

#### MATERIALS

1. f'c 8" precast lintel = 3500 psi 2. f'c prestressed lintel = 6000 psi 3. Grout per ASTM C476 f'c = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump 4. Concrete Masonry Units (CMU) per ASTM C90 w/minimum net area compressive strength = 1900 psi 5. Rebar per ASTM A615 grade 60 6. Prestressing strand per ASTM A416 grade 270 low relaxation

7. Mortar per ASTM C270 type M or S

### GENERAL NOTES

in lieu of concrete masonry units.

1. Provide full mortar bed and head joints. 2. Shore filled lintels as required.

3. Installation of lintel must comply with the architectural and/or structural documents. 4. U-Lintels are manufactured with 5 1/2" long notches at the ends to accomodate vertical cell reinforcing and grouting 5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or

exceed L/180 deflection. 6. Bottom field added rebar to be located at the bottom of the lintel cavity. 7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage 8. Cast-in-place concrete may be provided in composite lintel

9. Safe load rating based on rational design analysis per ACI 318 and ACI 530 10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05

11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating. 12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:

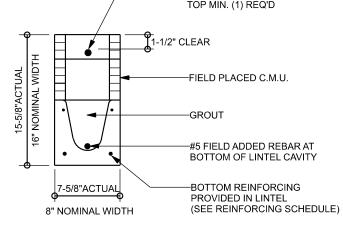
Applied vertical load
Safe vertical load
Safe vertical load
Safe horizontal load

≤1.0 13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

## TYPE DESIGNATION

F = FILLED WITH GROUT / U = UNFILLED / S = SOLID -OUANTITY OF #5 FIELD ADDED REBAR AT

NOMINAL HEIGHT	QUANTITY OF #5 FIELD ADDED REBAR AT TOP
	#5 FIELD ADDED REBAR AT TOP MIN. (1) REQ'D



## SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing. Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches.

2. N.R. = Not Rated 3. Safe loads are superimposed allowable loads. 4. Safe loads based on grade 40 or grade 60

5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only 6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from face of support.

7. For composite lintel heights not shown, use safe load from next lower height shown. 8. For lintels lengths not shown, use safe load from next longest length shown 9. All safe loads in units of pounds per linear

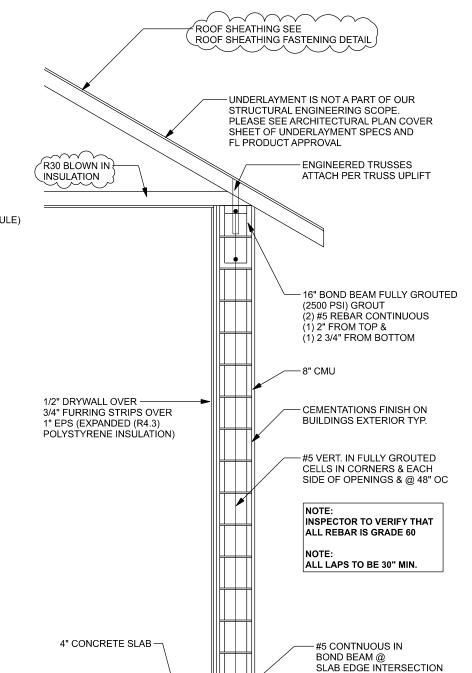
10. All safe loads based on simply supported 11. The number in the the parenthesis indicates the percent reduction for grade 40

field added rebar. Example 7'-6" lintel type 8F32-1B safe gravity load = 6472\H0.0469;(15)\H0.0781; w/ 15% reduction  $6472 \Rightarrow (.85) = 5501 \text{ plf}$ 

	SA	FE GRAVITY LC	ADS FO	R 8" PR	ECAST	& PRES	TRESSE	D U-LIN	TELS		
SAFE LOAD - POUNDS PER LINEAR FOOT											
		TYPE	01.10	8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0E	
LENGT	1		8U8	8F8-1B	8F12-1B	8F16-1B	8F20-1B	8F24-1B	8F28-1B	8F32-1	
01.40"	(0.41) DDE0.107	0004	3069	4605	6113	7547	8974	10394	11809		
2'-10"	(34")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809	
01.01	(4011)	DDEGAGE	0004	3069	3719	5163	6607	8054	9502	10951	
3'-6"	(42")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809	
4'-0"	(48")	PRECAST	1966	2561	2751	3820	4890	5961	7034	8107	
4-0	(40 )	FILLOAGT	1966	2693	4605	6113	7547	8974	10394	11809	
4'-6"	(54")	PRECAST	4500	1969	2110	2931	3753	4576	5400	6224	
4-0	(34 )	TREGAST	1599	2189	4375	6113	7547 <sub>(7)</sub>	8672	10294	11809	
51 4"	(0.411)	DDEGAGE	4047	1349	1438	1999	2560	3123	3686	4249	
5'-4"	(64")	PRECAST	1217	1663	3090	5365	7547(36)	7342 (19)	8733(19)	10127(1	
EL 40"	(70")	DDECACT	4000	1105	1173	1631	2090	2549	3009	3470	
5'-10"	(70")	PRECAST	1062	1451	2622	4360	7168 (45)	6036(19)	7181 <sub>(19)</sub>	8328 (2	
01.011 (7011)	(70!!)	DDEGAGE		1238	2177	3480	3031	3707	4383	5061	
6'-6"	(78")	PRECAST	908	1238	2177	3480	5381	8360	10394(37)	8825 (1	
					1011	1729	2632	2205	2698	3191	3685
7'-6"	(90")	PRECAST	743	1011	1729	2661	3898	5681	8467(44)	6472 (1	
				699	1160	1625	2564	3486	2818	3302	
9'-4"	(112")	PRECAST	554	752	1245	1843	2564	3486	4705(37)	6390(4	
				535	890	1247	2093	2777	2163	2536	
10'-6"	(126")	(126") PRECAST	475	643	1052	1533	2093	2781	3643 (38)	4754 (4	
		PRECAST		582	945	1366	1846	2423	3127	4006	
11'-4"	(136")		PRECAST	362	582	945	1366	1846	2423	3127	4006
		PRECAST			540	873	1254	1684	2193	2805	3552
12'-0"	(144")		337	540	873	1254	1684	2193	2805	3552	
				471	755	1075	1428	1838	2316	2883	
13'-4"	(160")	PRECAST	296	471	755	1075	1428	1838	2316	2883	
				424	706	1002	1326	1697	2127	2630	
14'-0"	(168")	PRECAST	279	442	706	1002	1326	1697	2127	2630	
				NR	NR	NR	NR	NR	NR	NR	
14'-8"	(176")	PRESTRESSED	N.R.	458	783	1370	1902	2245	2517	2712	
	(184")			NR	NR	NR	NR	NR	NR	NR	
15'-4"		PRESTRESSED	N.R.	412	710	1250	1733	2058	2320	2513	
				NR	NR	NR	NR	NR	NR	NR	
17'-4"	(208")	PRESTRESSED	N.R.	300	536	950	1326	1609	1849	2047	
				NR	NR	NR	NR	NR	NR	NR	
19'-4"	(232")	PRESTRESSED	N.R.		418						
				235 ND		750	1037	1282	1515 ND	1716 NR	
21'-4"	(256")	PRESTRESSED	N.R.	NR 190	NR 340	NR 509	NR 045	NR	NR 1350		
				180	340	598	845 ND	1114 ND	1359 ND	1468	
22'-0"	(264")	PRESTRESSED	N.R.	NR 105	NR 245	NR 550	NR 704	NR 1017	NR 4205	NR	
				165	315 ND	550	784	1047	1285	1399	
24'-0"	(288")	PRESTRESSED	N.R.	NR 400	NR	NR 450	NR C54	NR 004	NR 4000	NR	
			1	129	250	450	654	884	1092	1222	

SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

<	(Caete Caete			SAFE LOAD - POUNDS PER LINEAR FOOT						_								
		TYPE	00110	8RF6-0B	8RF10-0B	8RF14-0B	8RF18-0B	8RF22-0B	8RF26-0B	8RF30-0B								
LEN	GTH		8RU6	8RF6-1B	8RF10-1B	8RF14-1B	8RF18-1B	8RF22-1B	8RF26-1B	8RF30-1B								
4'-4"	/EQ!!\	DDECAST	1625	1749	3355	3280	4349	5421	6493	7567								
4 -4	(52")	PRECAST	1635	1891	3699	5206	6639	8060	9479	10893								
4'-6"	(E 4")	DDEGAGE	1494	1596	3063	2992	3968	4946	5924	6904								
4-6	(54")	PRECAST	1494	1756	3699	5206	6639	8060	9479	10893								
5'-8"	(68")	(68") PRECAST	DDECAST	866	920	1770	1716	2277	2839	3402	3966							
0-0	(00)		800	1167	2481	4567	6389	8060 (34)	7917 <sub>(19)</sub>	9311 (19)								
5'-10	" (70")	PRECAST	810	859	1653	1600	2124	2649	3174	3700								
	(, 0 )			010	1113	2342	4242	6639 (10)	8060 (39)	7402 <sub>(19)</sub>	8706 <sub>(19)</sub>							
6'-8"	(80")	PRECAST	797	901	1825	3120	5048	7747	9448	7360								
0-0	(00)	FRECASI	FILLOAGI	TILOAGI	TILOAGI	TRECACT		TILOAGI	TRECAGI	INLUAUI	191	901	1825	3120	5048	7915	9479	10893 (32)
7'-6"	(90")	PRECAST	669	755	1490	2459	3776	5743	7239	5623								
	(90)	INLUAGI	009	755	1490	2459	3776	5743	8998 (19)	10893 (48)								
9'-8"	(116")	PRECAST	411	466	999	1568	2253	3129	4091	3146								
L	(110)	11123,101	411	526	999	1568	2253	3129	4150	5891 <sub>(47)</sub>								



w/ STEMWALL

-#5 STEEL DOWEL

STD HOOK IN FOOTING

UNDISTURBED SOIL OR ENGINEERED FILL

# TYPICAL WALL SECTION (1 STORY)

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL PER FBC 2020 7th Edition, (107.3.5 building)

ATTACH CMU WALL TO SLAB @-

) LEG LAPPED w/

SPACED @ 48" OC

24" X 10" POURED -

CONCRETE STRIP FOOTING

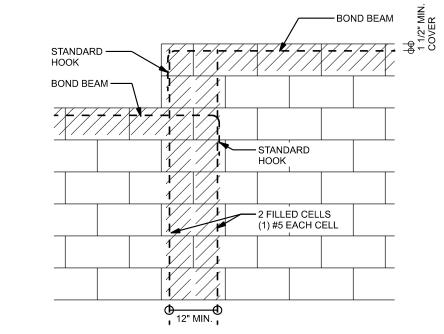
(3) #5 REBAR CONTINUOUS

BOND BEAM w/ 2' X 2' #5 CORNER REBAR 1) LEG EXTENDING INTO SLAB &

THE HORIZONTAL BOND BEAM REBAR

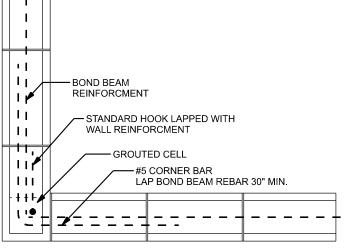
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" oc	6" oc

of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



#### CHANGES IN BOND BEAM HEIGHT (BASED ON FBC FIG. R609.2.5)

SCALE: 1/2" = 1'-0"

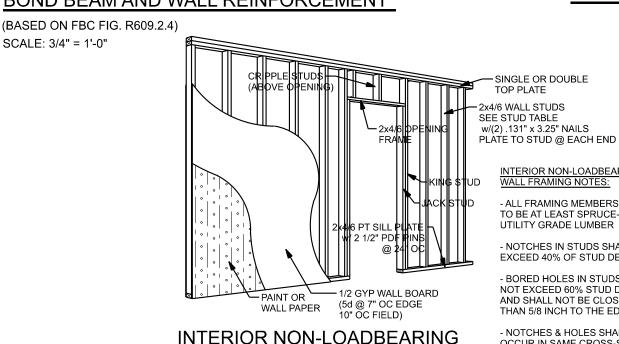


## CORNER CONTINUITY OF

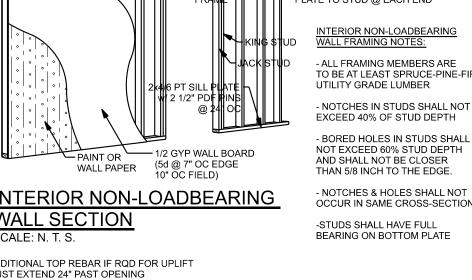
SCALE: 1/2" = 1'-0"

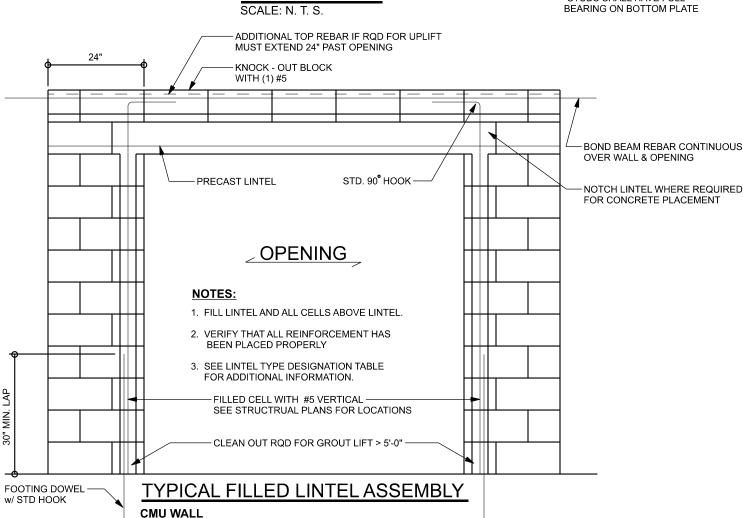
SCALE: 3/4" = 1'-0"

BOND BEAM AND WALL REINFORCEMENT



**INTERIOR NON-LOADBEARING** WALL SECTION



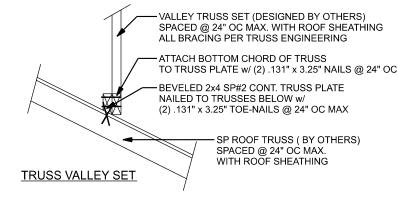


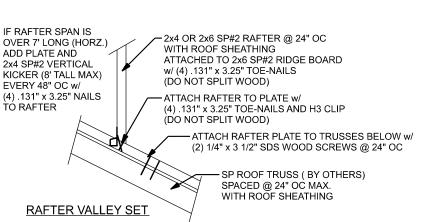
(6) 1/2" x 8" ANCHOR BOLTS PER BUCK EVENLY SPACED (ANCHOR BOLTS ARE INTO REINFORCED CELL AT EACH SIDE OF DOOR)

ACCORDANCE WITH ANSI/AMMA/NWWDA 101/IS2 STANDARDS AND BEAR AN AMMA OR WDMA LABEL IDENTIFYING THE MANUFACTURER,

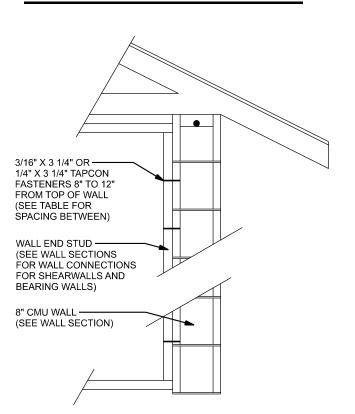
ENTITY (FBC 1707.4.21)

FOR WINDOW AND DOOR ANCHORAGE



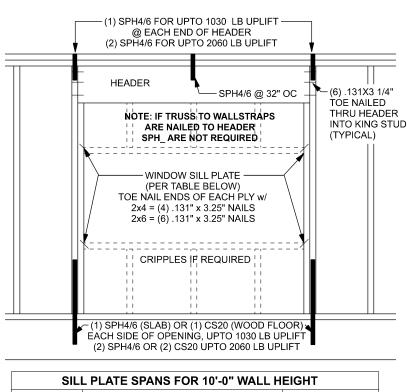


## VALLEY SET FRAMING DETAIL



WALL TYPE:	3/16" X 3 1/4" TAPCON MAX SPACING	1/4" X 3 1/4" TAPCON MAX SPACING
INTERIOR SHEAR WALL	6" OC	8" OC
INTERIOR BEARING WALL	16" OC	16" OC
INTERIOR NON-BEARING WALL	48" OC	48" OC

#### INT. FRAME WALL TO CMU CONNECTION



MAX. SPANS FOR SPF #2 (1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6 FOR OTHER WALL 4'-4" 6'-6" 6'-5" 9'-6" HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

#### TYP. FRAME OPENING DETAILS STRAPS ON WD FLOOR OR SLAB N.T.S.

HIP ROOF  $(7^{\circ} < 0 < 45^{\circ})$ a = 10% of least horizontal dimension or 0.4 a h, whichever is smaller, but not less than either 4% of least horizontal dimension or 3 ft (0.9 m). **Exception:** For buildings with  $\theta = 0^{\circ}$  to  $7^{\circ}$  and a least horizontal dimension greater than 300 ft (90 m), dimension a shall be limited to a maximum of 0.8 h. h = Mean roof height, in ft (m), except that eave height shall be used for roof  $\theta = Angle of plane of roof from horizontal, in degrees.$ Pressures shown are applied normal to the surface, for Exposure B, at h = 30

ft (9.1 m). Adjust to other conditions using Eq. (30.4-1). . Plus and minus signs signify pressures acting toward and away from the For hip roofs with  $\theta \le 25^{\circ}$ , Zone 3 shall be treated as Zone 2e and 2r. For effective wind areas between those given, values may be in terpolated;

effective wind area. If overhangs exist, the lesser horizontal dimension of the building shall not include any overhang dimension, but the edge distance, a, shall be measured from the outside edge of the overhang.

otherwise use the value associated with the lower

## GARAGE DOOR BUCK ATTACHMENT

GARAGE DOOR BUCKS w/ AB UP TO 18'W

ALL EXTERIOR WIDOWS AND GLASS GLASS DOORS ARE TO BE TESTED IN 1000 860 MTS12-30 PERFORMANCE CHARACTERISTICS, AND APPROVED PRODUCT TESTING

SEE ATTACHED MANUFACTURERS RECOMMENDATAIONS



CONNECTOR TABLE

Jolift SP Uplift SPF Truss Connector

To Truss/Rafter

12-16d		5/8"x7" Drill	& Epox	y
GRA	DE & SP	ECIES TA	BLE	
			Fb	ı
2x8	SF	<sup>2</sup> #2	925	1
2x10	SF	° #2	800	1
2x12	SF	<sup>9</sup> #2	750	1
GLB	24F-\	V3 SP	2600	1
LSL	TIMBER	STRAND	1700	1
LVL	MICR	OLAM	2950	2
PSL	PARA	ALAM	2900	2

#### NOTE: THIS BUILDING IS LOCATED IN THE WINDBORNE DEBRIS REGION THE BUILDING MUST PROVIDE PROTECTION OF OPENINGS PER FBCR R301.2.1.2 IF PRECUT 7/16" MIN WOOD STRUCTURAL PANELS ARE USED THEY ARE TO BE ATTACHED AS PER FBCR TABLE R301.2.1.2 (SHOWN BELOW)

TABLE R301.2.1.2 WINDBORNE DEBRIS PROTECTION FASTENING SCHEDULE FOR WOOD STRUCTURAL PANEL Sa, b, c, d

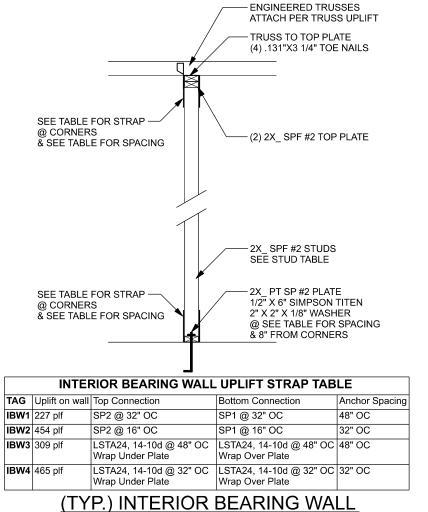
	FASTENER SPACING (inches)a, b				
FASTENER TYPE	Panel span ≤ 4 feet	THE RESERVE OF THE PARTY OF THE	6 feet < panel span ≤ 8 feet		
lo. 8 wood screw based nchor with 2-inch embedment ength	16	10	8		
lo. 10 wood screw based nchor with 2-inch embedment ength	16	12	9		
/ <sub>4</sub> -inch lag screw based nchor with 2-inch embedment ength	16	16	16		

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound = 4.448 N, 1 mile per hour = 0.447 m/s. a. This table is based on V<sub>sed</sub>, as determined in accordance with Section R301.2.1.3, of 130 mph and a 33-foot mean roof height.

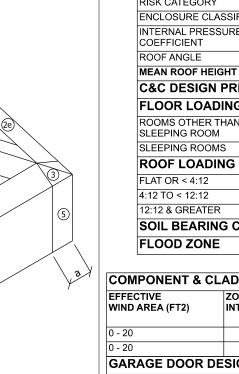
b. Fasteners shall be installed at opposing ends of the wood structural panel. Fasteners shall be located a minimum of 1 inch from the edge of the panel.

c. Anchors shall penetrate through the exterior wall covering with an embedment length of 2 inches minimum into the building frame. Fasteners shall be located a minimum of 2<sup>1</sup>/<sub>2</sub> inches from the edge of concrete block or concrete.

d. Where panels are attached to masonry or masonry/stucco, they shall be attached using vibrationresistant anchors having a minimum ultimate withdrawal capacity of 1500 pounds.



ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



# 9x7 GARAGE DOOF 16x7 GARAGE DOOR +24.8(Vasd) -27.6(Vasd)

### **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS

WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END. SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN OUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302, JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 \* DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

RACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

#### **BUILDER'S RESPONSIBILITY:** THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE

SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS. FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL

THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

#### **ROOF SYSTEM DESIGN:**

TRUSS SHEETS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIETS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN RECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approve
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet met ties not completely embedded in mortar ogrout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for typ and location of movement joints if not detailed on project drawings.

DESIGN CRITERIA & LOAD	
BUILDING CODE	7TH EDITION   FLORIDA BUILDING CODE RESIDENTIAL   (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	140 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	С
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
<b>C&amp;C DESIGN PRESSURES</b>	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD Z

COMPONENT &	CLADING DESIG	N PRESSUR	RES 140 MPH (	Vult) (EXP
EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR		ZONE 5 END 4' FROM AL OUTSIDE CORN	_
0 - 20	+29.7(Vasd)	-30.8(Vasd)	+29.7(Vasd)	-39.2(Vasd)
0 - 20	+47.6(Vult)	-49.3(Vult)	+47.6(Vult)	-62.8(Vult)
<b>GARAGE DOOR</b>	<b>DESIGN PRESS</b>	URES 140 M	PH (Vult) (EXP	C)
DVZ CARACE DOOR	125	0(\/ood) 20.2	(Vaad)	

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DIMENSIONS:

Stated dimensions supercede scaled

dimensions. Refer all questions to

Mark Disosway, P.E. for resolution

Do not proceed without clarification

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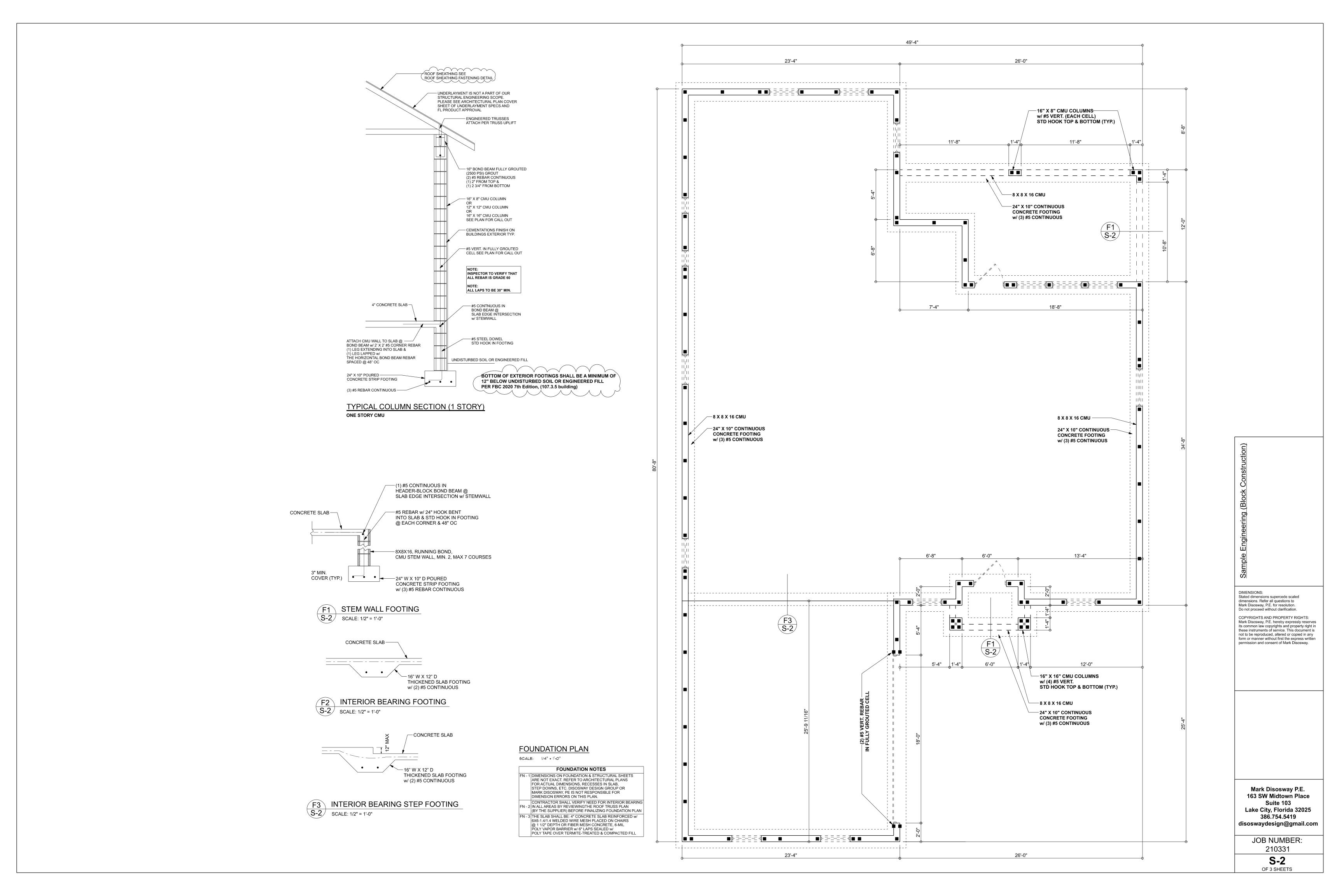
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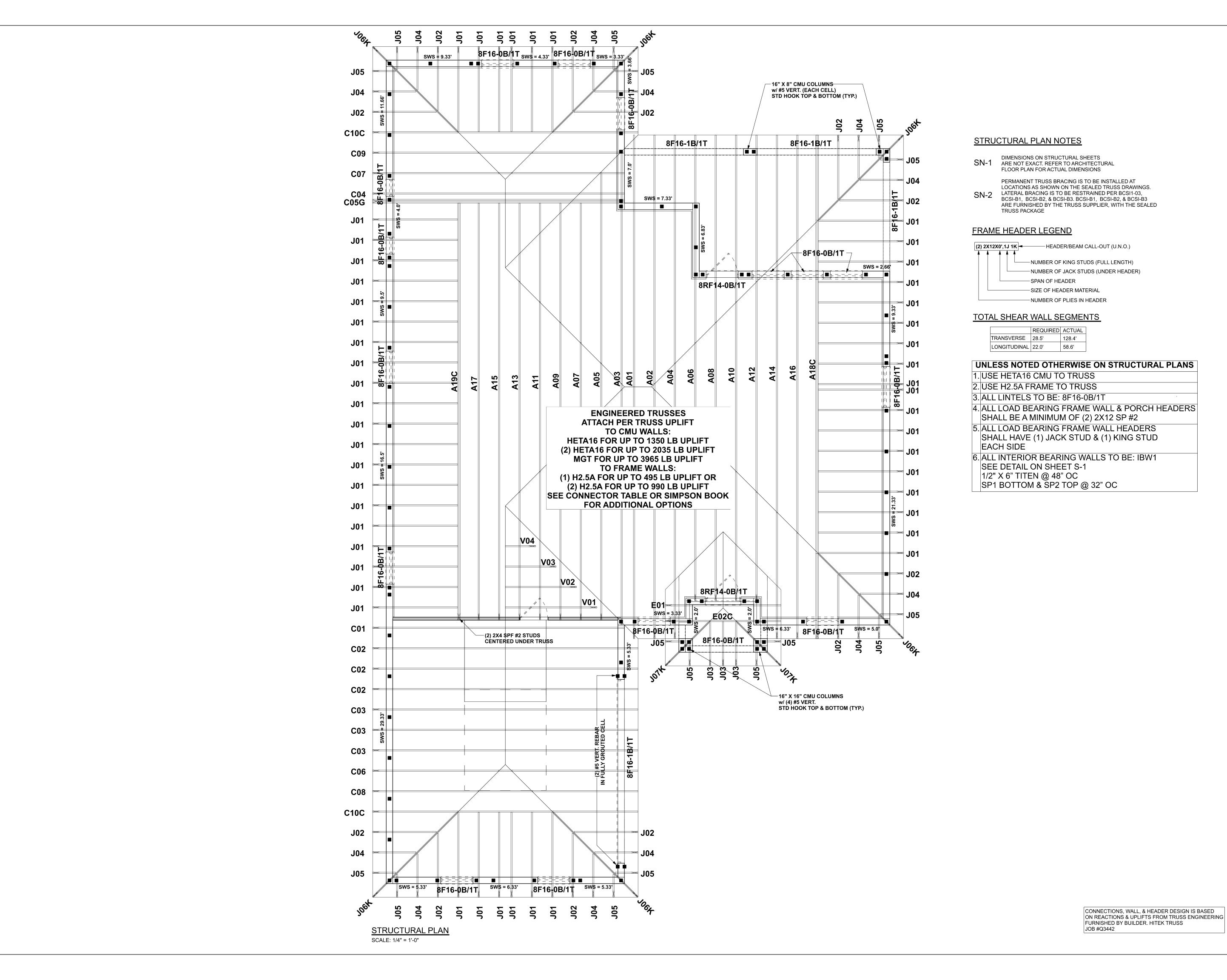
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JOB NUMBER:

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