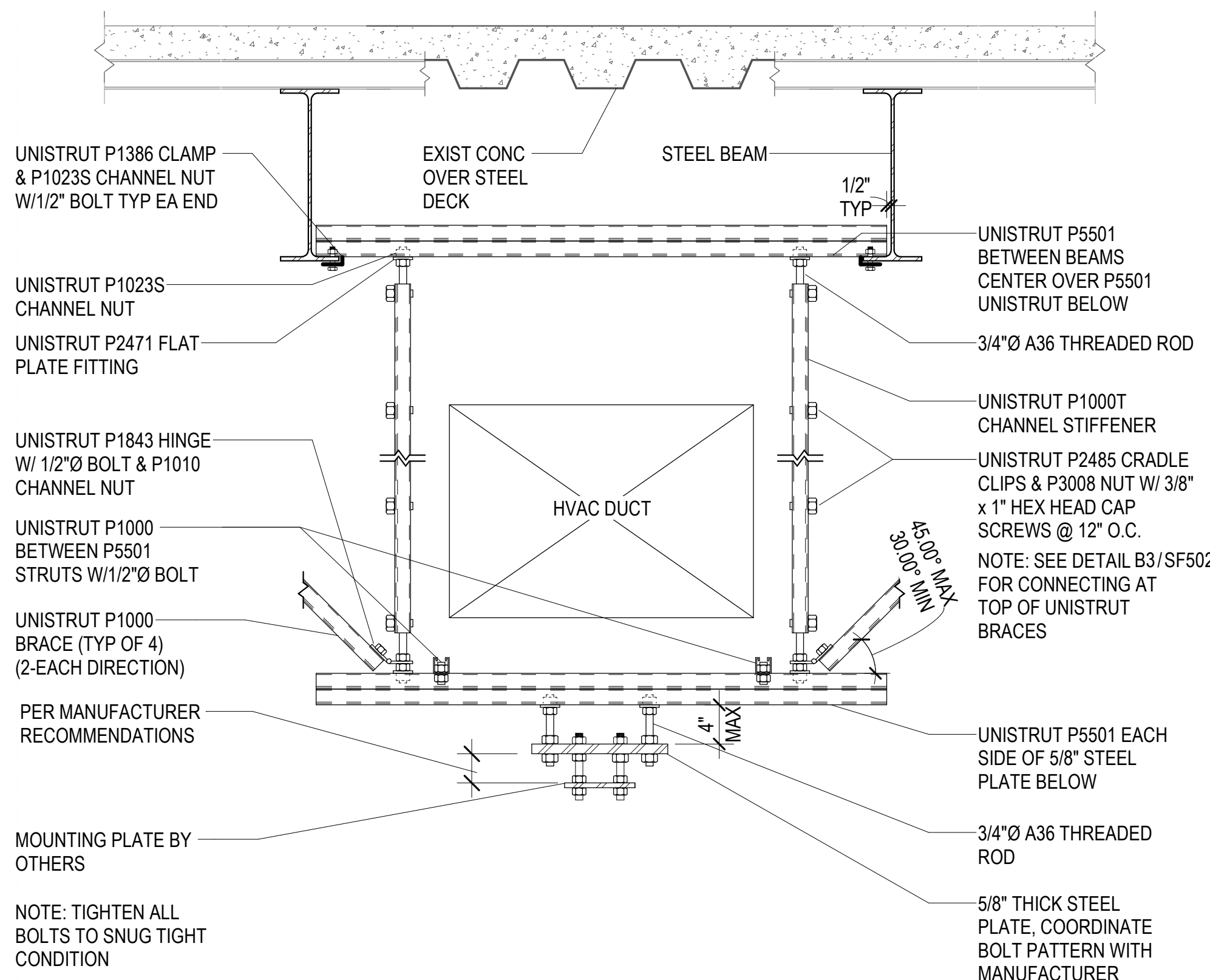
CONSTRUCTION
DOCUMENTS

SHEET TITLE

MEDICAL
EQUIPMENT
SUPPORT DETAILS

SHEET NO.

SF502

B3
SF502
TYPICAL UNISTRUT CONNECTION TO CONC OVER STEEL DECK
NO SCALEB4
SF502
TYPICAL SINGLE LIGHT SUPPORT FROM STRUCTURE
NO SCALEB5
SF502
TYPICAL SINGLE LIGHT SUPPORT FROM STRUCTURE AT MECHANICAL DUCT
NO SCALE