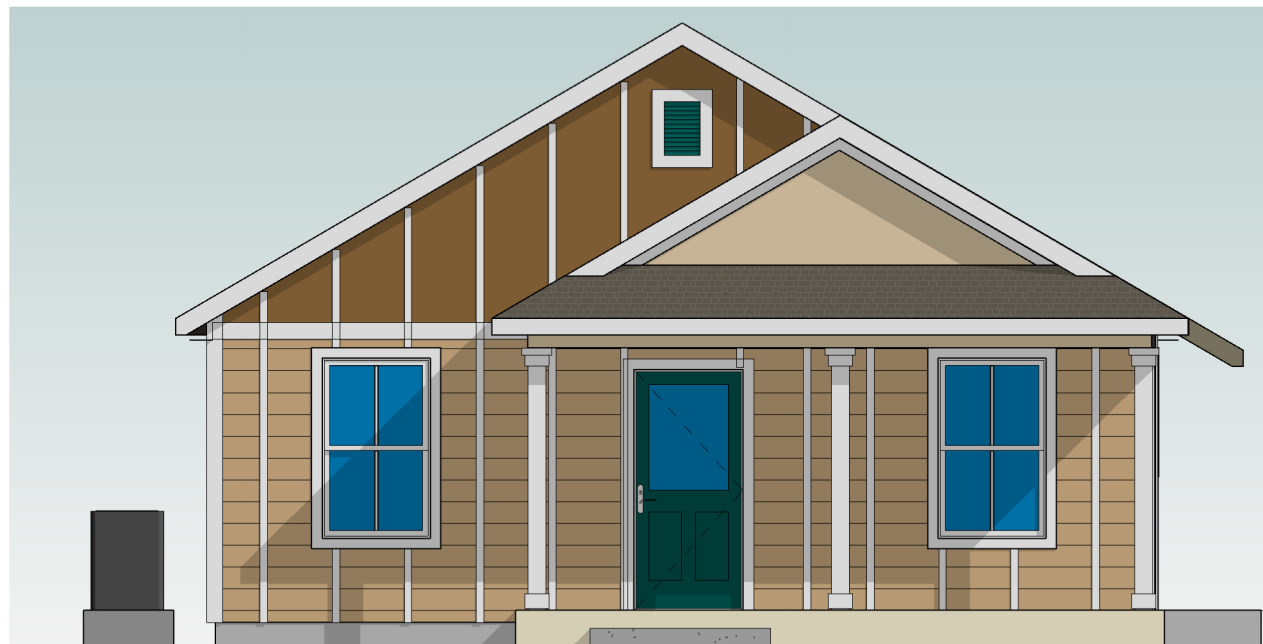


# Adoration Place - 3 Bedroom -Ver. 3



3BR Version 3 Add Carport



3BR Version 3 Base Model

## INDEX OF DRAWINGS

### ARCHITECTURAL

A000	cover sheet
A010	site plan
A100	3bed floor plan v. 3 base model
A110	3 bed floor plan v.3 add carport
A130	electrical & lighting base model & carport
A200	3br ver.3 exterior elevations
A210	3br ver.3 exterior elevations
A220	3br v.3 ext elevations add carport
A230	3BR v.3 ext elevations add carport
A300	3br v. 3 interior elevations
A410	code notes
S101	foundation plan
S201	framing plan
S301	foundation details
S401	framing schedules and legends
S501	shearwall details
S601	structural notes

## CODE INFORMATION

This 3 bedroom slab-on-grade foundation residential unit has been designed in accordance with the requirements of the following codes.

International Residential Code (IRC) 2021

Basic Wind Speed per figure R301.2.1.1: 140 MPH  
Exposure Category: B

(Direct shoreline placement of structure is not assumed. If this placement is to occur, the Architect and Engineer must be notified for re-evaluation of structure.)

Contractor must provide Protection of Openings in accordance with section R301.2.1.2. If wood structural panels are provided for openings, they are to be fastened in accordance with Table R301.2.1.2.

If applicable to home construction, pre-engineered wood trusses shall be in accordance with IRC 2021 Section R802.10.

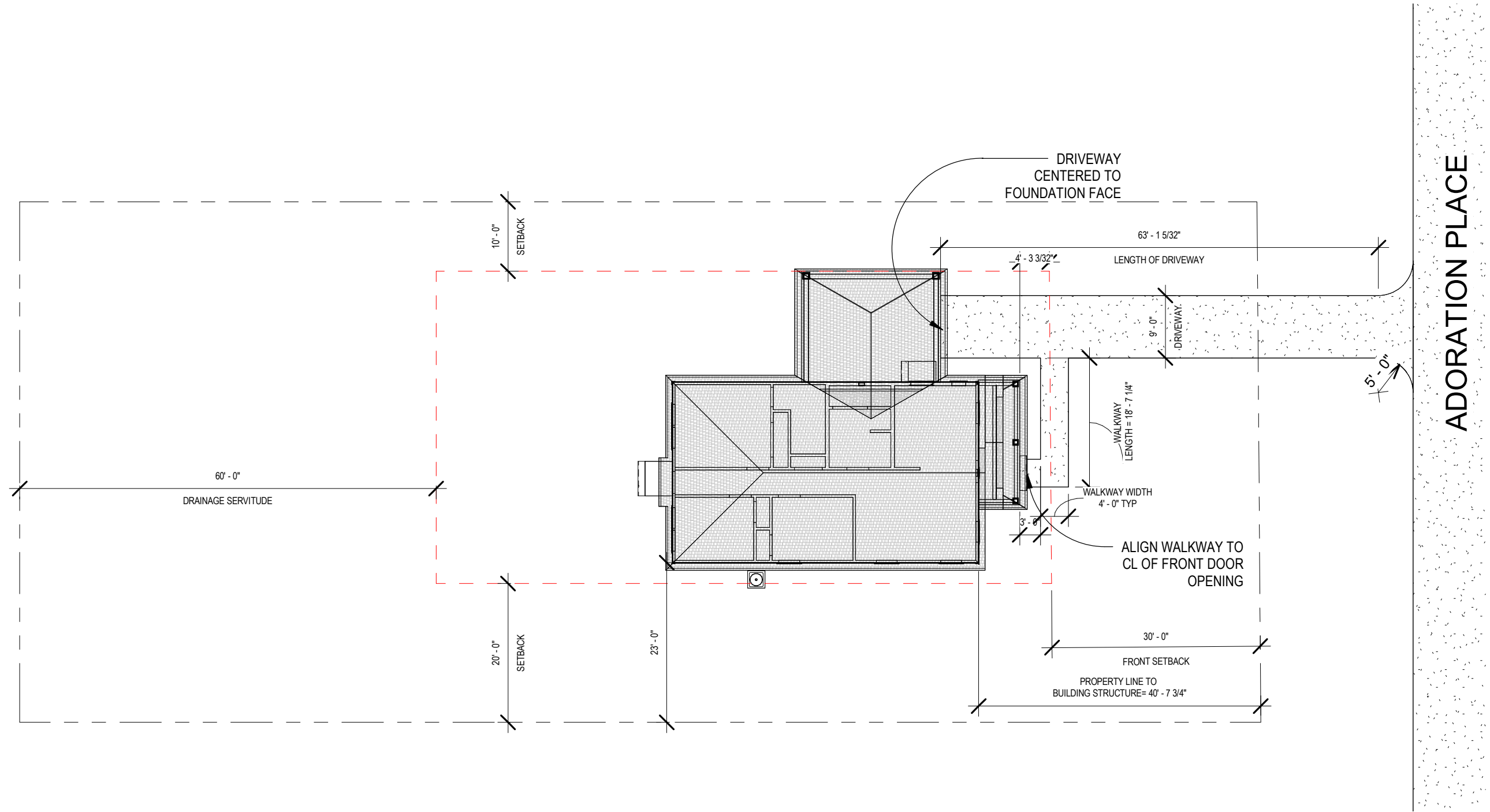
ANSI/ AF&PA WFCM-2018

Tables provided on sheets A410 & A411

## SYMBOL LEGEND

	DRAWING NUMBER DRAWING TITLE SHEET NUMBER	DRAWING SCALE	DRAWING TITLE
	DRAWING NUMBER SHEET NUMBER	EXTERIOR ELEVATION REFERENCE MARK	
	DRAWING NUMBER SHEET NUMBER	INTERIOR ELEVATION REFERENCE MARK	
	DRAWING NUMBER SHEET NUMBER		DETAIL REFERENCE MARK
	DRAWING NUMBER SHEET NUMBER	SECTION REFERENCE MARK	
	DRAWING NUMBER SHEET NUMBER	DETAIL SECTION REFERENCE MARK	
	DOOR NUMBER	DOOR INFORMATION	
	WINDOW MARK	WINDOW INFORMATION	







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job no.  
**2440**

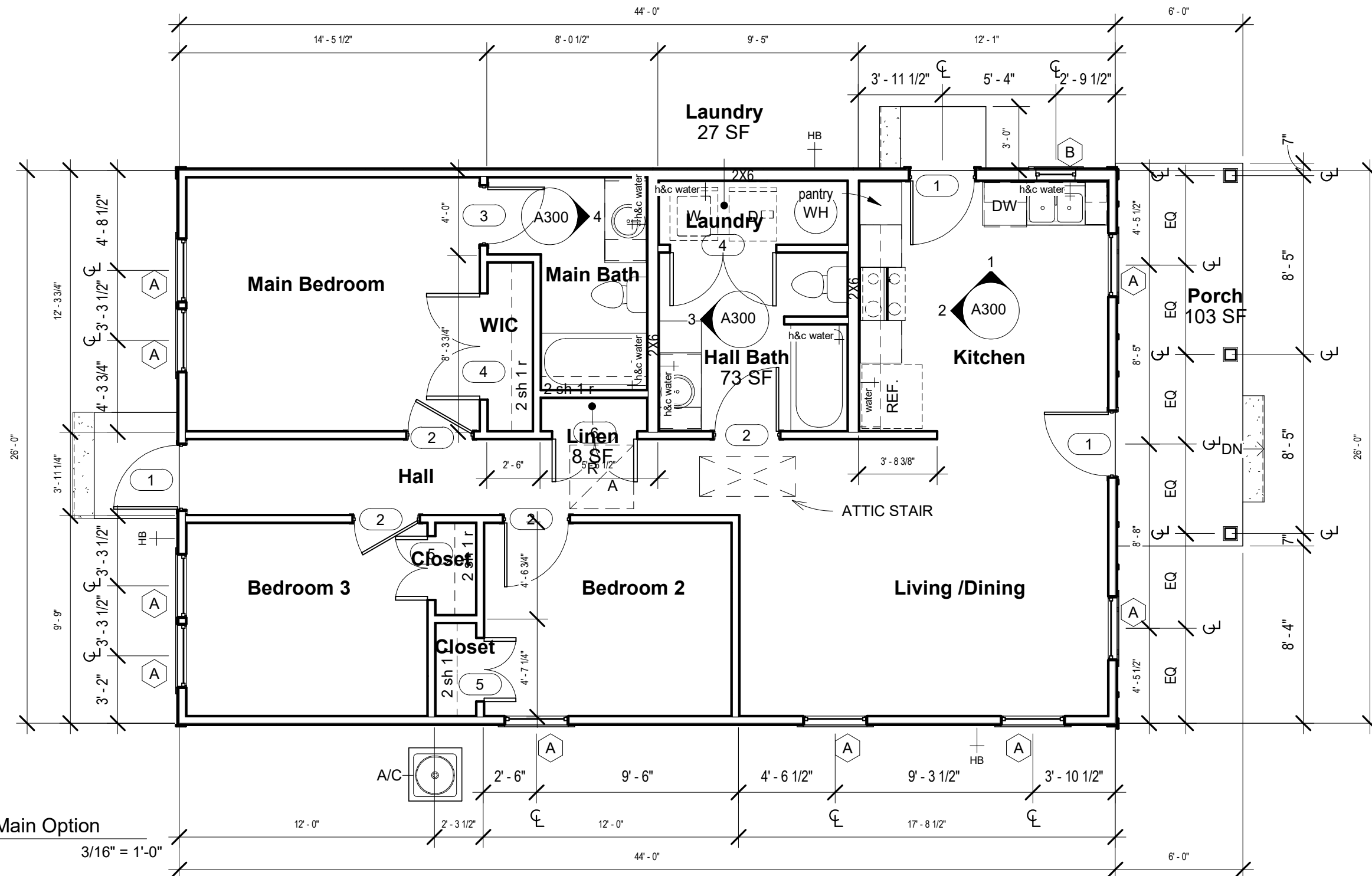
sheet title  
 3bed floor plan  
 v. 3 base model

sheet no.  
**A100**

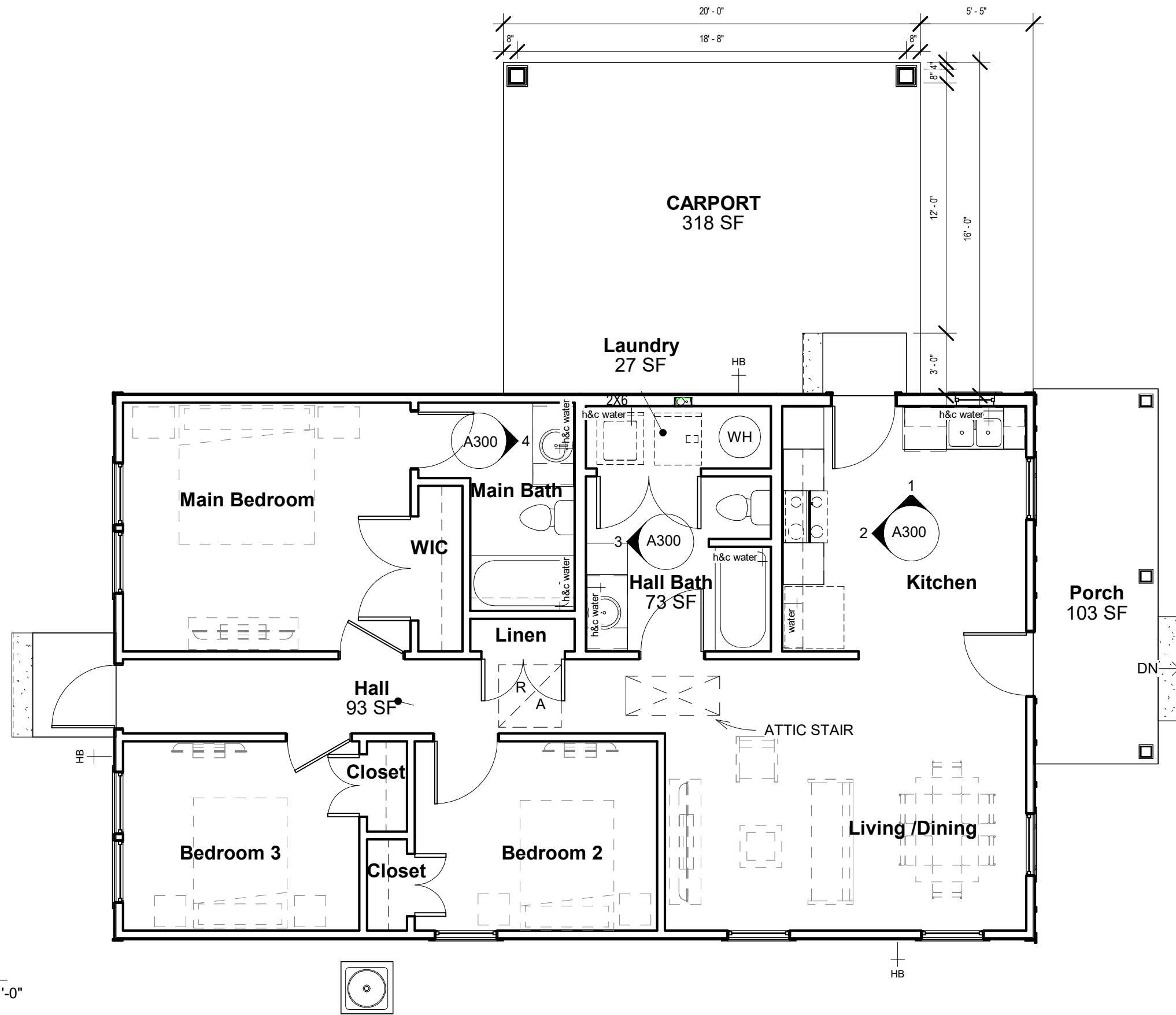
Door Schedule				
Type Mark	Count	Door Width	Door Height	Description
1	3	3' - 0"	6' - 8"	exterior two panel fiberglass door with half lite on wood frame
2	4	3' - 0"	6' - 8"	interior four panel masonite door on wood frame
3	1	2' - 8"	6' - 8"	interior four panel masonite door on wood frame
4	2	5' - 0"	6' - 8"	interior pair of 30" four panel masonite doors on wood frame
5	2	3' - 0"	6' - 8"	interior pair of 18" four panel masonite doors on wood frame
6	1	4' - 0"	6' - 8"	interior pair of 24" four panel masonite doors on wood frame

Window Schedule					
Type Mark	Count	R.O.		Head Height	Description
		Width	Height		
A	9	3' - 0"	5' - 0"	7' - 0"	2 over 2 insulated vinyl single hung; must meet 5.7 egress
B	1	2' - 0"	3' - 0"	7' - 0"	2 over 2 insulated vinyl single hung

Gross Area	
Name	Area
A/C Area	1144 SF
Front Porch	108 SF
Gross Under Beam	1252 SF



**1** First Floor Main Option  
 A100  
 3/16" = 1'-0"



1 First Floor Carport Option  
 A110 3/16" = 1'-0"



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job no.

2440

sheet title  
 3 bed floor plan  
 v.3 add carport

sheet no.

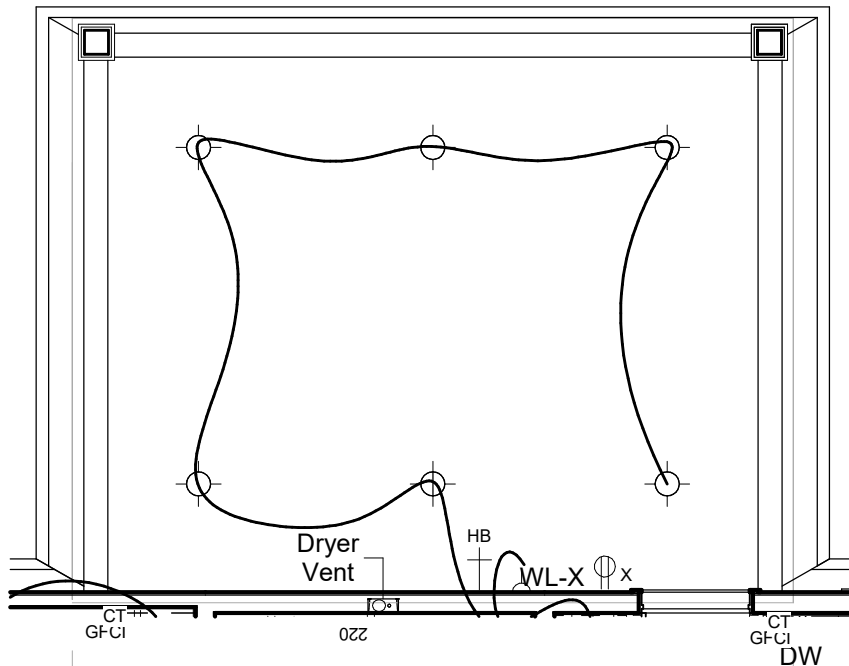
A110

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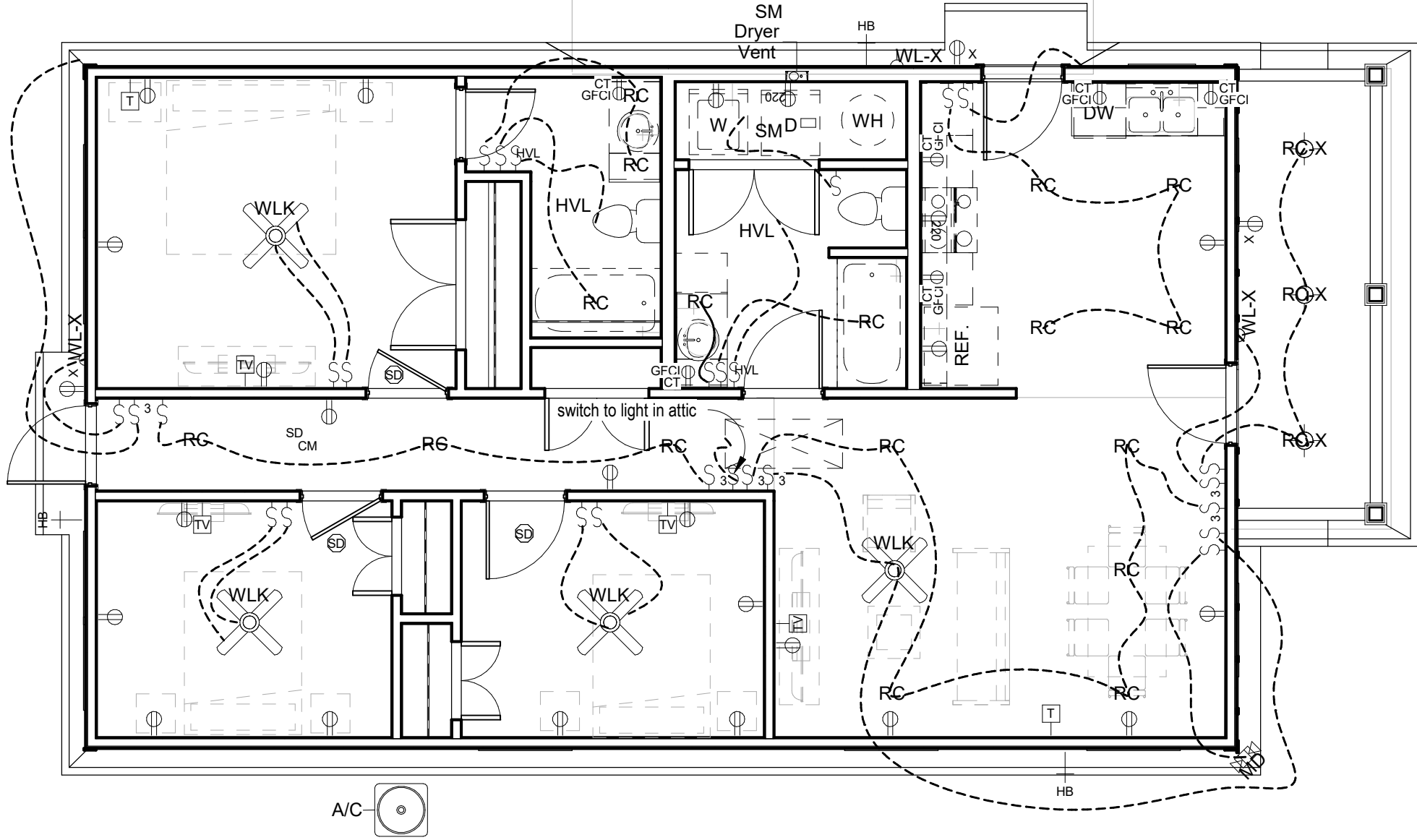
LIGHTING SYMBOLS	
MARK	DESCRIPTION
S	SWITCH
S-3	3 WAY SWITCH
S-HVL	SWITCH/ HEATER/VENT & LIGHT
HVL	HEATER/VENT W/ LIGHT
RC	RECESSED CAN LED LIGHT FIXTURE
RC-X	EXTERIOR GRADE RECESSED CAN LIGHT FIXTURE
WL	WALL LIGHT
PF	PENDANT FIXTURE
SM	SURFACE MOUNTED FIXTURE
JB	JUNCTION BOX
FL	FLOOD LIGHTS WITH MOTION DETECTOR
SCM	SMOKE AND CARBON MONOXIDE COMBO DETECTOR IN HALLWAY. HARDWIRE INTERCONNECTED W/ BATTERY BACKUP
SD	SMOKE DETECTOR. HARDWIRE INTERCONNECTED W/ BATTERY BACKUP
FLUO	2 BULB 4'-0" FLUORESCENT.
WLK	CEILING FAN W/LIGHT KIT

ELECTRICAL SYMBOLS	
MARK	DESCRIPTION
φ	DUPLEX OUTLET. MOUNT AT 12" AFF, TYPICAL U.N.O.
φ-220	220v OUTLET
φ-GFCI	DUPLEX OUTLET, GROUND FAULT CIRCUIT INTERRUPT MOUNT ABOVE COUNTERTOP
φ-R	DUPLEX OUTLET, EXTERIOR GRADE
△	TELEPHONE OUTLET. MOUNT AT STANDARD HEIGHT
△-W	TELEPHONE OUTLET. WALL MOUNTED AT 42"
TV	CABLE TV OUTLET. MOUNT AT STANDARD HEIGHT

COORDINATE EXACT LOCATIONS OF POWER AND LIGHTING W/ OWNER.



2 First Floor Lighting/Power Plan  
Carport Option  
A130 3/16" = 1'-0"



1 First Floor Lighting/Power Plan  
A130 3/16" = 1'-0"



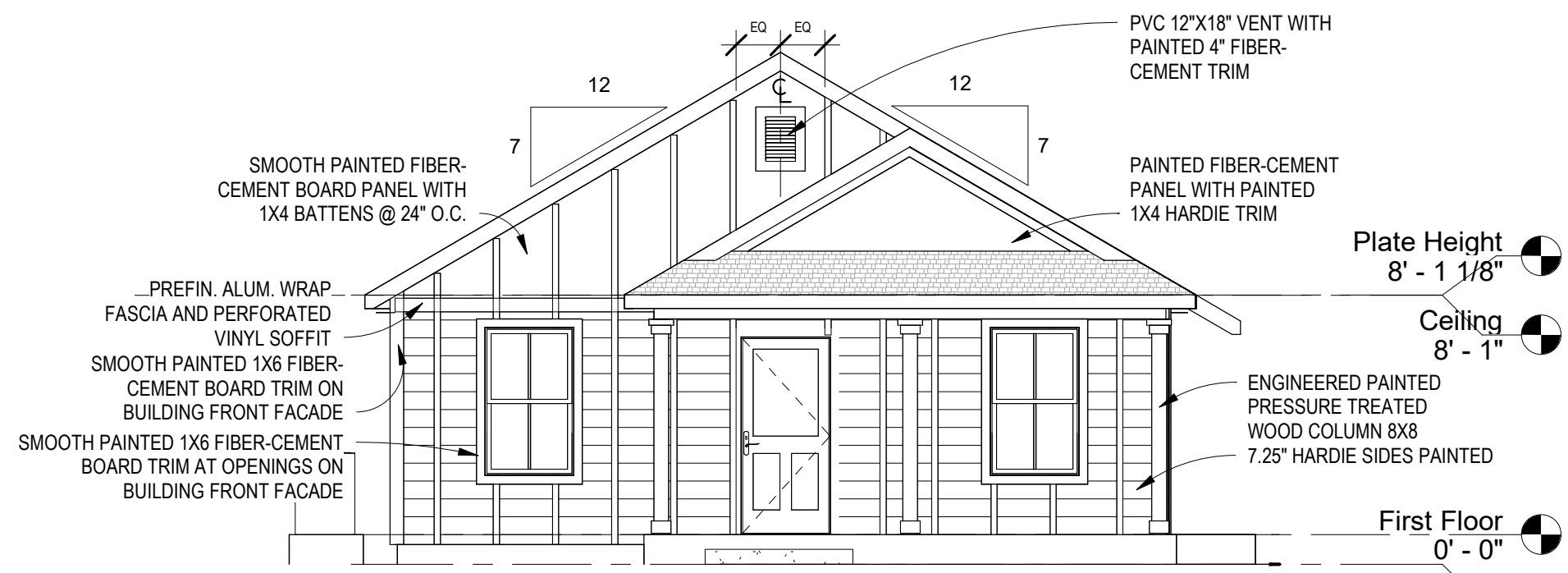
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for construction  
11/12/2025  
revisions

job no.  
**2440**  
sheet title  
electrical &  
lighting base  
model & carport  
sheet no.  
**A130**  
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1 Rear Elevation Version 3  
A200 3/16" = 1'-0"

- Plate Height 8' - 1 1/8"
- Ceiling 8' - 1"
- First Floor 0' - 0"
- Grade -1' - 0"



2 Front Elevation Version 3  
A200 3/16" = 1'-0"

- Plate Height 8' - 1 1/8"
- Ceiling 8' - 1"
- First Floor 0' - 0"
- Grade -1' - 0"



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job no.  
**2440**

sheet title  
 3br ver.3 exterior elevations

sheet no.  
**A200**

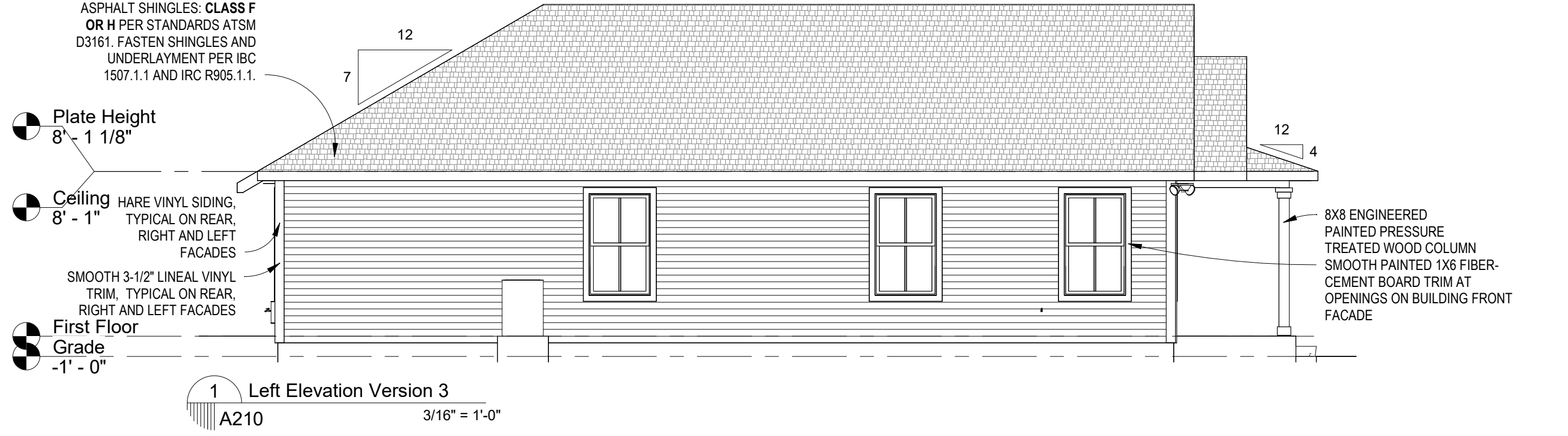
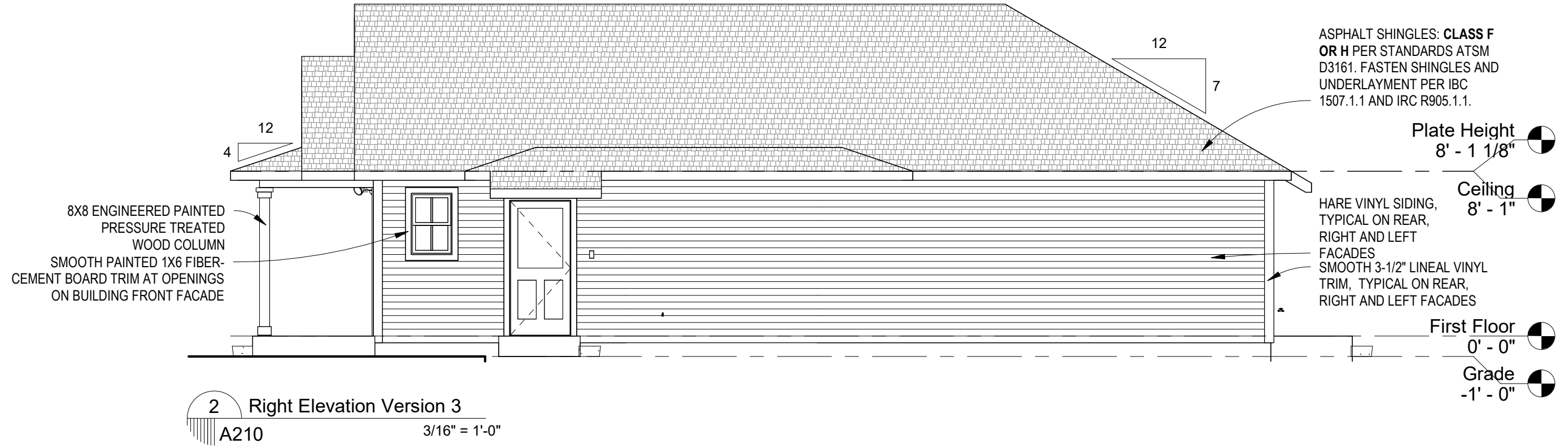


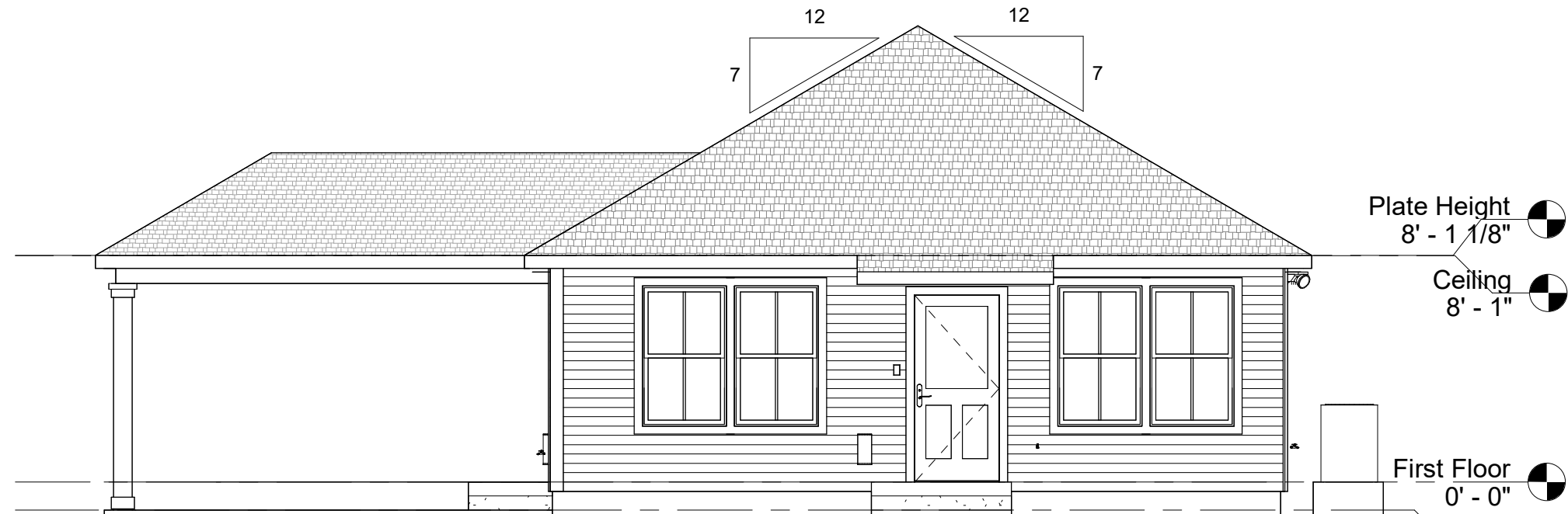
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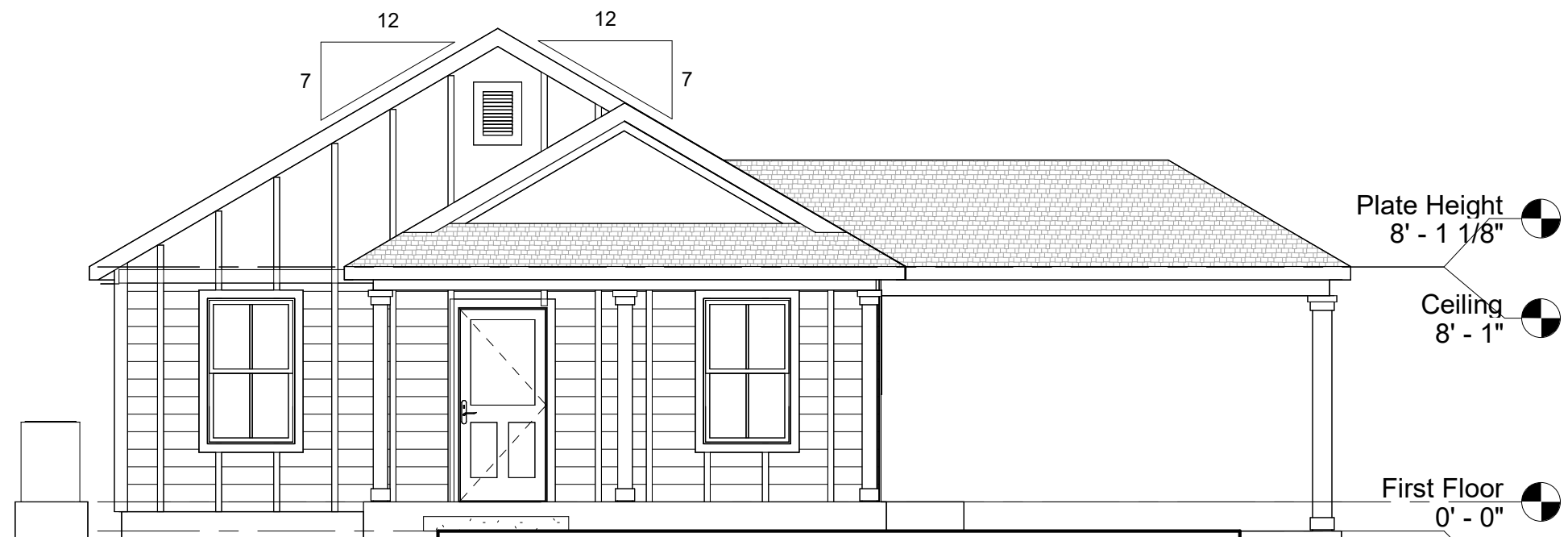
job no.  
**2440**  
 sheet title  
 3br ver.3  
 exterior  
 elevations

sheet no.  
**A210**  
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2 Rear Elevation Version 3 Carport Option  
A220 3/16" = 1'-0"



1 Front Elevation Version 3 Carport Option  
A220 3/16" = 1'-0"



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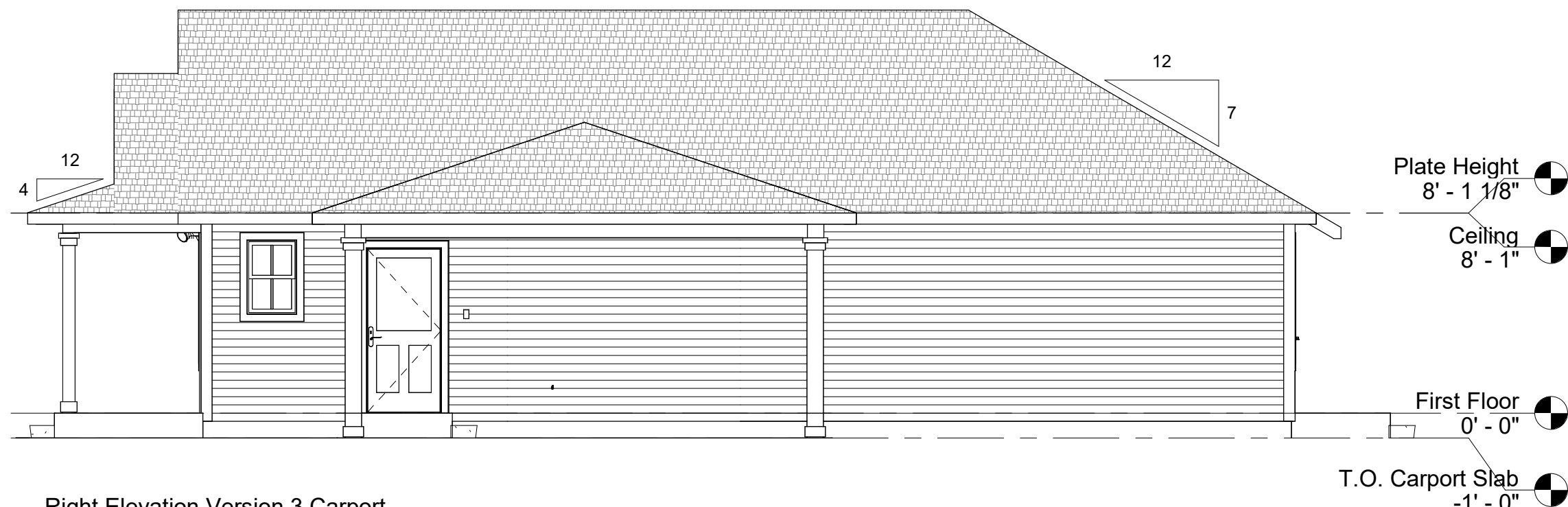
job no.

2440

sheet title  
 3br v.3 ext elevations add carport

sheet no.

A220



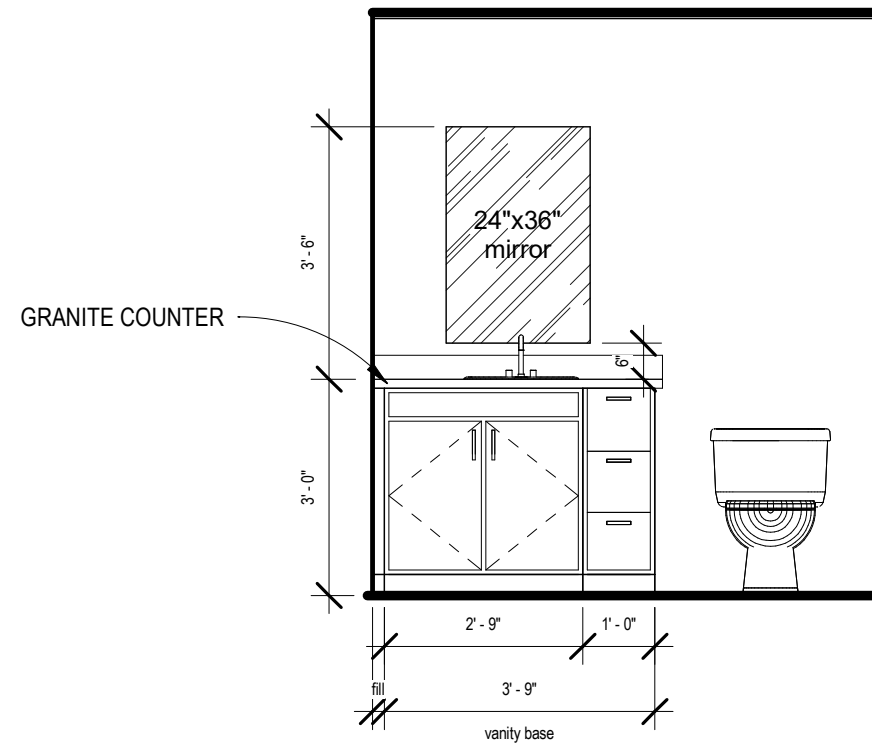
1 Right Elevation Version 3 Carport  
 Option  
 3/16" = 1'-0"



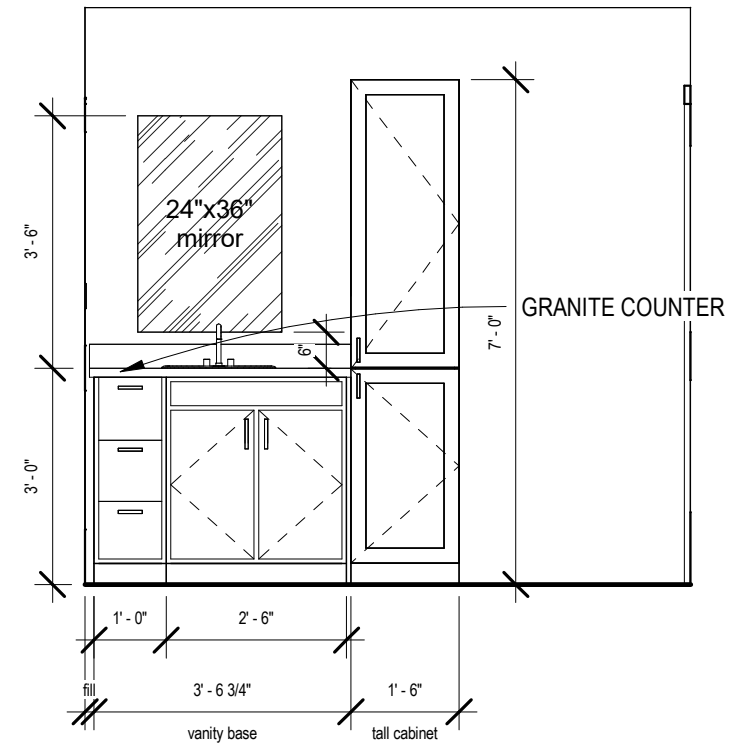
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job no.  
**2440**  
 sheet title  
 3BR v.3 ext  
 elevations add  
 carport

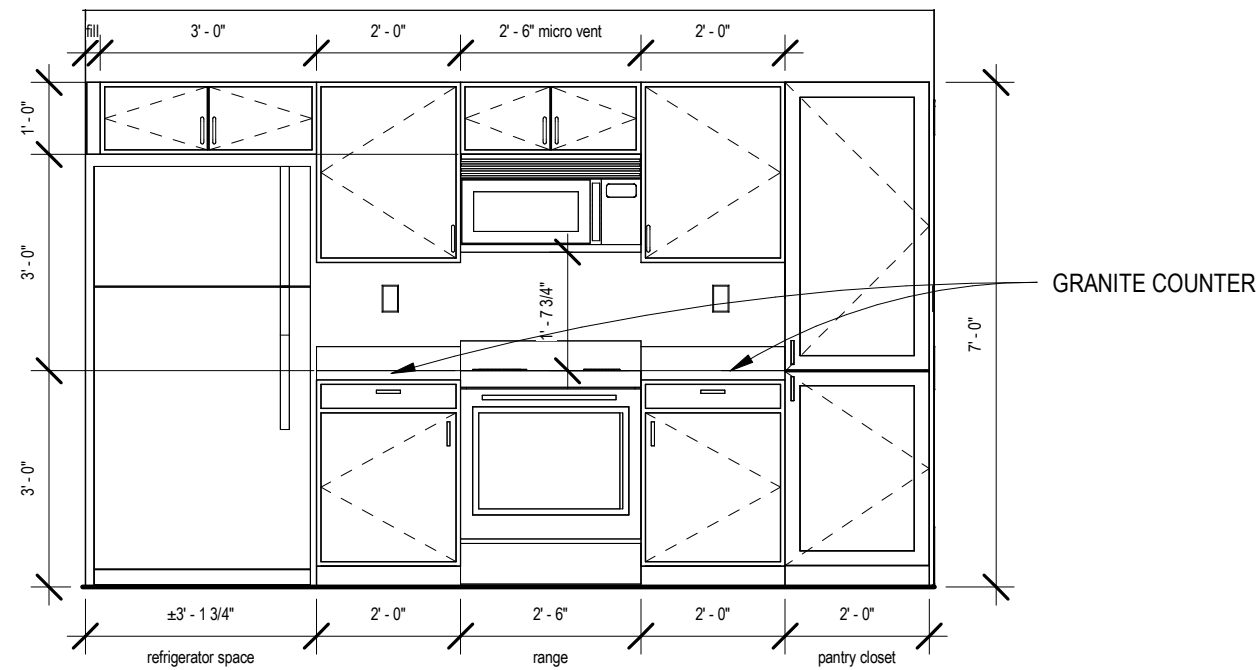
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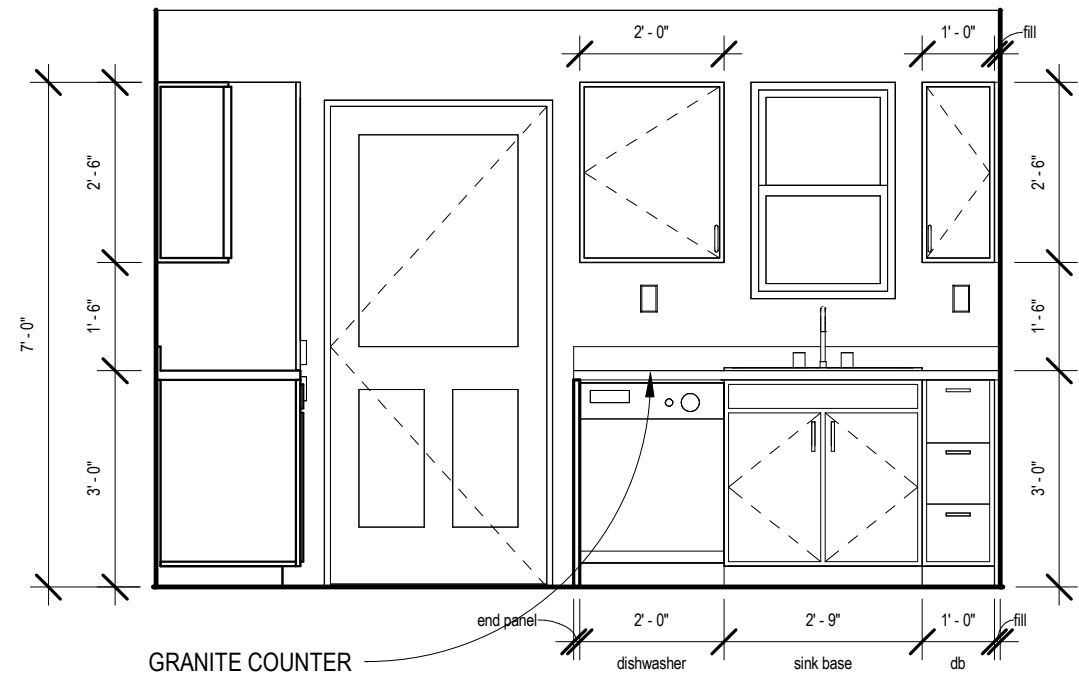
4 Master Bath  
A300 3/8" = 1'-0"



3 Hall Bath  
A300 3/8" = 1'-0"



2 Kitchen Elevation B  
A300 3/8" = 1'-0"



1 Kitchen Elevation A  
A300 3/8" = 1'-0"



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job no.  
**2440**  
sheet title  
3br v. 3 interior  
elevations

sheet no.  
**A300**  
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**International Residential Code 2021:**

**R301.2.1.2 Protection of Openings.** Exterior glazing in buildings located in windborne debris regions shall be protected from windborne debris. Glazed opening protection for windborne debris shall meet the requirements of the Large Missile Test of ASTM E1886 and ASTM E1996 as modified in Section 301.2.1.2.1. Garage door glazed opening protection for windborne debris shall meet the requirements of an approved impact-resisting standard or ANSI/DASMA 115.

**Exception:** Wood structural panels with a thickness of not less than 7/16 inch (11 mm) and a span of not more than 8 feet (2438 mm) shall be permitted for opening protection. Panels shall be precut and attached to the framing surrounding the opening containing the product with the glazed opening. Panels shall be predrilled as required for the anchorage method and shall be secured with the attachment hardware provided. Attachments shall be designed to resist the component and cladding loads determined in accordance with either Table R301.2.1(1) or ASCE 7, with the permanent corrosion-resistant attachment hardware provided and anchors permanently installed on the building. Attachment in accordance with Table R301.2.1.2 is permitted for buildings with a mean roof height of 45 feet (13 728 mm) or less where the ultimate design wind speed, Vult, is 180 mph (290 kph) or less.

**R302.11 Fireblocking.** In combustible construction, fireblocking shall be provided to cut off both vertical and horizontal concealed draft openings and to form an effective fire barrier between stories, and between a top story and the roof space.

Fireblocking shall be provided in wood-framed construction in the following locations:

1. In concealed spaces of stud walls and partitions, including furred spaces and parallel rows of studs or staggered studs, as follows:

1.1. Vertically at the ceiling and floor levels.

1.2. Horizontally at intervals not exceeding 10 feet (3048 mm).

2. At interconnections between concealed vertical and horizontal spaces such as occur at soffits, drop ceilings and cove ceilings.

3. In concealed spaces between stair stringers at the top and bottom of the run. Enclosed spaces under stairs shall comply with Section R302.7.

4. At openings around vents, pipes, ducts, cables and wires at ceiling and floor level, with an approved material to resist the free passage of flame and products of combustion. The material filling this annular space shall not be required to meet the ASTM E136 requirements.

5. For the fireblocking of chimneys and fireplaces, see Section R1003.19.

6. Fireblocking of cornices of a two-family dwelling is required at the line of dwelling unit separation.

**R310.1 Emergency Escape and Rescue Required.** Basements, habitable attics and every sleeping room shall have not less than one operable emergency escape and rescue opening. Where basements contain one or more sleeping rooms, an emergency escape and rescue opening shall be required in each sleeping room. Emergency escape and rescue openings shall open directly into a public way, or to a yard or court having a minimum width of 36 inches (914 mm) that opens to a public way.

3.A yard shall not be required to open directly into a public way where the yard opens to an unobstructed path from the yard to the public way. Such path shall have a width of not less than 36 inches (914 mm).

**R310.1.1 Operational constraints and opening control devices.** Emergency escape and rescue openings shall be operational from the inside of the room without the use of keys, tools or special knowledge. Window opening control devices and fall prevention devices complying with ASTM F2090 shall be permitted for use on windows serving as a required emergency escape and rescue opening and shall be not more than 70 inches (178 cm) above the finished floor.

**R310.2 Emergency escape and rescue openings.** Emergency escape and rescue openings shall have minimum dimensions in accordance with Sections R310.2.1 through R310.2.4.

**R310.2.1 Minimum size.** Emergency escape and rescue openings shall have a net clear opening of not less than 5.7 square feet (0.530 m2).

**Exception:** The minimum net clear opening for grade-floor emergency escape and rescue openings shall be 5 square feet (0.465 m2).

**R310.2.2 Minimum dimensions.** The minimum net clear opening height dimension shall be 24 inches (610 mm). The minimum net clear opening width dimension shall be 20 inches (508 mm). The net clear opening dimensions shall be the result of normal operation of the opening.

**R310.2.3 Maximum height from floor.** Emergency escape and rescue openings shall have the bottom of the clear opening not greater than 44 inches (1118 mm) above the floor.

**R311.3 Floors and landings at exterior doors.** There shall be a landing or floor on each side of each exterior door. The width of each landing shall be not less than the door served. Landings shall have a dimension of not less than 36 inches (914 mm) measured in the direction of travel. The slope at exterior landings shall not exceed 1/4 unit vertical in 12 units horizontal (2 percent).

**Exception:** Exterior balconies less than 60 square feet (5.6 m2) and only accessed from a door are permitted to have a landing less than 36 inches (914 mm) measured in the direction of travel.

**SECTION R317 Protection of Wood and Wood-based Products against Decay**

**R317.1 Location required.** Protection of wood and wood-based products from decay shall be provided in the following locations by the use of naturally durable wood or wood that is preservative-treated in accordance with AWWA U1.

8. Portions of wood structural members that form the structural supports of buildings, balconies, porches or similar permanent building appurtenances where those members are exposed to the weather without adequate protection from a roof, eave, overhang or other covering that would prevent moisture or water accumulation on the surface or at joints between members.

**Exception:** Sawn lumber used in buildings located in a geographical region where experience has demonstrated that climatic conditions preclude the need to use naturally durable or preservative-treated wood where the structure is exposed to the weather.

**R319.1 Address Identification.** Buildings shall be provided with approved address identification. The address identification shall be legible and placed in a position that is visible from the street or road fronting the property. Address identification characters shall contrast with their background. Address numbers shall be Arabic numbers or alphabetical letters. Numbers shall not be spelled out. Each character shall be not less than 4 inches (102 mm) in height with a stroke width of not less than 0.5 inch (12.7 mm). Where required by the fire code official, address identification shall be provided in additional approved locations to facilitate emergency response. Where access is by means of a private road and the building address cannot be viewed from the public way, a monument, pole or other sign or means shall be used to identify the structure. Address identification shall be maintained.

**R322.2.1 Elevation requirements.**

1. Buildings and structures in flood hazard areas, including flood hazard areas designated as Coastal A Zones, shall have the lowest floors elevated to or above the base flood elevation plus 1 foot (305 mm), or the design flood elevation, whichever is higher.

2. In areas of shallow flooding (AO Zones), buildings and structures shall have the lowest floor (including basement) elevated to a height above the highest adjacent grade of not less than the depth number specified in feet (mm) on the FIRM plus 1 foot (305 mm), or not less than 3 feet (915 mm) if a depth number is not specified.

**R401.2 Foundation Requirements.** Foundation construction shall be capable of accommodating all loads in accordance with Section R301 and of transmitting the resulting loads to the supporting soil. Fill soils that support footings and foundations shall be designed, installed and tested in accordance with accepted engineering practice.

**R403.1.7.3 Foundation elevation.** On graded sites, the top of any exterior foundation shall extend above the elevation of the street gutter at point of discharge or the inlet of an approved drainage device a minimum of 12 inches (305 mm) plus 2 percent. Alternate elevations are permitted subject to the approval of the building official, provided it can be demonstrated that required drainage to the point of discharge and away from the structure is provided at all locations on the site.

**R502.8.1 Sawn lumber.** Notches in solid lumber joists, rafters and beams shall not exceed one-sixth of the depth of the member, shall not be longer than one-third of the depth of the member and shall not be located in the middle one-third of the span. Notches at the ends of the member shall not exceed one-fourth the depth of the member. The tension side of members 4 inches (102 mm) or greater in nominal thickness shall not be notched except at the ends of the members. The diameter of holes bored or cut into members shall not exceed one-third the depth of the member. Holes shall not be closer than 2 inches (51 mm) to the top or bottom of the member, or to any other hole located in the member. Where the member is notched, the hole shall not be closer than 2 inches (51 mm) to the notch.

**R502.8.2 Engineered wood products.** Cuts, notches and holes bored in trusses, structural composite lumber, structural glue-laminated members, cross-laminated timber members or I-joists are prohibited except where permitted by the manufacturer's recommendations or where the effects of such alterations are specifically considered in the design of the member by a registered design professional.

**R506.2.3 Vapor retarder.** A minimum 6-mil (0.006 inch; 0.1524 mm) vapor retarder conforming to ASTM E1745 Class A requirements with joints lapped not less than 6 inches (152 mm) shall be placed between the concrete floor slab and the base course or the prepared subgrade where a base course does not exist.

**R602.6 Drilling and notching-studs.** Drilling and notching of studs shall be in accordance with the following:

**1. Notching.** A stud in an exterior wall or bearing partition shall not be cut or notched to a depth exceeding 25 percent of its depth. Studs in nonbearing partitions shall not be notched to a depth exceeding 40 percent of a single stud depth.

**2. Boring.** The diameter of bored holes in studs shall not exceed 60 percent of the stud depth, the edge of the hole shall not be less than 5/8 inch (16 mm) from the edge of the stud, and the hole shall not be located in the same section as a cut or notch. Where the diameter of a bored hole in a stud located in exterior walls or bearing partitions is over 40 percent, such stud shall be doubled and not more than two successive doubled studs shall be so bored. See Figures R602.6(1) and R602.6(2).

**R602.10 Wall bracing.** Buildings shall be braced in accordance with this section or, when applicable, Section R602.12. Where a building, or portion thereof, does not comply with one or more of the bracing requirements in this section, those portions shall be designed and constructed in accordance with Section R301.1.

**[Note per LSUCCC]:** In Climate Zone 2A, one and two family dwellings shall be continuously sheathed with a minimum 7/16" wood structural panels (Table R602.10.4 CS-WSP), or it's structural equivalent as per an ICC-ESR and approved by the local building official.



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2440

sheet title  
code notes

sheet no.

A410

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**FOUNDATION NOTES:**

**SLAB A:** 4" THICK POST-TENSIONED SLAB OVER A LOW PLASTICITY COMPACTED CLAY FILL (PI BETWEEN 15-25) WITH TENDON REINFORCEMENT IN ACCORDANCE WITH THE PLAN, UNLESS NOTED OTHERWISE. PROVIDE 10 MIL VAPOR BARRIER TAPED AND SEALED UNDER ALL SLAB CONCRETE.

IN ALL AREAS OF EXPOSED CONCRETE, PLACE 6x6x10/10 WWF ON TOP OF AND TIE SECURELY TO ALL P.T. TENDONS.

VERIFY ALL DROPS, OFFSETS, AND LEDGES WITH THE ARCHITECTURAL PLANS.

ALL STRUCTURAL FILL SHALL BE COMPACTED TO 95% STANDARD PROCTOR DENSITY IN A MAXIMUM OF 8" LIFTS.

DIMENSIONS ARE TAKEN FROM EDGE OF SLAB AND CENTER OF GRADE BEAM - REFER TO ARCHITECTURAL FOR FORM SETTING DIMENSIONS.

REFER TO S301 FOR FOUNDATION DETAILS, NOTES, AND SCHEDULES.

TOTAL SLAB AREA = 1,572.0 SF

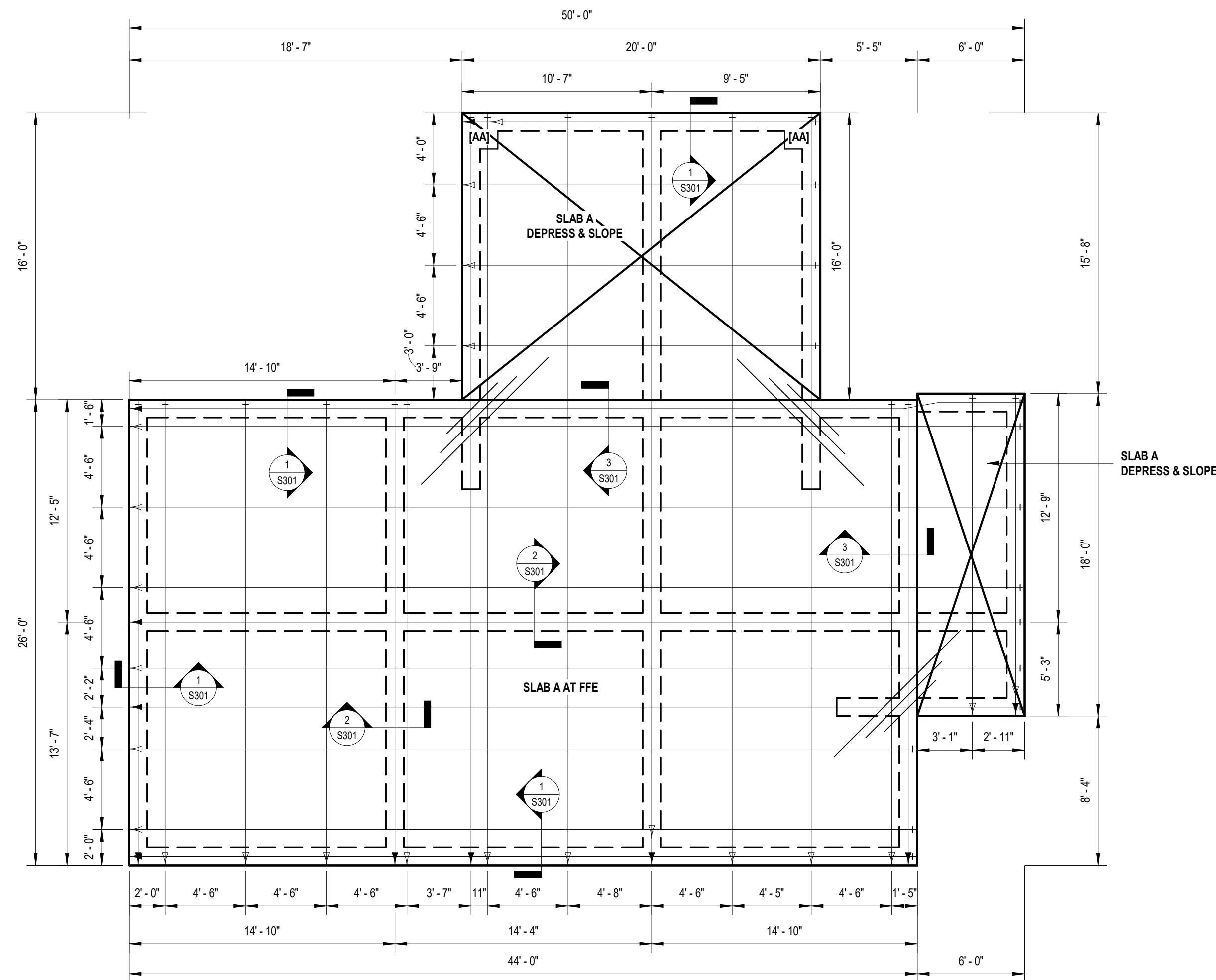
**NOTE (AA):**

PROVIDE 24"x24"x24" DEEP CONCRETE FOOTING AT CAST IN PLACE POST BASE. RE: DETAILS

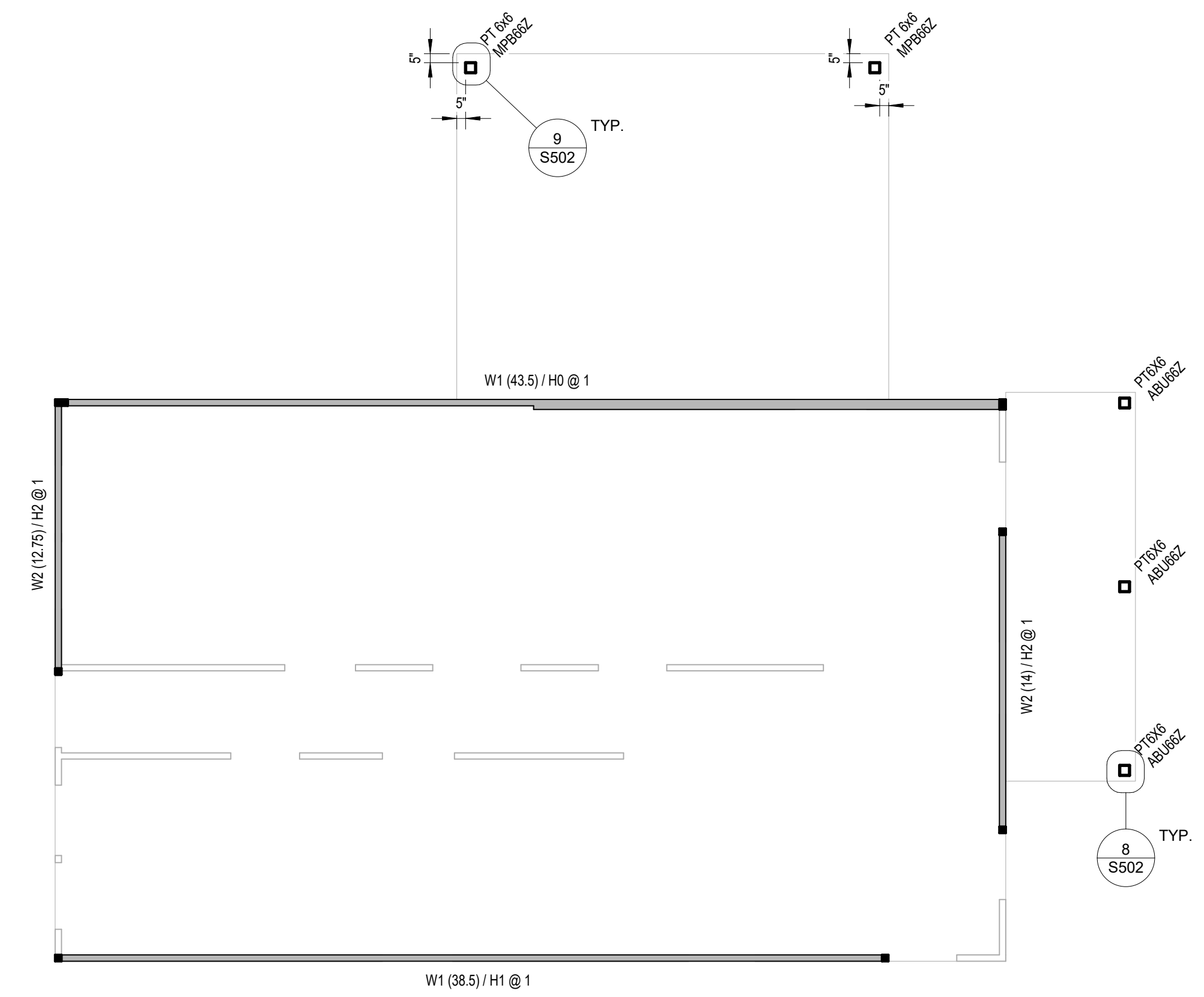
**SHEARWALL NOTES:**

REFER TO SHEET S401 FOR SHEARWALL LEGENDS, NOTES, AND SCHEDULES.

ALL EXTERIOR WALLS SHALL BE SHEATHED AS A W1 WALL MINIMUM, U.N.O.

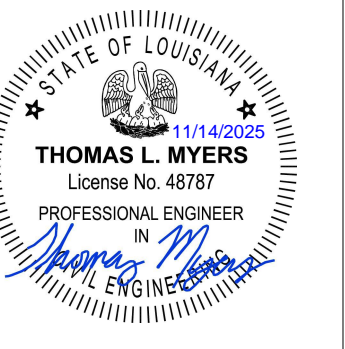


**1 P.T. FOUNDATION PLAN**  
 3/16" = 1'-0"



**2 SHEARWALL PLAN**  
 3/16" = 1'-0"

C:\Users\ThomasMyers\OneDrive\Documents\Projects\SELS\25-0780\SELS-25-0783 MSH - 3 BR Model - Lot # 1 - Adoration Phase, Covington, LA 70435\Progress\SE-25-783.rvt



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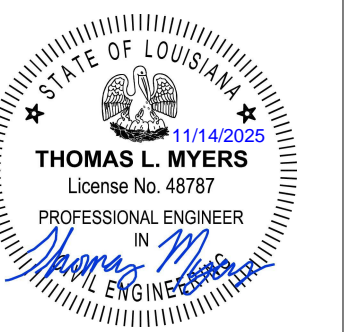
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job no.  
**SE-25-783**  
 sheet title  
**FOUNDATION PLAN**

sheet no.  
**S101**







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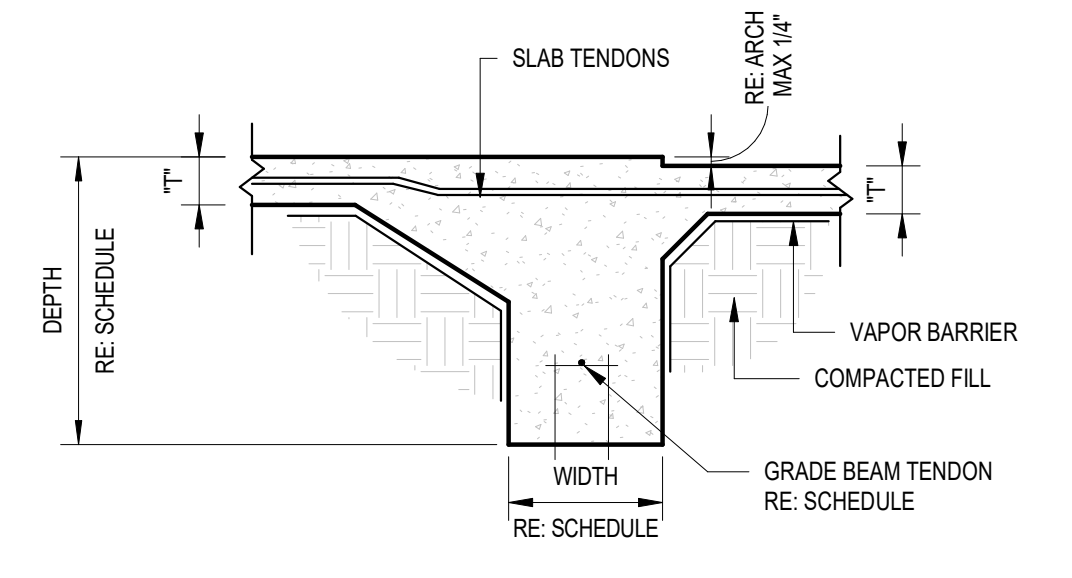
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job no.  
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 sheet title  
**FOUNDATION  
 DETAILS**

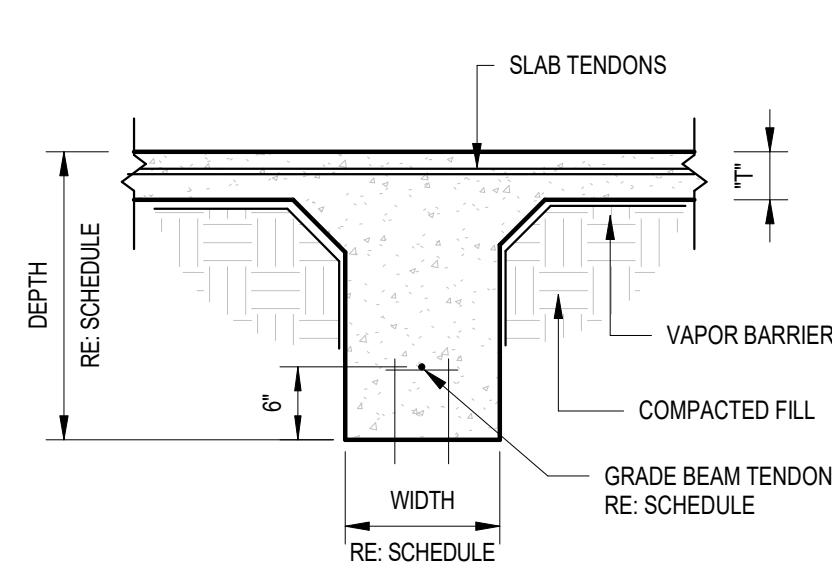
sheet no.  
**S301**



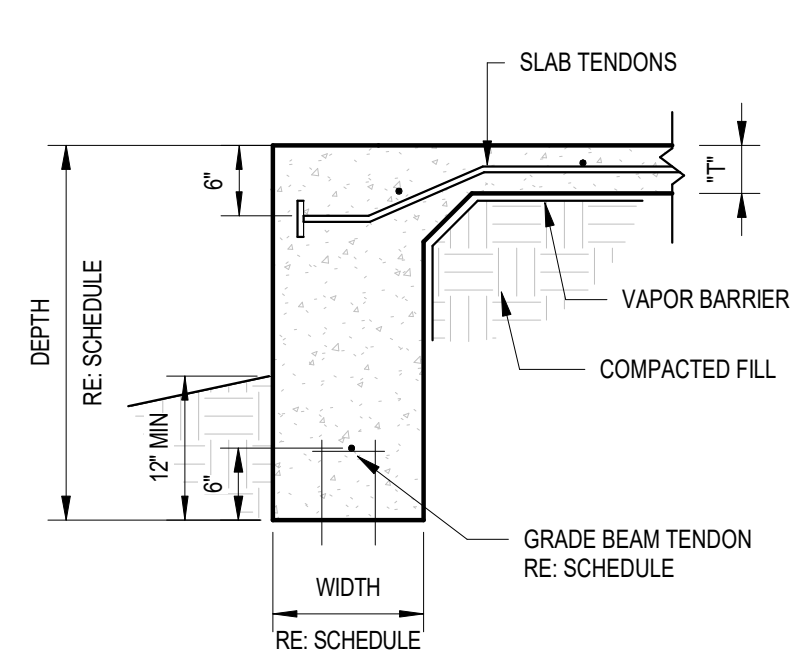
POST-TENSION FOUNDATION GRADE BEAM SCHEDULE				
MARK	GRADE BEAM WIDTH	GRADE BEAM DEPTH	NUMBER OF GRADE BEAM TENDONS	NOTES
PT-1	12"	24"	1	
PLAN LEGEND				
CONVENTIONALLY REINFORCED BEAM WITH 2 #S TOP AND BOTTOM			CRB	
POST-TENSIONING TENDON			SLAB CABLE	GRADE BEAM CABLE
#4 SLAB REINFORCEMENT @ 18" O.C. ONE-WAY AS SHOWN				
#4 BARS 4', 6', 10' @ 6" O.C. TYPICAL RE-ENTRANT CORNER REINFORCEMENT				
<b>NOTE:</b> ALL GRADE BEAMS TO BE PT-1 UNLESS NOTED OTHERWISE.				



**3 INTERIOR GRADE BEAM AT SLAB DEPRESSION**  
 3/4" = 1'-0"



**2 TYPICAL INTERIOR GRADE BEAM**  
 3/4" = 1'-0"



**1 TYPICAL EXTERIOR GRADE BEAM**  
 3/4" = 1'-0"

BEAM AND HEADER SCHEDULE table with columns: MARK, SIZE, GRADE, MARK, SIZE, GRADE. Rows include 228, 2210, 2212, 328, 3210, 3212, 182, 183, 242, 243.

PLAN LEGEND showing 228 - HEADER OR DROPPED BEAM and 228 - FLUSH BEAM. Includes NOTES about conventional headers and beam supports.

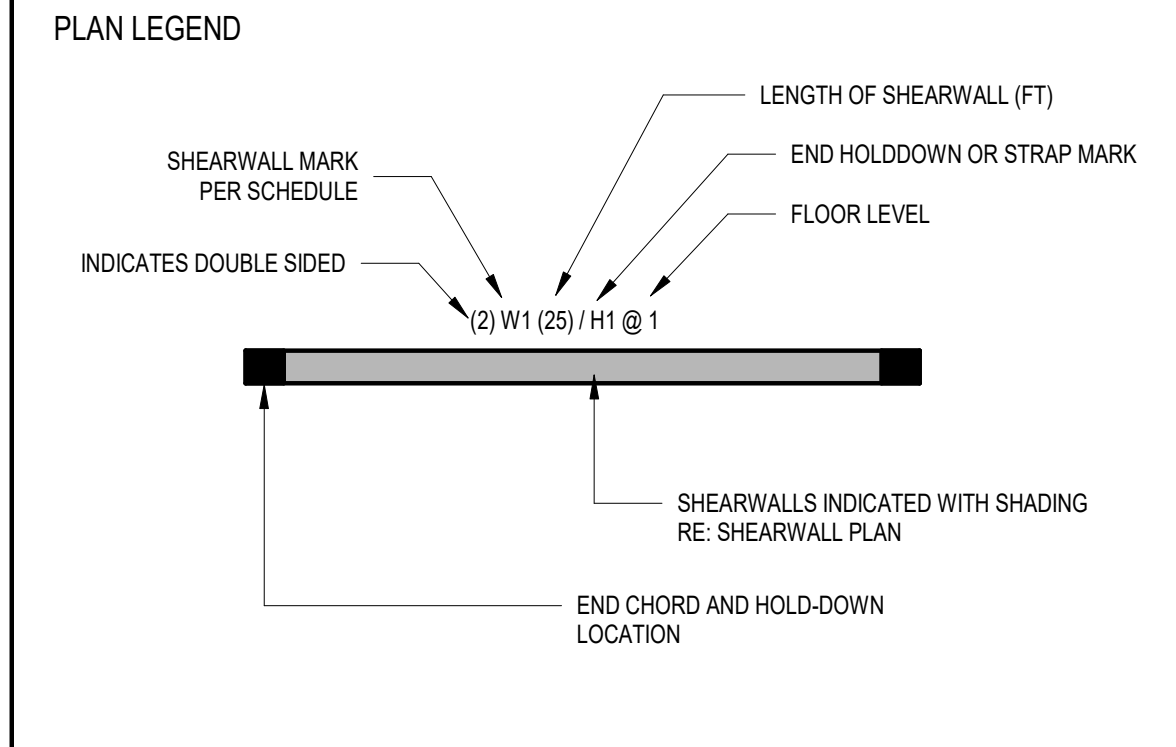
SIMPSON HOLD-DOWN SCHEDULE table with columns: MARK, TYPE, HOLD DOWN CAPACITY (LB) ASD, ANCHOR ROD DIAMETER, DRILL & EPOXY EMBEDMENT (IN), REQUIRED STUD PACK PILES, ANCHOR CAPACITY (LB) 1-EDGE/2-EDGE (LRFD). Rows include H0 through H6.

SIMPSON STRAP SCHEDULE table with columns: MARK, TYPE, HOLD DOWN CAPACITY (LB) ASD, MIN. END LENGTH (IN), TOTAL # OF 10d NAILS, REQUIRED STUD PACK PILES. Rows include S1 through S6.

NOTES: INSTALL PRODUCTS AS REQUIRED BY SIMPSON INSTALLATION INSTRUCTIONS TO OBTAIN CAPACITIES LISTED AS BASIS OF DESIGN. ALTERNATE PRODUCTS MAY BE USED PROVIDED THE ABOVE DESIGN LOADS ARE MET. THREADED ANCHOR RODS SHALL CONFORM TO A-307 GRADE C OR A36 MATERIAL. EPOXY SHALL BE SIMPSON SET-XP OR EQUAL INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER INSTRUCTIONS. HALF OF THE REQUIRED NAILS LISTED MUST BE INSTALLED INTO EACH OF THE STUD OR STUD PACKS BEING TIED TOGETHER. CUT STRAP LENGTH AND CENTER STRAP BETWEEN LEVELS AS NECESSARY TO MEET THIS REQUIREMENT. ALL STUD PACKS SHALL BE SYP #2. ALL STUD PACKS SHALL BE SECURED TOGETHER IN ACCORDANCE WITH THE BUILT-UP STUD DETAIL. WHERE HOLD-DOWNS ARE CALLED OUT AT UPPER LEVEL SHEARWALLS ABOVE STEEL FRAMING ELEMENTS, FULLY WELD ANCHOR RODS TO STRUCTURAL STEEL BELOW.

ROOF DECK SCHEDULE table with columns: TYPE, THICKNESS AND GRADE, NAIL TYPE, BOUNDARY NAILING, FIELD NAILING, BLOCKED. Includes NOTES about nail length and blocking requirements.

SHEARWALL SCHEDULE table with columns: MARK, SHEATHING, FASTENER, BOUNDARY NAILING, FIELD NAILING, ALLOWABLE SHEAR VALUE. Rows include G1, G2, G3, W1, W2, W3, W4.



NOTES: NAILS SHALL NOT TEAR THE PAPER OF GYPSUM SHEARWALLS. FASTENERS SHALL NOT BE PERMITTED TO BE DRIVEN PAST THE FLUSH FACE OF PLYWOOD SHEATHING. MATERIAL THICKNESSES AND FASTENER SIZES SHOWN ARE MINIMUMS. ORIENTED STRAND BOARD (OSB) MAY BE SUBSTITUTED FOR PLYWOOD SHEATHING. "BOUNDARY NAILING" REFERS TO THE ATTACHMENT REQUIRED ALONG THE SUPPORTED EDGES OF EACH SHEATHING PANEL. "FIELD NAILING" REFERS TO THE ATTACHMENT REQUIRED ALONG THE INTERMEDIATE SUPPORTS BEHIND EACH SHEATHING PANEL. ALL SHEARWALL SHEATHING MUST HAVE ALL FREE EDGES SUPPORTED BY BLOCKING OR THE PANELS MUST BE ORIENTED VERTICALLY & SIZED TO EXTEND ENTIRELY FROM BOTTOM PLATE TO TOP PLATE SUCH THAT NO EDGES ARE UNSUPPORTED. LIGHTEST ATTACHMENT SHOWN ABOVE SHALL BE USED FOR ALL NON-SHEARWALL APPLICATIONS FOR RESPECTIVE SHEATHING TYPE AND THICKNESS SHOWN ON THE ARCHITECTURAL PLANS. ALL SHEARWALLS WITH NAILING AT 3" O.C. OR LESS REQUIRE DOUBLE STUDS AT ALL PANEL POINTS THAT SHALL BE FASTENED TOGETHER WITH 8d NAILS @ 2" O.C. STAGGERED. SHEATH ALL EXTERIOR WALLS AS W1, U.N.O. SHEARWALL MARKS ARE SHOWN ON THE SIDE OF THE SHEARWALL THAT RECEIVES THE SCHEDULED SHEATHING AND NAILING PATTERN. HOLD-DOWNS AND STRAPS SHALL BE INSTALLED ON ALL END CHORDS.

SILL AND SOLE PLATE ANCHORAGE SCHEDULE table with columns: LOCATION, TYPE OF WALL, ANCHORAGE, PLATE ANCHOR SPACING. Rows include exterior wall sill & plates, interior wall sill & plates, and sole plates.

NOTES: SHEARWALLS SHEATHED ON BOTH SIDES SHALL USE TWICE THE ANCHORS REQUIRED BY THE MOST STRINGENT SCHEDULED ANCHORAGE OF THE SIDES INDIVIDUALLY. THE 1/2" WET-SET BOLTS SCHEDULED ABOVE SHALL BE GALVANIZED AND HAVE A MIN. 1" HOOK EMBEDDED MIN. 7" INTO CONCRETE AND BE FITTED WITH STANDARD WASHER AND NUT. THERE SHALL ALWAYS BE ONE ANCHOR PLACED NOT MORE THAN 12" NOR LESS THAN 4" FROM END SILL PLATE END. THERE SHALL BE A MINIMUM OF TWO (2) ANCHORS PER SILL PLATE. THE 1/2" WET-SET SILL BOLTS SCHEDULED MAY BE REPLACED WITH 1/2" X 6" SIMPSON TITEN HD ANCHORS ON A 1:1 BASIS. EXPANSION ANCHORS SHALL NOT BE PERMITTED. PAF ANCHORS SHALL BE HILTI X-CP. SILL BOLTS IN WOOD SHEARWALLS SHALL HAVE A 0.229" X 3" X 3" WASHERS AND EXTEND TO WITHIN 1/2" OF SHEATED SIDES OF PLATE.

LOAD BEARING WALL STUD SCHEDULE table with columns: WALL TYPE, LEVEL, 2x4 STUD SPACING, 2x6 STUD SPACING. Rows include 1-story exterior walls and 1-story interior walls.

NOTES: SEE ARCHITECTURAL PLANS FOR WALL WIDTHS WHERE 2X4 AND 2X6 STUDS ARE ALLOWED BY THE SCHEDULE. SEE PLAN FOR POSSIBLE EXCEPTIONS TO THIS SCHEDULE. BEARING WALLS ARE INDICATED ON PLANS AS SUCH

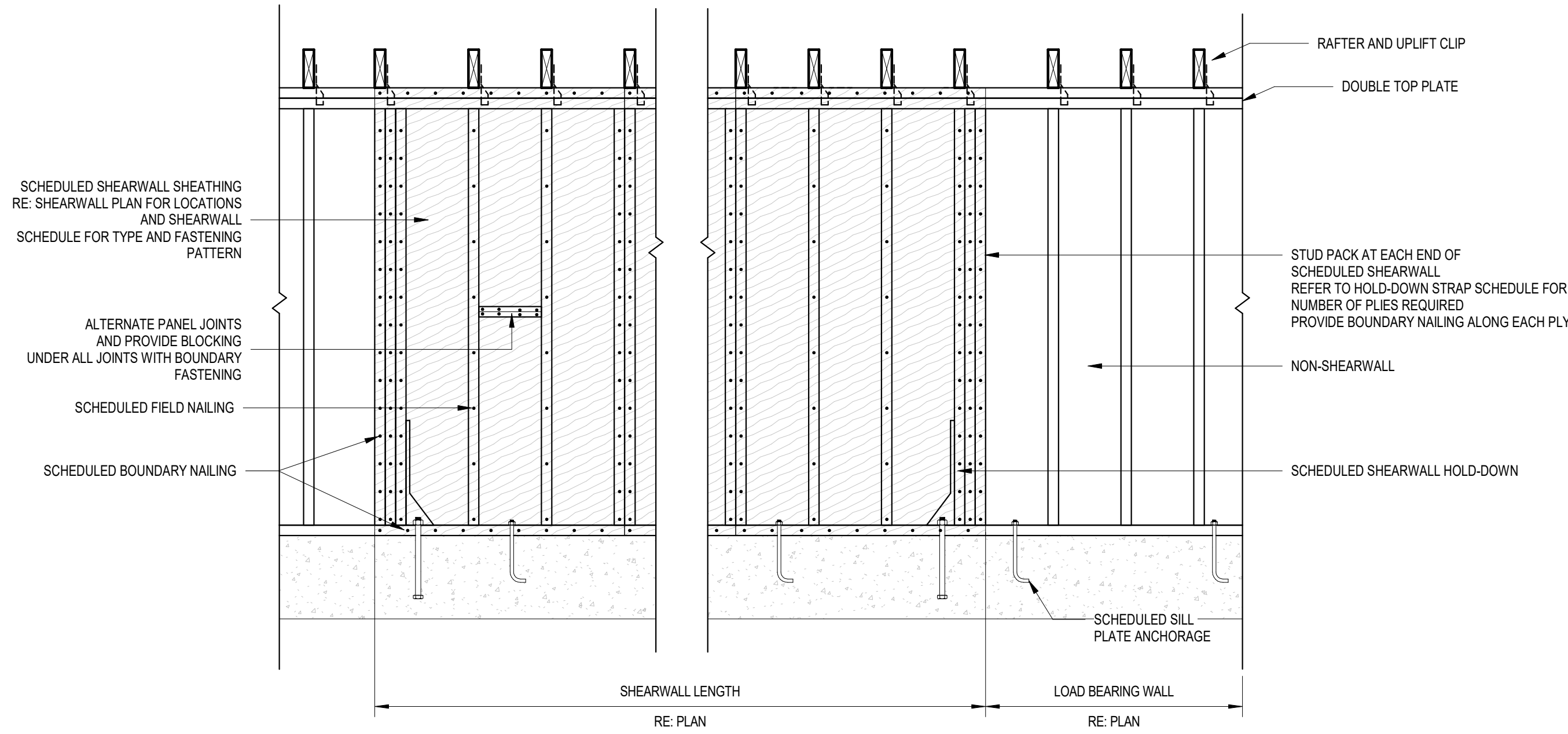
STANDARD WOOD CONNECTION FASTENING SCHEDULE table with columns: DESCRIPTION OF BUILDING ELEMENT, COMMON NAIL, 0.131"x3" NAIL. Rows include joist to sill, bridging, sole plate anchorage, stud to sole plate, built-up stud packs, top plate, and ceiling joist connections.

NOTES: PENNY SIZES ARE FOR COMMON NAILS (NOT SINKER OR BOX) AND ARE DEFINED AS FOLLOWS: PENNYWEIGHT, DIAMETER, LENGTH. ANY DESCRIPTION OF BUILDING ELEMENTS NOT NOTED ABOVE MAY BE FOUND IN THE IBC, TABLE 2304.9.1. SEE ICC ESR-1539 FOR ALL ADDITIONAL FASTENER REQUIREMENTS NOT NOTED HERE.

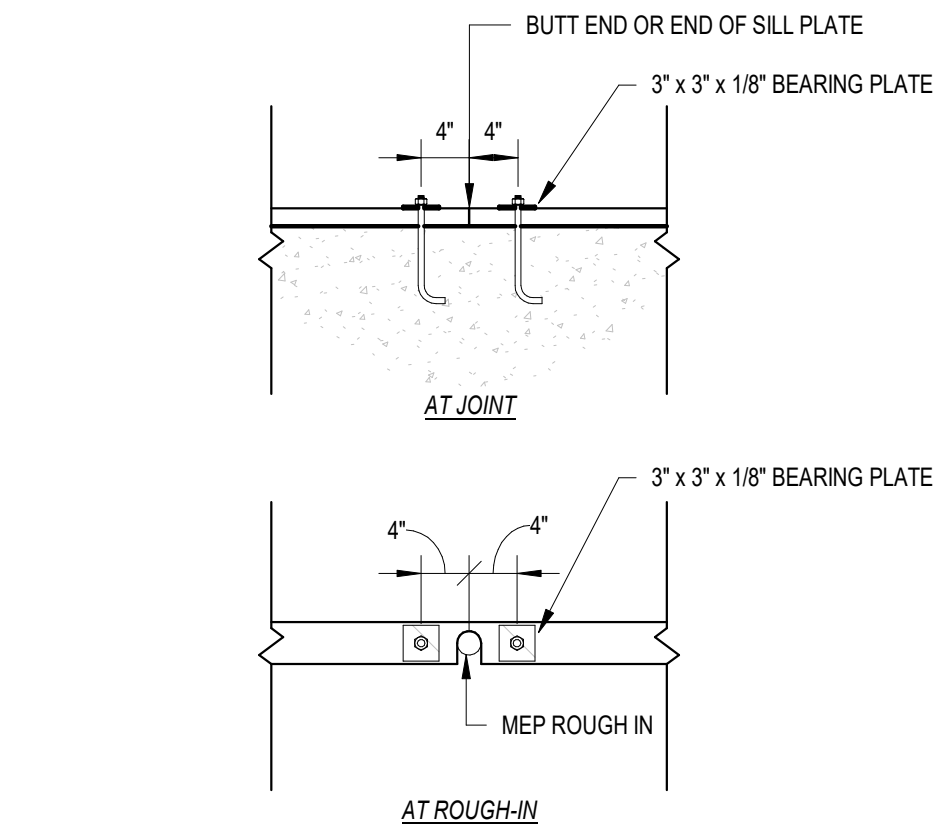
ABBREVIATIONS table with columns: Symbol, Description, Symbol, Description. Includes terms like AT, A.F.F., ARCH, BM, B.O.S., BOT., C.G., COL., CONT., CNX, DL, EL., ELEC., E.O.A., E.O.S., FF, PLF, GA, GB, MECH., O.C., O.C.E.W., P.T., SIM., T.O., T.O.C., T.O.J., T.O.S., U.N.O., V.O.J., PL., CL., U.W.A., GAGE GRADE BEAM, MECHANICAL, ON CENTER, POST-TENSION, TOP OF, TOP OF CONCRETE, TOP OF JOIST, TOP OF SLAB, UNLESS NOTED OTHERWISE, VERIFY ON JOBSITE, PLATE, CENTER LINE, UNDER WALL ABOVE.

SOLID SAWN JOIST AND BEAM HANGER SCHEDULE table with columns: BEAM OR JOIST SIZE, SIMPSON HANGER, BEAM OR JOIST SIZE, SIMPSON HANGER. Rows include 2x6, 2x8, 2x10, 2x12, 2x6, 2x8, 2x10, 2x12.

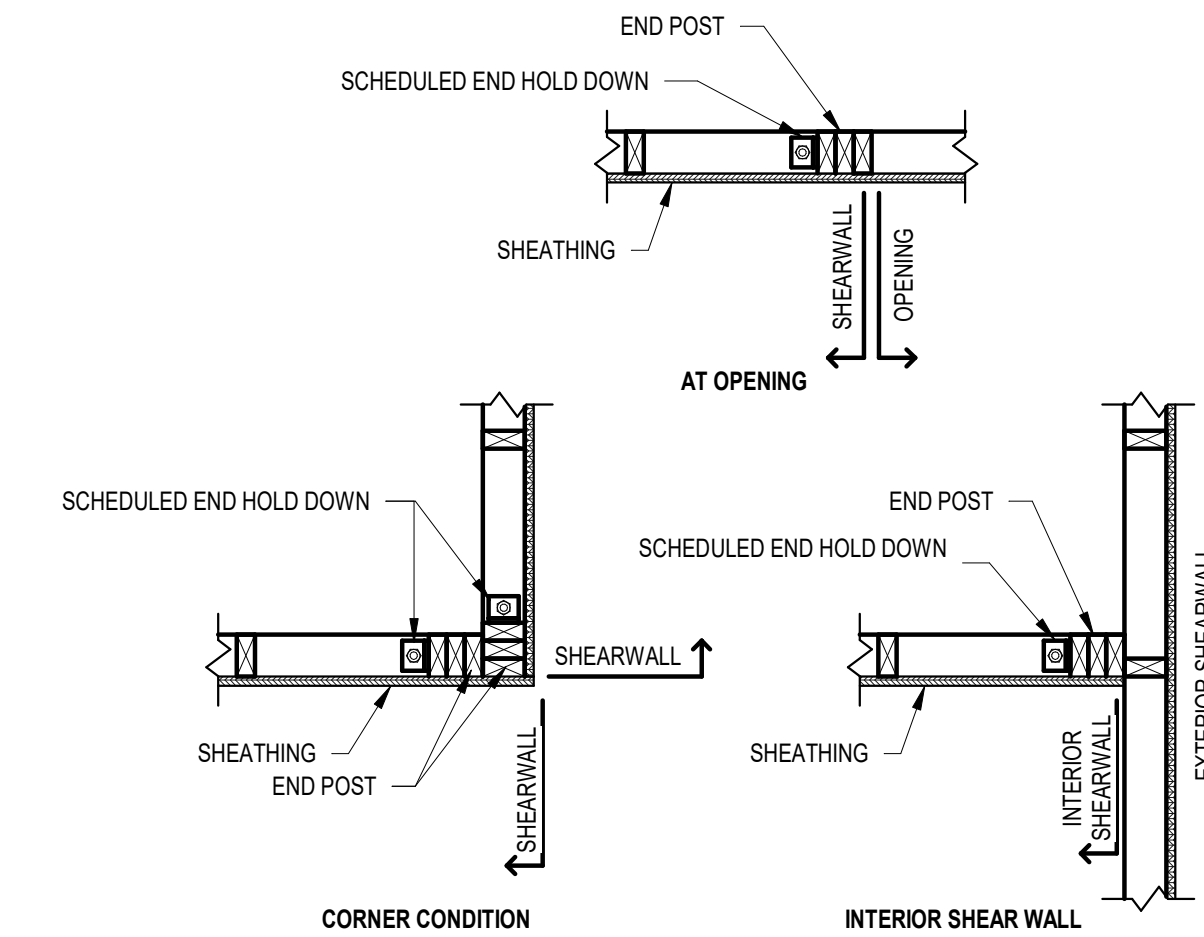
NOTES: ALL FLUSH JOIST AND BEAM CONNECTORS SHALL BE MADE WITH MANUFACTURED JOIST OR BEAM HANGERS MANUFACTURED BY SIMPSON STRONG TIE OR APPROVED EQUAL. IF MULTIPLE OPTIONS FOR FASTENING ARE PROVIDED BY THE MANUFACTURER, PROVIDE THE TYPE, QUANTITY, AND SIZE OF FASTENERS REQUIRED TO ACHIEVE THE MAXIMUM RATED CONNECTOR CAPACITY. ENGINEERED GRADE LUMBER BEAM CONNECTORS HAVE BEEN DESIGNATED ON THE PLANS, SECTIONS, OR DETAILS. Diagram showing beam hanger connection with labels: BEAM SEE PLAN, JOIST OR BEAM HANGER PER SCHEDULE, FLOOR JOIST OR BEAM SEE PLAN.



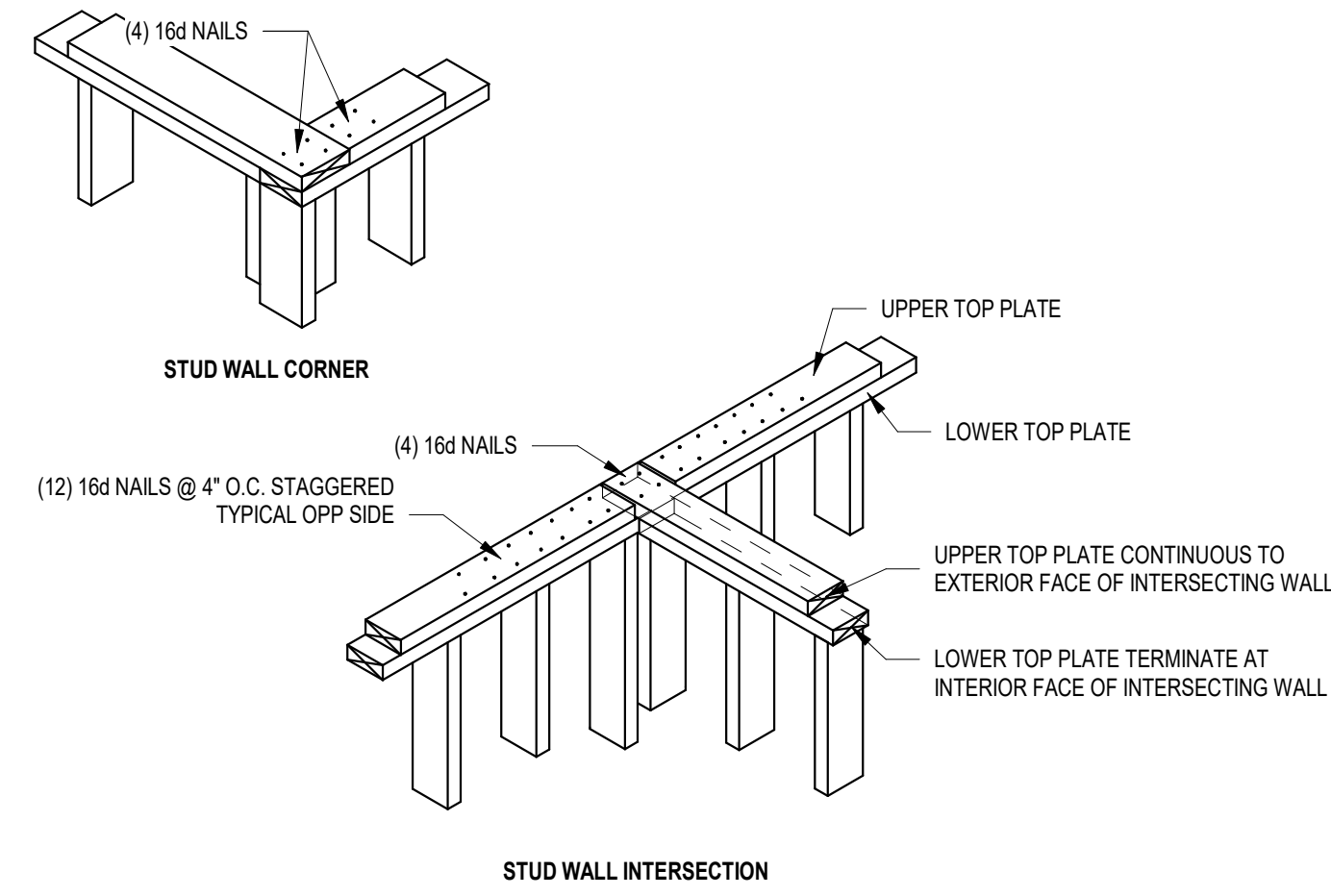
**1 TYPICAL SINGLE-STORY SHEARWALL**  
 3/4" = 1'-0"



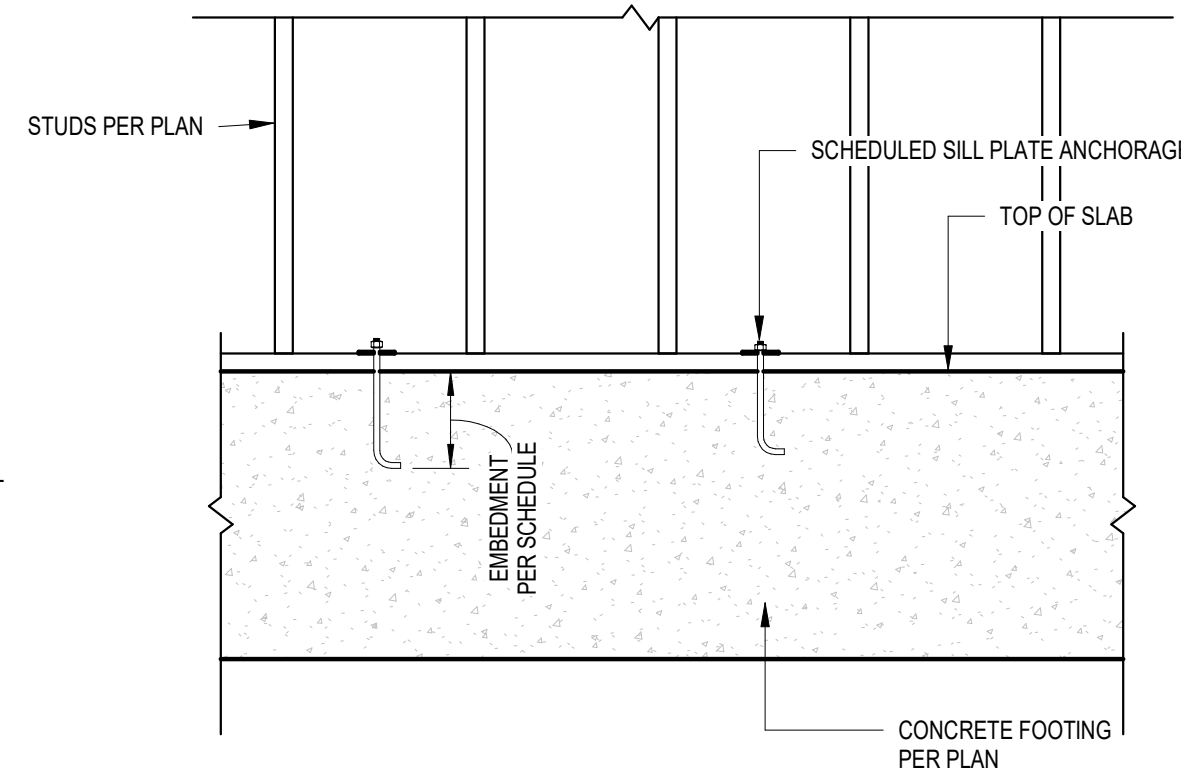
**2 TYPICAL SILL ANCHORAGE AT JOINT**  
 3/4" = 1'-0"



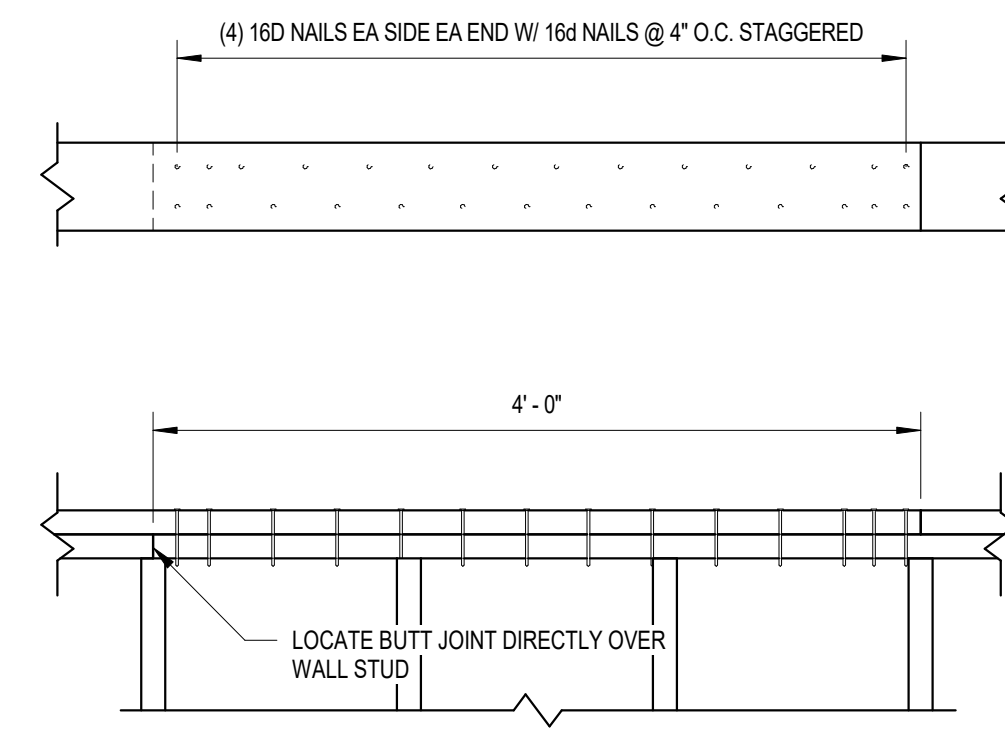
**3 TYPICAL SHEARWALL CONDITIONS**  
 3/4" = 1'-0"



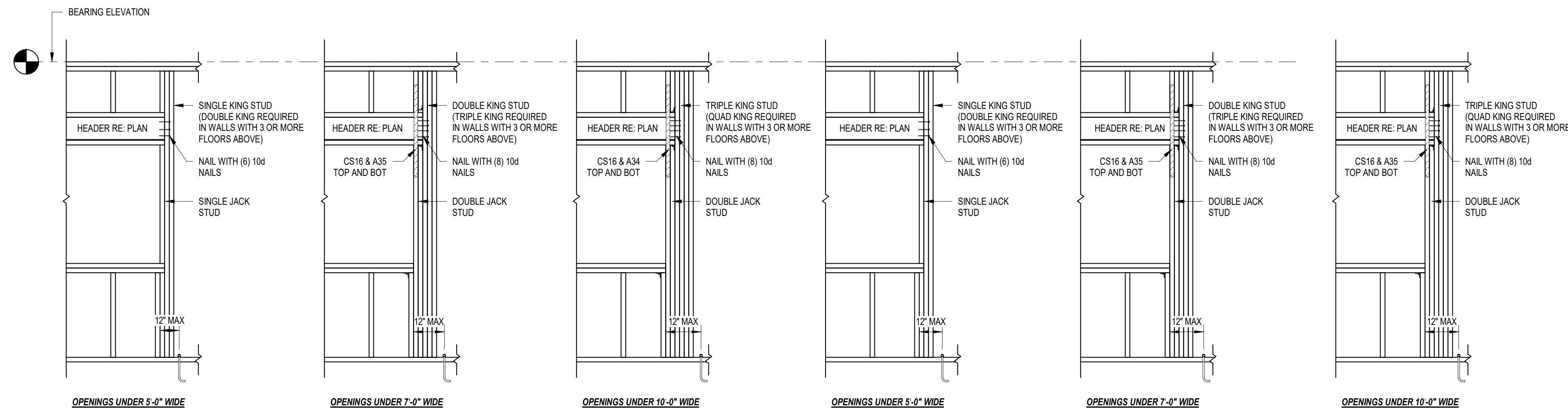
**4 TYPICAL TOP PLATE CONDITIONS**  
 1/2" = 1'-0"



**5 TYPICAL SILL ANCHORAGE DETAIL**  
 3/4" = 1'-0"



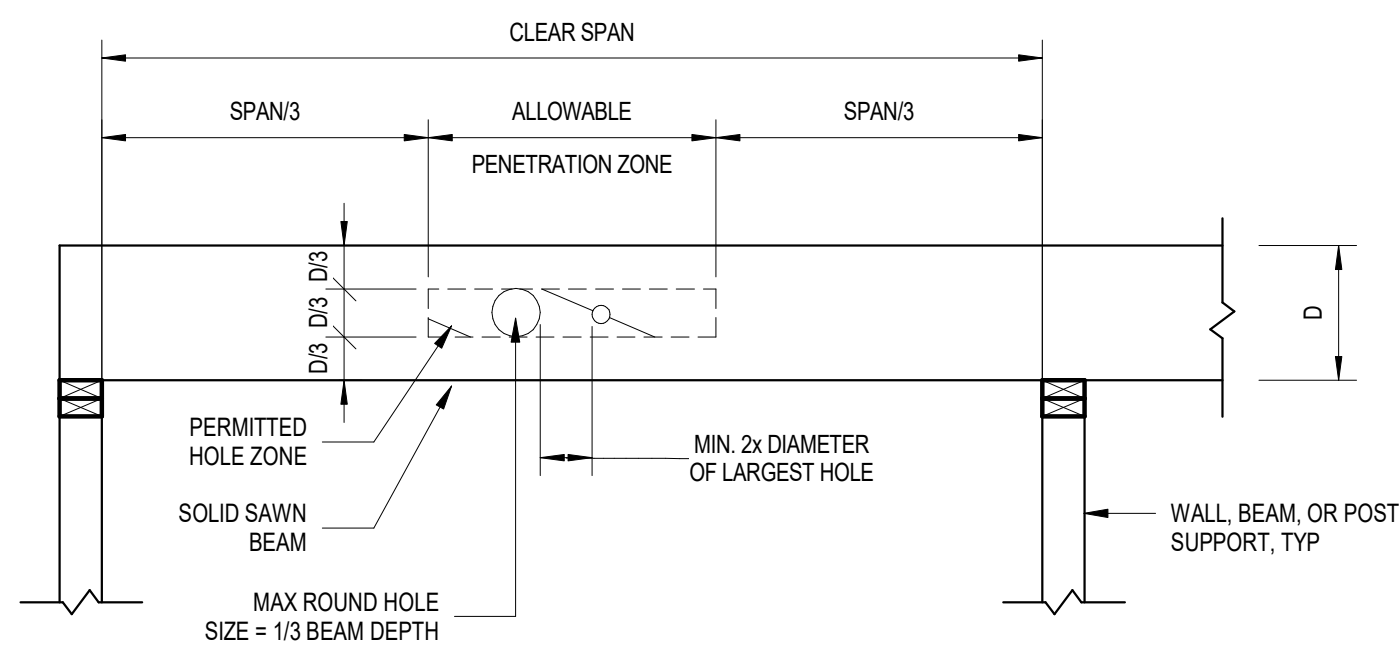
**6 TYPICAL TOP PLATE SPLICE**  
 1" = 1'-0"



**7 TYPICAL JACK AND KING STUD DETAIL**  
 1/2" = 1'-0"

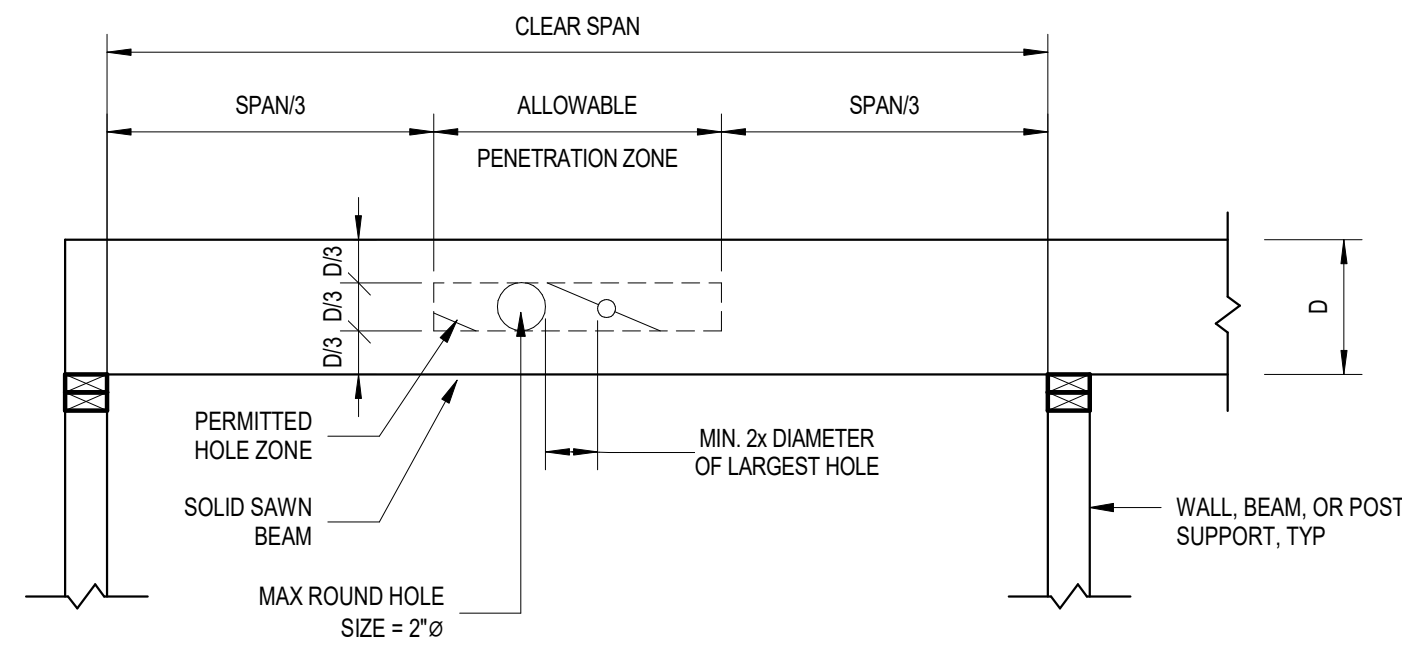
**TYPICAL 2X4 LOAD BEARING WALL HEADER SUPPORT DETAIL**

**TYPICAL 2X6 LOAD BEARING WALL HEADER SUPPORT DETAIL**



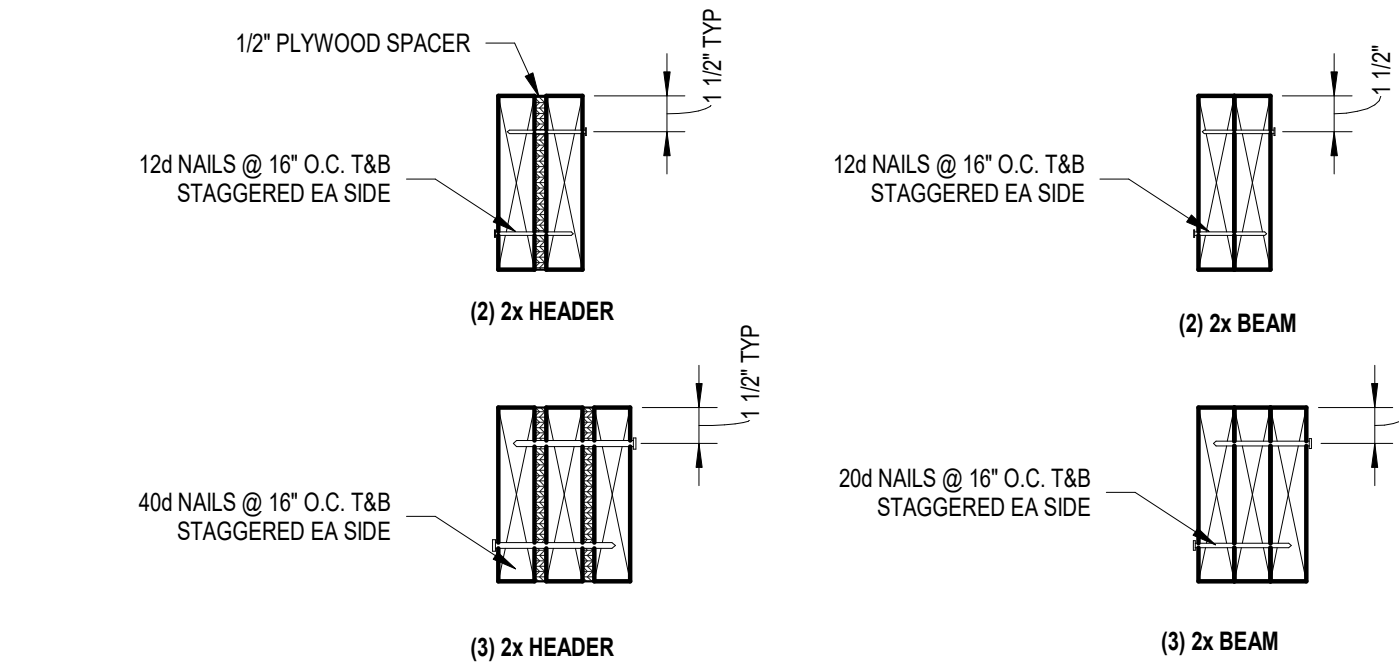
- NOTES:**
1. DETAIL APPLIES TO SOLID SAWN HEADERS OR BEAMS WITH DEPTHS GREATER THAN 7 1/4".
  2. ALLOWABLE HOLES INDICATED ARE SUITABLE FOR HEADERS AND BEAMS WITH UNIFORM LOADING ONLY. HOLES MUST BE ROUND. SQUARE HOLES NOT PERMITTED.
  3. HOLES ARE PERMITTED TO BE PLACED IN THE MIDDLE 1/3 OF THE SPAN IN THE MIDDLE 1/3 OF THE BEAM ONLY.
  4. DETAIL APPLIES TO BOTH SIMPLE AND CONTINUOUS SPANS.
  5. HOLES ARE NOT PERMITTED TO BE PLACED IN CANTILEVERS.
  6. ANY CONDITION NOT COVERED BY THIS DETAIL SHALL BE SUBMITTED TO THE SEOR FOR REVIEW AND APPROVAL PRIOR TO FIELD CUTTING HOLES.

**1 ALLOWABLE HOLES IN SOLID SAWN BEAMS AND HEADERS**  
 3/4" = 1'-0"

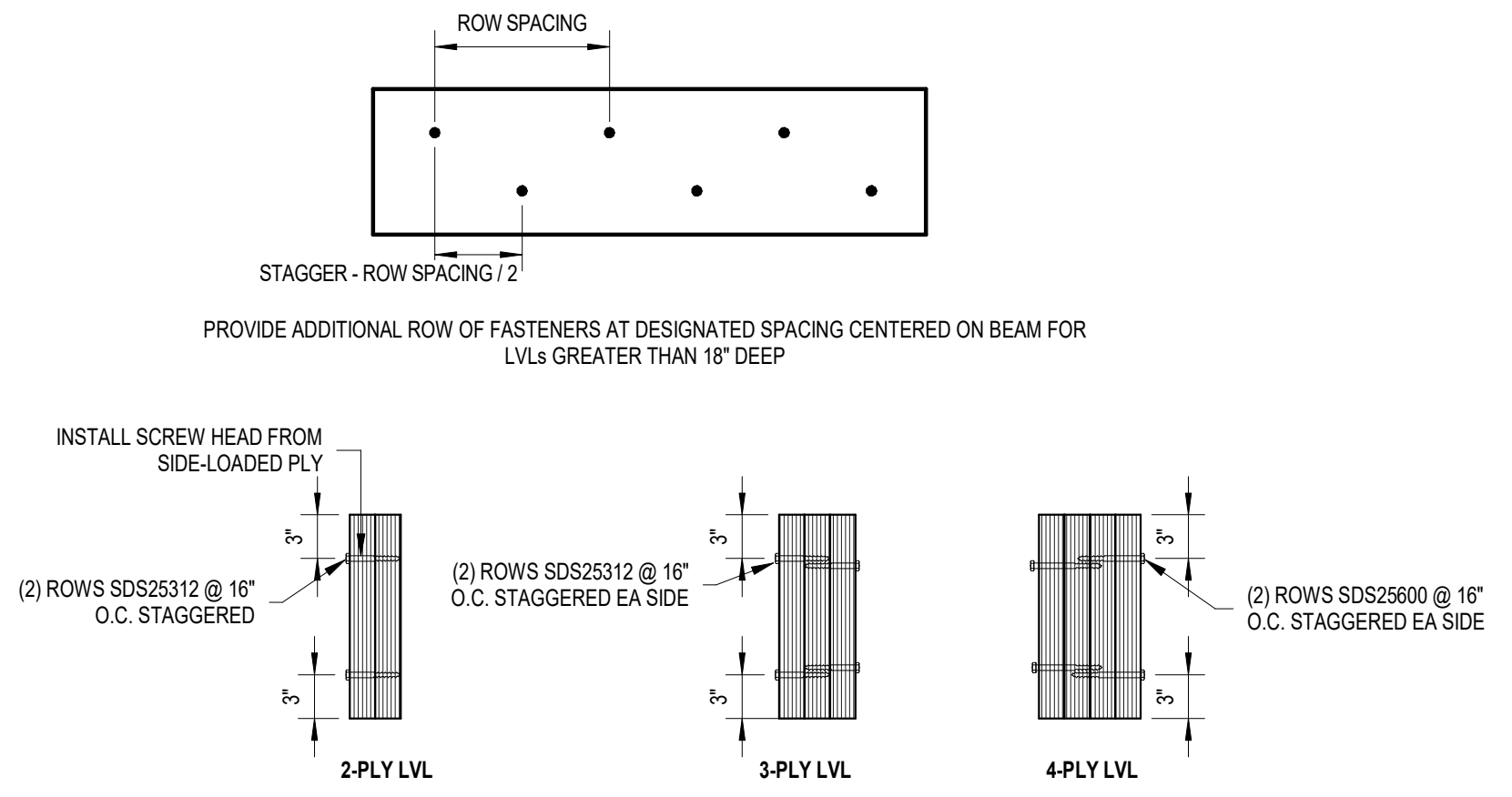


- NOTES:**
1. DETAIL APPLIES TO LVL AND PSL HEADERS OR BEAMS WITH DEPTHS GREATER THAN 7 1/4".
  2. ALLOWABLE HOLES INDICATED ARE SUITABLE FOR HEADERS AND BEAMS WITH UNIFORM LOADING ONLY. HOLES MUST BE ROUND. SQUARE HOLES NOT PERMITTED.
  3. HOLES ARE PERMITTED TO BE PLACED IN THE MIDDLE 1/3 OF THE SPAN IN THE MIDDLE 1/3 OF THE BEAM ONLY.
  4. DETAIL APPLIES TO BOTH SIMPLE AND CONTINUOUS SPANS.
  5. HOLES ARE NOT PERMITTED TO BE PLACED IN CANTILEVERS.
  6. ANY CONDITION NOT COVERED BY THIS DETAIL SHALL BE SUBMITTED TO THE SEOR FOR REVIEW AND APPROVAL PRIOR TO FIELD CUTTING HOLES.

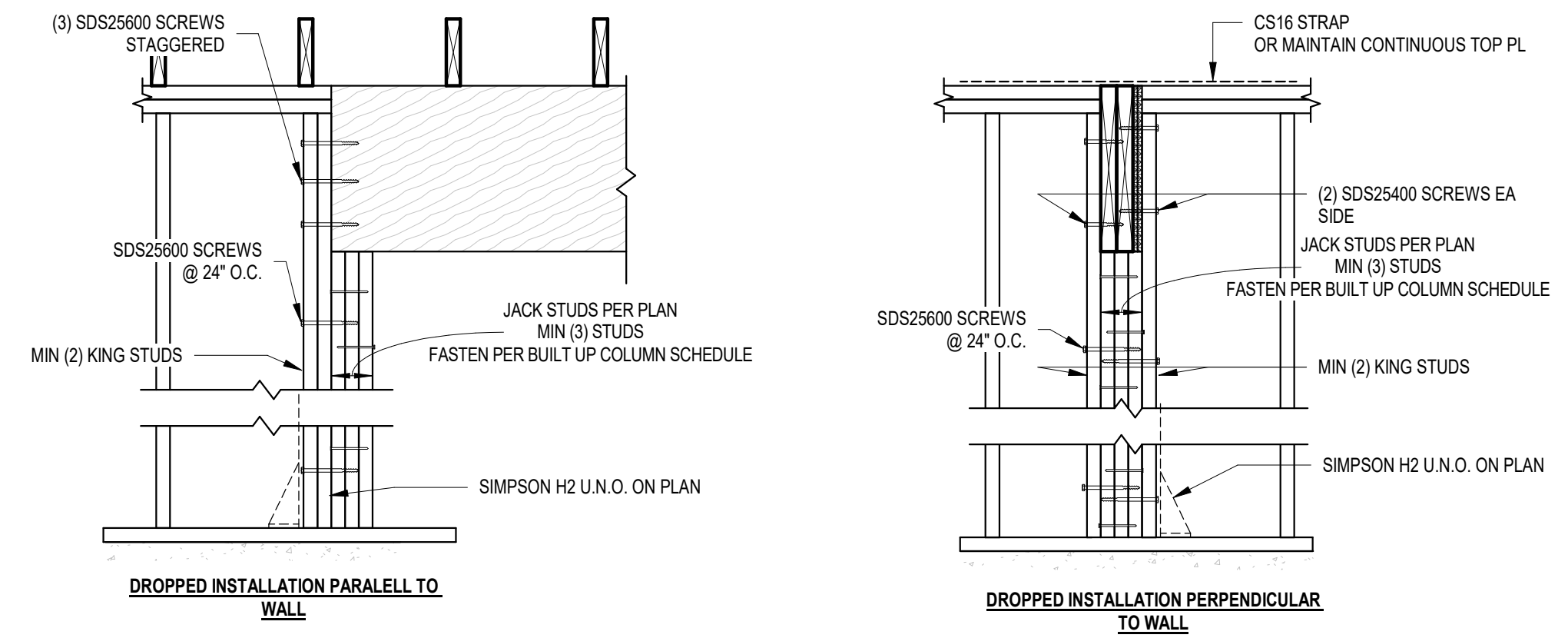
**2 ALLOWABLE HOLES IN ENGINEERED BEAMS AND HEADERS**  
 3/4" = 1'-0"



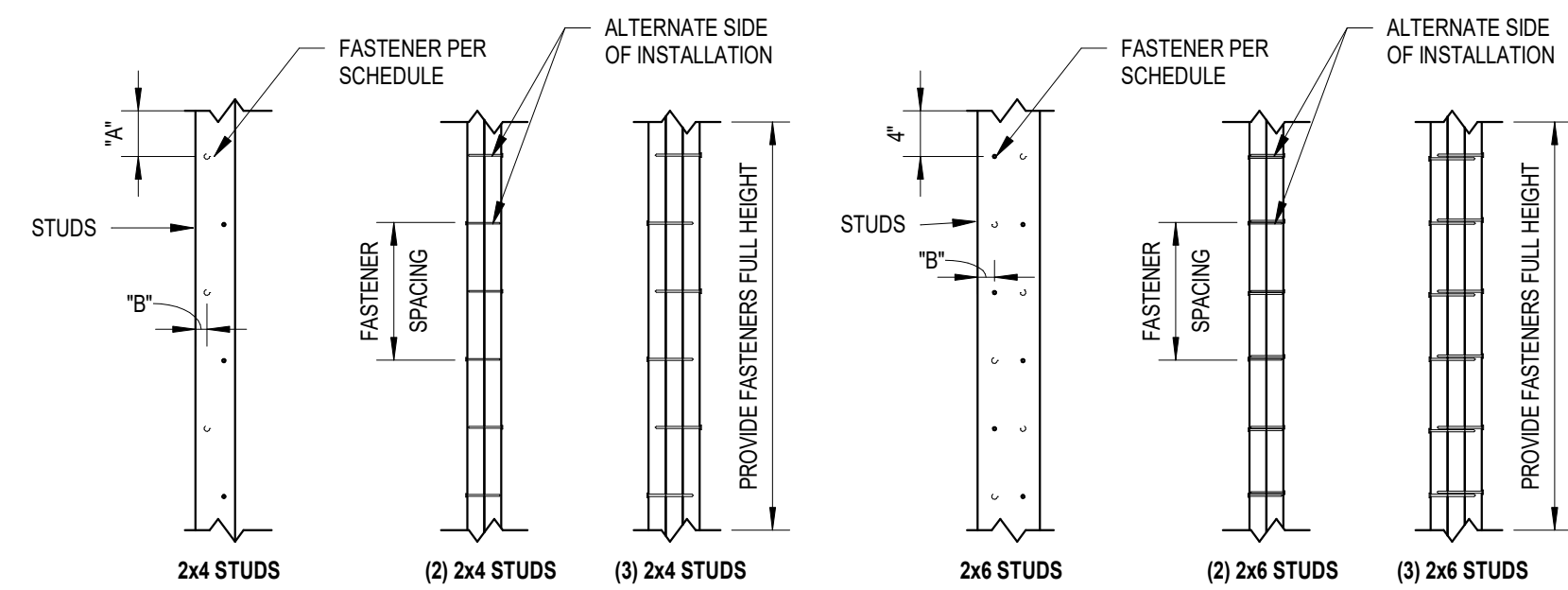
**4 BUILT UP HEADER / BEAM DETAIL**  
 1 1/2" = 1'-0"



**5 MULTI-PLY LVL FASTENING**  
 1" = 1'-0"

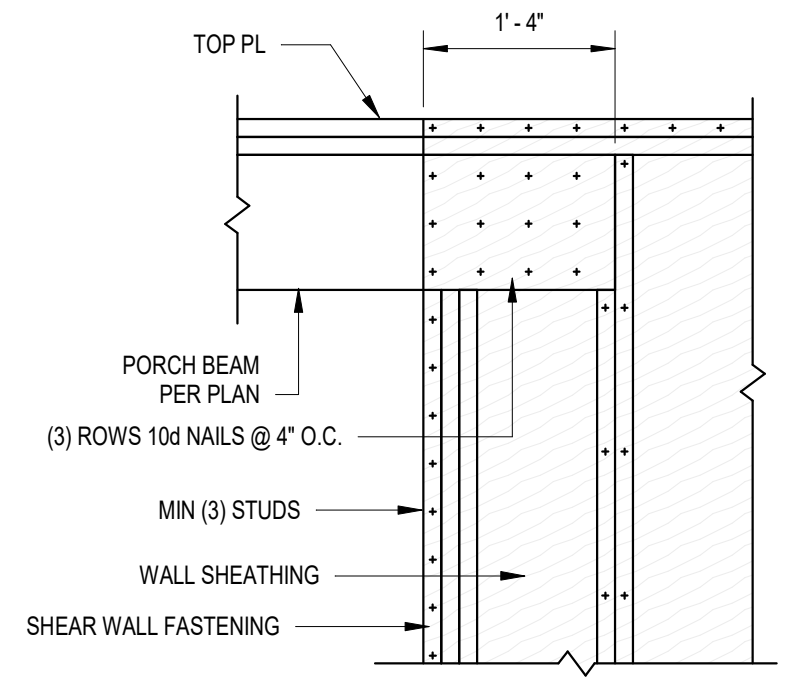


**3 TYPICAL ENGINEERED BEAM BEARING**  
 3/4" = 1'-0"

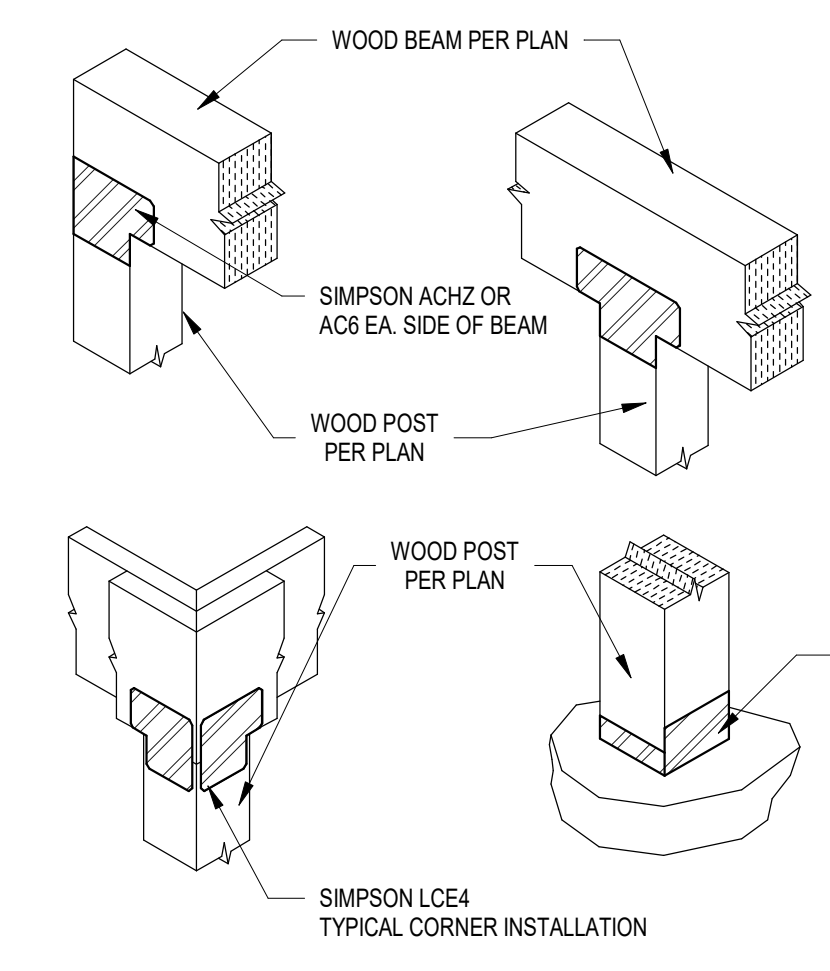


**6 BUILT UP COLUMN DETAIL/SCHEDULE**  
 3/4" = 1'-0"

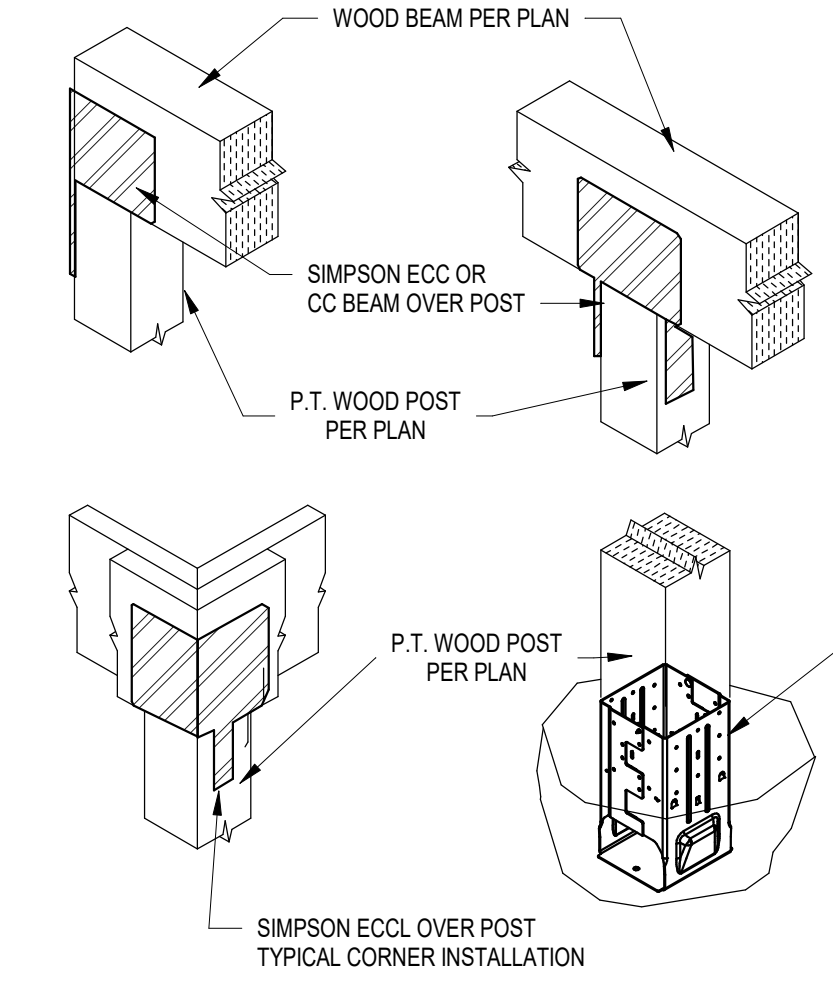
2x4 BUILT UP COLUMN				
QUANTITY OF STUDS	FASTENERS	SPACING	"A" END DISTANCE	"B" EDGE DISTANCE
(2) 2x	10d COMMON NAILS	(1) ROW @ 6" STAGGERED	2 1/2"	1"
(3) 2x	20d COMMON NAILS	(1) ROW @ 8" STAGGERED	3 1/2"	1 1/2"
(4) 2x	SIMPSON SDW22600	(1) ROW @ 8" STAGGERED	4"	1 1/2"
(5) 2x	1/2" Ø THRU BOLTS	(1) ROW @ 9" STAGGERED	3 1/2"	1 1/2"
2x6 BUILT UP COLUMN				
QUANTITY OF STUDS	FASTENERS	SPACING	"A" END DISTANCE	"B" EDGE DISTANCE
(2) 2x	10d COMMON NAILS	(2) ROW @ 6"	2 1/2"	1"
(3) 2x	20d COMMON NAILS	(2) ROW @ 8"	3 1/2"	1 1/2"
(4) 2x	SIMPSON SDW22600	(2) ROW @ 8"	4"	1 1/2"
(5) 2x	1/2" Ø THRU BOLTS	(2) ROW @ 9"	3 1/2"	1 1/2"



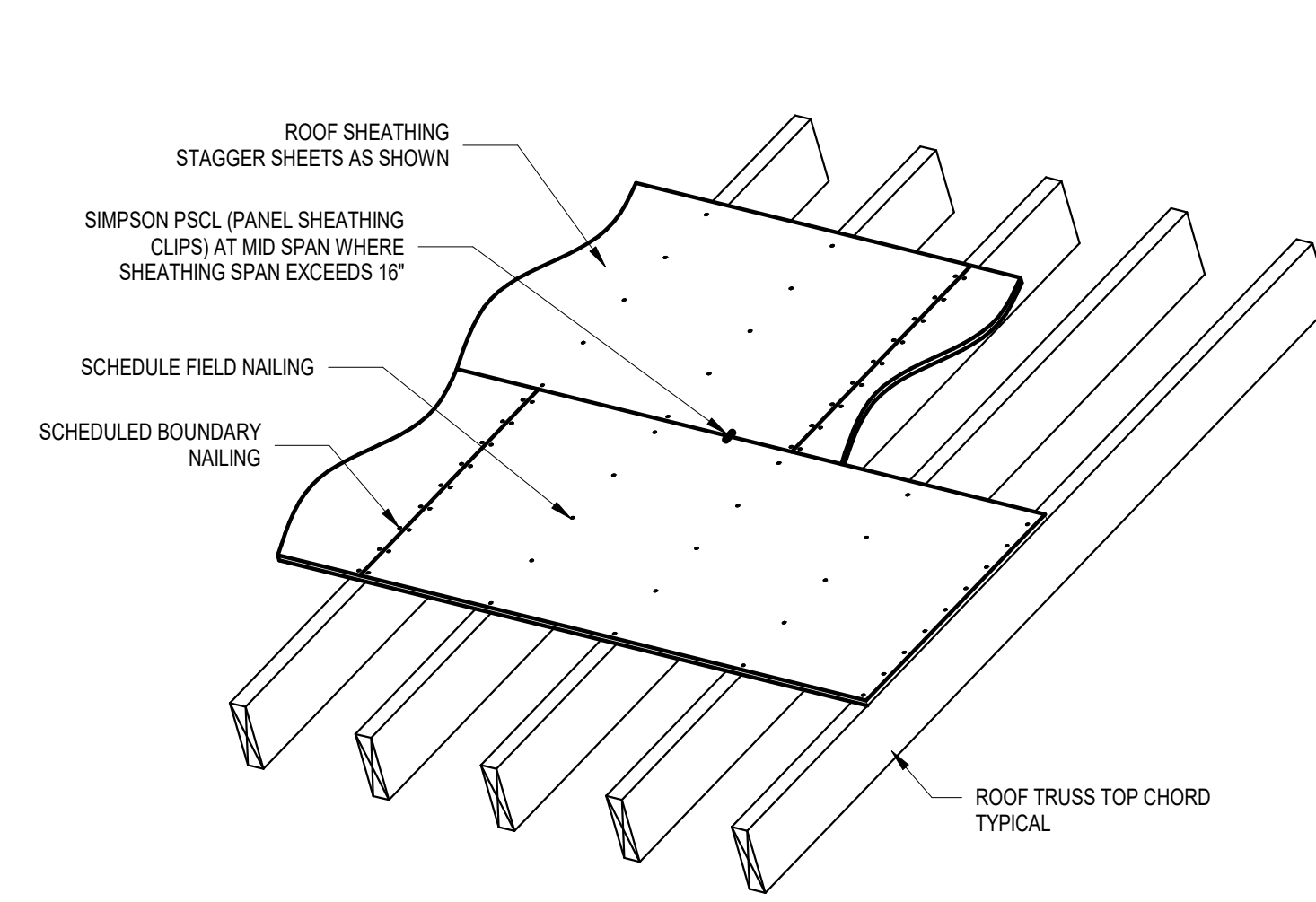
**7 PORCH BEAM AT EXTERIOR WALL DETAIL**  
 3/4" = 1'-0"



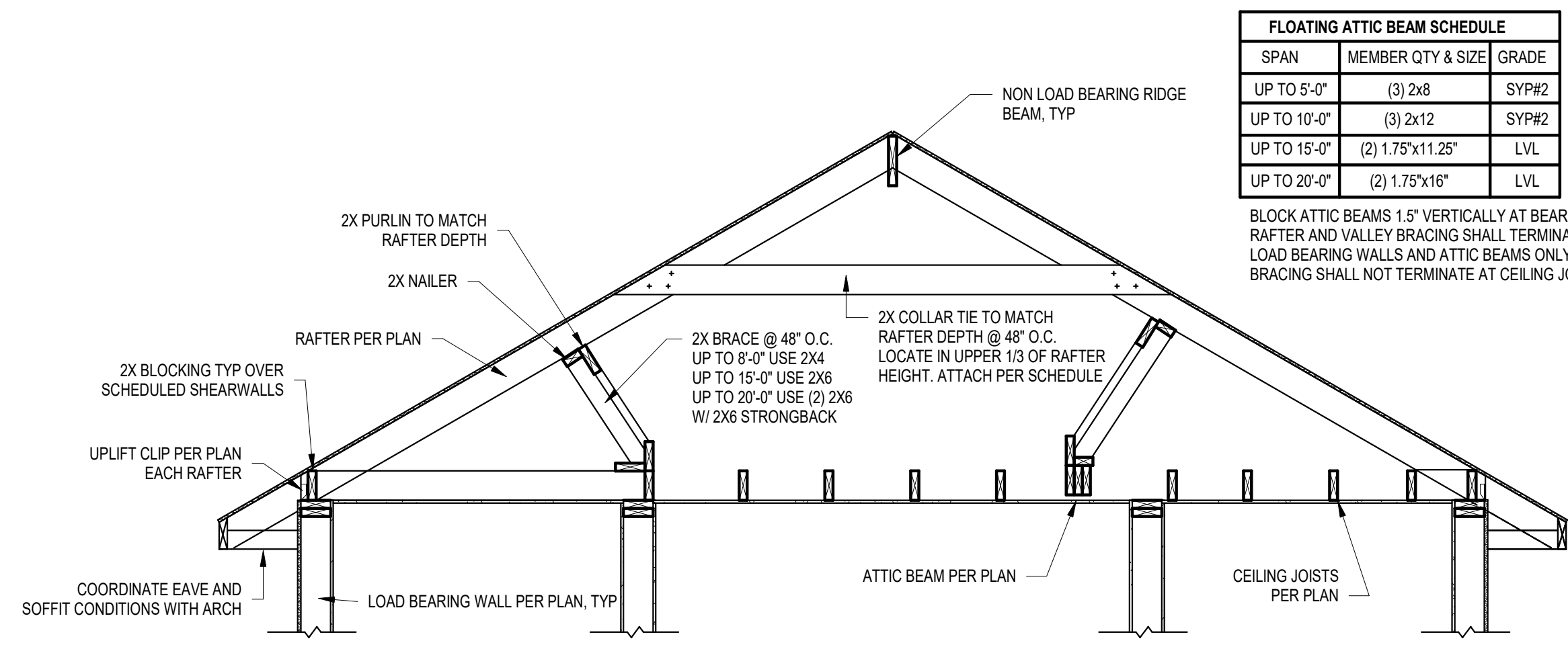
**8 TYPICAL STAND ALONE POST DETAIL**  
 3/4" = 1'-0"



**9 TYPICAL MPBZ STAND ALONE POST DETAIL**  
 3/4" = 1'-0"



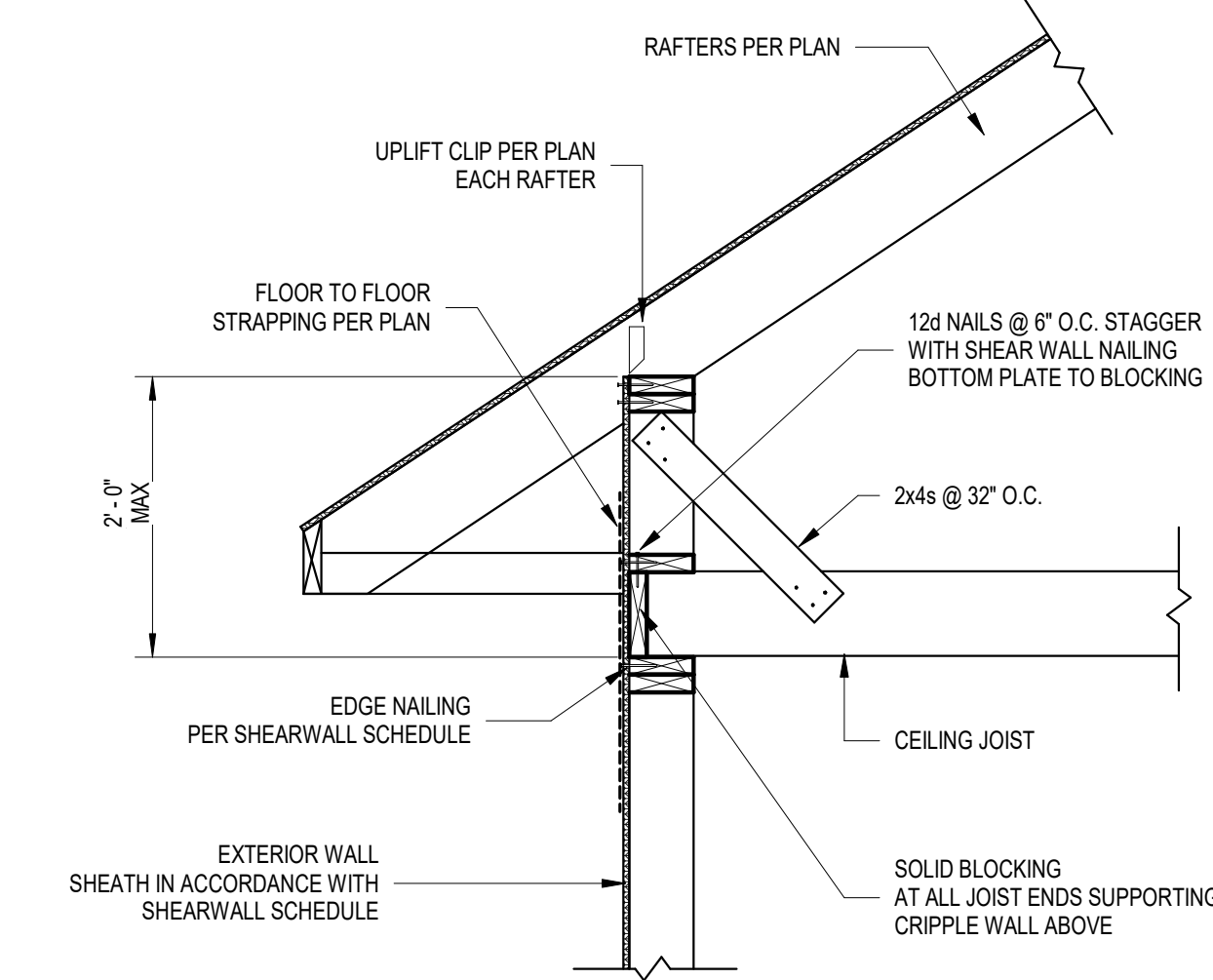
**1 TYPICAL ROOF SHEATHING DETAIL**  
 3/4" = 1'-0"



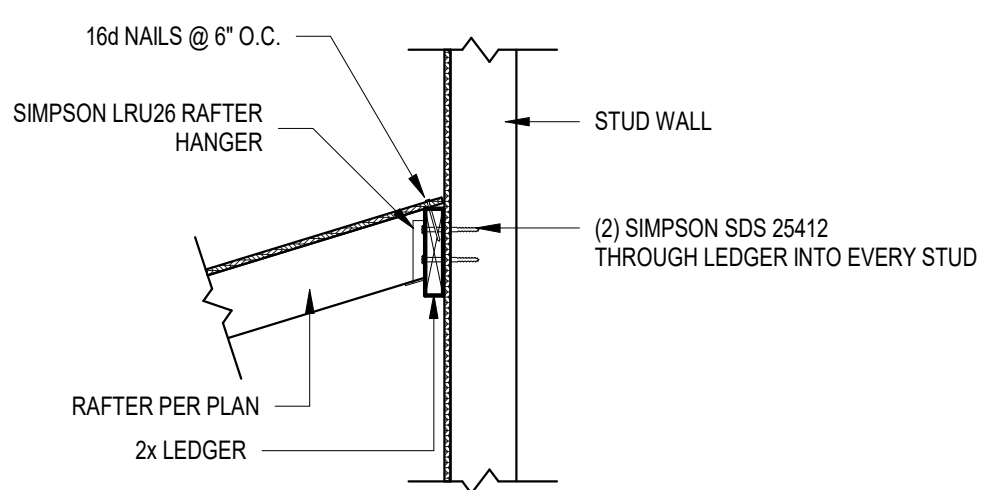
**2 TYPICAL ROOF FRAMING**  
 1/2" = 1'-0"

SPAN	MEMBER QTY & SIZE	GRADE
UP TO 5'-0"	(3) 2x8	SYP#2
UP TO 10'-0"	(3) 2x12	SYP#2
UP TO 15'-0"	(2) 1.75"x11.25"	LVL
UP TO 20'-0"	(2) 1.75"x16"	LVL

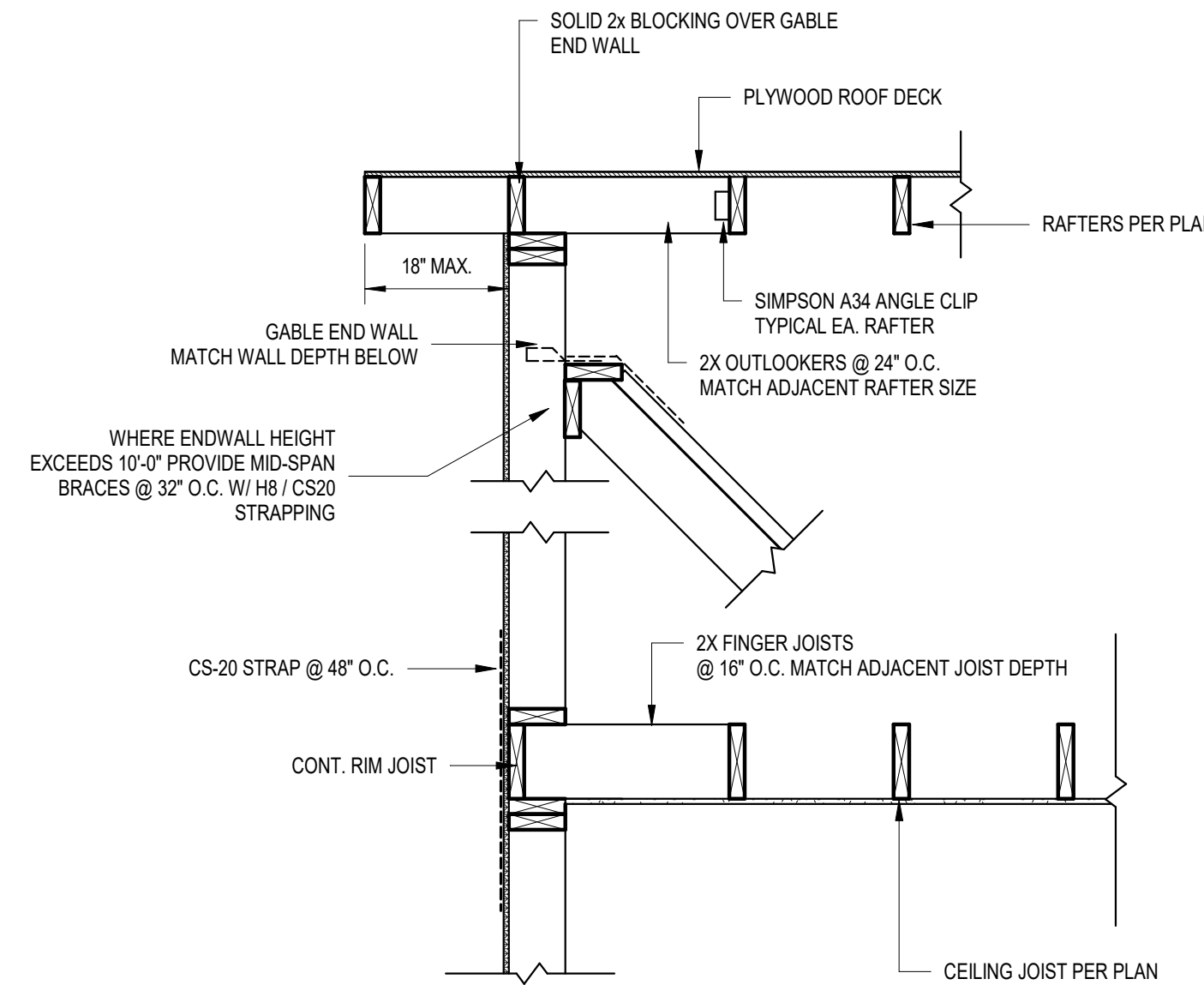
BLOCK ATTIC BEAMS 1.5" VERTICALLY AT BEARING. RAFTER AND VALLEY BRACING SHALL TERMINATE AT LOAD BEARING WALLS AND ATTIC BEAMS ONLY. BRACING SHALL NOT TERMINATE AT CEILING JOISTS.



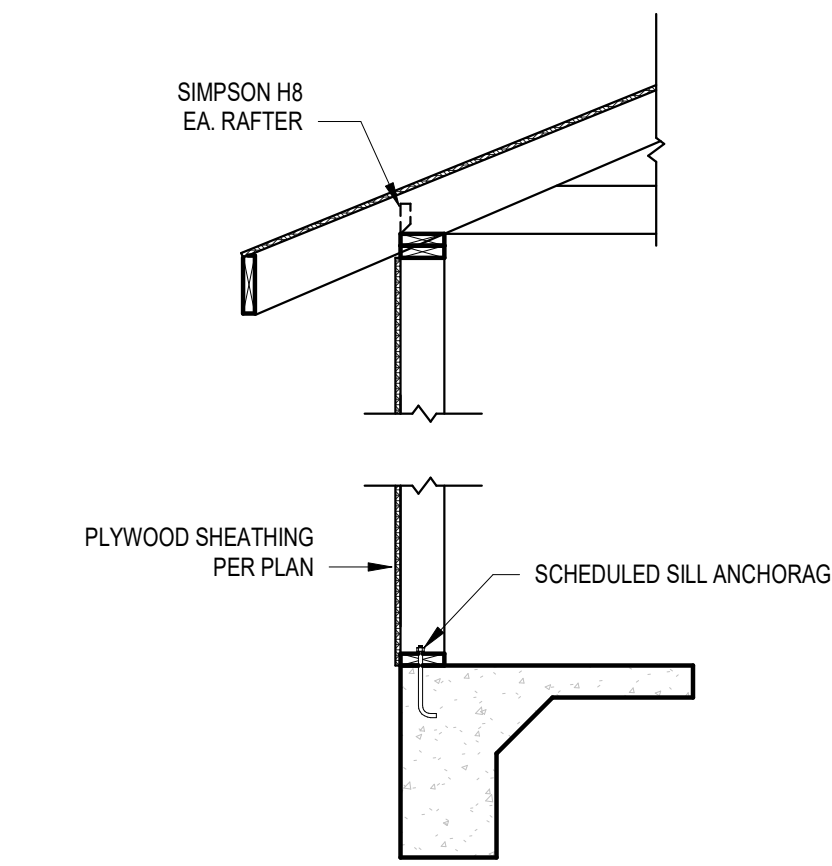
**3 TYPICAL CRIPPLE WALL RAFTER BEARING**  
 3/4" = 1'-0"



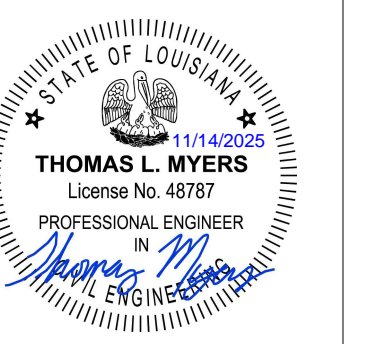
**4 TYPICAL RAFTER LEDGER ATTACHMENT DETAIL**  
 3/4" = 1'-0"



**5 TYPICAL GABLE ENDWALL FRAMING**  
 3/4" = 1'-0"



**6 UPLIFT STRAP CONNECTIONS AT ROOF BEARING WALLS (SINGLE-STORY)**  
 1/2" = 1'-0"



CD SET

issue date  
**8.15.2025**

revisions

job no.

**SE-25-783**

sheet title  
**STRUCTURAL NOTES**

sheet no.

**S601**

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**GENERAL**

ALL DESIGN LOADS SHOWN ON THE STRUCTURAL PLANS ARE ALLOWABLE STRESS DESIGN (UNFACTORED) SERVICE LOADS.

ALL METHODS, PROCEDURES, AND SEQUENCES OF WORK ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL ENSURE ALL PRECAUTIONS ARE TAKEN TO MAINTAIN THE INTEGRITY OF THE STRUCTURE THROUGHOUT ALL STAGES OF CONSTRUCTION.

REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING PLANS FOR ALL ITEMS OMITTED FROM THE STRUCTURAL PLANS.

IF A CONFLICT IS OBSERVED IN THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD FOR PROPER GUIDANCE AND CLARIFICATION PRIOR TO CONSTRUCTION.

THE STRUCTURAL INTEGRITY OF THE DESIGN DEPENDS ON THE FULL INTERACTION OF ALL ITS FRAMING MEMBERS AND CLADDING. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROPERLY SHORE AND BRACE ALL STAGES OF CONSTRUCTION UNTIL THE STRUCTURE IS COMPLETED.

THE GENERAL CONTRACTOR SHALL COORDINATE THE STRUCTURAL DRAWINGS WITH ALL OTHER DRAWINGS.

ALL EXISTING CONDITIONS, DIMENSIONS, ELEVATIONS, AND QUANTITIES SHALL BE VERIFIED BY THE GENERAL CONTRACTOR PRIOR TO ANY FABRICATION OR INSTALLATION.

SCALING OF THE STRUCTURAL DRAWINGS SHALL NOT BE PERMITTED. IF ANY DIMENSIONS VITAL TO CONSTRUCTION ARE NOT SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS, CONTRACTOR SHALL SEND FORMAL REQUEST TO THE ARCHITECT OR ENGINEER OF RECORD FOR CLARIFICATION.

IN THE EVENT OF CONFLICTING OR DIFFERING REQUIREMENTS INDICATED ON THE STRUCTURAL DRAWINGS AND/OR SPECIFICATIONS THAT HAVE NOT BEEN SUBSEQUENTLY CLARIFIED OR CHANGED, THE CONTRACTOR SHALL PROVIDE THE BETTER QUALITY, GREATER QUANTITY, OR MORE STRINGENT UNLESS OTHERWISE DIRECTED BY THE ENGINEER OF RECORD.

**CODES AND DESIGN SPECIFICATIONS**

BUILDING CODE: INTERNATIONAL BUILDING CODE (IBC) 2021

AMERICAN CONCRETE INSTITUTE (ACI 318 LATEST EDITION)

NATIONAL DESIGN SPECIFICATION FOR WOOD FRAMED CONSTRUCTION 2018

ASCE 7-16

**DESIGN LOADS**

- ROOF DESIGN LOADS

CEILING COLLATERAL DEAD LOAD = 10 PSF  
 CEILING LIVE LOAD = 20 PSF  
 ROOF COLLATERAL DEAD LOAD = 10 PSF  
 ROOF LIVE LOAD = 20 PSF

- WIND LOAD CRITERIA

ULTIMATE WINDSPEED = 140 MPH  
 NOMINAL WINDSPEED = 108 MPH  
 OCCUPANCY CATEGORY = II  
 EXPOSURE = B  
 INTERNAL PRESSURE COEFFICIENT = +/- 0.18

TOTAL SHEAR PERPENDICULAR TO LENGTH = 18 KIP (ULTIMATE)  
 TOTAL SHEAR PERPENDICULAR TO WIDTH = 10 KIP (ULTIMATE)

**GEOTECHNICAL**

NO GEOTECHNICAL INFORMATION WAS FURNISHED FOR THIS PROJECT. THIS ENGINEER HAS ASSUMED THAT THE SOIL CONDITIONS FOR THIS DESIGN ARE FIRM, STABLE, AND NON-EXPANSIVE AND ARE CAPABLE OF SAFELY SUPPORTING 1,500 PSF FOR SPOT FOOTINGS AND FOR CONTINUOUS FOOTINGS. BY USING THESE PLANS, THE OWNER HOLDS HARMLESS THE ENGINEER FROM ALL LIABILITY AND DAMAGES ARISING FROM UNSTABLE SOIL CONDITIONS OR FROM CONDITIONS WHERE FACTORS CAUSE THE SOIL CONDITIONS TO BECOME UNSTABLE AND ALLOW MOVEMENT WITHIN THE FOUNDATION AND STRUCTURAL SYSTEM.

IF CONDITIONS ARE FOUND TO BE OTHER THAN HEREIN STATED, STOP OPERATIONS AND NOTIFY ENGINEER SO THEY CAN HAVE THE NECESSARY INVESTIGATION AND MODIFY PLANS AS REQUIRED.

STRIP A MINIMUM OF 6" OF EXISTING SOIL AND REMOVE ALL ROOTS, ORGANIC MATERIALS AND UNSTABLE SOILS PRIOR TO PLACING ANY FILL.

A MAXIMUM OF 2.0 FEET OF FILL MAY BE PLACED ON THE SITE. MAXIMUM DIFFERENTIAL FILL SHALL NOT EXCEED 20%.

STRUCTURAL FILL SHALL BE LEAN CLAY WITH A PI BETWEEN 15 AND 25 COMPACTED TO 95% STANDARD PROCTOR DENSITY IN MAXIMUM OF 6" LIFTS.

CONTRACTOR AND OWNER TO PROVIDE PROPER DRAINAGE AWAY FROM THE FOUNDATION DURING AND THROUGHOUT THE STRUCTURE LIFE.

**CAST-IN-PLACE CONCRETE**

ALL CONCRETE WORK SHALL BE CONDUCTED IN ACCORDANCE WITH THE ACI STANDARD "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318 LATEST EDITION).

CONCRETE MIX DESIGNED SHALL BE ESTABLISHED BY THE SUPPLIER IN ACCORDANCE WITH ACI 318-14. MIX DESIGNS SHALL BE SUBMITTED WITH BACKUP DATA PER ACI 318-14 TO THE ARCHITECT/ENGINEER FOR REVIEW AND APPROVAL PRIOR TO CONCRETE BATCHING. THE PROPOSED MATERIALS AND MIX SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE TESTING LABORATORY. RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS ON THE GENERAL CONTRACTOR.

ALL CONCRETE SHALL CONFORM TO THE REQUIREMENTS SPECIFIED IN THE TABLE BELOW:

USE	COMPRESSIVE STRENGTH	MAXIMUM AGGREGATE SIZE
CONTINUOUS FOOTINGS & SLABS ON GRADE.....	3,000 PSI.....	3/4"

WATER/CEMENT RATIO SHALL BE LIMITED TO 0.50 MAXIMUM.

SLEEVES, MECHANICAL OPENINGS, CONDUITS, PIPES, RECESSES, DEPRESSIONS, AND ALL EMBEDDED ITEMS SHALL BE PROVIDED FOR AS PER THE MECHANICAL, ELECTRICAL, AND ARCHITECTURAL DRAWINGS.

ENGINEER OF RECORD DOES NOT PERMIT THE USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS THAT MAY CAUSE EXCESSIVE CORROSION IN THE REINFORCING STEEL.

A MINIMUM 10 MIL POLYETHYLENE VAPOR BARRIER SHALL BE PROPERLY INSTALLED AND SEALED UNDER ALL CONCRETE DESIGNS UNDER THIS SEAL.

CONTRACTOR SHALL REFER TO THE FOUNDATION DRAWINGS FOR ADDITIONAL INFORMATION.

ALL FLOOR DRAINS, DRIPS, CURBS, LEDGES, ETC. SHALL BE COORDINATED WITH THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.

CONTRACTOR SHALL PLACE AND SECURE ALL REINFORCEMENT, ANCHOR BOLTS, ETC. PRIOR TO PLACING ANY CONCRETE. WET PLACING ANY OF THESE ITEMS DURING CONCRETE PLACEMENT IS NOT PERMITTED.

WATER MAY NOT BE ADDED TO BATCH AT THE SITE UNLESS IT IS SPECIFICALLY NOTED THAT IT MAY BE ADDED BY THE READY-MIX COMPANY.

**CONCRETE REINFORCEMENT**

ALL CONCRETE REINFORCEMENT SHALL BE DETAILED AND ACCESSORIES PROVIDED IN ACCORDANCE WITH THE PROVISIONS OF ACI 318 (LATEST EDITION).

ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60 (Fy = 60,000 PSI MIN.) UNLESS OTHERWISE SHOWN.

WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 AND SHALL BE PROVIDED IN FLAT SHEETS ONLY.

WELDED WIRE FABRIC SHALL BE LAPPED A MINIMUM OF 2 MESHES, BUT NOT LESS THAN 12".

ALL REINFORCING STEEL SHALL BE SUPPORTED ON PLASTIC CHAIRS AT 48" O.C. E.W.

ALL BAR SPLICES, WHERE REQUIRED SHALL CLASS "B" TENSION LAP SPLICES (2'-0" MINIMUM).

CONCRETE PROTECTION FOR REINFORCEMENT OF CAST-IN-PLACE CONCRETE MEMBERS SHALL BE IN ACCORDANCE WITH ACI 318-14 UNLESS OTHERWISE NOTED. MINIMUM CLEAR COVER OF CONCRETE OVER OUTER REINFORCING BARS SHALL BE IN ACCORDANCE WITH BELOW.

1. CONCRETE PERMANENTLY CAST AGAINST EARTH ..... 3"
2. CONCRETE EXPOSED TO EARTH OR WEATHER  
 #6 THROUGH #11 BARS ..... 2"  
 #5 BAR AND SMALLER ..... 1 1/2"

ALL REINFORCING STEEL SHALL BE ADEQUATELY TIED TOGETHER AND SUPPORTED TO ENSURE PROPER LOCATION OF REINFORCEMENT IN ACCORDANCE WITH THE STRUCTURAL DESIGN.

BARs SHALL BE IN CONTACT WHEN FORMING A LAP SPLICE, UNLESS NOTED OTHERWISE.

ALL DOWELS SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED WALL OR COLUMN REINFORCEMENT AND SHALL BE LAPPED WITH A CLASS "B" TENSION LAP SPLICE.

**POST-INSTALLED ANCHORS**

POST-INSTALLED ANCHORS SHALL ONLY BE USED WITH THE PRIOR WRITTEN APPROVAL OF THE ENGINEER OF RECORD OR WHERE SPECIFIED IN THE STRUCTURAL DOCUMENTS.

ALL HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. PROVIDE CONTINUOUS SPECIAL INSPECTION FOR ALL MECHANICAL AND ADHESIVE ANCHORS PER THE APPLICABLE EVALUATION REPORT (ICC-ES-ESR).

CONTRACTOR SHALL AVOID DAMAGING ANY REINFORCING STEEL OR POST-TENSIONED TENDONS WHEN DRILLING HOLES FOR POST-INSTALLED ANCHORS.

**SLAB-ON-GRADE POST-TENSIONED CONCRETE**

ALL POST-TENSIONING MATERIAL AND ANCHORAGES SHALL CONFORM TO THE REQUIREMENTS OF THE POST TENSIONING INSTITUTE (PTI) "SPECIFICATIONS FOR UNBONDED SINGLE STRAND TENDONS."

ALL TENDONS SHALL BE FABRICATED FROM 1/2" Ø, 270 KSI LOW-RELAXATION STRANDS CONFORMING TO THE REQUIREMENTS OF ASTM A418. TENDONS SHALL BE COATED WITH A PERMANENT RUST PREVENTATIVE LUBRICANT AND PLASTIC SHEATH OF AT LEAST 0.025" THICK.

DEAD ENDS AND LIVE ENDS MAY BE REVERSED IN THE FIELD AT THE CONTRACTOR'S OPTION.

AT DEAD ENDS, TENDON SHEATHING MAY BE CUT BACK AS MUCH AS 12" FROM THE ANCHOR LOCATION.

AT LIVE ENDS, SHEATHING MAY BE CUT BACK A MAXIMUM OF 2", TAPING IS SUFFICIENT FOR PATCHING OR REPLACEMENT OF SHEATHING.

MINIMUM CLEARANCE OF 3" SHALL BE MAINTAINED BETWEEN TENDONS AROUND ALL BLOCK-OUTS AND PIPE PENETRATIONS.

ALL CONCRETE SHALL BE WELL CONSOLIDATED.

CONCRETE FOR POST-TENSIONED FOUNDATIONS SHALL ACHIEVE A MINIMUM STRENGTH OF 1,500 PSI PRIOR TO FULL TENDON ELONGATIONS. TENDONS SHALL BE STRESSED AS EARLY AS POSSIBLE AFTER THIS STRENGTH HAS BEEN OBTAINED TO REDUCE SHRINKAGE CRACKING.

TENDONS SHALL BE ANCHORAGED AT 28.7 KIPS. THESE TENDONS MAY BE INITIALLY AND TEMPORARILY STRESSED AT 33 KIPS IN ORDER TO OVERCOME FRICTION AND COMPENSATE FOR SEATING LOSSES.

EACH TENDON ELONGATION SHALL BE MEASURED AND COMPARED TO THE REQUIRED ELONGATIONS. AN ELONGATION REPORT SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO CUTTING AND GROUTING LIVE ENDS.

TENDON STRESSING AND FINISHING SHALL FOLLOW THE PROCEDURES REQUIRED IN PTIS "CONSTRUCTION AND MAINTENANCE PROCEDURES MANUAL FOR POST-TENSIONED SLAB-ON-GRADE CONSTRUCTION," LATEST EDITION.

**WOOD FRAMING - WALLS**

LOAD BEARING WALL STUDS SHALL BE AS SCHEDULED ON THE STRUCTURAL DRAWINGS.

HEADERS FOR 2x6 WOOD FRAMED WALLS TO BE (3) 2x8, UNLESS NOTED OTHERWISE.

HEADERS FOR 2X4 WOOD FRAMED WALLS TO BE (2) 2x8, UNLESS NOTED OTHERWISE.

STUD WALLS SHALL BE BRACED UNTIL ALL DECKING, SHEAR PANELS AND ROOF TRUSSES ARE IN PLACE. PROVIDE 2x BLOCKING AT MID HEIGHT OF ALL BEARING WALLS OR AT 4'-0" MAXIMUM SPACING.

WOOD BEAMS AND MULTI-PLY TRUSSES SHALL BEAR ON MULTIPLE STUDS AND HAVE THE SAME NUMBER OF STUDS AS NUMBER OF BEAM/TRUSS PLIES (2 MINIMUM), UNLESS A GREATER NUMBER IS NOTED ON THE DRAWINGS. CONTINUE STUD PACKS DOWN TO FOUNDATION.

JAMB, BEAM, MULTI-PLY TRUSS SUPPORT STUDS/POSTS SHALL BE CONTINUOUS DOWN TO FOUNDATION OR TRANSFER BEAM/HEADER BELOW. PROVIDE VERTICAL 2x STUDS OR SOLID BLOCKING THROUGH JOIST/TRUSS CAVITY TO MATCH STUD GROUP ABOVE. AT BEAMS, ADD SUFFICIENT BLOCKS TO EACH END OF BEAM TO MATCH STUD GROUP ABOVE.

WALL STUDS SHALL BE CONTINUOUS BETWEEN FLOORS, CEILING, AND ROOF.

WHERE EXTERIOR WALLS SPAN TWO STORIES WITHOUT INTERMEDIATE SUPPORT FROM A FLOOR, STUDS SHALL BE FULL LENGTH FOR THE TWO STORIES. STUDS SHALL NOT BE SPLICED AT THE "PLATE HEIGHT" WHEN NO ROOF, CEILING, OR FLOOR PROVIDES LATERAL SUPPORT AT THAT HEIGHT.

EXTERIOR WALLS SHALL BE COMPLETELY SHEATHED IN ACCORDANCE WITH THE LOWEST SHEARWALL SHEATHING REQUIREMENTS.

EXTERIOR SHEATHING SHALL BE MADE CONTINUOUS FROM FOUNDATION TO ROOF AND HORIZONTALLY CONTINUOUS FROM BUILDING CORNER TO BUILDING CORNER AND AROUND OPENINGS BY NAILING TO STUDS, BLOCKING, ETC. IN WALL TO CONTINUE THROUGH INTERSECTING WALLS AND FLOORS.

INTERIOR SHEATHING AT SHEAR WALLS SHALL BE MADE CONTINUOUS FROM FOUNDATION TO TOP OF SHEAR WALL AND HORIZONTALLY CONTINUOUS FROM END OF SHEAR WALL TO END OF SHEAR WALL BY FASTENING TO STUDS, BLOCKING, ETC. IN WALL TO CONTINUE THROUGH INTERSECTING WALLS AND FLOORS.

REFER TO PLANS FOR SHEARWALL LOCATIONS, DETAILS AND ANCHORAGE REQUIREMENTS.

WALL DOUBLE TOPPED PLATES SHALL BE LAPPED AT CORNERS AND INTERSECTIONS. ALL DOUBLE PLATE END JOINTS SHALL BE OFFSET AT LEAST 48", UNO.

SILL, SOLE, AND MUD PLATES SHALL BE SECURED IN ACCORDANCE WITH THE ANCHORING SCHEDULE.

**WOOD SHEATHING:**

ROOF, WALL, AND FLOOR SHEATHING SHALL BE AS SCHEDULED IN THE STRUCTURAL DOCUMENTS.

ORIENTED STRAND BOARD (OSB) MAY BE USED INTERCHANGEABLY WITH PLYWOOD AT VERTICAL APPLICATIONS.

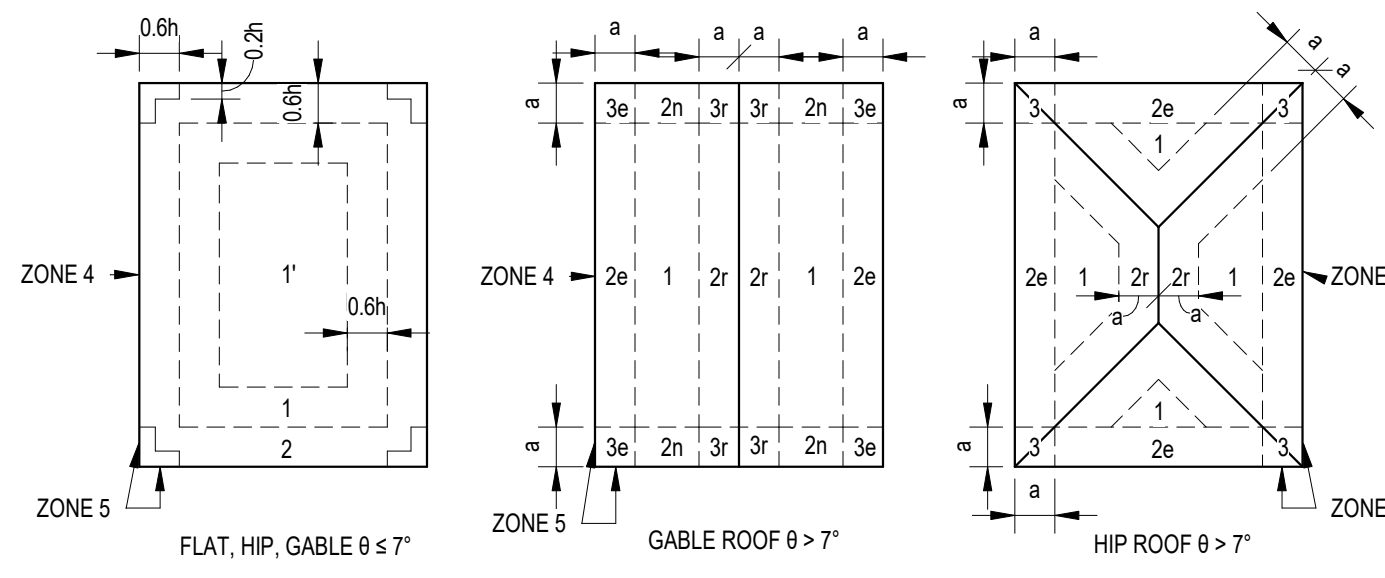
ALL WOOD SHEATHING WITHIN 4'-0" ON EACH SIDE OF FIRE WALLS SHALL BE FIRE RETARDANT.

INTERIOR GYPSUM SHEARWALLS SHALL BE 5/8" THICK TYPE X GYPSUM REGULAR CONFORMING TO THE REQUIREMENTS OF ASTM C 36 AND INSTALLED PER GA-216.

REFER TO ARCHITECTURAL DRAWINGS FOR PROPOSED LOCATIONS OF FIRE RETARDANT PLYWOOD AT ROOF DECKING.

PROVIDE 1/4" GAPS EVERY 80 FEET IN PLYWOOD DECKING OR PLYWOOD RUNS LONGER THAN 80 FEET.

ALL ROOF SHEATHING SHALL RUN CONTINUOUS BELOW ALL DORMERS, CUPOLAS, AND VALLEY / BUILT-UP TRUSSES UNLESS NOTED OTHERWISE. ANCHOR VALLEY TRUSSES TO SUPPORTING ELEMENTS BELOW WITH SIMPSON VTOR AT 24" O.C. MAXIMUM.



**GENERAL NOTES:**

1. PRESSURES SHOWN IN THE SCHEDULE ARE ULTIMATE LEVEL (FACTORED) LOADS PER ASCE 7-16 AND IBC 2021, WHERE SERVICE LEVEL (UNFACTORED) WIND PRESSURES ARE NEEDED FOR DESIGN, THE TABLE PRESSURES SHALL BE MULTIPLIED BY A FACTOR OF 0.60.
2. TABLE PRESSURES ARE FOR THE EFFECTIVE AREA OF COMPONENTS SHOWN. FOR OTHER EFFECTIVE AREAS, LINEAR INTERPOLATE BETWEEN THE TABULATED VALUES.
3. POSITIVE PRESSURE INDICATES PRESSURE ACTING TOWARDS THE BUILDING. NEGATIVE PRESSURES ACT AWAY FROM THE BUILDING.
4. A MINIMUM DEAD LOAD EQUAL TO 5 PSF MAY BE INCLUDED FOR NET ROOF UPLIFT PRESSURES.

h = MEAN ROOF HEIGHT  
 a = 10% OF THE LEAST HORIZONTAL DIMENSION OR 0.4h, WHICHEVER IS SMALLER, BUT NOT LESS THAN EITHER 4% OF THE LEAST HORIZONTAL DIMENSION OR 3 FT

COMPONENTS AND CLADDING DESIGN PRESSURES				
ZONE	ROOF (PSF)			
	10 SF	25 SF	50 SF	100 SF
1	32 / -59	27.5 / -47	24 / -38	20 / -29
2	-	-	-	-
3	-	-	-	-
1'	-	-	-	-
2e	32 / -59	27.5 / -47	24 / -38	20 / -29
2n	32 / -65	27.5 / -56	27 / -49	20 / -42
2r	32 / -89	27.5 / -47	27 / -38	20 / -29
3e	32 / -80	27.5 / -68	27 / -59	20 / -50
3r	32 / -65	27.5 / -56	27 / -49	20 / -42
WALLS (PSF)				
ZONE	10 SF	25 SF	50 SF	100 SF
4	35 / -38	33 / -36	31.5 / -34.5	30 / -33
5	35 / -47	33 / -43	31.5 / -40	30 / -37