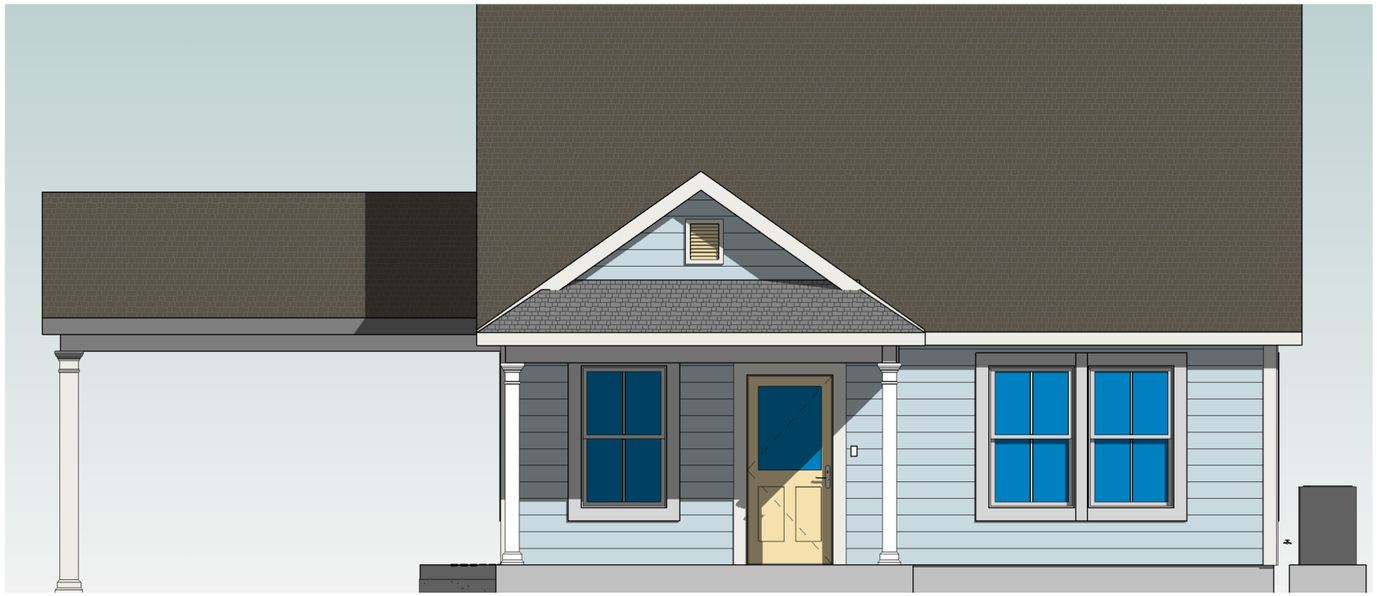
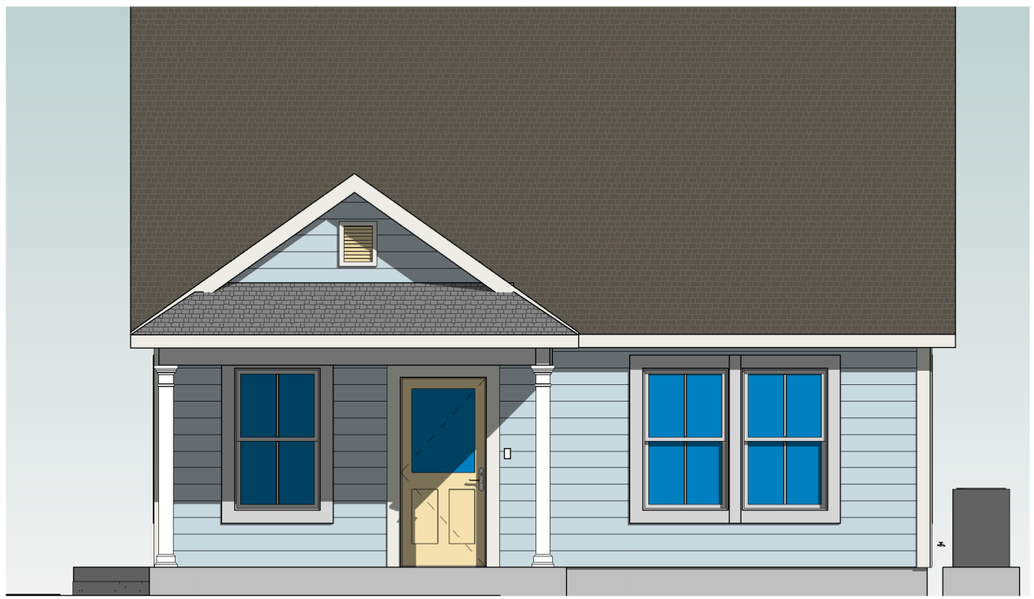
Adoration Place - 4 Bedroom - VER. 3



4BR Version 3 Option 2



4BR Version 3 Option 1

INDEX OF DRAWINGS

ARCHITECTURAL

CODE INFORMATION

This 3 bedroom slab-on-grade foundation residential unit has been designed in accordance with the requirements of the following codes.

International Residential Code (IRC) 2021

Basic Wind Speed per figure R301.2.1.1: 140 MPH
Exposure Category: B
(Direct shoreline placement of structure is not assumed. If this placement is to occur, the Architect and Engineer must be notified for re-evaluation of structure.)

Contractor must provide Protection of Openings in accordance with section R301.2.1.2. If wood structural panels are provided for openings, they are to be fastened in accordance with Table R301.2.1.2.

If applicable to home construction, pre-engineered wood trusses shall be in accordance with IRC 2021 Section R802.10.

ANSI/ AF&PA WFCM-2018
Tables provided on sheets A410 & A411

SYMBOL LEGEND

DRAWING NUMBER / DRAWING TITLE / SHEET NUMBER / DRAWING SCALE — DRAWING TITLE

DRAWING NUMBER / SHEET NUMBER — EXTERIOR ELEVATION REFERENCE MARK

DRAWING NUMBER / SHEET NUMBER — INTERIOR ELEVATION REFERENCE MARK

DRAWING NUMBER / SHEET NUMBER — DETAIL REFERENCE MARK

DRAWING NUMBER / SHEET NUMBER — SECTION REFERENCE MARK

DRAWING NUMBER / SHEET NUMBER — DETAIL SECTION REFERENCE MARK

DOOR NUMBER — DOOR INFORMATION

WINDOW MARK — WINDOW INFORMATION

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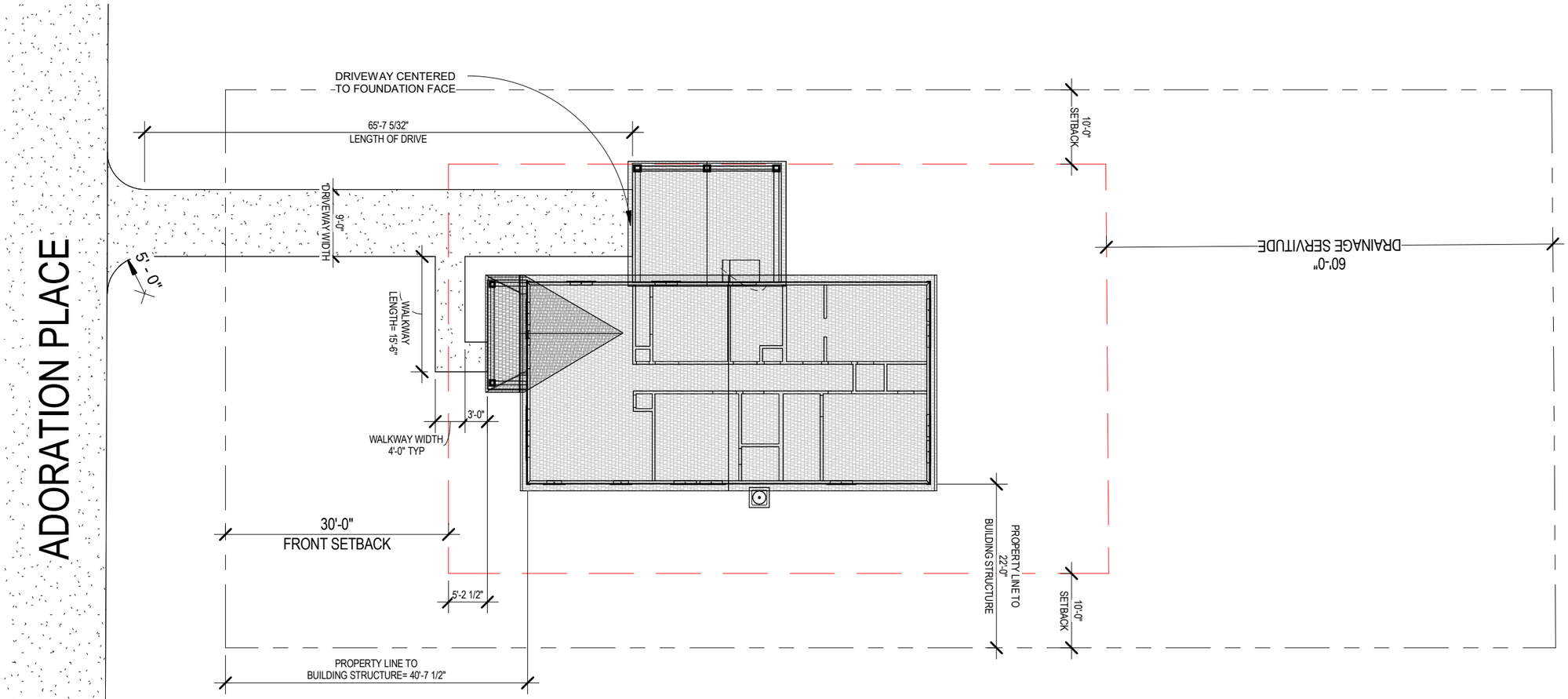
job no.
2440

sheet title
cover sheet

sheet no.
A000



1 Site Plan
A010
1/16" = 1'-0"

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2440

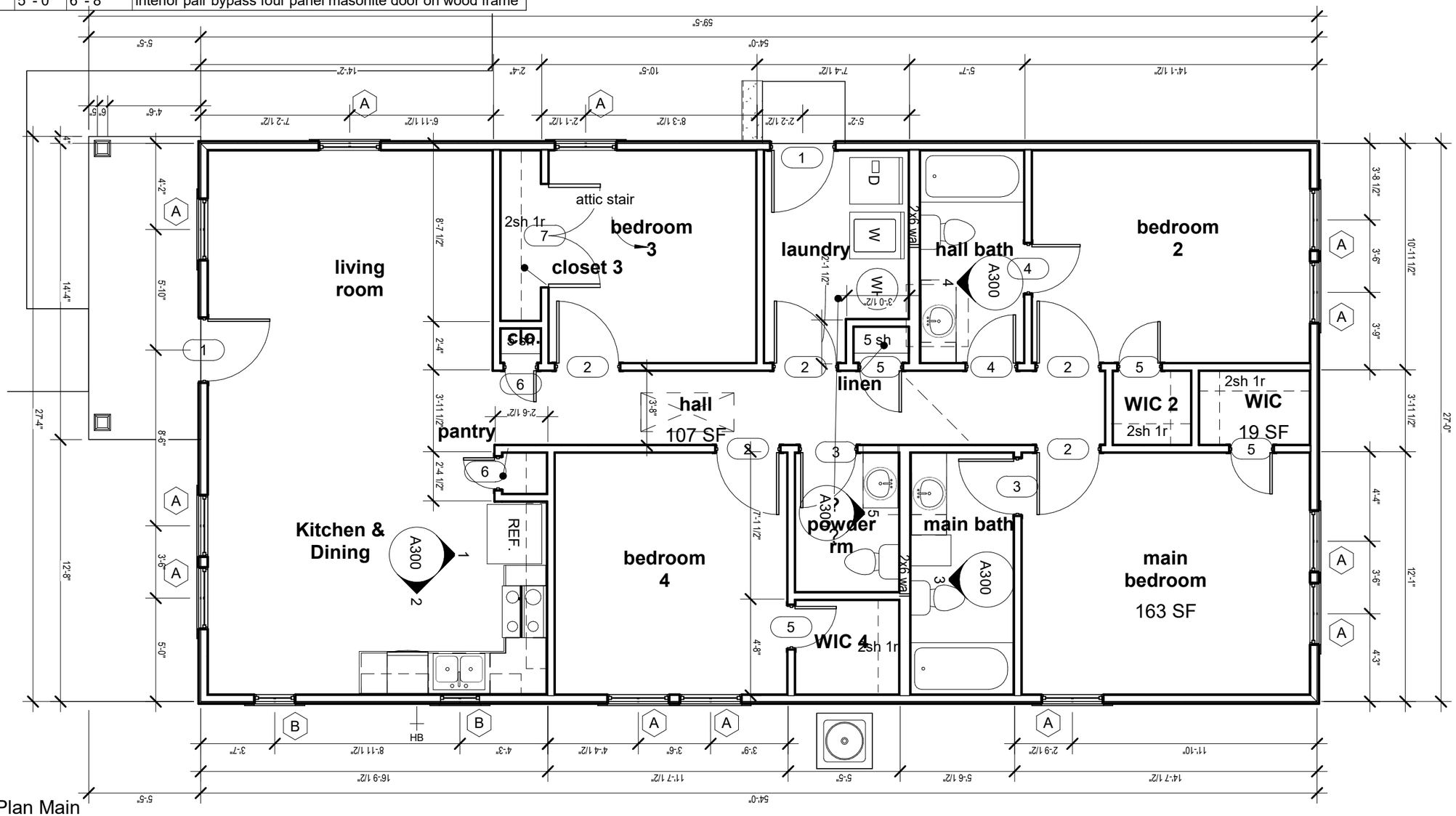
sheet title
site plan

sheet no.
A010

Door Schedule				
Type Mark	Count	Door Width	Door Height	Door Material
1	2	3' - 0"	6' - 8"	exterior two panel fiberglass door with half lite on wood frame
2	5	3' - 0"	6' - 8"	interior four panel masonite door on wood frame
3	2	2' - 8"	6' - 8"	interior four panel masonite door on wood frame
4	2	2' - 4"	6' - 8"	interior four panel masonite door on wood frame
5	4	2' - 0"	6' - 8"	interior four panel masonite door on wood frame
6	2	1' - 6"	6' - 8"	interior four panel masonite door on wood frame
7	1	5' - 0"	6' - 8"	interior pair bypass four panel masonite door on wood frame

Window Schedule					
Type Mark	Count	Width	Height	Head Height	Description
A	12	3' - 0"	5' - 0"	7' - 0"	2 over 2 insulated vinyl single hung: must meet 5.7 egress
B	2	2' - 0"	3' - 0"	<varies>	2 over 2 insulated vinyl single hung

Gross Area	
Name	Area
A/C Arear	1439 SF
Front Porch	78 SF
Gross Area Total	1517 SF



1 Floor Plan Main
A100
3/16" = 1'-0"

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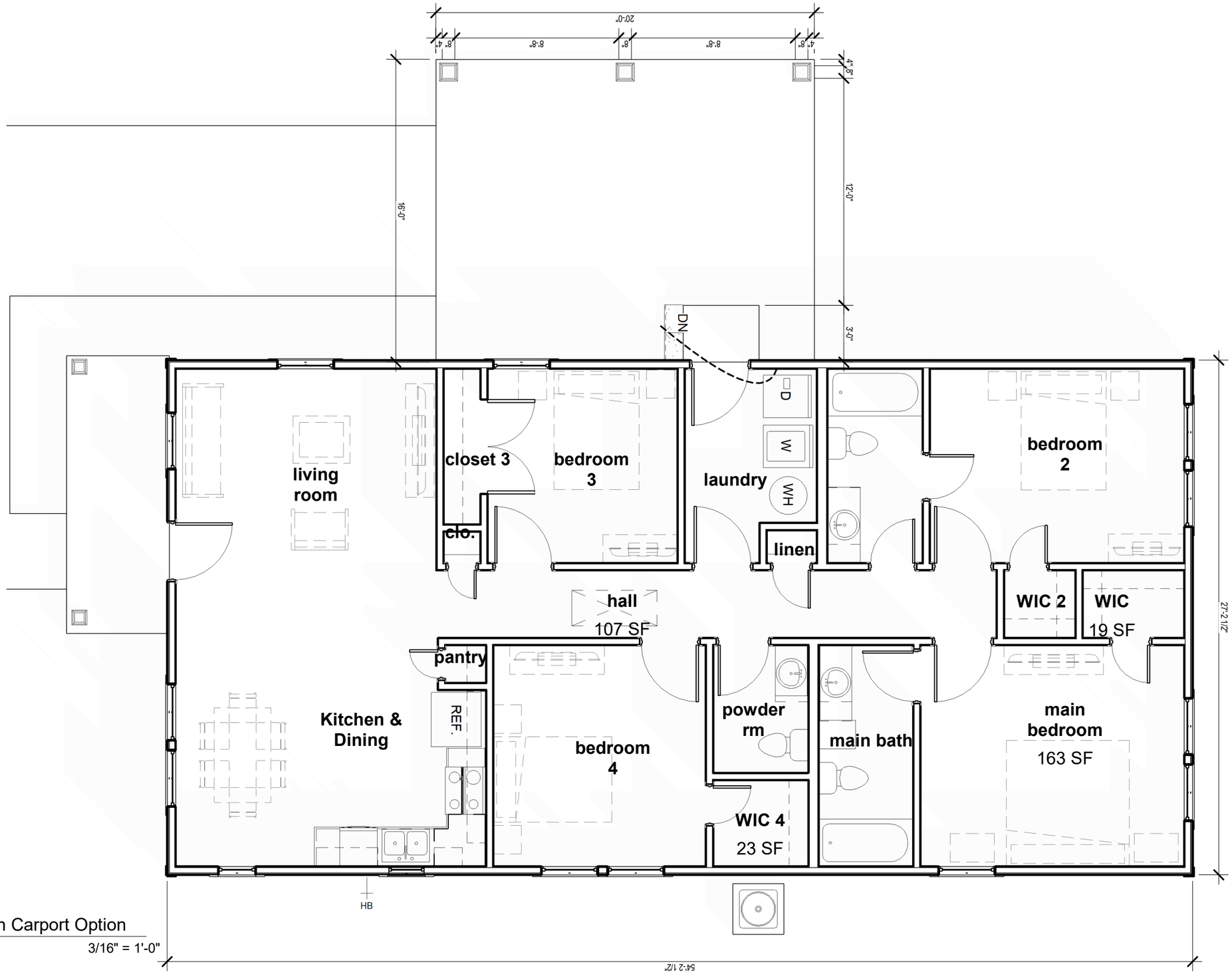
job no.
2440

sheet title
4BR floor plan
ver. 3 main

sheet no.
A100



1 Floor Plan Carport Option
A110
3/16" = 1'-0"

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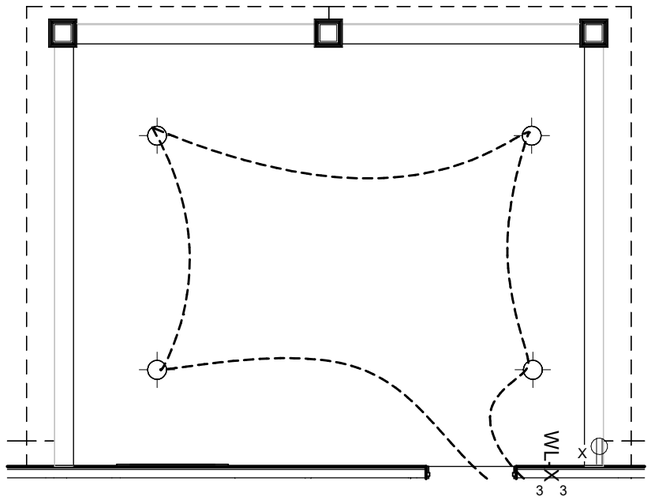
job no.
2440

sheet title
floor plan ver. 3
carport op

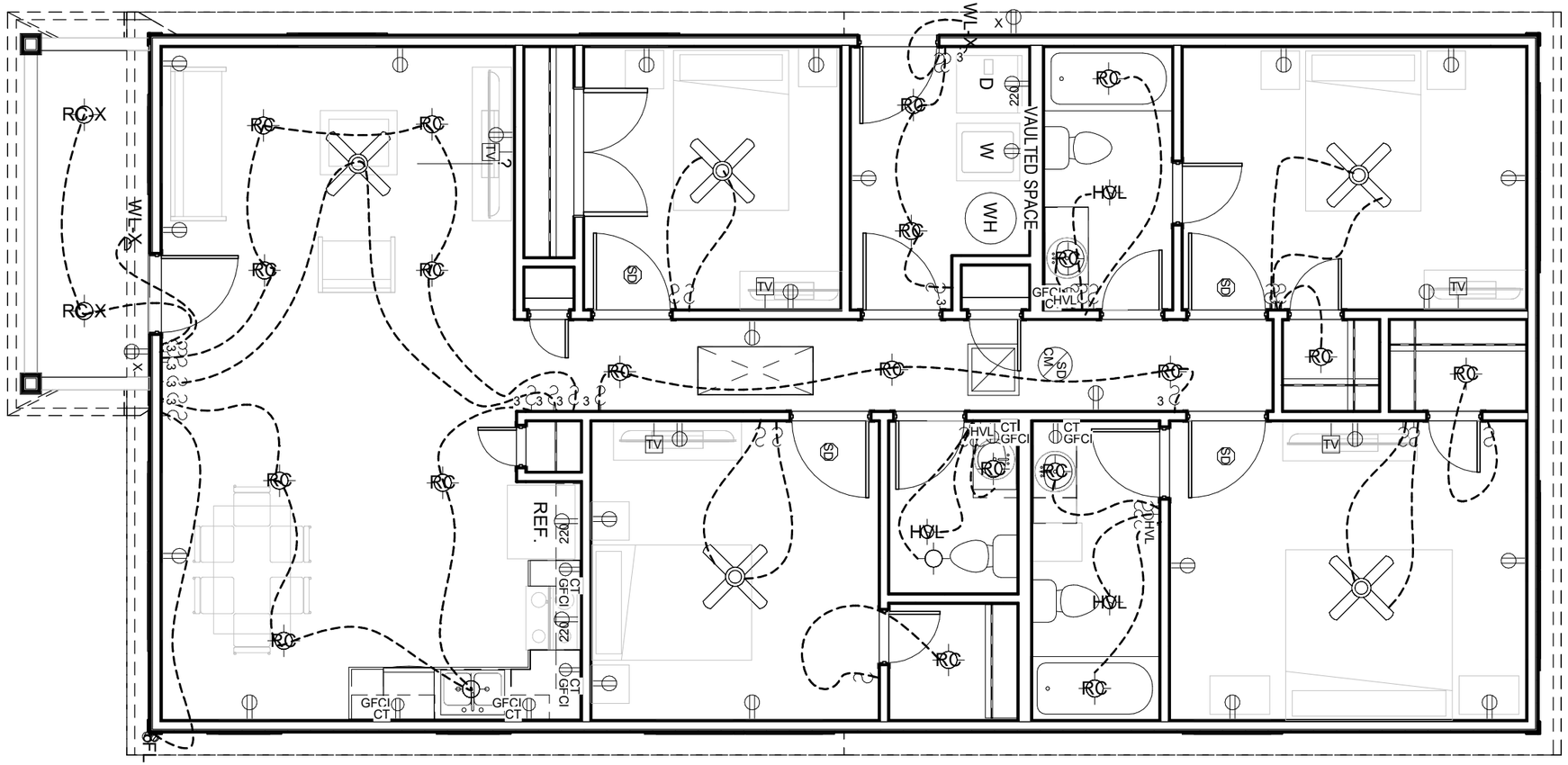
sheet no.
A110



2 Lighting/Power Plan Carport Option

A130

3/16" = 1'-0"



1 Lighting/Power Plan

A130

3/16" = 1'-0"

LIGHTING SYMBOLS

MARK	DESCRIPTION
S	SWITCH
S3	3 WAY SWITCH
SD	SWITCH/ DIMMER
SVL	SWITCH/ VENT & LIGHT
SHVL	SWITCH/ HEATER/VENT & LIGHT
V	VENT
VL	VENT W/ LIGHT
HVL	HEATER/VENT W/ LIGHT
PC	PENDANT CAN LIGHT FIXTURE
RC	RECESSED CAN LIGHT FIXTURE
RC-X	RECESSED CAN LIGHT FIXTURE, EXTERIOR
RC MR	RECESSED CAN LIGHT FIXTURE W/ MOISTURE RESISTANT LENS COVER
RC DS	RECESSED CAN LIGHT W/ DIRECTIONAL SPOTLIGHT FIXTURE
WL	WALL LIGHT
WL-X	EXTERIOR GRADE WALL LIGHT
PF	PENDANT FIXTURE
SM	SURFACE MOUNTED FIXTURE
MD	FLOOD LIGHTS WITH MOTION DETECTOR
SD	SMOKE DETECTOR. HARDWIRE INTERCONNECTED W/ BATTERY BACKUP
CM SD	COMBO SMOKE AND CARBON MONOXIDE DETECTOR. HARDWIRE INTERCONNECTED W/ BATTERY BACKUP.
	CONTINUOUS LED TAPE LIGHTING
WLK	CEILING FAN W/LIGHT KIT

ELECTRICAL SYMBOLS

MARK	DESCRIPTION
	DUPLEX OUTLET. MOUNT AT 12" AFF, TYPICAL U.N.O.
	QUADRUPLEX OUTLET. MOUNT AT 12" AFF, TYPICAL U.N.O.
CT	DUPLEX OUTLET, MOUNT ABOVE COUNTERTOP
GFCI	DUPLEX OUTLET, GROUND FAULT CIRCUIT INTERRUPT, MOUNT AT 12" AFF, TYPICAL U.N.O.
220	220v OUTLET
R	RANGE OUTLET
GFCI CT	DUPLEX OUTLET, GROUND FAULT CIRCUIT INTERRUPT MOUNT ABOVE COUNTERTOP
X	DUPLEX OUTLET, EXTERIOR GRADE
	RECESSED FLOOR OUTLET, FLUSH W/ FINISHED FLOOR
T	THERMOSTAT
T	TELEPHONE OUTLET. WALL MOUNTED AT 42"
DATA	CATEGORY 5 DATA CABLE OUTLET. MOUNT AT STANDARD HEIGHT
TV	CABLE TV OUTLET. MOUNT AT STANDARD HEIGHT
J	JUNCTION BOX
JX	JUNCTION BOX/ EXTERIOR GRADE
AC	AIR CONDITION CONDENSER UNIT SERVICE W/ DISCONNECT
	GAS

1. COORDINATE EXACT LOCATIONS OF POWER AND LIGHTING W/ OWNER.

3 Electrical Symbols

A130

1/8" = 1'-0"

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job no.

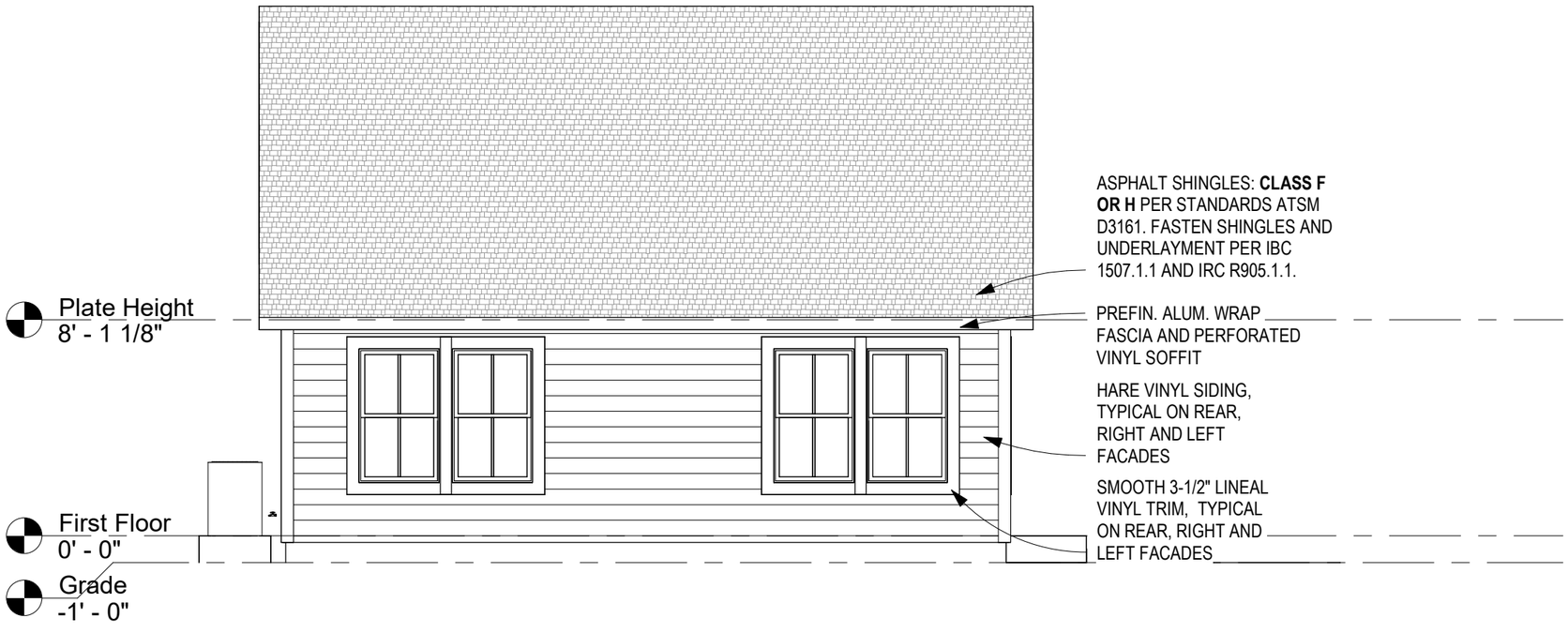
2440

sheet title

electrical & lighting base model & carport

sheet no.

A130



2 Rear Elevation Version 3
A200
3/16" = 1'-0"

1 Front Elevation Verion 3
A200
3/16" = 1'-0"

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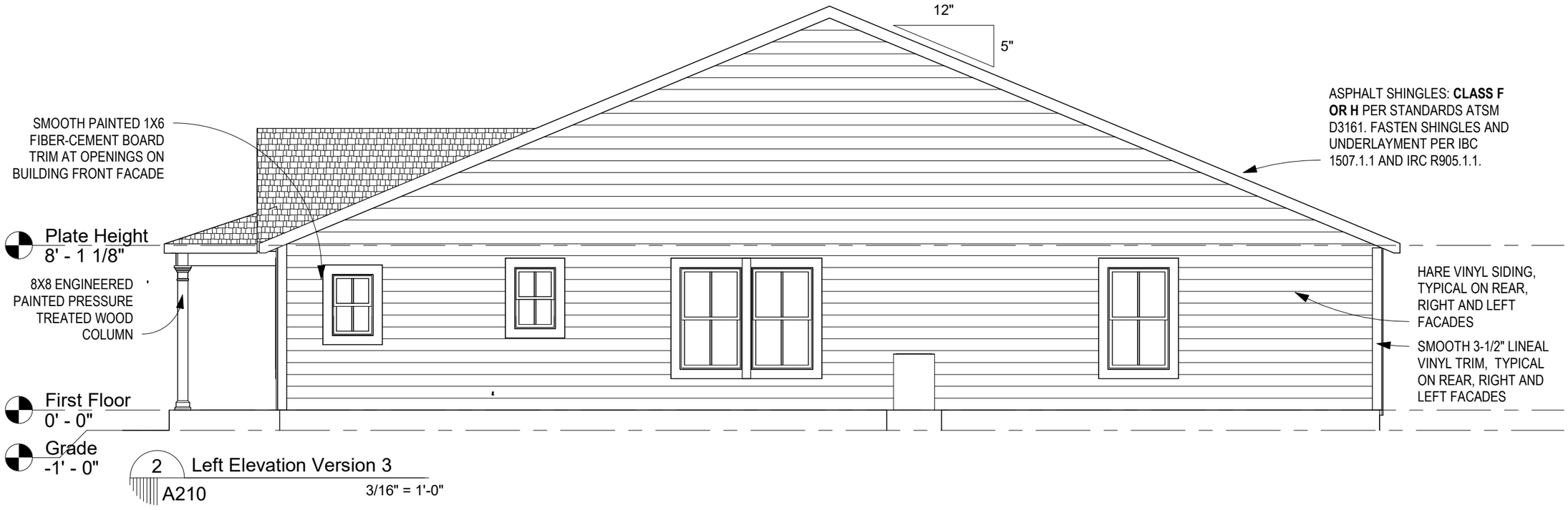
job no.
2440

sheet title
4BR ver. 3 ext elevations

sheet no.
A200

3 Right Elevation Version 3
A210
3/16" = 1'-0"



2 Left Elevation Version 3
A210
3/16" = 1'-0"

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job no.
2440

sheet title
4BR ver. 3 ext elevations

sheet no.
A210

2 A220 Rear Elevation Version 3 Carport

3/16" = 1'-0"

1 A220 Front Elevation Verion 3 Carport

3/16" = 1'-0"

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job no.
2440

sheet title
4BR ver. 3 ext elev carport

sheet no.
A220

1 Right Elevation Version 3 Carport
A230 3/16" = 1'-0"

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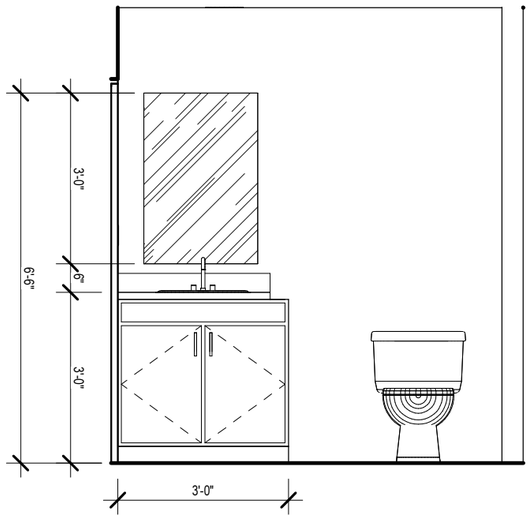
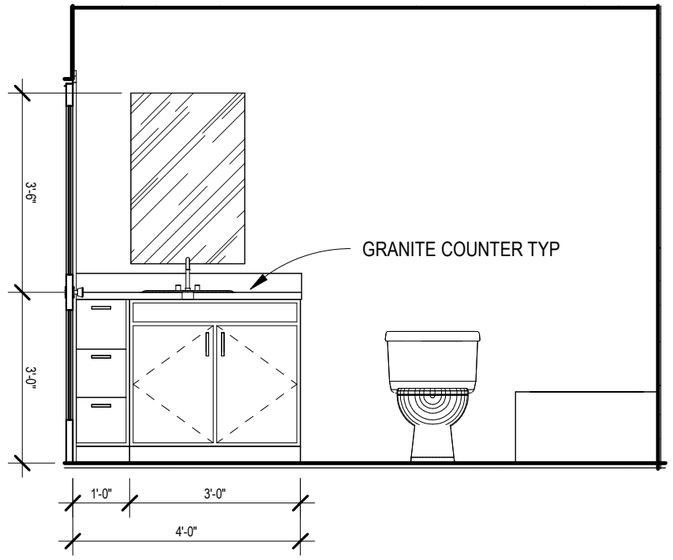
job no.
2440

sheet title
4BR ver. 3 ext elev carport

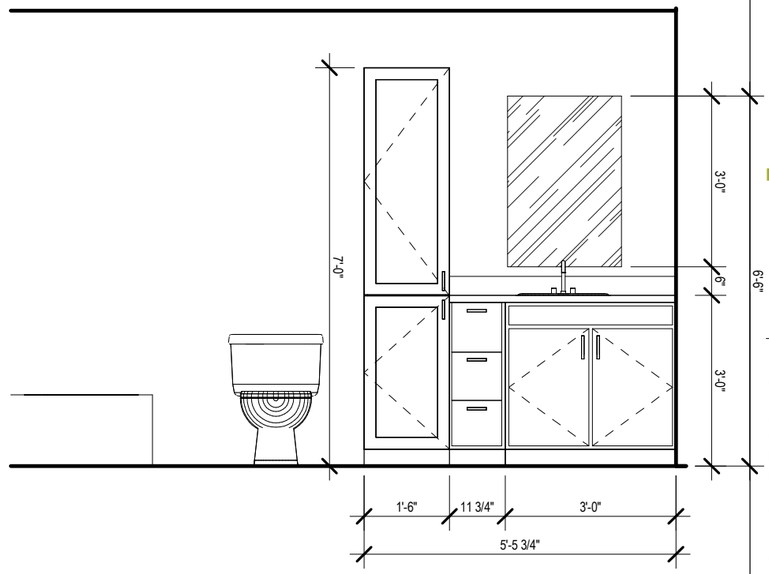
sheet no.
A230



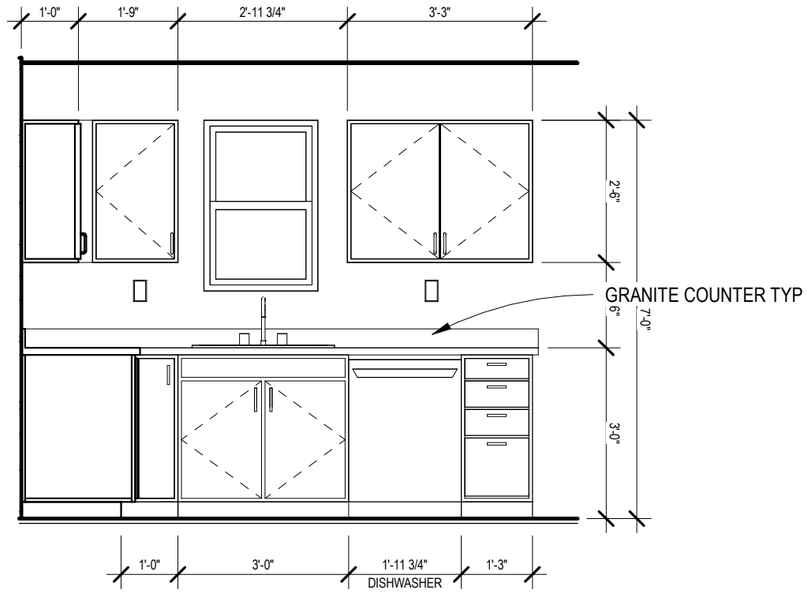
5 Powder Room Interior Elevation
A300 3/8" = 1'-0"



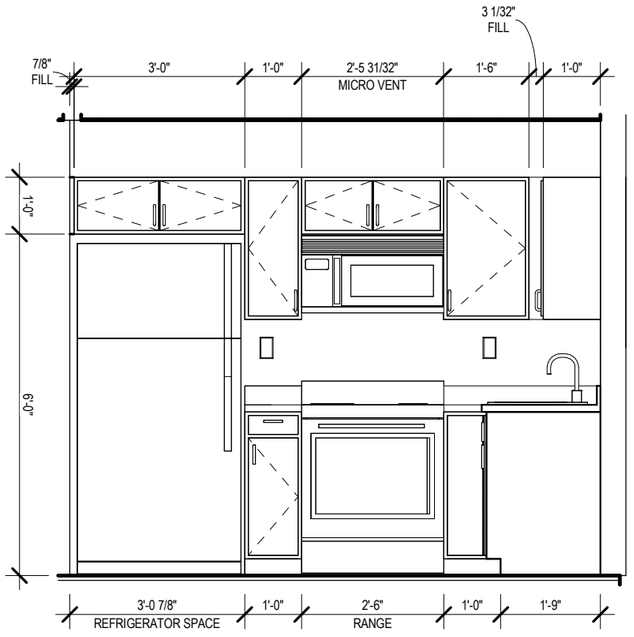
4 Bath Interior Elevation
A300 3/8" = 1'-0"



3 Primary Bath Interior Elevation
A300 3/8" = 1'-0"



2 Kitchen Interior Elevation B
A300 3/8" = 1'-0"



1 Kitchen Interior Elevation A
A300 3/8" = 1'-0"

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job no.
2440

sheet title
interior elevations

sheet no.
A300

International Residential Codes:

R301.2.1.2 Protection of Openings. Exterior glazing in buildings located in windborne debris regions shall be protected from windborne debris. Glazed opening protection for windborne debris shall meet the requirements of the Large Missile Test of ASTM E 1996 and ASTM E 1886 as modified in Section 301.2.1.2.1. Garage door glazed opening protection for windborne debris shall meet the requirements of an approved impact-resisting standard or ANSI/DASMA 115.

Exception: Wood structural panels with a thickness of not less than 7/16 inch (11 mm) and a span of not more than 8 feet (2438 mm) shall be permitted for opening protection. Panels shall be precut and attached to the framing surrounding the opening containing the product with the glazed opening. Panels shall be predrilled as required for the anchorage method and shall be secured with the attachment hardware provided. Attachments shall be designed to resist the component and cladding loads determined in accordance with either Table R301.2(2) or ASCE 7, with the permanent corrosion-resistant attachment hardware provided and anchors permanently installed on the building. Attachment in accordance with Table R301.2.1.2 is permitted for buildings with a mean roof height of 45 feet (13, 728 mm) or less where the ultimate design wind speed, Vult, is 180 mph (290 kph) or less.

R302.11 Fireblocking. In combustible construction, fireblocking shall be provided to cut off both vertical and horizontal concealed draft openings and to form an effective fire barrier between stories, and between a top story and the roof space.

Fireblocking shall be provided in wood-framed construction in the following locations:

1. In concealed spaces of stud walls and partitions, including furred spaces and parallel rows of studs or staggered studs, as follows:
1.1. Vertically at the ceiling and floor levels.
1.2. Horizontally at intervals not exceeding 10 feet (3048 mm).
2. At interconnections between concealed vertical and horizontal spaces such as occur at soffits, drop ceilings and cove ceilings.
3. In concealed spaces between stair stringers at the top and bottom of the run. Enclosed spaces under stairs shall comply with Section R302.7.
4. At openings around vents, pipes, ducts, cables and wires at ceiling and floor level, with an approved material to resist the free passage of flame and products of combustion. The material filling this annular space shall not be required to meet the ASTM E 136 requirements.
5. For the fireblocking of chimneys and fireplaces, see Section R1003.19.
6. Fireblocking of cornices of a two-family dwelling is required at the line of dwelling unit separation.

R307.2 Bathtub and Shower Spaces. Bathtub and shower floors and walls above bathtubs with installed shower heads and in shower compartments shall be finished with a nonabsorbent surface. Such wall surfaces shall extend to a height of not less than 6 feet (1829 mm) above the floor.

R310.1 Emergency Escape and Rescue Required. Basements, habitable attics and every sleeping room shall have not less than one operable emergency escape and rescue opening. Where basements contain one or more sleeping rooms, an emergency escape and rescue opening shall be required in each sleeping room. Emergency escape and rescue openings shall open directly into a public way, or to a yard or court that opens to a public way.

Exception: Storm shelters and basements used only to house mechanical equipment not exceeding a total floor area of 200 square feet (18.58 m2).

R310.1.1 Minimum Opening Area. Emergency and escape rescue openings shall have a net clear opening of not less than 5.7 square feet (0.530 m2).

Exception: Grade floor or below grade openings shall have a net clear opening of not less than 5 square feet (0.465 m2).

R311.3 Floors and landings at exterior doors. There shall be a landing or floor on each side of each exterior door. The width of each landing shall be not less than the door served. Every landing shall have a dimension of not less than 36 inches (914 mm) measured in the direction of travel. The slope at exterior landings shall not exceed 1/4 unit vertical in 12 units horizontal (2 percent).

Exception: Exterior balconies less than 60 square feet (5.6 m2) and only accessible from a door are permitted to have a landing less than 36 inches (914 mm) measured in the direction of travel.

R312.2 Window fall protection. Window fall protection shall be provided in accordance with Sections R312.2.1 and R312.2.2.

R312.2.1 Window Sills. In dwelling units, where the top of the sill of an operable window opening is located less than 24 inches (610 mm) above the finished floor and greater than 72 inches (1829 mm) above the finished grade or other surface below on the exterior of the building, the operable window shall comply with one of the following:

1. Operable windows with openings that will not allow a 4-inch-diameter (102 mm) sphere to pass through the opening where the opening is in its largest opened position.
2. Operable windows that are provided with window fall prevention devices that comply with ASTM F 2090.
3. Operable windows that are provided with window opening control devices that comply with Section R312.2.2.

R314.3 Smoke Alarm Locations: Smoke alarms shall be installed in the following locations:

1. In each sleeping room.
2. Outside each separate sleeping area in the immediate vicinity of the bedrooms.
3. On each additional story of the dwelling, including basements and habitable attics and not including crawl spaces and uninhabitable attics. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full story below the upper level.
4. Smoke alarms shall be installed not less than 3 feet (914 mm) horizontally from the door or opening of a bathroom that contains a bathtub or shower unless this would prevent placement of a smoke alarm required by Section R314.3.

R314.4 Interconnection. Where more than one smoke alarm is required to be installed within an individual dwelling unit in accordance with Section R314.3, the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of the alarms in the individual dwelling unit. Physical interconnection of smoke alarms shall not be required where listed wireless alarms are installed and all alarms sound upon activation of one alarm.

R316.5.3 Attics and **R316.5.4 Crawl Spaces**. The thermal barrier specified in Section R316.4 is not required where all of the following apply:

1. Attic access is required by Section R807.1.
2. The space is entered only for purposes of repairs or maintenance.
3. The foam plastic insulation has been tested in accordance with Section R316.6 or the foam plastic insulation is protected against ignition using one of the following ignition barrier materials:
3.1. 11/2-inch-thick (38 mm) mineral fiber insulation.
3.2. 1/4-inch-thick (6.4 mm) wood structural panels.
3.3. 3/8-inch (9.5 mm) particleboard.
3.4. 1/4-inch (6.4 mm) hardboard.
3.5. 3/8-inch (9.5 mm) gypsum board.
3.6. Corrosion-resistant steel having a base metal thickness of 0.016 inch (0.406 mm).
3.7. 11/2-inch-thick (38 mm) cellulose insulation [attics only]; or
3.8. 1/4-inch (6.4 mm) fiber-cement panel, soffit or backer board [attics only].

The ignition barrier is not required where the foam plastic insulation has been tested in accordance with Section R316.6.

R319.1 Address Identification. Buildings shall be provided with approved address identification. The address identification shall be legible and placed in a position that is visible from the street or road fronting the property. Address identification characters shall contrast with their background. Address numbers shall be Arabic numbers or alphabetical letters. Numbers shall not be spelled out. Each character shall be not less than 4 inches (102 mm) in height with a stroke width of not less than 0.5 inch (12.7 mm). Where required by the fire code official, address identification shall be provided in additional approved locations to facilitate emergency response. Where access is by means of a private road and the building address cannot be viewed from the public way, a monument, pole or other sign or means shall be used to identify the structure. Address identification shall be maintained.

R401.2 Foundation Requirements. Foundation construction shall be capable of accommodating all loads in accordance with Section R301 and of transmitting the resulting loads to the supporting soil. Fill soils that support footings and foundations shall be designed, installed and tested in accordance with accepted engineering practice. Gravel fill used as footings for wood and precast concrete foundations shall comply with Section R403.

R403.1.7.3 Foundation elevation. On graded sites, the top of any exterior foundation shall extend above the elevation of the street gutter at point of discharge or the inlet of an approved drainage device a minimum of 12 inches (305 mm) plus 2 percent. Alternate elevations are permitted subject to the approval of the building official, provided it can be demonstrated that required drainage to the point of discharge and away from the structure is provided at all locations on the site.

R502.8.1 Sawn lumber.Notches in solid lumber joists, rafters and beams shall not exceed one-sixth of the depth of the member, shall not be longer than one-third of the depth of the member and shall not be located in the middle one-third of the span. Notches at the ends of the member shall not exceed one-fourth the depth of the member. The tension side of members 4 inches (102 mm) or greater in nominal thickness shall not be notched except at the ends of the members. The diameter of holes bored or cut into members shall not exceed one-third the depth of the member. Holes shall not be closer than 2 inches (51 mm) to the top or bottom of the member, or to any other hole located in the member. Where the member is also notched, the hole shall not be closer than 2 inches (51 mm) to the notch.

R502.8.2 Engineered wood products. Cuts, notches and holes bored in trusses, structural composite lumber, structural glue-laminated members, cross-laminated timber members or I-joists are prohibited except where permitted by the manufacturer s recommendations or where the effects of such alterations are specifically considered in the design of the member by a registered design professional.

R602.6 Drilling and notching-studs. Drilling and notching of studs shall be in accordance with the following:

1. Notching. Any stud in an exterior wall or bearing partition shall be permitted to be cut or notched to a depth not exceeding 25 percent of its width. Studs in nonbearing partitions shall be permitted to be notched to a depth not to exceed 40 percent of a single stud width.

2. Drilling. Any stud shall be permitted to be bored or drilled, provided that the diameter of the resulting hole is not more than 60 percent of the stud width, the edge of the hole is not more than 5/8 inch (16 mm) to the edge of the stud, and the hole is not located in the same section as a cut or notch. Studs located in exterior walls or bearing partitions drilled over 40 percent and up to 60 percent shall be doubled with not more than two successive doubled studs bored. See Figures R602.6(1) and R602.6(2).

Exception: Use of approved stud shoes is permitted where they are installed in accordance with the manufacturer s recommendations.

R602.6.1 Drilling and notching of top plate. When piping or ductwork is placed in or partly in an exterior wall or interior load-bearing wall, necessitating cutting, drilling or notching of the top plate by more than 50 percent of its width, a galvanized metal tie not less than 0.054 inch thick (1.37 mm) (16 ga) and 11/2 inches (38 mm) wide shall be fastened across and to the plate at each side of the opening with not less than eight 10d (0.148 inch diameter) nails having a minimum length of 11/2 inches (38 mm) at each side or equivalent. The metal tie must extend a minimum of 6 inches past the opening. See Figure R602.6.1.

Exception: When the entire side of the wall with the notch or cut is covered by wood structural panel sheathing.

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2440

sheet title
code notes

sheet no.
A410

FOUNDATION NOTES:

SLAB A: 4" THICK POST-TENSIONED SLAB OVER A LOW PLASTICITY COMPACTED CLAY FILL (PI BETWEEN 15-25) WITH TENDON REINFORCEMENT IN ACCORDANCE WITH THE PLAN, UNLESS NOTED OTHERWISE. PROVIDE 10 MIL VAPOR BARRIER TAPED AND SEALED UNDER ALL SLAB CONCRETE.

IN ALL AREAS OF EXPOSED CONCRETE, PLACE 6x6x10/10 WWF ON TOP OF AND TIE SECURELY TO ALL P.T. TENDONS.

VERIFY ALL DROPS, OFFSETS, AND LEDGES WITH THE ARCHITECTURAL PLANS.

ALL STRUCTURAL FILL SHALL BE COMPACTED TO 95% STANDARD PROCTOR DENSITY IN A MAXIMUM OF 8" LIFTS.

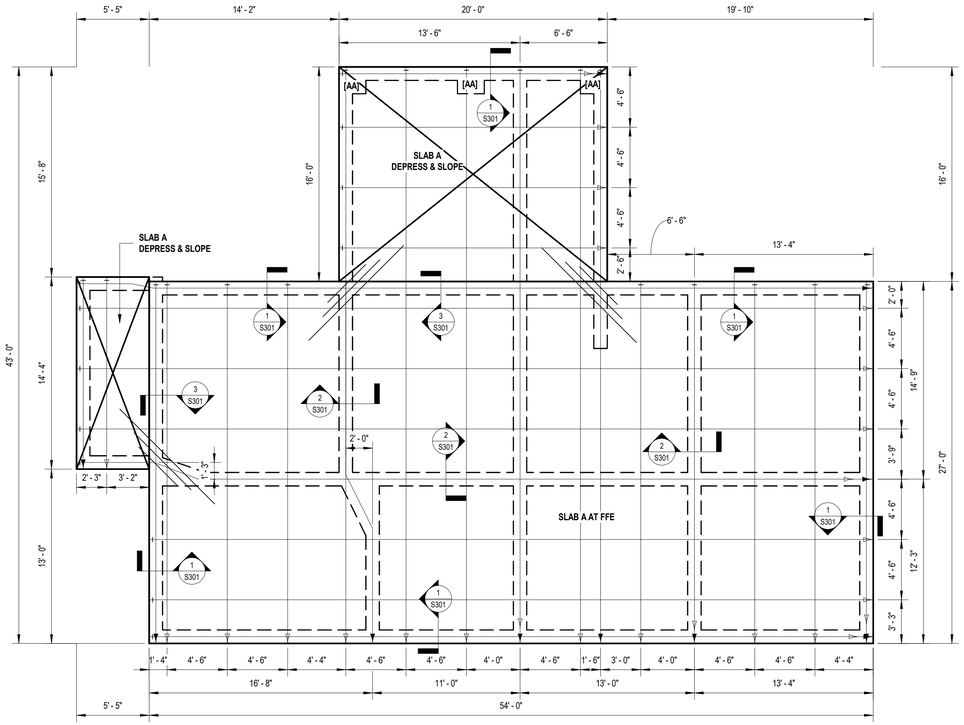
DIMENSIONS ARE TAKEN FROM EDGE OF SLAB AND CENTER OF GRADE BEAM - **REFER TO ARCHITECTURAL FOR FORM SETTING DIMENSIONS.**

REFER TO S301 FOR FOUNDATION DETAILS, NOTES, AND SCHEDULES.

TOTAL SLAB AREA = 1,855.64 SF

NOTE [AA]:

PROVIDE 24"X24"x24" DEEP CONCRETE FOOTING AT CAST IN PLACE POST BASE. RE: DETAILS

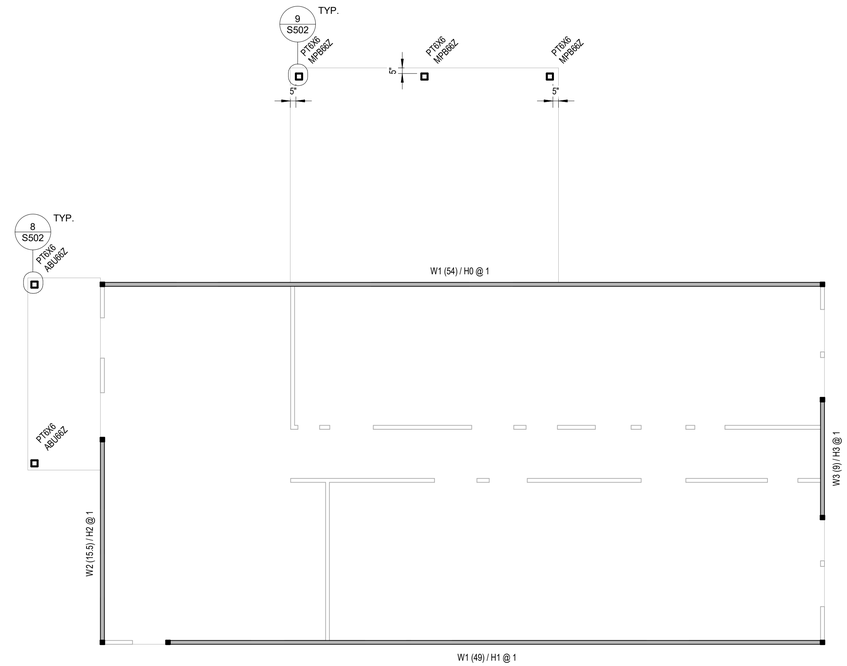


1 P.T. FOUNDATION PLAN
3/16" = 1'-0"

SHEARWALL NOTES:

REFER TO SHEET S401 FOR SHEARWALL LEGENDS, NOTES, AND SCHEDULES.

ALL EXTERIOR WALLS SHALL BE SHEATHED AS A W1 WALL MINIMUM, U.N.O.



2 SHEARWALL PLAN
3/16" = 1'-0"

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S101

SOUTHEAST ENGINEERS & LAND SURVEYORS
4880 BLUEBONNET BOULEVARD, SUITE A
BATON ROUGE, LOUISIANA 70809

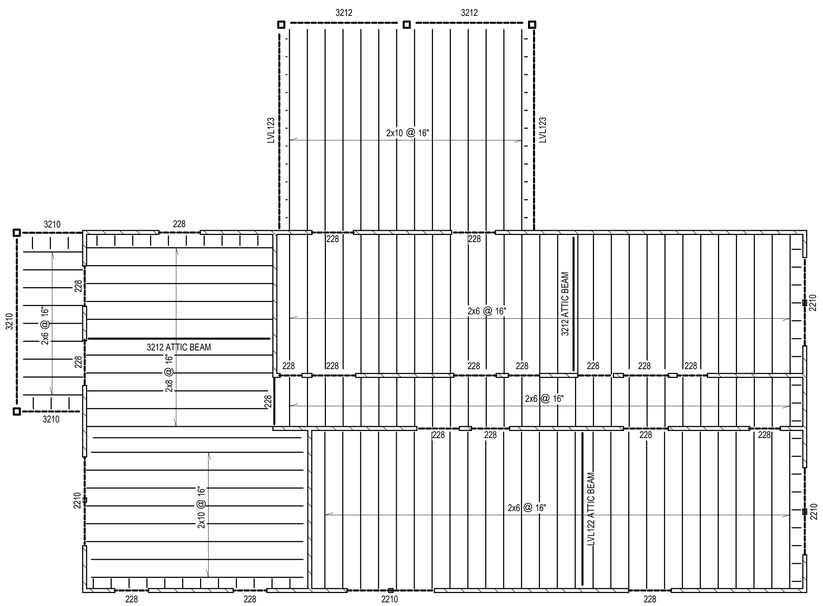
FRAMING NOTES:

REFER TO ARCHITECTURAL PLANS FOR BEARING ELEVATIONS, PITCHES, AND DIMENSIONS.

COORDINATE THE LOCATIONS OF PLUMBING SLEEVES, ROOF DRAINS, PENETRATIONS, AND GUTTERS WITH THE ARCHITECTURAL PLANS.

ALL LUMBER EXPOSED TO WEATHER SHALL BE PRESSURE TREATED.

REFER TO S401 FOR ALL FRAMING LEGENDS, NOTES, AND SCHEDULES.



1 FRAMING PLAN
3/16" = 1'-0"

ROOF FRAMING NOTES:

ALL RAFTERS SHALL BE 2x6 SOUTHERN YELLOW PINE #2 OR EQUAL (UNLESS OTHERWISE NOTED ON PLAN), SPACED AT 16" O.C. PROVIDE INTERMEDIATE BRACING FOR RAFTERS AS GIVEN BELOW:

2x6 -13'-5" MAX

RIDGE, HIP, AND VALLEY RAFTERS SHALL BE OF ADEQUATE DEPTH FOR FULL BEARING OF OPPOSING RAFTERS. USE 1.75" LVL MEMBERS FOR DEPTH GREATER THAN 11.25"

VALLEY RAFTERS SHALL BE VERTICALLY SUPPORTED EVERY 4'-0" O.C.

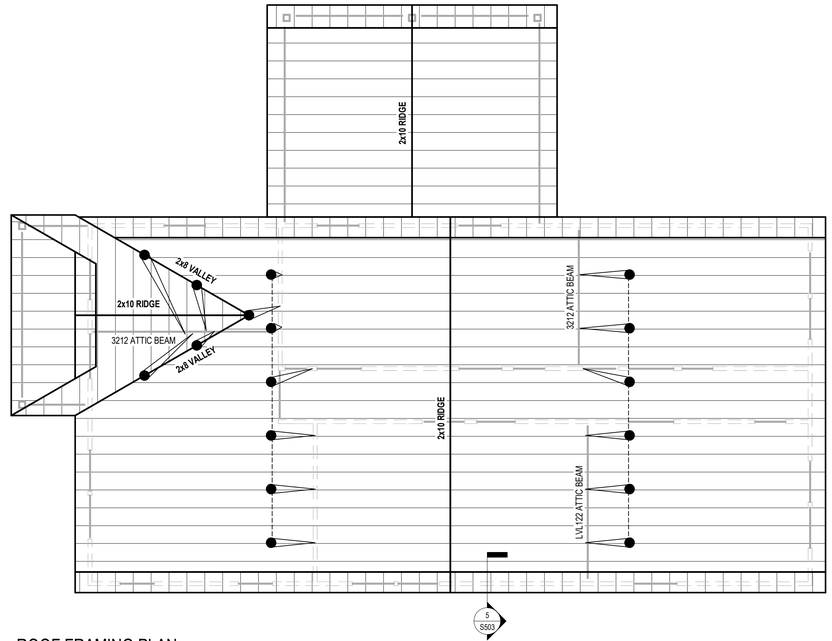
RIDGE BEAMS SHALL BE BRACED AT EACH END.

REFER TO TYPICAL ROOF BEARING UPLIFT CONNECTION DETAILS FOR STRAPPING REQUIREMENTS.

COORDINATE LOCATIONS OF CHIMNEY, PLUMBING SLEEVES, ROOF DRAINS, PENETRATIONS AND GUTTERS WITH ARCHITECTURAL PLANS.

REFER TO ARCHITECTURAL PLANS FOR BEARING ELEVATIONS, PITCHES, AND DIMENSIONS.

REFER TO S401 FOR ALL FRAMING LEGENDS, NOTES, AND SCHEDULES.



2 ROOF FRAMING PLAN
3/16" = 1'-0"

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SOUTHEAST ENGINEERS & LAND SURVEYORS
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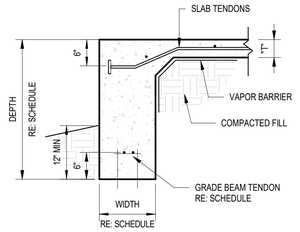
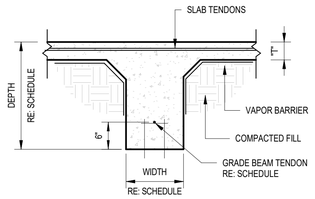
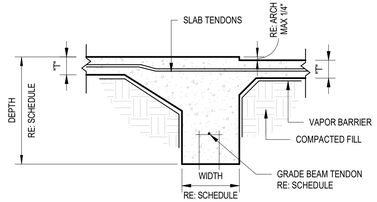
BATON ROUGE, LOUISIANA 70809



1 TYPICAL EXTERIOR GRADE BEAM
3/4" = 1'-0"



2 TYPICAL INTERIOR GRADE BEAM
3/4" = 1'-0"



3 INTERIOR GRADE BEAM AT SLAB DEPRESSION
3/4" = 1'-0"

POST-TENSION FOUNDATION GRADE BEAM SCHEDULE				
MARK	GRADE BEAM WIDTH	GRADE BEAM DEPTH	NUMBER OF GRADE BEAM TENDONS	NOTES
PT-1	12"	24"	1	

PLAN LEGEND

CONVENTIONALLY REINFORCED BEAM WITH 2 #4'S TOP AND BOTTOM	CRB
POST-TENSIONING TENDON	SLAB CABLE / GRADE BEAM CABLE
#4 SLAB REINFORCEMENT @ 18" O.C. ONE-WAY AS SHOWN	----------
#4 BARS 4', 6', 10' @ 6" O.C. TYPICAL RE-ENTRANT CORNER REINFORCEMENT	

NOTE:
ALL GRADE BEAMS TO BE PT-1 U.N.O.

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S301

SOUTHEAST ENGINEERS & LAND SURVEYORS
4880 BLUEBONNET BOULEVARD, SUITE A
BATON ROUGE, LOUISIANA 70809

BEAM AND HEADER SCHEDULE

MARK	SIZE	GRADE	MARK	SIZE	GRADE
228	(2) 2x8	SYP#2	LVL122	(2) 1.75" X 11.875"	2.0E-3100
2210	(2) 2x10	SYP#2	LVL123	(3) 1.75" X 11.875"	2.0E-3100
2212	(2) 2x12	SYP#2	LVL142	(2) 1.75" X 14"	2.0E-3100
328	(3) 2x8	SYP#2	LVL143	(3) 1.75" X 14"	2.0E-3100
3210	(3) 2x10	SYP#2	LVL162	(2) 1.75" X 16"	2.0E-3100
3212	(3) 2x12	SYP#2	LVL163	(3) 1.75" X 16"	2.0E-3100
			LVL182	(2) 1.75" X 18"	2.0E-3100
			LVL183	(3) 1.75" X 18"	2.0E-3100
			LVL242	(2) 1.75" X 24"	2.0E-3100
			LVL243	(3) 1.75" X 24"	2.0E-3100

PLAN LEGEND

228 - HEADER OR DROPPED BEAM

228 - FLUSH BEAM

NOTES:

CONVENTIONAL HEADERS SHALL HAVE 1/2" PLYWOOD FLITCHES BETWEEN EACH PLY WHEN FRAMED INTO WALLS.

REFER TO TYPICAL DETAILS FOR BUILT-UP BEAM/HEADER FASTENING DETAILS.

ALL BEAMS SHALL BE SUPPORTED BY STUD PACKS THAT MATCH THE BEAM WIDTH, U.N.O. IN NO CASE SHALL ANY BEAM BEARING BE LESS THAN 3 1/2".

SIMPSON HOLD-DOWN SCHEDULE

MARK	TYPE	HOLD DOWN CAPACITY (LB) ASD	ANCHOR ROAD DIAMETER	DRILL & EPOXY EMBEDMENT (IN.)	REQUIRED STUD PACK PLIES	ANCHOR CAPACITY (LB) 1-EDGE/2-EDGE (LRFD)
H0	STD ANCHOR	1,500	5/8"	8"	(2) 2x	8,400 / 6,400
H1	HDU2	3,075	5/8"	8"	(2) 2x	8,400 / 6,400
H2	HDU4	4,565	5/8"	8"	(3) 2x	8,400 / 6,400
H3	HDU5	5,645	5/8"	12.5"	(3) 2x	9,800 / 9,800
H4	HDU8	7,870	7/8"	17.5"	(4) 2x	20,000 / 15,600
H5	HDU11	11,175	1"	20"	(5) 2x	26,400 / 19,000
H6	HDU14	14,445	1"	20"	(6) 2x	26,400 / 19,000

SIMPSON STRAP SCHEDULE

MARK	TYPE	HOLD DOWN CAPACITY (LB) ASD	MIN. END LENGTH (IN.	TOTAL # OF 10d NAILS	REQUIRED STUD PACK PLIES
S1	CS16	1,705	13	22	1
S2	CS14	2,490	16	30	2
S3	(2) CS14	4,980	16	30 (EACH)	3 (TOTAL)
S4	CMST14	6,490	34	76	3
S5	CMST12	9,215	44	98	4
S6	(Z) CMST14	12,980	34	76 (EACH)	5

NOTES:

INSTALL PRODUCTS AS REQUIRED BY SIMPSON INSTALLATION INSTRUCTIONS TO OBTAIN CAPACITIES LISTED AS BASIS OF DESIGN.

ALTERNATE PRODUCTS MAY BE USED PROVIDED THE ABOVE DESIGN LOADS ARE MET.

THREADED ANCHOR RODS SHALL CONFORM TO A-307 GRADE C OR A36 MATERIAL.

EPOXY SHALL BE SIMPSON SET-XP OR EQUAL INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER INSTRUCTIONS.

HALF OF THE REQUIRED NAILS LISTED MUST BE INSTALLED INTO EACH OF THE STUD OR STUD PACKS BEING TIED TOGETHER. CUT STRAP LENGTH AND CENTER STRAP BETWEEN LEVELS AS NECESSARY TO MEET THIS REQUIREMENT.

ALL STUD PACKS SHALL BE SYP #2. ALL STUD PACKS SHALL BE SECURED TOGETHER IN ACCORDANCE WITH THE BUILT-UP STUD DETAIL.

WHERE HOLD-DOWNS ARE CALLED OUT AT UPPER LEVEL SHEARWALLS ABOVE STEEL FRAMING ELEMENTS, FULLY WELD ANCHOR RODS TO STRUCTURAL STEEL BELOW.

ROOF DECK SCHEDULE

TYPE	THICKNESS AND GRADE	NAIL TYPE	BOUNDARY NAILING	FIELD NAILING	BLOCKED
ROOF DECK	15/32" EXT. STR. 1 PLYWOOD	10d	6" O.C.	12" O.C.	NO

NOTES:

NAILS SHALL HAVE A SUFFICIENT LENGTH TO PENETRATE ROOF AND FLOOR FRAMING MEMBERS BY A MINIMUM OF 1 1/2".

ALL BLOCKED DIAPHRAGMS SHALL HAVE ALL PANEL EDGES BLOCKED WITH MIN. 2x4 MATERIAL AND NAILED PER BOUNDARY NAILING REQUIREMENTS.

BOUNDARY NAILING REFERS TO THE NAILS ALONG THE EDGES OF EACH PANEL.

FIELD NAILING REFERS TO NAILING REQUIRED ALONG ALL INTERMEDIATE SUPPORTS UNDER EACH PANEL.

CUT NAIL SPACING IN HALF AT OVERHANGS.

GLUE DECKING AS REQUIRED BY ARCHITECTURAL PLANS.

SHEARWALL SCHEDULE

MARK	SHEATHING	FASTENER	BOUNDARY NAILING	FIELD NAILING	ALLOWABLE SHEAR VALUE
G1	1/2" GYPSUM	6d COOLER	7" O.C.	7" O.C.	100 PLF
G2	5/8" GYPSUM	6d COOLER	4" O.C.	4" O.C.	175 PLF
G3	DOUBLE LAYER 5/8" GYPSUM	6d COOLER	9" BASE PLY 7" FACE PLY	9" BASE PLY 7" FACE PLY	250 PLF
W1	7/16" EXP 1	8d COMMON	6" O.C.	12" O.C.	364 PLF
W2	7/16" EXP 1	8d COMMON	4" O.C.	12" O.C.	532 PLF
W3	7/16" EXP 1	8d COMMON	3" O.C.	12" O.C.	686 PLF
W4	7/16" EXP 1	8d COMMON	2" O.C.	12" O.C.	832 PLF

PLAN LEGEND

NOTES:

NAILS SHALL NOT TEAR THE PAPER OF GYPSUM SHEARWALLS.

FASTENERS SHALL NOT BE PERMITTED TO BE DRIVEN PAST THE FLUSH FACE OF PLYWOOD SHEATHING.

MATERIAL THICKNESSES AND FASTENER SIZES SHOWN ARE MINIMUMS.

ORIENTED STRAND BOARD (OSB) MAY BE SUBSTITUTED FOR PLYWOOD SHEATHING.

"BOUNDARY NAILING" REFERS TO THE ATTACHMENT REQUIRED ALONG THE SUPPORTED EDGES OF EACH SHEATHING PANEL.

"FIELD NAILING" REFERS TO THE ATTACHMENT REQUIRED ALONG THE INTERMEDIATE SUPPORTS BEHIND EACH SHEATHING PANEL.

ALL SHEARWALL SHEATHING MUST HAVE ALL FREE EDGES SUPPORTED BY BLOCKING OR THE PANELS MUST BE ORIENTED VERTICALLY & SIZED TO EXTEND ENTIRELY FROM BOTTOM PLATE TO TOP PLATE SUCH THAT NO EDGES ARE UNSUPPORTED.

LIGHTEST ATTACHMENT SHOWN ABOVE SHALL BE USED FOR ALL NON-SHEARWALL APPLICATIONS FOR RESPECTIVE SHEATHING TYPE AND THICKNESS SHOWN ON THE ARCHITECTURAL PLANS.

ALL SHEARWALLS WITH NAILING AT 3" O.C. OR LESS REQUIRE DOUBLE STUDS AT ALL PANEL POINTS THAT SHALL BE FASTENED TOGETHER WITH 8d NAILS @ 2" O.C. STAGGERED.

SHEATH ALL EXTERIOR WALLS AS W1, U.N.O.

SHEARWALL MARKS ARE SHOWN ON THE SIDE OF THE SHEARWALL THAT RECEIVES THE SCHEDULED SHEATHING AND NAILING PATTERN.

HOLDDOWNS AND STRAPS SHALL BE INSTALLED ON ALL END CHORDS.

SILL AND SOLE PLATE ANCHORAGE SCHEDULE

LOCATION	TYPE OF WALL	ANCHORAGE	PLATE ANCHOR SPACING
EXTERIOR WALL SILL & PLATES INDIVIDUALLY	NON-SHEARWALL	1/2" ø SILL BOLTS	@ 48" O.C.
	G1-G2 SHEARWALL	1/2" ø SILL BOLTS	@ 48" O.C.
	G3 & W1 SHEARWALL	1/2" ø SILL BOLTS	@ 35" O.C.
	W2-W3 SHEARWALL	1/2" ø SILL BOLTS	@ 22" O.C.
	W4-W5 SHEARWALL	1/2" ø SILL BOLTS	@ 17" O.C.
INTERIOR WALL SILL & PLATES INDIVIDUALLY	NON-SHEARWALL	0.145" X 2 7/8" PAF	@ 20" O.C.
	G1-G2 SHEARWALL	(2) 0.145" X 2 7/8" PAF	@ 20" O.C.
	G3 & W1 SHEARWALL	1/2" ø SILL BOLTS	@ 36" O.C.
	W2-W3 SHEARWALL	1/2" ø SILL BOLTS	@ 22" O.C.
	W4-W5 SHEARWALL	1/2" ø SILL BOLTS	@ 17" O.C.
SOLE PLATES	NON-SHEARWALL	8d NAIL	@ 20" O.C.
	G1-G2 SHEARWALL	(2) 8d NAILS	@ 20" O.C.
	G3 & W1 SHEARWALL	(2) 8d NAILS	@ 8" O.C.
	W2-W3 SHEARWALL	(2) 8d NAILS	@ 6" O.C.
	W4-W5 SHEARWALL	(2) 8d NAILS	@ 4" O.C.

NOTES:

SHEARWALLS SHEATHED ON BOTH SIDES SHALL USE TWICE THE ANCHORS REQUIRED BY THE MOST STRINGENT SCHEDULED ANCHORAGE OF THE SIDES INDIVIDUALLY.

THE 1/2" WET-SET BOLTS SCHEDULED ABOVE SHALL BE GALVANIZED AND HAVE A MIN. 1" HOOK EMBEDDED MIN. 7" INTO CONCRETE AND BE FITTED WITH STANDARD WASHER AND NUT.

THERE SHALL ALWAYS BE ONE ANCHOR PLACED NOT MORE THAN 12" NOR LESS THAN 4" FROM END SILL PLATE END. THERE SHALL BE A MINIMUM OF TWO (2) ANCHORS PER SILL PLATE.

THE 1/2" WET-SET SILL BOLTS SCHEDULED MAY BE REPLACED WITH 1/2" X 6" SIMPSON TITEN HD ANCHORS ON A 1:1 BASIS.

EXPANSION ANCHORS SHALL NOT BE PERMITTED.

PAF ANCHORS SHALL BE HILTI X-CP.

SILL BOLTS IN WOOD SHEARWALLS SHALL HAVE A 0.229" X 3" X 3" WASHERS AND EXTEND TO WITHIN 1/2" OF SHEATED SIDES OF PLATE.

LOAD BEARING WALL STUD SCHEDULE

WALL TYPE	LEVEL	2x4 STUD SPACING	2x6 STUD SPACING
1-STORY EXTERIOR WALLS UP TO 8'-2"	1	#2 SYP @ 16" O.C.	#2 SYP @ 16" O.C.
1-STORY INTERIOR WALLS UP TO 8'-2"	1	#2 SYP @ 16" O.C.	#2 SYP @ 16" O.C.

NOTES:

SEE ARCHITECTURAL PLANS FOR WALL WIDTHS WHERE 2X4 AND 2X6 STUDS ARE ALLOWED BY THE SCHEDULE.

SEE PLAN FOR POSSIBLE EXCEPTIONS TO THIS SCHEDULE.

BEARING WALLS ARE INDICATED ON PLANS AS SUCH

STANDARD WOOD CONNECTION FASTENING SCHEDULE

DESCRIPTION OF BUILDING ELEMENT	COMMON NAIL	0.131"x3" NAIL
JOIST TO SILL OR GIRDER, TOENAIL	(3) 8d	3
BRIDGING TO JOIST, TOENAIL	(2) 8d	2
SOLE PLATE ANCHORAGE	SEE SCHEDULE	SEE SCHEDULE
SOLE OR TOP (BOTTOM PLY) PLATE TO STUD, END NAIL	(2) 16d	3
STUD TO SOLE PLATE, TOENAIL	(4) 8d	4
BUILT-UP STUD PACKS, FACE NAIL	SEE DETAIL	SEE DETAIL
TOP PLATE PLY NAILING, FACE NAIL	16d @ 16" O.C.	@ 8" O.C.
TOP PLATE SPLICE NAILING, FACE NAIL	SEE DETAIL	SEE DETAIL
JOIST OR RAFTER BLOCKING TO TOP PLATE	(3) 8d	3
RIM JOIST TO TOP PLATE, TOENAIL	8d @ 6" O.C.	6" O.C.
TOP PLATE CORNER/INTERSECTION LAPS, FACE NAIL.	SEE DETAIL	SEE DETAIL
BUILT-UP 2X HEADERS AND BEAMS (CONT.)	SEE DETAIL	SEE DETAIL
BUILT-UP 2X HEADERS AND BEAMS (END)	SEE DETAIL	SEE DETAIL
CEILING JOIST TO TOP PLATE, TOENAIL	(3) 8d	3
CONTINUOUS HEADER TO STUD, TOENAIL	(4) 8d	4
CEILING JOIST LAPS AND RAFTER LAPS, FACE NAIL	(3) 16d	4
RAFTER TO PLATE, TOENAIL	(3) 8d	3
BUILT-UP CORNER STUDS, FACE NAIL	16d @ 16" O.C.	@ 16" O.C.
COLLAR TIE TO RAFTER, TOENAIL	(3) 10d	4
JACK RAFTER TO HIP, TOENAIL	(3) 10d	4
RAFTER TO RIDGE BEAM, TOENAIL	(2) 16d	3
RAFTER TO RIDGE BEAM, FACE NAIL	(3) 16d	3
JACK RAFTER TO HIP, FACE NAIL	(3) 16d	3
JOIST TO RIM/BAND JOIST, END NAIL	(3) 16d	4
2x RIBBON TO END OF FLOOR TRUSS FACE NAIL	(2) 16d	3
TOP CHORD OF ROOF TRUSSES, TOENAIL	(2) 16d	3

NOTES:

PENNY SIZES ARE FOR COMMON NAILS (NOT SINKER OR BOX) AND ARE DEFINED AS FOLLOWS:

PENNYWEIGHT	DIAMETER	LENGTH
6d	0.113"	2"
8d	0.131"	2 1/2"
10d	0.148"	3"
16d	0.162"	3 1/2"

ANY DESCRIPTION OF BUILDING ELEMENTS NOT NNOTED ABOVE MAY BE FOUND IN THE IBC, TABLE 2304.9.1.

SEE ICC ESR-1539 FOR ALL ADDITIONAL FASTENER REQUIREMENTS NOT NOTED HERE.

ABBREVIATIONS

@	AT	GA.	GAGE
A.F.F.	ABOVE FINISHED FLOOR	GB	GRADE BEAM
ARCH.	ARCHITECTURAL	MECH.	MECHANICAL
BM.	BEAM	O.C.	ON CENTER
B.O.S.	BOTTOM OF STEEL	O.C.E.W.	ON CENTER EACH WAY
BOT.	BOTTOM	P.T.	POST-TENSION
C.G.	CENTER OF GRAVITY	SIM.	SIMILAR
COL.	COLUMN	T.O.	TOP OF
CONT.	CONTINUOUS	T.O.C.	TOP OF CONCRETE
CXN	CONNECTION	T.O.J.	TOP OF JOIST
DL	DEAD LOAD	T.O.S.	TOP OF SLAB
LL	LIVE LOAD	U.N.O.	UNLESS NOTED OTHERWISE
EL.	ELEVATION	V.O.J.	VERIFY ON JOBSITE
ELEC.	ELECTRICAL	PL.	PLATE
E.O.A.	EDGE OF ANGLE	CL.	CENTER LINE
E.O.S.	EDGE OF SLAB	U.W.A.	UNDER WALL ABOVE
FF.	FINISHED FLOOR		
PLF	LBF PER LINEAR FOOT		

SOLID SAWN JOIST AND BEAM HANGER SCHEDULE

BEAM OR JOIST SIZE	SIMPSON HANGER	BEAM OR JOIST SIZE	SIMPSON HANGER
2x6	LUS26	(3) 2x10	HHUS210-3
2x8	LUS28	(3) 2x12	HHUS210-3
2x10	LUS210	LVL122	HHUS410
2x12	LUS210		
(2) 2x6	HHUS26-2		
(2) 2x8	HHUS28-2		
(2) 2x10	HHUS210-2		
(2) 2x12	HHUS212-2		

NOTES:

ALL FLUSH JOIST AND BEAM CONNECTORS SHALL BE MADE WITH MANUFACTURED JOIST OR BEAM HANGERS MANUFACTURED BY SIMPSON STRONG TIE OR APPROVED EQUAL.

IF MULTIPLE OPTIONS FOR FASTENING ARE PROVIDED BY THE MANUFACTURER, PROVIDE THE TYPE, QUANTITY, AND SIZE OF FASTENERS REQUIRED TO ACHIEVE THE MAXIMUM RATED CONNECTOR CAPACITY.

ENGINEERED GRADE LUMBER BEAM CONNECTORS HAVE BEEN DESIGNATED ON THE PLANS, SECTIONS, OR DETAILS.

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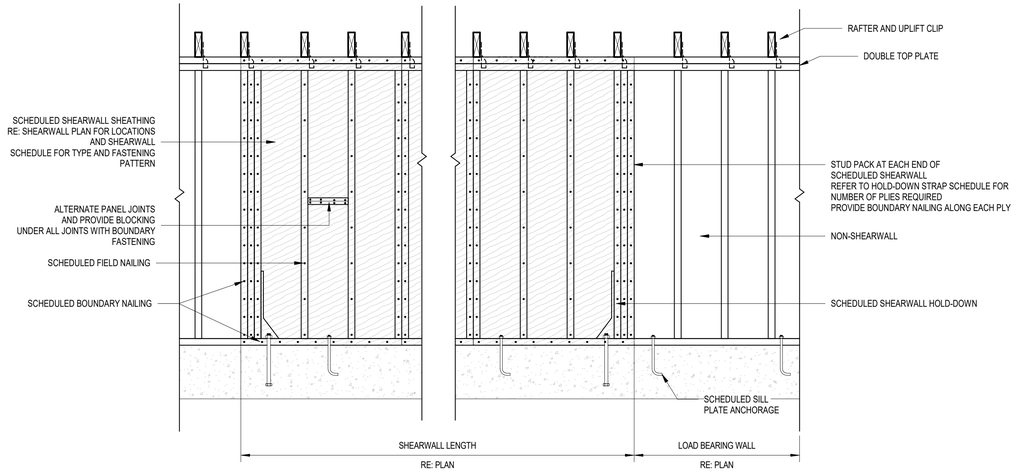
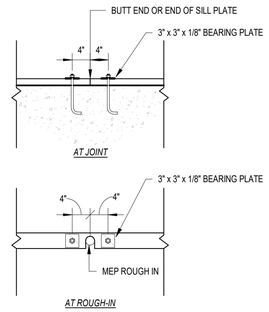
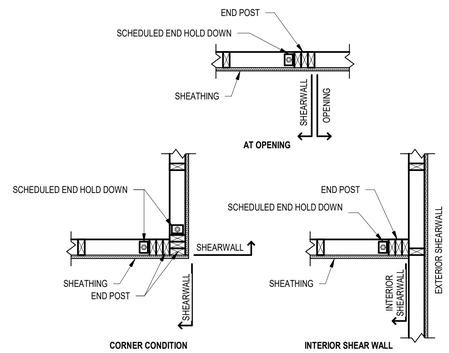


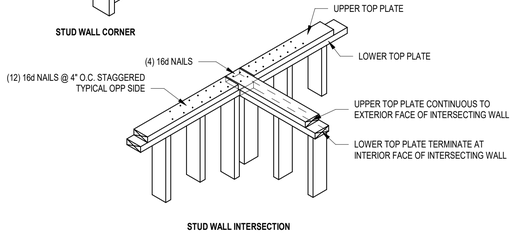
1 TYPICAL SINGLE-STORY SHEARWALL
3/4" = 1'-0"



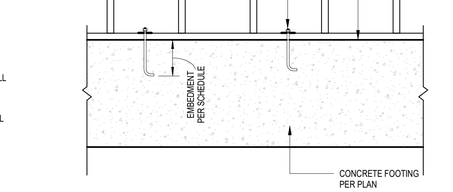
2 TYPICAL SILL ANCHORAGE AT JOINT
3/4" = 1'-0"



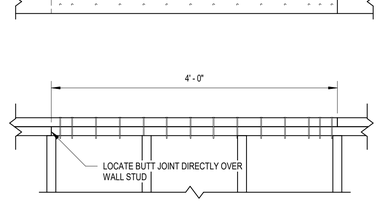
3 TYPICAL SHEARWALL CONDITIONS
3/4" = 1'-0"



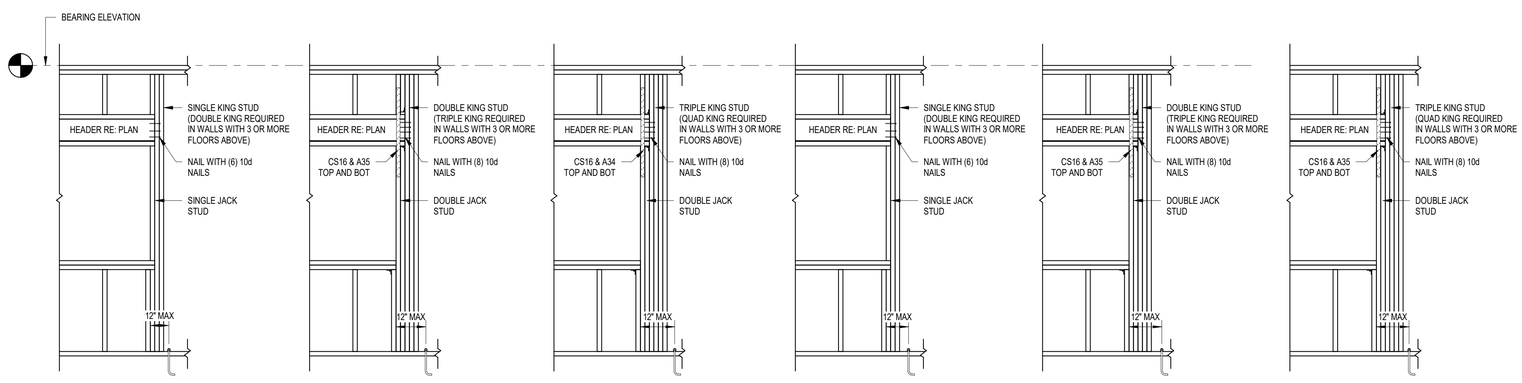
4 TYPICAL TOP PLATE CONDITIONS
1/2" = 1'-0"



5 TYPICAL SILL ANCHORAGE DETAIL
3/4" = 1'-0"



6 TYPICAL TOP PLATE SPLICE
1" = 1'-0"



7 TYPICAL JACK AND KING STUD DETAIL
1/2" = 1'-0"

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SHEARWALL DETAILS

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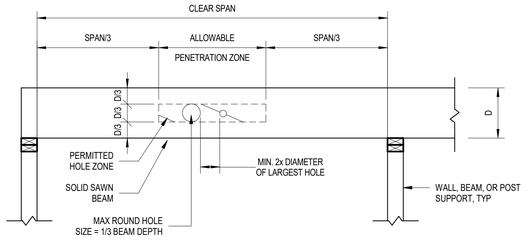
S501

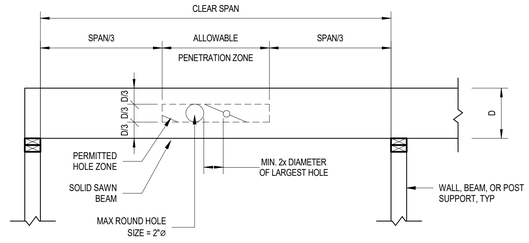
SOUTHEAST ENGINEERS & LAND SURVEYORS
4880 BLUEBONNET BOULEVARD, SUITE A
BATON ROUGE, LOUISIANA 70809



NOTES:

1. DETAIL APPLIES TO SOLID SAWN HEADERS OR BEAMS WITH DEPTHS GREATER THAN 7 1/4".
2. ALLOWABLE HOLES INDICATED ARE SUITABLE FOR HEADERS AND BEAMS WITH UNIFORM LOADING ONLY.
3. HOLES MUST BE ROUND. SQUARE HOLES NOT PERMITTED.
4. HOLES ARE PERMITTED TO BE PLACED IN THE MIDDLE 1/3 OF THE SPAN IN THE MIDDLE 1/3 OF THE BEAM ONLY.
5. DETAIL APPLIES TO BOTH SIMPLE AND CONTINUOUS SPANS.
6. HOLES ARE NOT PERMITTED TO BE PLACED IN CANTILEVERS
7. ANY CONDITION NOT COVERED BY THIS DETAIL SHALL BE SUBMITTED TO THE SEOR FOR REVIEW AND APPROVAL PRIOR TO FIELD CUTTING HOLES.

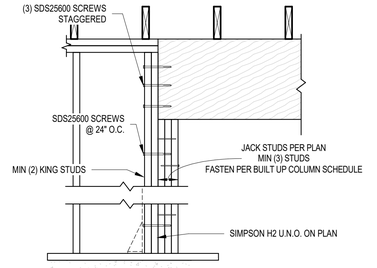
1 ALLOWABLE HOLES IN SOLID SAWN BEAMS AND HEADERS
3/4" = 1'-0"



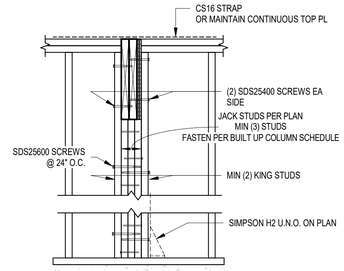
NOTES:

1. DETAIL APPLIES TO LVL AND PSL HEADERS OR BEAMS WITH DEPTHS GREATER THAN 7 1/4".
2. ALLOWABLE HOLES INDICATED ARE SUITABLE FOR HEADERS AND BEAMS WITH UNIFORM LOADING ONLY.
3. HOLES MUST BE ROUND. SQUARE HOLES NOT PERMITTED.
4. HOLES ARE PERMITTED TO BE PLACED IN THE MIDDLE 1/3 OF THE SPAN IN THE MIDDLE 1/3 OF THE BEAM ONLY.
5. DETAIL APPLIES TO BOTH SIMPLE AND CONTINUOUS SPANS.
6. HOLES ARE NOT PERMITTED TO BE PLACED IN CANTILEVERS
7. ANY CONDITION NOT COVERED BY THIS DETAIL SHALL BE SUBMITTED TO THE SEOR FOR REVIEW AND APPROVAL PRIOR TO FIELD CUTTING HOLES.

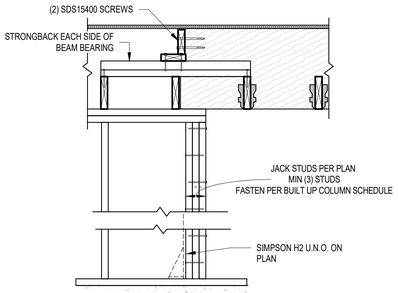
2 ALLOWABLE HOLES IN ENGINEERED BEAMS AND HEADERS
3/4" = 1'-0"



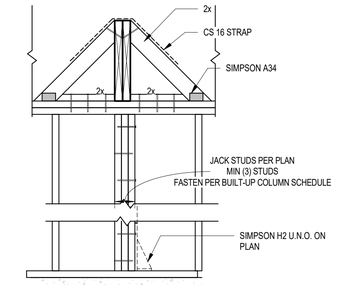
DROPPED INSTALLATION PARALELL TO WALL



DROPPED INSTALLATION PERPENDICULAR TO WALL

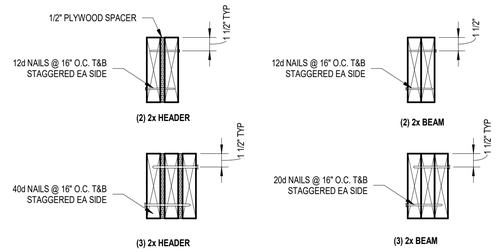


FLUSH INSTALLATION PARALELL TO WALL

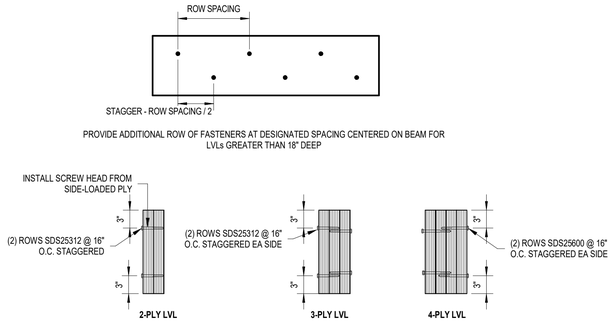


FLUSH INSTALLATION PERPENDICULAR TO WALL

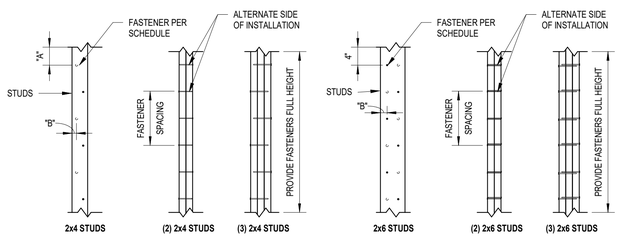
3 TYPICAL ENGINEERED BEAM BEARING
3/4" = 1'-0"



4 BUIT UP HEADER / BEAM DETAIL
1 1/2" = 1'-0"

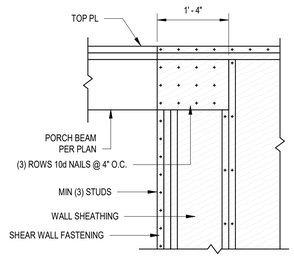


5 MULTI-PLY LVL FASTENING
1" = 1'-0"

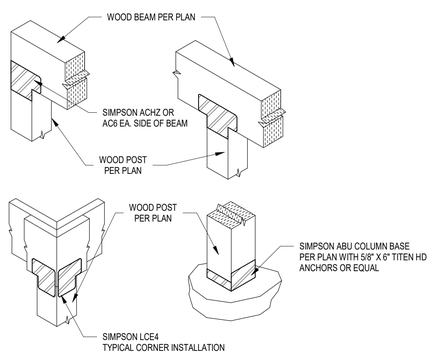


BUILT UP COLUMN	2x4 BUILT UP COLUMN				
	QUANTITY OF STUDS	FASTENERS	SPACING	"A" END DISTANCE	"B" EDGE DISTANCE
	(2) 2x	10d COMMON NAILS	(1) ROW @ 6" STAGGERED	2 1/2"	1"
	(3) 2x	20d COMMON NAILS	(1) ROW @ 8" STAGGERED	3 1/2"	1 1/2"
	(4) 2x	SIMPSON SDW22600	(1) ROW @ 8" STAGGERED	4"	1 1/2"
	(5) 2x	1/2" ø THRU BOLTS	(1) ROW @ 9" STAGGERED	3 1/2"	1 1/2"
	2x6 BUILT UP COLUMN				
	QUANTITY OF STUDS	FASTENERS	SPACING	"A" END DISTANCE	"B" EDGE DISTANCE
	(2) 2x	10d COMMON NAILS	(2) ROW @ 6"	2 1/2"	1"
	(3) 2x	20d COMMON NAILS	(2) ROW @ 8"	3 1/2"	1 1/2"
	(4) 2x	SIMPSON SDW22600	(2) ROW @ 8"	4"	1 1/2"
	(5) 2x	1/2" ø THRU BOLTS	(2) ROW @ 9"	3 1/2"	1 1/2"

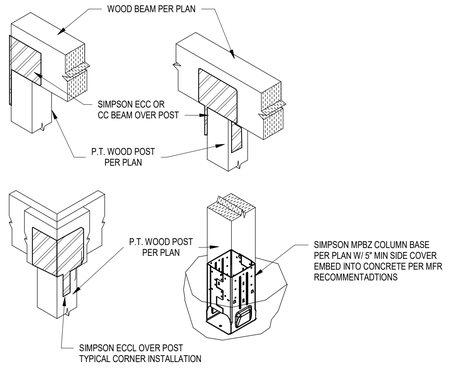
6 BUILT UP COLUMN DETAIL/SCHEDULE
3/4" = 1'-0"



7 PORCH BEAM AT EXTERIOR WALL DETAIL
3/4" = 1'-0"



8 TYPICAL STAND ALONE POST DETAIL
3/4" = 1'-0"



9 TYPICAL MPBZ STAND ALONE POST DETAIL
3/4" = 1'-0"

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FRAMING DETAILS

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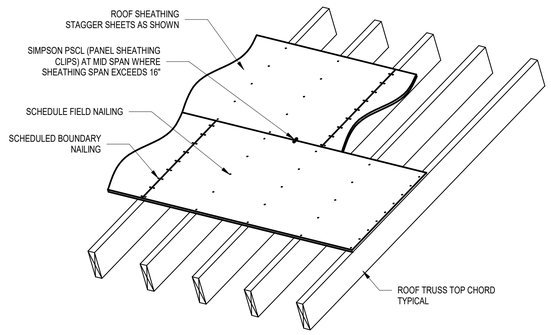
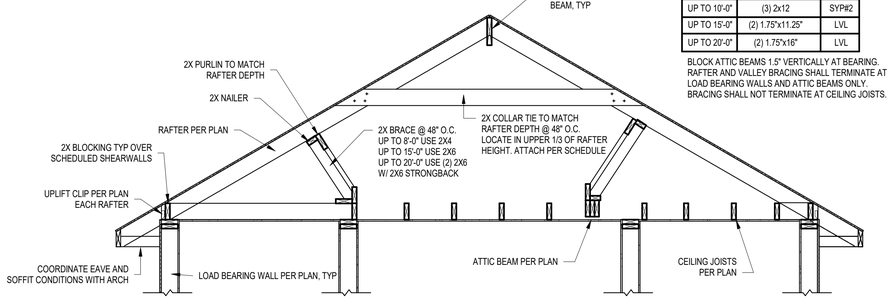
S502

SOUTHEAST ENGINEERS & LAND SURVEYORS
4880 BLUEBONNET BOULEVARD, SUITE A
BATON ROUGE, LOUISIANA 70809



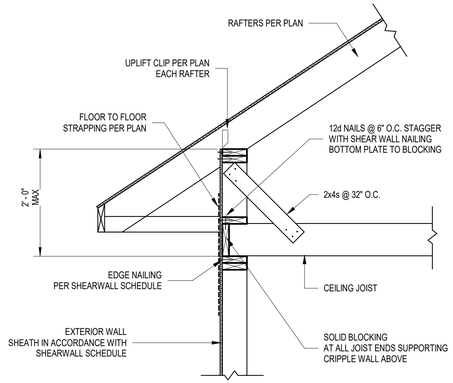
1 TYPICAL ROOF SHEATHING DETAIL
3/4" = 1'-0"



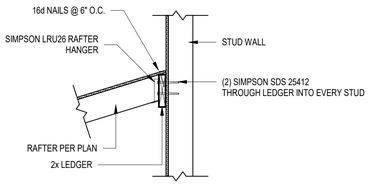
FLOATING ATTIC BEAM SCHEDULE		
SPAN	MEMBER QTY & SIZE	GRADE
UP TO 5'-0"	(3) 2x8	SYP#2
UP TO 10'-0"	(3) 2x12	SYP#2
UP TO 15'-0"	(2) 1.75"x11.25"	LVL
UP TO 20'-0"	(2) 1.75"x16"	LVL

BLOCK ATTIC BEAMS 1.5" VERTICALLY AT BEARING. RAFTER AND VALLEY BRACING SHALL TERMINATE AT LOAD BEARING WALLS AND ATTIC BEAMS ONLY. BRACING SHALL NOT TERMINATE AT CEILING JOISTS.

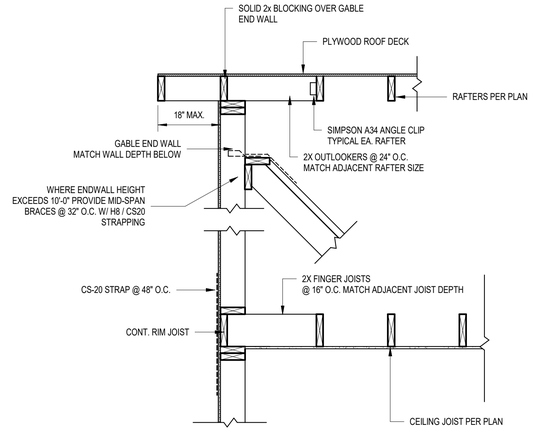
2 TYPICAL ROOF FRAMING
1/2" = 1'-0"



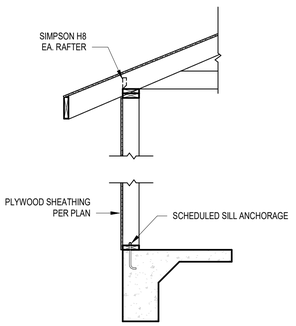
3 TYPICAL CRIPPLE WALL RAFTER BEARING
3/4" = 1'-0"



4 TYPICAL RAFTER LEDGER ATTACHMENT DETAIL
3/4" = 1'-0"



5 TYPICAL GABLE ENDWALL FRAMING
3/4" = 1'-0"



6 UPLIFT STRAP CONNECTIONS AT ROOF BEARING WALLS (SINGLE-STORY)
1/2" = 1'-0"

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ROOF FRAMING DETAILS

sheet no.
S503

SOUTHEAST ENGINEERS & LAND SURVEYORS
4880 BLUEBONNET BOULEVARD, SUITE A
BATON ROUGE, LOUISIANA 70809

GENERAL

ALL DESIGN LOADS SHOWN ON THE STRUCTURAL PLANS ARE ALLOWABLE STRESS DESIGN (UNFACTORED) SERVICE LOADS.

ALL METHODS, PROCEDURES, AND SEQUENCES OF WORK ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL ENSURE ALL PRECAUTIONS ARE TAKEN TO MAINTAIN THE INTEGRITY OF THE STRUCTURE THROUGHOUT ALL STAGES OF CONSTRUCTION.

REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING PLANS FOR ALL ITEMS OMITTED FROM THE STRUCTURAL PLANS.

IF A CONFLICT IS OBSERVED IN THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD FOR PROPER GUIDANCE AND CLARIFICATION PRIOR TO CONSTRUCTION.

THE STRUCTURAL INTEGRITY OF THE DESIGN DEPENDS ON THE FULL INTERACTION OF ALL ITS FRAMING MEMBERS AND CLADDING. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROPERLY SHORE AND BRACE ALL STAGES OF CONSTRUCTION UNTIL THE STRUCTURE IS COMPLETED.

THE GENERAL CONTRACTOR SHALL COORDINATE THE STRUCTURAL DRAWINGS WITH ALL OTHER DRAWINGS.

ALL EXISTING CONDITIONS, DIMENSIONS, ELEVATIONS, AND QUANTITIES SHALL BE VERIFIED BY THE GENERAL CONTRACTOR PRIOR TO ANY FABRICATION OR INSTALLATION.

SCALING OF THE STRUCTURAL DRAWINGS SHALL NOT BE PERMITTED. IF ANY DIMENSIONS VITAL TO CONSTRUCTION ARE NOT SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS, CONTRACTOR SHALL SEND FORMAL REQUEST TO THE ARCHITECT OR ENGINEER OF RECORD FOR CLARIFICATION.

IN THE EVENT OF CONFLICTING OR DIFFERING REQUIREMENTS INDICATED ON THE STRUCTURAL DRAWINGS AND/OR SPECIFICATIONS THAT HAVE NOT BEEN SUBSEQUENTLY CLARIFIED OR CHANGED, THE CONTRACTOR SHALL PROVIDE THE BETTER QUALITY, GREATER QUANTITY, OR MORE STRINGENT UNLESS OTHERWISE DIRECTED BY THE ENGINEER OF RECORD.

CODES AND DESIGN SPECIFICATIONS

BUILDING CODE: INTERNATIONAL BUILDING CODE (IBC) 2021

AMERICAN CONCRETE INSTITUTE (ACI 318 LATEST EDITION)

NATIONAL DESIGN SPECIFICATION FOR WOOD FRAMED CONSTRUCTION 2018

ASCE 7-16

DESIGN LOADS

- ROOF DESIGN LOADS

  CEILING COLLATERAL DEAD LOAD = 10 PSF
  CEILING LIVE LOAD = 20 PSF
  ROOF COLLATERAL DEAD LOAD = 10 PSF
  ROOF LIVE LOAD = 20 PSF

- WIND LOAD CRITERIA

  ULTIMATE WINDSPEED = 140 MPH
  NOMINAL WINDSPEED = 108 MPH
  OCCUPANCY CATEGORY = II
  EXPOSURE = B
  INTERNAL PRESSURE COEFFICIENT = +/- 0.18

  TOTAL SHEAR PERPENDICULAR TO LENGTH = 25 KIP (ULTIMATE)
  TOTAL SHEAR PERPENDICULAR TO WIDTH = 10 KIP (ULTIMATE)

GEOTECHNICAL:

NO GEOTECHNICAL INFORMATION WAS FURNISHED FOR THIS PROJECT. THIS ENGINEER HAS ASSUMED THAT THE SOIL CONDITIONS FOR THIS DESIGN ARE FIRM, STABLE, AND NON-EXPANSIVE AND ARE CAPABLE OF SAFELY SUPPORTING 1,500 PSF FOR SPOT FOOTINGS AND FOR CONTINUOUS FOOTINGS. BY USING THESE PLANS, THE OWNER HOLDS HARMLESS THE ENGINEER FROM ALL LIABILITY AND DAMAGES ARISING FROM UNSTABLE SOIL CONDITIONS OR FROM CONDITIONS WHERE FACTORS CAUSE THE SOIL CONDITIONS TO BECOME UNSTABLE AND ALLOW MOVEMENT WITHIN THE FOUNDATION AND STRUCTURAL SYSTEM.

IF CONDITIONS ARE FOUND TO BE OTHER THAN HEREIN STATED, STOP OPERATIONS AND NOTIFY ENGINEER SO THEY CAN HAVE THE NECESSARY INVESTIGATION AND MODIFY PLANS AS REQUIRED STRIP A MINIMUM OF 6" OF EXISTING SOIL AND REMOVE ALL ROOTS, ORGANIC MATERIALS AND UNSTABLE SOILS PRIOR TO PLACING ANY FILL.

A MAXIMUM OF 2.0 FEET OF FILL MAY BE PLACED ON THE SITE. MAXIMUM DIFFERENTIAL FILL SHALL NOT EXCEED 20%.

STRUCTURAL FILL SHALL BE LEAN CLAY WITH A PI BETWEEN 15 AND 25 COMPACTED TO 95% STANDARD PROCTOR DENSITY IN MAXIMUM OF 6" LIFTS.

CONTRACTOR AND OWNER TO PROVIDE PROPER DRAINAGE AWAY FROM THE FOUNDATION DURING AND THROUGHOUT THE STRUCTURE LIFE.

CAST-IN-PLACE CONCRETE

ALL CONCRETE WORK SHALL BE CONDUCTED IN ACCORDANCE WITH THE ACI STANDARD "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318 LATEST EDITION).

CONCRETE MIX DESIGNED SHALL BE ESTABLISHED BY THE SUPPLIER IN ACCORDANCE WITH ACI 318-14. MIX DESIGNS SHALL BE SUBMITTED WITH BACKUP DATA PER ACI 318-14 TO THE ARCHITECT/ENGINEER FOR REVIEW AND APPROVAL PRIOR TO CONCRETE BATCHING. THE PROPOSED MATERIALS AND MIX SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE TESTING LABORATORY. RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS ON THE GENERAL CONTRACTOR.

ALL CONCRETE SHALL CONFORM TO THE REQUIREMENTS SPECIFIED IN THE TABLE BELOW:

USE	COMPRESSIVE STRENGTH	MAXIMUM AGGREGATE SIZE
CONTINUOUS FOOTINGS & SLABS ON GRADE	3,000 PSI	3/4"

WATER/CEMENT RATIO SHALL BE LIMITED TO 0.50 MAXIMUM.

SLEEVES, MECHANICAL OPENINGS, CONDUITS, PIPES, RECESSES, DEPRESSIONS, AND ALL EMBEDDED ITEMS SHALL BE PROVIDED FOR AS PER THE MECHANICAL, ELECTRICAL, AND ARCHITECTURAL DRAWINGS.

ENGINEER OF RECORD DOES NOT PERMIT THE USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS THAT MAY CAUSE EXCESSIVE CORROSION IN THE REINFORCING STEEL.

A MINIMUM 10 MIL POLYETHYLENE VAPOR BARRIER SHALL BE PROPERLY INSTALLED AND SEALED UNDER ALL CONCRETE DESIGNS UNDER THIS SEAL.

CONTRACTOR SHALL REFER TO THE FOUNDATION DRAWINGS FOR ADDITIONAL INFORMATION.

ALL FLOOR DRAINS, DROPS, CURBS, LEDGES, ETC. SHALL BE COORDINATED WITH THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.

CONTRACTOR SHALL PLACE AND SECURE ALL REINFORCEMENT, ANCHOR BOLTS, ETC. PRIOR TO PLACING ANY CONCRETE. WET PLACING ANY OF THESE ITEMS DURING CONCRETE PLACEMENT IS NOT PERMITTED.

WATER MAY NOT BE ADDED TO BATCH AT THE SITE UNLESS IT IS SPECIFICALLY NOTED THAT IT MAY BE ADDED BY THE READY-MIX COMPANY.

CONCRETE REINFORCEMENT

ALL CONCRETE REINFORCEMENT SHALL BE DETAILED AND ACCESSORIES PROVIDED IN ACCORDANCE WITH THE PROVISIONS OF ACI 318 (LATEST EDITION).

ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60 (Fy = 60,000 PSI MIN.) UNLESS OTHERWISE SHOWN.

WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 AND SHALL BE PROVIDED IN FLAT SHEETS ONLY.

WELDED WIRE FABRIC SHALL BE LAPPED A MINIMUM OF 2 MESHES, BUT NOT LESS THAN 12".

ALL REINFORCING STEEL SHALL BE SUPPORTED ON PLASTIC CHAIRS AT 48" O.C. E.W.

ALL BAR SPLICES, WHERE REQUIRED SHALL CLASS "B" TENSION LAP SPLICES (2'-0" MINIMUM).

CONCRETE PROTECTION FOR REINFORCEMENT OF CAST-IN-PLACE CONCRETE MEMBERS SHALL BE IN ACCORDANCE WITH ACI 318-14 UNLESS OTHERWISE NOTED. MINIMUM CLEAR COVER OF CONCRETE OVER OUTER REINFORCING BARS SHALL BE IN ACCORDANCE WITH BELOW.

1. CONCRETE PERMANENTLY CAST AGAINST EARTH ..... 3"
2. CONCRETE EXPOSED TO EARTH OR WEATHER
   #6 THROUGH #11 BARS ..... 2"
   #5 BAR AND SMALLER..... 1 1/2"

ALL REINFORCING STEEL SHALL BE ADEQUATLY TIED TOGETHER AND SUPPORTED TO ENSURE PROPER LOCATION OF REINFORCEMENT IN ACCORDANCE WITH THE STRUCTURAL DESIGN.

BARS SHALL BE IN CONTACT WHEN FORMING A LAP SPLICE, UNLESS NOTED OTHERWISE.

ALL DOWELS SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED WALL OR COLUMN REINFORCEMENT AND SHALL BE LAPPED WITH A CLASS "B" TENSION LAP SPLICE.

POST-INSTALLED ANCHORS

POST-INSTALLED ANCHORS SHALL ONLY BE USED WITH THE PRIOR WRITTEN APPROVAL OF THE ENGINEER OF RECORD OR WHERE SPECIFIED IN THE STRUCTURAL DOCUMENTS.

ALL HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. PROVIDE CONTINUOUS SPECIAL INSPECTION FOR ALL MECHANICAL AND ADHESIVE ANCHORS PER THE APPLICABLE EVALUATION REPORT (ICC-ES-ESR).

CONTRACTOR SHALL AVOID DAMAGING ANY REINFORCING STEEL OR POST-TENSIONED TENDONS WHEN DRILLING HOLES FOR POST-INSTALLED ANCHORS.

SLAB-ON-GRADE POST-TENSIONED CONCRETE

ALL POST-TENSIONING MATERIAL AND ANCHORAGES SHALL CONFORM TO THE REQUIREMENTS OF THE POST TENSIONING INSTITUTE (PTI) "SPECIFICATIONS FOR UNBONDED SINGLE STRAND TENDONS."

ALL TENDONS SHALL BE FABRICATED FROM 1/2" ø, 270 KSI LOW-RELAXATION STRANDS CONFORMING TO THE REQUIREMENTS OF ASTM A416. TENDONS SHALL BE COATED WITH A PERMANENT RUST PREVENTATIVE LUBRICANT AND PLASTIC SHEATH OF AT LEAST 0.025" THICK.

DEAD ENDS AND LIVE ENDS MAY BE REVERSED IN THE FIELD AT THE CONTRACTOR'S OPTION.

AT DEAD ENDS, TENDON SHEATHING MAY BE CUT BACK AS MUCH AS 12" FROM THE ANCHOR LOCATION.

AT LIVE ENDS, SHEATHING MAY BE CUT BACK A MAXIMUM OF 2", TAPING IS SUFFICIENT FOR PATCHING OR REPLACEMENT OF SHEATHING.

MINIMUM CLEARANCE OF 3" SHALL BE MAINTAINED BETWEEN TENDONS AROUND ALL BLOCK-OUTS AND PIPE PENETRATIONS.

ALL CONCRETE SHALL BE WELL CONSOLIDATED.

CONCRETE FOR POST-TENSIONED FOUNDATIONS SHALL ACHIEVE A MINIMUM STRENGTH OF 1,500 PSI PRIOR TO FULL TENDON ELONGATIONS. TENDONS SHALL BE STRESSED AS EARLY AS POSSIBLE AFTER THIS STRENGTH HAS BEEN OBTAINED TO REDUCE SHRINKAGE CRACKING.

TENDONS SHALL BE ANCHORAGED AT 28.7 KIPS. THESE TENDONS MAY BE INITIALLY AND TEMPORARILY STRESSED AT 33 KIPS IN ORDER TO OVERCOME FRICTION AND COMPENSATE FOR SEATING LOSSES.

EACH TENDON ELONGATION SHALL BE MEASURED AND COMPARED TO THE REQUIRED ELONGATIONS. AN ELONGATION REPORT SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO CUTTING AND GROUTING LIVE ENDS.

TENDON STRESSING AN FINISHING SHALL FOLLOW THE PROCEDURES REQUIRED IN PTI'S "CONSTRUCTION AND MAINTENANCE PROCEDURES MANUAL FOR POST-TENSIONED SLAB-ON-GRADE CONSTRUCTION," LATEST EDITION.

WOOD FRAMING - WALLS

LOAD BEARING WALL STUDS SHALL BE AS SCHEDULED ON THE STRUCTURAL DRAWINGS.

HEADERS FOR 2x6 WOOD FRAMED WALLS TO BE (3) 2x8, UNLESS NOTED OTHERWISE.

HEADERS FOR 2X4 WOOD FRAMED WALLS TO BE (2) 2x8, UNLESS NOTED OTHERWISE.

STUD WALLS SHALL BE BRACED UNTIL ALL DECKING, SHEAR PANELS AND ROOF TRUSSES ARE IN PLACE. PROVIDE 2x BLOCKING AT MID HEIGHT OF ALL BEARING WALLS OR AT 4'-0" MAXIMUM SPACING.

WOOD BEAMS AND MULTI-PLY TRUSSES SHALL BEAR ON MULTIPLE STUDS AND HAVE THE SAME NUMBER OF STUDS AS NUMBER OF BEAM/TRUSS PLIES (2 MINIMUM), UNLESS A GREATER NUMBER IS NOTED ON THE DRAWINGS. CONTINUE STUD PACKS DOWN TO FOUNDATION .

JAMB, BEAM, MULTI-PLY TRUSS SUPPORT STUDS/POSTS SHALL BE CONTINUOUS DOWN TO FOUNDATION OR TRANSFER BEAM/HEADER BELOW. PROVIDE VERTICAL 2X STUDS OR SOLID BLOCKING THROUGH JOIST/TRUSS CAVITY TO MATCH STUD GROUP ABOVE. AT BEAMS, ADD SUFFICIENT BLOCKS TO EACH END OF BEAM TO MATCH STUD GROUP ABOVE.

WALL STUDS SHALL BE CONTINUOUS BETWEEN FLOORS, CEILING, AND ROOF.

WHERE EXTERIOR WALLS SPAN TWO STORIES WITHOUT INTERMEDIATE SUPPORT FROM A FLOOR, STUDS SHALL BE FULL LENGTH FOR THE TWO STORIES. STUDS SHALL NOT BE SPLICED AT THE "PLATE HEIGHT" WHEN NO ROOF, CEILING, OR FLOOR PROVIDES LATERAL SUPPORT AT THAT HEIGHT.

EXTERIOR WALLS SHALL BE COMPLETELY SHEATHED IN ACCORDANCE WITH THE LOWEST SHEARWALL SHEATHING REQUIREMENTS.

EXTERIOR SHEATHING SHALL BE MADE CONTINUOUS FROM FOUNDATION TO ROOF AND HORIZONTALLY CONTINUOUS FROM BUILDING CORNER TO BUILDING CORNER AND AROUND OPENINGS BY NAILING TO STUDS, BLOCKING, ETC. IN WALL TO CONTINUE THROUGH INTERSECTING WALLS AND FLOORS.

INTERIOR SHEATHING AT SHEAR WALLS SHALL BE MADE CONTINUOUS FROM FOUNDATION TO TOP OF SHEAR WALL AND HORIZONTALLY CONTINUOUS FROM END OF SHEAR WALL TO END OF SHEAR WALL BY FASTENING TO STUDS, BLOCKING, ETC. IN WALL TO CONTINUE THROUGH INTERSECTING WALLS AND FLOORS.

REFER TO PLANS FOR SHEARWALL LOCATIONS, DETAILS AND ANCHORAGE REQUIREMENTS.

WALL DOUBLE TOPPED PLATES SHALL BE LAPPED AT CORNERS AND INTERSECTIONS. ALL DOUBLE PLATE END JOINTS SHALL BE OFFSET AT LEAST 48", UNO.

SILL, SOLE, AND MUD PLATES SHALL BE SECURED IN ACCORDANCE WITH THE ANCHORING SCHEDULE.

WOOD SHEATHING:

ROOF, WALL, AND FLOOR SHEATHING SHALL BE AS SCHEDULED IN THE STRUCTURAL DOCUMENTS.

ORIENTED STRAND BOARD (OSB) MAY BE USED INTERCHANGEABLY WITH PLYWOOD AT VERTICAL APPLICATIONS.

ALL WOOD SHEATHING WITHIN 4'-0" ON EACH SIDE OF FIRE WALLS SHALL BE FIRE RETARDENT.

INTERIOR GYPSUM SHEARWALLS SHALL BE 5/8" THICK TYPE X GYPSUM REGULAR CONFORMING TO THE REQUIREMENTS OF ASTM C 36 AND INSTALLED PER GA-216.

REFER TO ARCHITECTURAL DRAWINGS FOR PROPOSED LOCATIONS OF FIRE RETARDENT PLYWOOD AT ROOF DECKING.

PROVIDE 1/4" GAPS EVERY 80 FEET IN PLYWOOD DECKING OR PLYWOOD RUNS LONGER THAN 80 FEET.

ALL ROOF SHEATHING SHALL RUN CONTINUOUS BELOW ALL DORMERS, CUPOLAS, AND VALLEY / BUILT-UP TRUSSES UNLESS NOTED OTHERWISE. ANCHOR VALLEY TRUSSES TO SUPPORTING ELEMENTS BELOW WITH SIMPSON VTCR AT 24" O.C. MAXIMUM.

WOOD FRAMING - GENERAL:

ALL WORK SHALL BE IN ACCORDANCE WITH THE "NATIONAL DESIGN SPECIFICATION FOR WOOD PRODUCTS", LATEST EDITION, BY THE AMERICAN FOREST & PAPER ASSOCIATION, AMERICAN WOOD COUNCIL AND "PANEL DESIGN SPECIFICATIONS", LATEST EDITION, BY APA - THE ENGINEERED WOOD ASSOCIATION.

ALL SOLID SAWN WOOD SPECIES SHALL BE SOUTHERN YELLOW PINE #2 WITH A MAXIMUM MOISTURE CONTENT OF 19% UNLESS NOTED OTHERWISE.

ALL STUDS SHALL BE BRACED HORIZONTALLY AT THIRD POINTS OR FULLY SHEATED WHEN SUBJECTED TO LOAD BEARING CONDITION DURING CONSTRUCTION.

ALL WOOD MEMBERS IN DIRECT CONTACT WITH CONCRETE OR EXPOSED TO THE WEATHER SHALL BE PRESSURE TREATED.

MEMBERS MAY BE MULTIPLE OR SINGLE PLY TO ACHIEVE THE THICKNESS SPECIFIED. WHERE MULTIPLE PLY MEMBERS ARE USED, FASTEN MEMBERS PER THE MANUFACTURER REQUIREMENTS TO OBTAIN THE CAPACITY OF A SINGLE PLY MEMBER.

SPLICES ARE NOT PERMITTED IN RAFTERS, BEAMS, OR POSTS EXCEPT AT SUPPORTS.

RAFTERS, JOISTS, AND BEAMS SHALL BE SUPPORTED LATERALLY AT THE ENDS AND AT EACH INTERIOR SUPPORT BY SOLID BLOCKING EXCEPT WHERE THE ENDS OF THE MEMBERS ARE FASTENED TO A HEADER, BAND OR RIM BOARD.

ROOF AND FLOOR SHEATHING ARE DESIGNED AS DIAPHRAGMS. SPAN DIRECTION OF SHEETS SHALL BE PERPENDICULAR TO FRAMING AND JOINTS SHALL BE STAGGERED. MAINTAIN 1/8" GAP BETWEEN ENDS AND EDGES OF PANELS. REFER TO TYPICAL FRAMING DETAILS FOR ADDITIONAL INFORMATION AND ATTACHMENT SPACING.

WOOD FRAMING - CONNECTIONS:

FASTEN WOOD FRAME ELEMENTS IN ACCORDANCE WITH IBC TABLE 2304.10.1 UNLESS NOTED OTHERWISE IN THE PLANS. PROVIDE IBC FASTENING IN ADDITION TO MANUFACTURED CONNECTIONS.

THE NUMBER AND SIZE OF FASTENERS CONNECTING WOOD MEMBERS SHALL NOT BE LESS THAN THAT SET FORTH IN THE IBC 2021 "FASTENING SCHEDULE" UNLESS NOTED AS GREATER ON DRAWINGS.

BEAM TO BEAM FLUSH CONNECTIONS SHALL BE MADE USING MANUFACTURED BEAM HANGERS PER THE STRUCTURAL DETAILS. NO STRUCTURAL BEAMS OR HEADER CONNECTIONS SHALL RELY ON TOE-NAILING FOR LOAD TRANSFER.

CONNECTORS AND FASTENERS FOR PRESERVATIVE-TREATED AND FIRE-RETARDANT-TREATED WOOD SHALL BE OF HOT DIPPED ZINC COATED GALVANIZED STEEL. INCLUDING BUT NOT LIMITED TO ANCHOR BOLTS, POWER ACTUATED FASTENERS, NAILS, SCREWS , BOLTS AND METAL FRAMING HARDWARE. THE ZINC COATING WEIGHTS SHALL BE IN ACCORDANCE WITH ASTM A153.

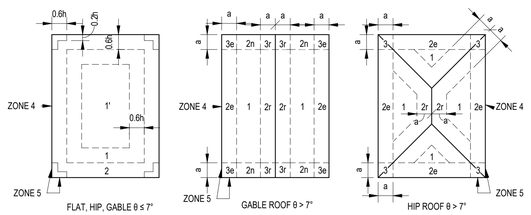
CONNECTORS SHALL BE GALVANIZED AND MANUFACTURED BY SIMPSON STRONG-TIE COMPANY OR APPROVED ALTERNATE. INSTALL ALL CONNECTORS PER MANUFACTURER'S RECOMMENDATIONS. USE TYPE, SIZE AND QUANTITY OF FASTENERS IN CONNECTORS SPECIFIED BY CONNECTOR MANUFACTURER. WHERE FASTENER OPTIONS ARE GIVEN BY THE MANUFACTURER, INSTALL TYPE, SIZE AND QUANTITY OF FASTENERS REQUIRED TO ACHIEVE THE MAXIMUM RATED CONNECTOR CAPACITY. ALL SPECIFIED MANUFACTURED PRODUCTS MAY BE SUBSTITUTED WITH AN EQUIVALENT PRODUCT HAVING THE SAME LOAD CAPACITY AT THE CONTRACTOR'S OPTION.

ALL STEEL CAPS, BRACKETS, HANGERS, BASE PLATES AND BOLTS EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED.

NAILS NOTED IN NOTES, SECTION, PLANS, AND DETAILS ARE COMMON WIRE NAILS, UNLESS NOTED OTHERWISE. PNEUMATIC GUN NAILING IS ALLOWED WITH UNITS OF EQUIVALENT GAGE AND LENGTH AS COMMON WIRE NAIL OF SIZE NOTED. ENSURE THAT ALL OVERDRIVEN OR CROOKED NAILS ARE REMOVED AND PROPERLY REPLACED.

ALL BOLT HEADS AND NUTS BEARING ON WOOD SHALL HAVE STANDARD CUT WASHERS, UNLESS NOTED OTHERWISE. ALL BOLT HOLES IN WOOD SHALL BE DRILLED 1/32" DIAMETER LARGER THAN NORMAL BOLT DIAMETER.

BOLTS IN WOOD SHALL NOT BE LESS THAN 7 DIAMETERS FROM THE END AND 4 DIAMETERS FROM THE EDGE OF THE MEMBER.



GENERAL NOTES:

1. PRESSURES SHOWN IN THE SCHEDULE ARE ULTIMATE LEVEL (FACTORED) LOADS PER ASCE 7-16 AND IBC 2021. WHERE SERVICE LEVEL (UNFACTORED WIND PRESSURES ARE NEEDED FOR DESIGN, THE TABLE PRESSURES SHALL BE MULTIPLIED BY A FACTOR OF 0.60.
2. TABLE PRESSURES ARE FOR THE EFFECTIVE AREA OF COMPONENTS SHOWN. FOR OTHER EFFECTIVE AREAS, LINEAR INTERPOLATE BETWEEN THE TABULATED VALUES.
3. POSITIVE PRESSURE INDICATES PRESSURE ACTING TOWARDS THE BUILDING. NEGATIVE PRESSURES ACT AWAY FROM THE BUILDING.
4. A MINIMUM DEAD LOAD EQUAL TO 5 PSF MAY BE INCLUDED FOR NET ROOF UPLIFT PRESSURES.

h = MEAN ROOF HEIGHT
a = 10% OF THE LEAST HORIZONTAL DIMENSION OR 0.4h, WHICHEVER IS SMALLER, BUT NOT LESS THAN EITHER 4% OF THE LEAST HORIZONTAL DIMENSION OR 3 FT

COMPONENTS AND CLADDING DESIGN PRESSURES

ROOF (PSF)				
ZONE	10 SF	25 SF	50 SF	100 SF
1	32 / -59	27.5 / -47	24 / -38	20 / -29
2	-	-	-	-
3	-	-	-	-
1'	-	-	-	-
2e	32 / -59	27.5 / -47	24 / -38	20 / -29
2n	32 / -65	27.5 / -56	27 / -49	20 / -42
2r	32 / -59	27.5 / -47	27 / -38	20 / -29
3e	32 / -80	27.5 / -68	27 / -59	20 / -50
3r	32 / -65	27.5 / -56	27 / -49	20 / -42
WALLS (PSF)				
ZONE	10 SF	25 SF	50 SF	100 SF
4	35 / -38	33 / -36	31.5 / -34.5	30 / -33
5	35 / -47	33 / -43	31.5 / -40	30 / -37

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