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ADDENDUM NUMBER THREE

DATE OF ISSUANCE: March 22, 2021 PROJECT NUMBER: 20029

PROJECT: SLCC Testing Center ARCHITECT: JRCA Architects, Inc. 577 South 200 East

Remodel Salt Lake City, Utah 84111

OWNER: SLCC

4600 Redwood Rd, Salt Lake City, UT 84123

Incorporate the following revisions to the Specifications, Drawings and other Contract Documents of the above named project. General Items are not referenced. Revisions to the Specifications are referenced by section, page number, and paragraph number. Revisions to the Drawings are referenced by drawing sheet number. This addendum forms part of the Construction Documents.

The end of this Addendum is indicated by the note "END OF ADDENDUM". Attachments are located at the end of the Addendum and are referenced in the Addendum.

Drawing Items:

Item No. Section or Sheet No. Description

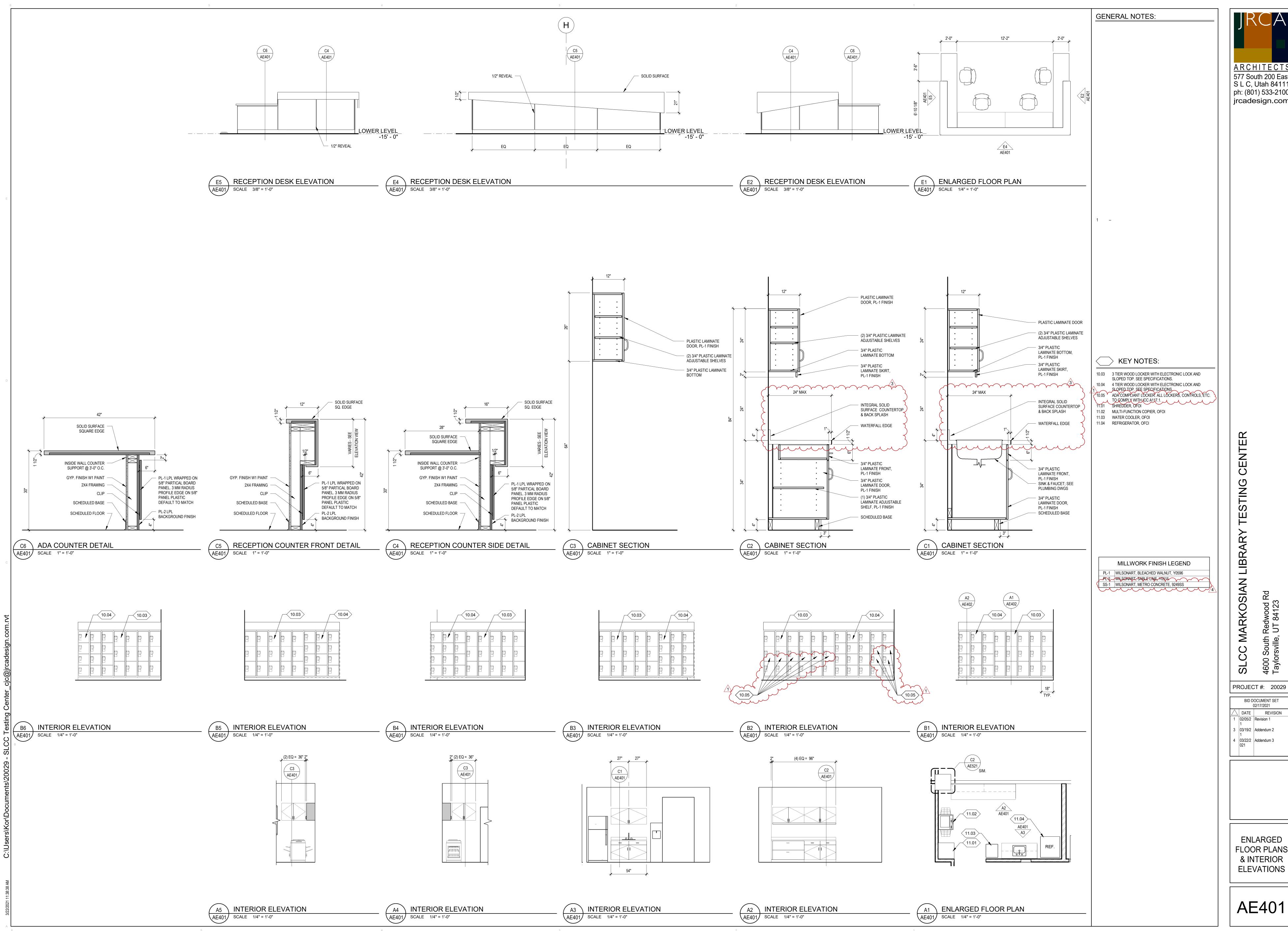
AD3-1 AE401 Revised Millwork Finish Schedule

Structural Items:

Item No. Section or Sheet No. Description

AD3-2 Structural Addendum Two.

END OF ADDENDUM THREE



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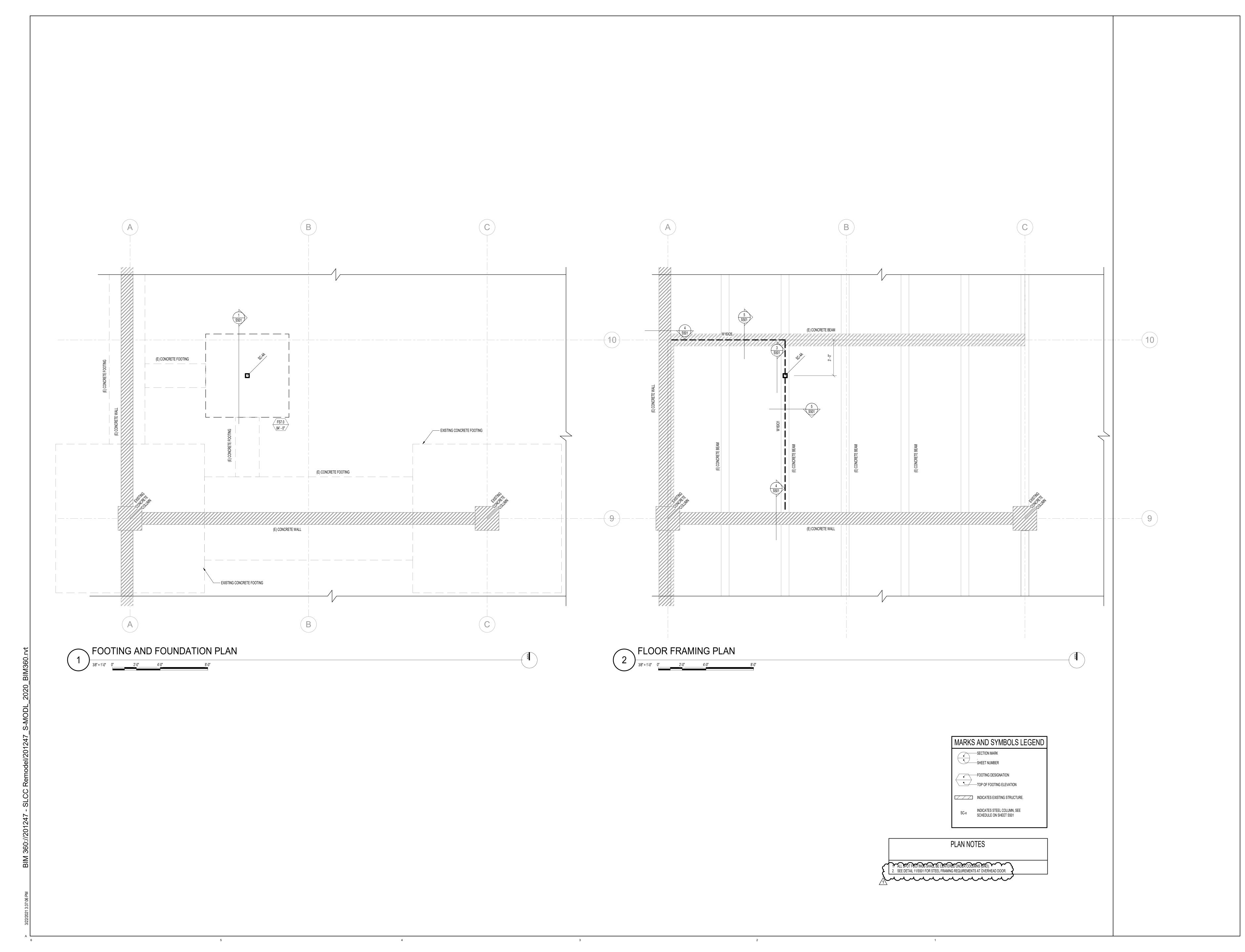
BID DOCUMENT SET 02/17/2021 DATE REVISION

02/05/2 Revision 1 3 03/19/2 Addendum 2

4 03/22/2 Addendum 3

ENLARGED FLOOR PLANS & INTERIOR **ELEVATIONS**

AE401



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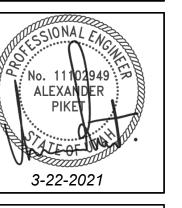
SLCC I ESTING CENTER
4600 South Redwood Rd
Taylorsville 117 84123

PROJECT #: 201247

BID SET 3/22/2021

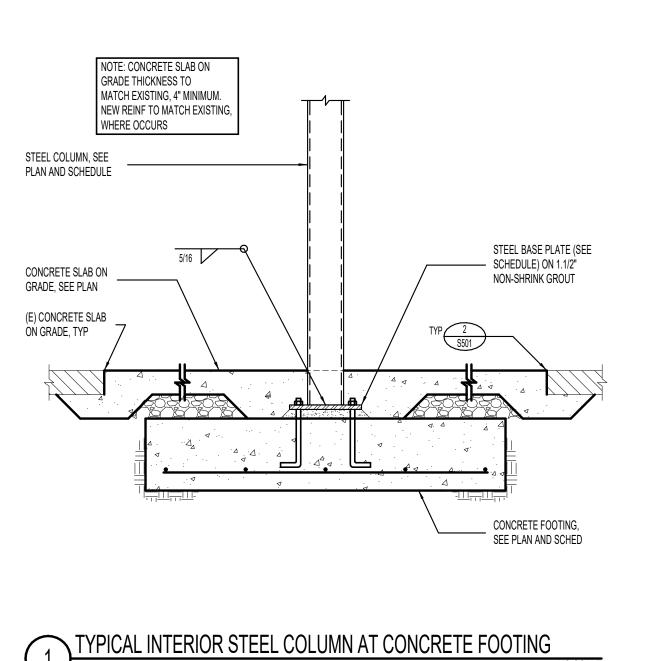
ADATE REVISION

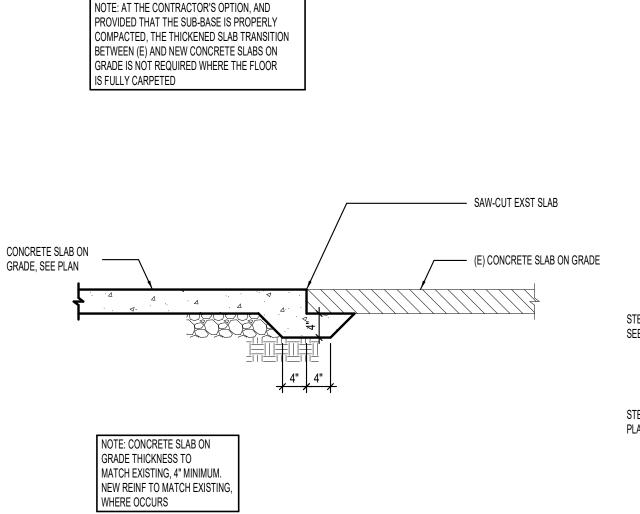
1 3/22/20 ADDENDUM 3
21

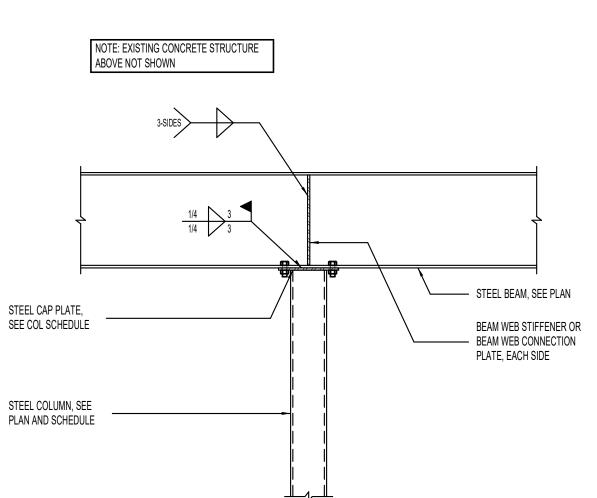


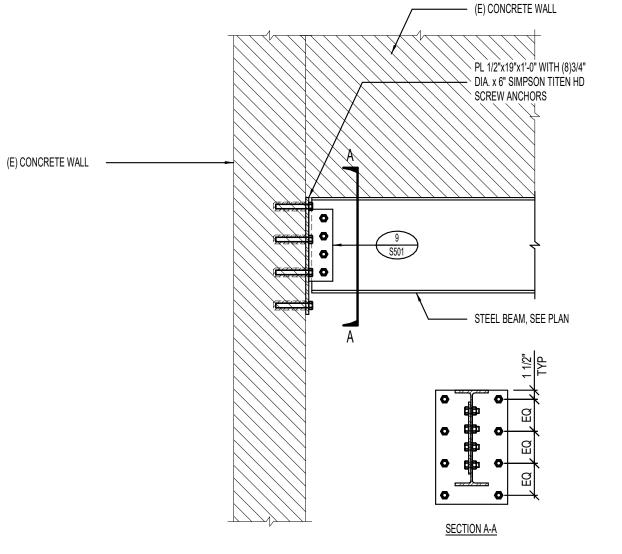
FLOOR
FRAMING AND
FOOTING AND
FOUNDATION
PLANS

S101









1/2" DIA.x 2.3/4" SIMPSON

'CS STRONG-BOLT 2', TYP DO NOT DRILL THROUGH

HSS3x3x1/4xCONT., TYP

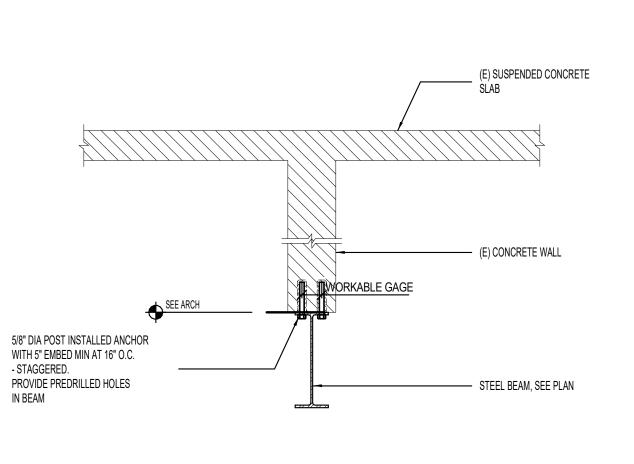
COORD. WITH ARCH

HSS3x3x1/4 TUBE

FRAME AS REQ'D

AT BASE (BEYOND), PROVIDE L3x3x1/4 ON (2) SIDES OF POST, WITH SAME SIMPSON ANCHOR SPECIFIED ABOVE. COORDINATE LOCATION OF ANGLES WITH

(E) REBAR



TYPICAL SLAB PATCH DETAIL

BEAM TO COLUMN CONNECTION

BEAM CONNECTION AT EXISTING WALL

STEEL FRAMING AT OVERHEAD DOOR

(E) FLOOR STRUCTURE ———

L6x4x5/16x0'-4" (LLV) EACH SIDE [(2) TOTAL] OF POST

WITH (1) 1/2" DIA. THRU

BOLT AND 2" VERT SLOT IN

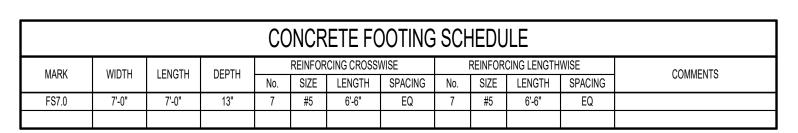
ANGLE, DO NOT OVERSIZE

HOLE, TYP.

HSS3x3x14 POST

EA. SIDE OF DOOR

EXISTING CONCRETE STRUCTURE AT STEEL BEAM



- **CONCRETE FOOTING NOTES:** PLACE ALL FOOTING REINFORCING IN THE BOTTOM OF THE FOOTING WITH 3" CLEAR CONCRETE COVER (UNO). TOP REINFORCING, WHERE OCCURS, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" MINIMUM CONCRETE COVER.
- 3. IF FOOTINGS ARE EARTH-FORMED, FOOTINGS SHALL BE 6" LONGER AND WIDER THAN SCHEDULED. 4. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
- 5. SOME SCHEDULED FOOTINGS MAY NOT BE USED, SEE FOOTING AND FOUNDATION PLAN FOR FOOTING MARKS.

	STANDARD	ADHESIVE EMBEDMENT SCHEDULE	
(TI	REBAR DOWEL HREADED ROD SIZE)	MINIMUM EMBEDMENT INTO CONCRETE OR GROUTED MASONRY	
	#3 (3/8")	3.3/8"	
	#4 (1/2")	4.1/2"	
	#5 (5/8")	5.5/8"	
	#6 (3/4")	6.3/4"	
<u>STAN</u> 1.		EMBEDMENT NOTES: MENTS, NOTES AND DETAILS IN DRAWINGS SHALL GOVERN OVER THIS	
)		SHALL BE DOWEL/ROD DIAMETER PLUS 1/8". FOLLOW	

N OF GENERAL STRUCTURAL DOWEL/THREADED SEE DETAILS MANUFACTURER'S INSTRUCTIONS FOR HOLE PREPARATION. PROVIDE A 3" MINIMUM EDGE DISTANCE TO CENTER OF HOLE. 4. CONTACT STRUCTURAL ENGINEER IF MINIMUM EMBEDMENTS INDICATED ABOVE

BEAM WEB STIFFENER ———

NOT ACHIEVABLE. SEE "POST INSTALLED ANCHORS" SECTION OF GENERAL STRUCTURAL NOTES F STANDARD ADHESIVE EMBEDMENT SCHEDULE

A-325 BOLT SCHEDULE

EACH BEAM DEPTH GROUP No. PER BEAM SIZE ASD CAPACITY

4 3/4"DIA 42.4K

		USE "STANDARD ADHESIVE" PER "POST INSTALLED ANCHORS" SECTION OF GENERAL STRUCTURA NOTES
OVER THIS	MINIMUM EMBED	REBAR DOWEL/THREADED ROD, SEE DETAILS
VE ARE		
S FOR		

6 CONCRETE FOOTING SCHEDULE

	f'c = 3000psi & f'c = 3500 psi				f'c = 4000psi & f'c = 4500 psi				f'c = 5000psi			f'c = 6000psi					
	REGULAR TOP		REGULAR TOP			REGULAR TOP		REGULAR		TOP							
BAR SIZE	CLA	CLASS		ASS	CLA	CLASS		CLASS		CLASS		CLASS		CLASS		CLASS	
	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	
#3	17"	22"	22"	28"	15"	19"	19"	24"	13"	17"	17"	22"	12"	16"	15"	20	
#4	22"	29"	29"	37"	19"	25"	25"	32"	17"	22"	22"	29"	16"	20"	20"	27	
#5	28"	36"	36"	47"	24"	31"	31"	40"	22"	28"	28"	36"	20"	26"	26"	33	
#6	33"	43"	43"	56"	29"	37"	37"	48"	26"	33"	33"	43"	24"	31"	31"	40	
#7	48"	63"	63"	81"	42"	54"	54"	70"	37"	49"	49"	63"	34"	44"	44"	58	
#8	55"	72"	72"	93"	48"	62"	62"	80"	43"	56"	55"	72"	39"	51"	51"	66	
#9	62"	81"	81"	105"	54"	70"	70"	91"	48"	63"	63"	81"	44"	57"	57"	74	
#10	70"	91"	91"	118"	61"	79"	79"	102"	54"	70"	70"	91"	50"	64"	64"	83	
#11	78"	101"	101"	131"	67"	87"	87"	113"	60"	78"	78"	101"	55"	71"	71"	93	

TABULATED VALUES ARE FOR CASE 1 REINFORCEMENT, WHERE THE REQUIREMENTS OF TABLE BELOW ARE MET. WHERE THESE CONDITIONS ARE NOT MET, MULTIPLY THE LAP LENGTHS (fd) BY 1.5.

		, ,	_
REC	UIREMEN	NT FOR CASE 1 LAP LENGTHS	d₀ = BAR [
BAR CLEAR SPACING	CLEAR COVER	STIRRUPS OR TIES	
>=d _b	>=db	>=CODE FOR MINIMUM THROUGHOUT f d	

>=2db >=db CONCRETE REINFORCING BAR LAP SPLICE NOTES:

- THIS SCHEDULE SHALL BE USED FOR ALL BAR SPLICES IN CONCRETE WALLS, UNLESS NOTED OTHERWISE. CLASS 'A' SPLICES MAY BE USED ONLY IN CASES WHERE 50% OR LESS OF THE BARS ARE SPLICED WITHIN THE LAP SPLICE LENGTH. CLASS 'B' SPLICES SHALL BE USED FOR ALL SPLICES UNLESS THE REQUIREMENTS OF NOTE №. 2 ABOVE ARE MET.
- TIES AND STIRRUPS SHALL NOT BE SPLICED.
- DO NOT SPLICE VERTICAL BARS IN RETAINING WALLS UNLESS SPECIFICALLY SHOWN. 6. THE VALUES TABULATED IN SCHEDULE ARE FOR GRADE 60 REINFORCING BARS. FOR GRADE 75, MULTIPLY LAP LENGTHS BY 1.25 AND FOR
- THE VALUES TABULATED IN SCHEDULE ARE MINIMUM REQUIREMENTS. LONGER LENGTHS MAY BE USED FOR CONSTRUCTIBILITY.
- 8. TOP BARS ARE CLASSIFIED AS HORIZONTAL BARS WHERE 12", OR MORE, OF FRESH CONCRETE IS CAST BELOW THE REINFORCING BAR. SPLICES FOR BUNDLED BARS:
- a. FOR BUNDLED BARS OF THREE OR LESS, LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.2. b. FOR BUNDLED BARS OF FOUR OR MORE, LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.33. c. INDIVIDUAL BAR SPLICES WITHIN A BUNDLE SHALL NOT OVERLAP.
- d. ENTIRE BUNDLES SHALL NOT BE LAP SPLICED.
- 10. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

CONCRETE REINFORCING BAR LAP SPLICE SCHEDULE

SHEAR TAB SHALL BE 1/2" THICK. 5/16" FILLET WELD EACH SIDE OF SHEAR TAB. STEEL BEAM BOLT SPACING SHALL BE 3" MIN, TYP. REDUCE BOLT SPACING TO 2.7/8 WHERE NEEDED TO FIT THE REQUIRED QUANTITY OF BOLTS.) WHEN MORE THAN ONE ROW OF BOLTS IS NEEDED, THE FIRST ROW SHALL BE A COMPLETE ROW WITH THE REMAINDER OF THE BOLTS PLACED IN THE SECOND ROW WITH (3) BEAM TO BEAM SHEAR TAB CONNECTION BOLTS MIN AT SECOND ROW. (5) HSS COLUMN THAT DO NOT HAVE A MINIMUM 1/4" WALL THICKNESS STEEL BEAM ----SHALL USE A SINGLE ANGLE CONNECTION WHERE STEEL TUBE WALL IS TOO THIN. 6) AT MOMENT FRAME COLUMNS, SEE MOMENT CONNECTION DETAILS FOR CONTINUITY PLATE REQUIREMENTS. 7 BOLT EDGE DISTANCE, Leh SHALL BE EQUAL TO TWICE THE BOLT DIAMETER FOR BOTH THE PLATE AND THE BEAM WEB. 8 BOLT EDGE DISTANCE, Lev SHALL BE 1.1/4" FOR BOLT DIAMETERS 7/8" OR LESS AND 1.3/4xBOLT DIAMETER FOR BOLT DIAMETERS GREATER THAN 7/8". BEAM TO BEAM WEB PLATE CONNECTION

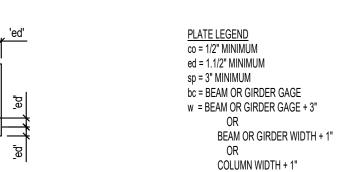
TYPICAL 3/4"DIA BOLTED WEB PLATE CONNECTIONS WITH BOLT SCHEDULE [SINGLE SHEAR STEEL TUBE AND WIDE FLANGE COLUMN]

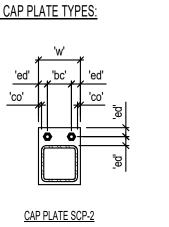
STEEL COLUMN SCHEDULE								
MARK	SIZE	STEEL BASE PLATE	STEEL CAP PLATE	COMMENTS				
SC-4A	HSS4x4x3/8	1" (SBP-1)	1/2" (SCP-2)					

2. ALL CAP PLATE BOLTS SHALL BE 3/4"DIA A325N BOLTS, TYPICAL UNLESS NOTED OTHERWISE.

3. ANCHOR RODS SHALL NOT BE WELDED (INCLUDING TACK WELDS). 4. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

BASE PLATE SBP-1



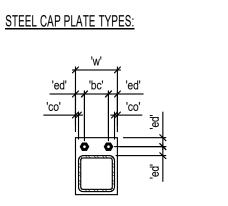


STEEL COLUMN SCHEDULE

WHICHEVER IS GREATER

1. UNLESS NOTED OTHERWISE, ALL COLUMNS SHALL BE INSTALLED WITH (4) 3/4"DIA ANCHOR RODS WITH 3" MINIMUM HOOKS. PROJECT ANCHOR RODS 3" MINIMUM ABOVE THE TOP OF THE BASE PLATE. EMBEDMENT SHALL BE 9" MINIMUM. ALL RODS SHALL BE INSTALLED WITH HARDENED WASHERS BENEATH THE NUT. ANY

BOLT HOLES LARGER THAN THE ROD DIAMETER PLUS 5/16" SHALL HAVE 5/16" PLATE WASHERS INSTALLED BENEATH THE HARDENED WASHERS.



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PROJECT #: 201247

3/22/2021 DATE REVISION 1 | 3/22/20 | ADDENDUM 3



DETAILS AND SCHEDULES

S501