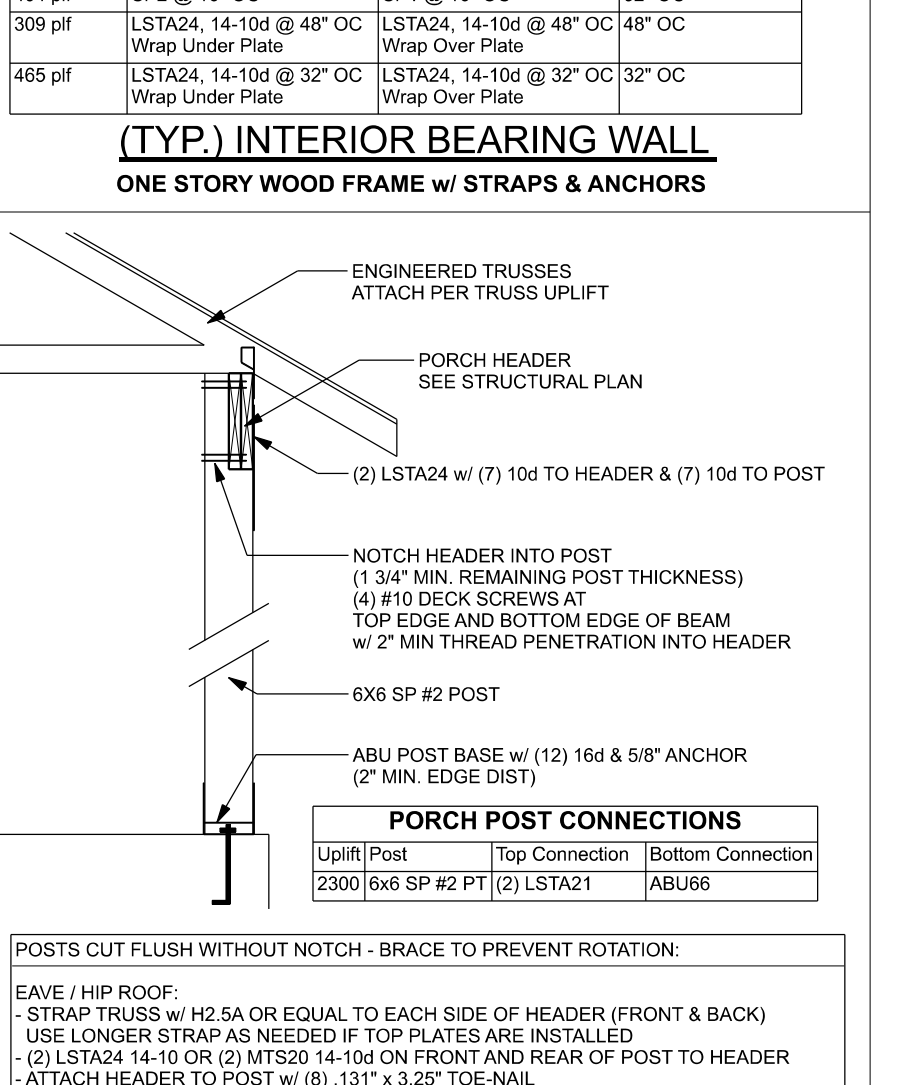
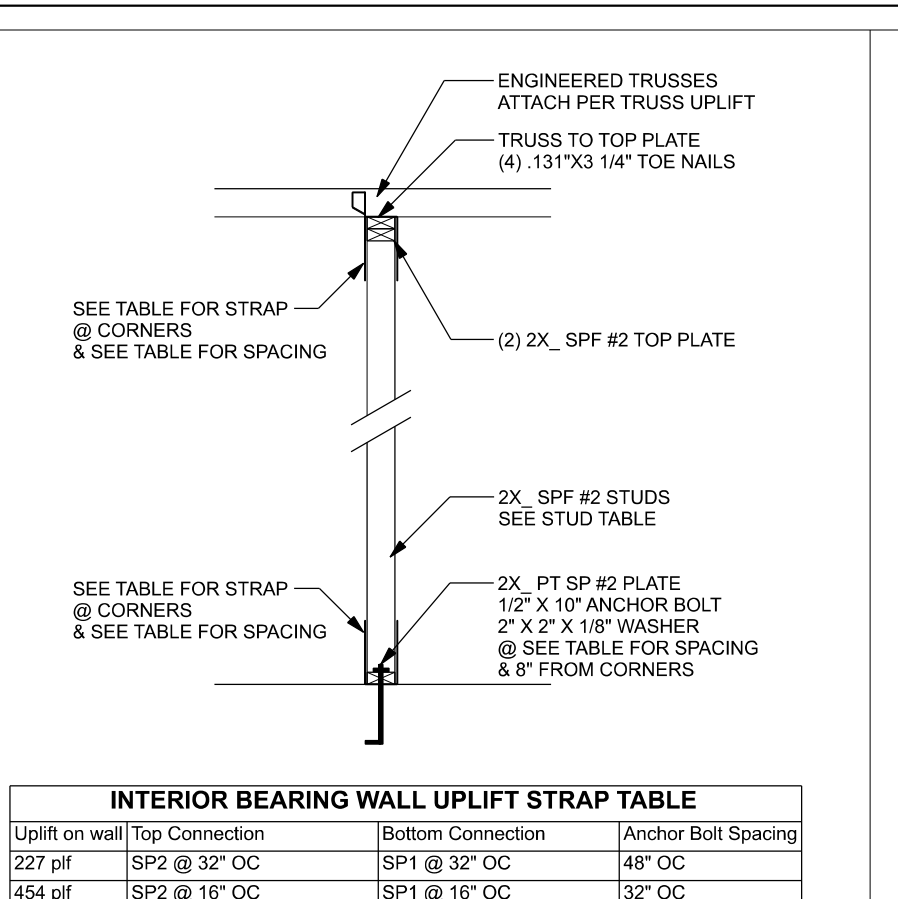
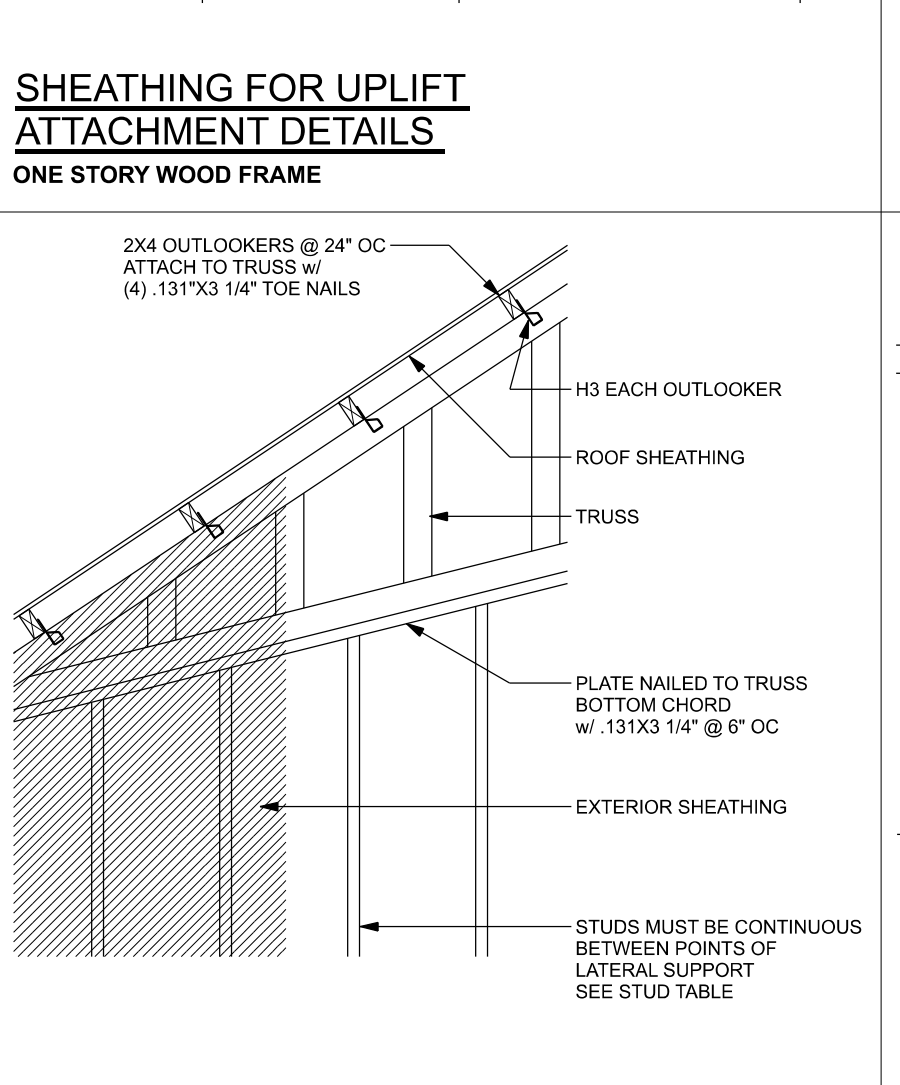
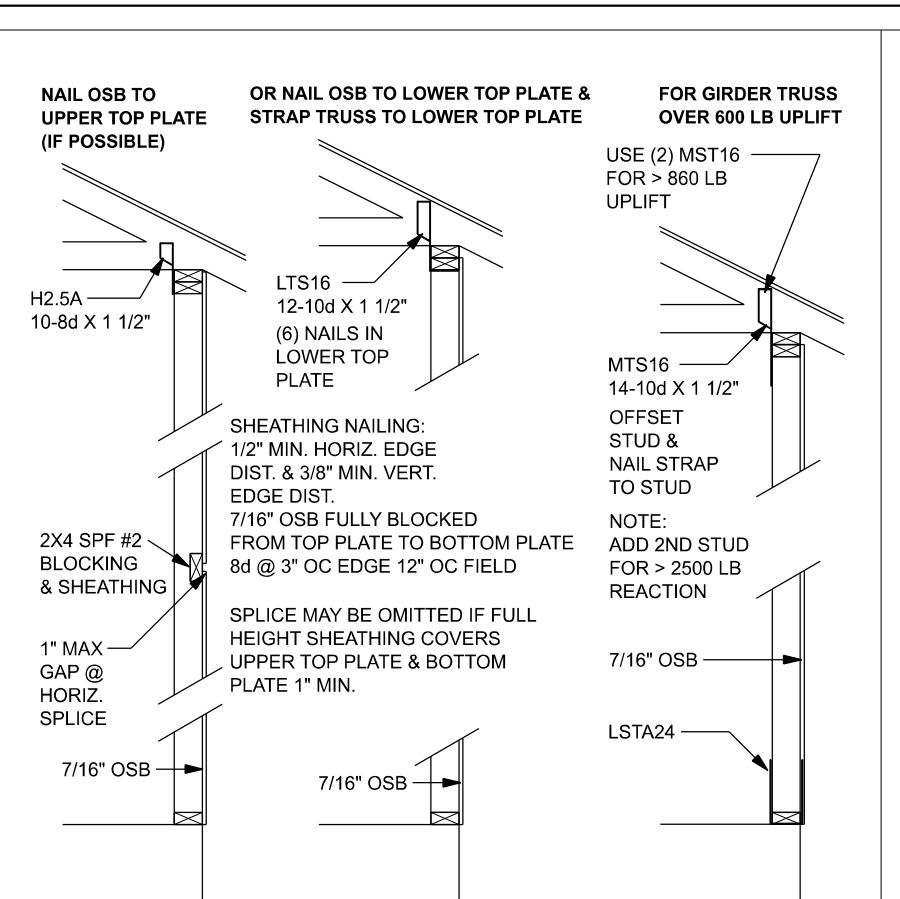
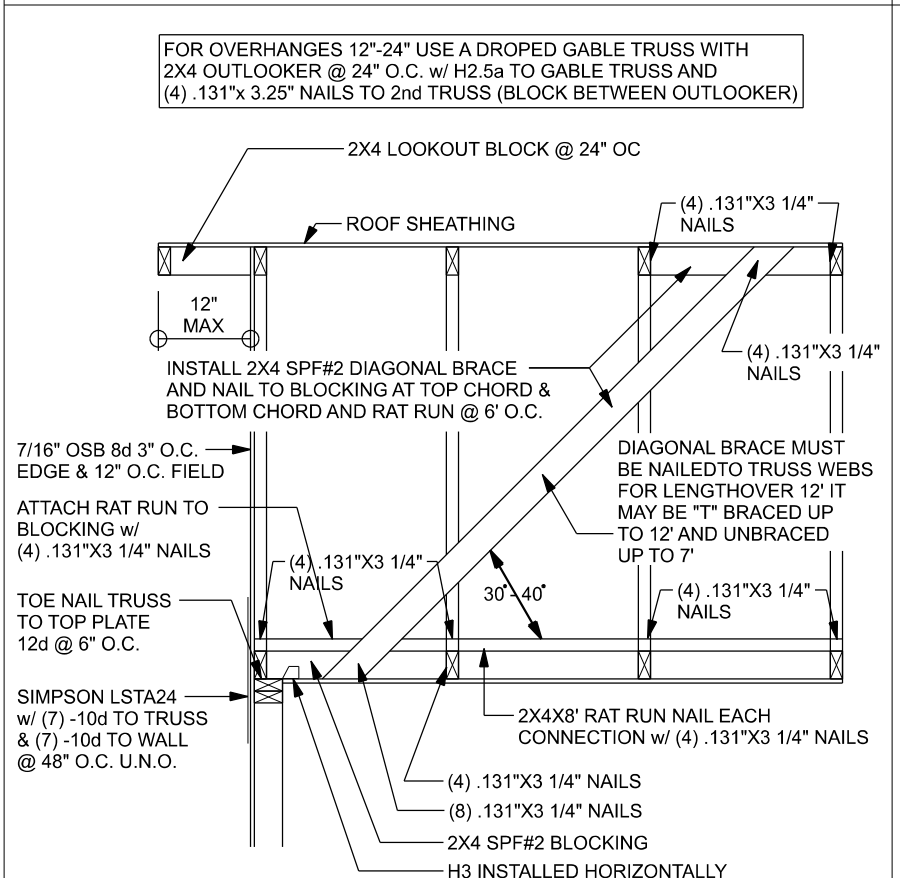


**ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)**

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	12" oc
120 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	6" oc
120 mph Exp. C	15/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
130 mph	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. B	15/32"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
140 mph	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. B	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
150 mph	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.  
Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



**CONNECTOR TABLE**

Uplift SP/ Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	
415	290	H3	4-8x1 1/2"
575	485	H2.5A	5-8x1 1/2"
1340	1015	H10A	9-10x1 1/2"
720	620	LTS12-20	6-10x1 1/2"
1000	860	MST12-30	7-10x1 1/2"
1450	1245	HTS20-30	12-10x1 1/2"
Uplift SP/ Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA1	8-10x1
1640	1455	MSTA24	9-10x1
1030	1030	CS20	7-10x1
Uplift SP/ Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10x1
1065	665	SP2	6-10x1
771	771	LSTA24	10-10x1
1235	1235	LSTA24	14-10x1
Uplift SP/ Uplift SPF	Holdowns @ Stenwall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"
4235	3940	HTT4	18-16x2 1/2"
Uplift SP/ Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"
4235	3940	HTT4	18-16x2 1/2"
Uplift SP/ Uplift SPF	Post Bases @ Stenwall	To Post	Anchor
2200		ABU44	12-16x1
2300		ABU66	12-16x1
Uplift SP/ Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200		ABU44	12-16x1
2300		ABU66	12-16x1

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD DEFLECTION SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

Stud Height	Stud Spacing	Stud Height
(1) 2x4 @ 16" OC		TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC		TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC		TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC		TO 17'-3" STUD HEIGHT

**GRADE & SPECIES TABLE**

Grade	Species	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, LAYOUT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED AND SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS SIGNED AND SEALED ALL TRUSS CONNECTIONS AND UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER TO FURNISH TRUSS ENGINEERS TO WIND LOAD ENGINEER FOR REVIEW OF THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SHEETS.

FOUNDATION: FOUNDATION DESIGN & PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OF SOILS TEST INDICES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x W1.4, F<sub>y</sub> = 80ksi, WELDED WIRE REINFORCEMENT FABRIC: (W.W.M.) CONFORMING TO ASTM A1815, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSEAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS SHOULD NOT BE INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F<sub>y</sub> = 40 KSI, ALL LAP SPICES 4' DB (25' FOR #5 BARS), UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-16, U.L.C.

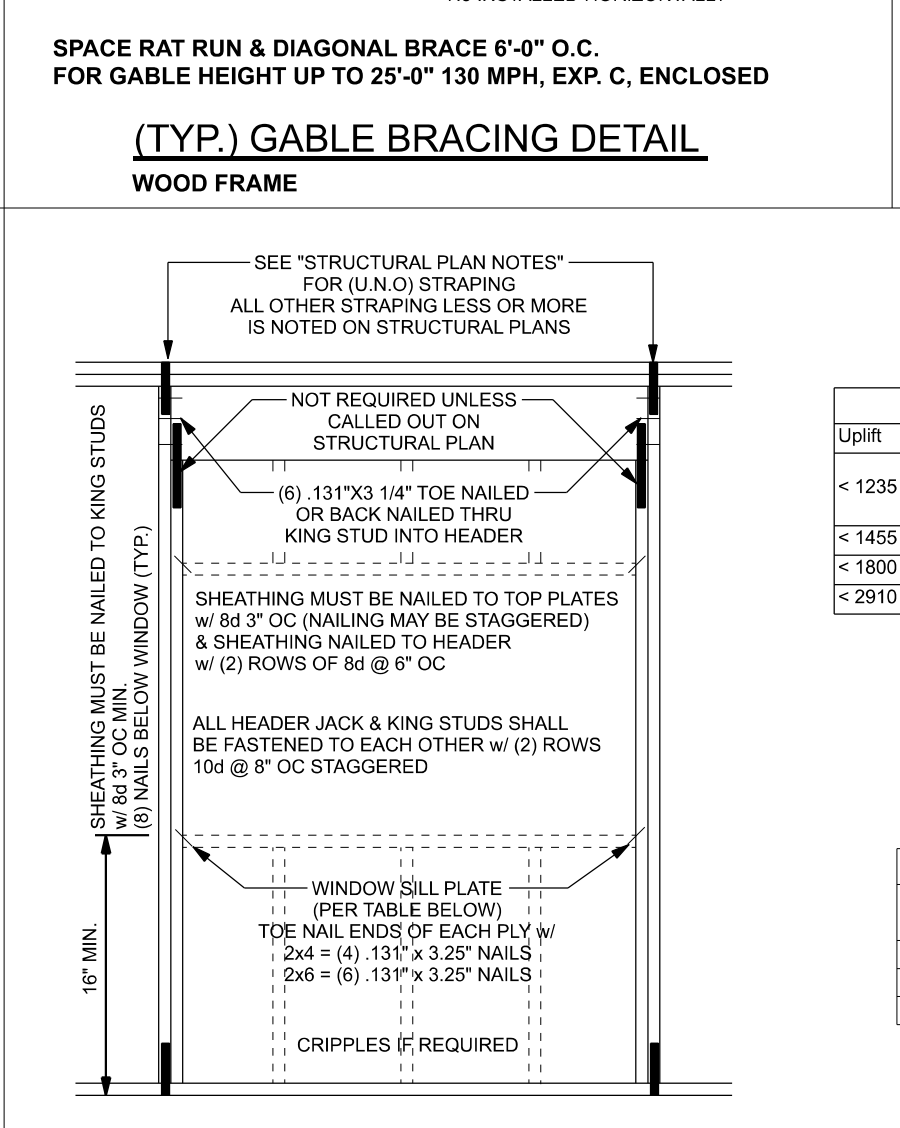
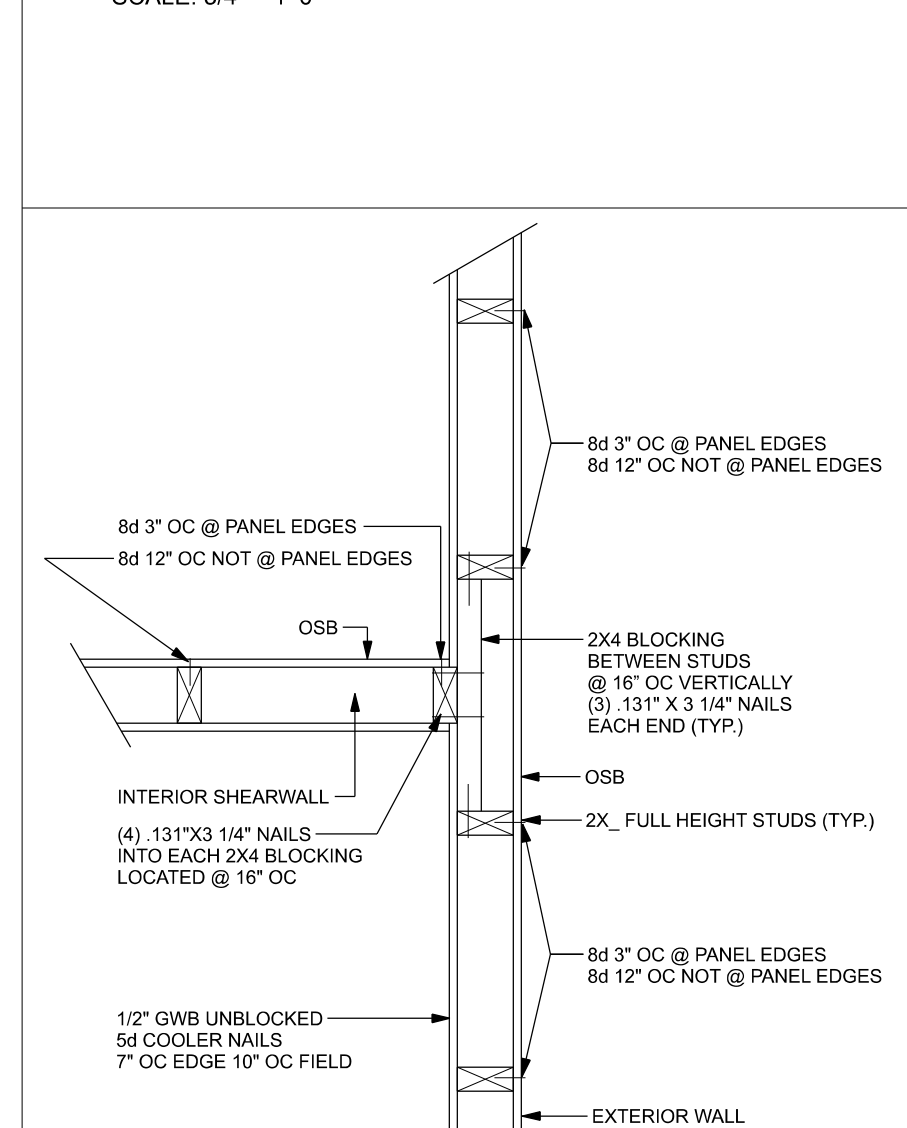
ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUTED CMU.

**BUILDER'S RESPONSIBILITY:**  
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:  
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.  
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBGIR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.  
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN O.M.T.S.A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.  
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, LAYOUT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN:**  
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBER REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

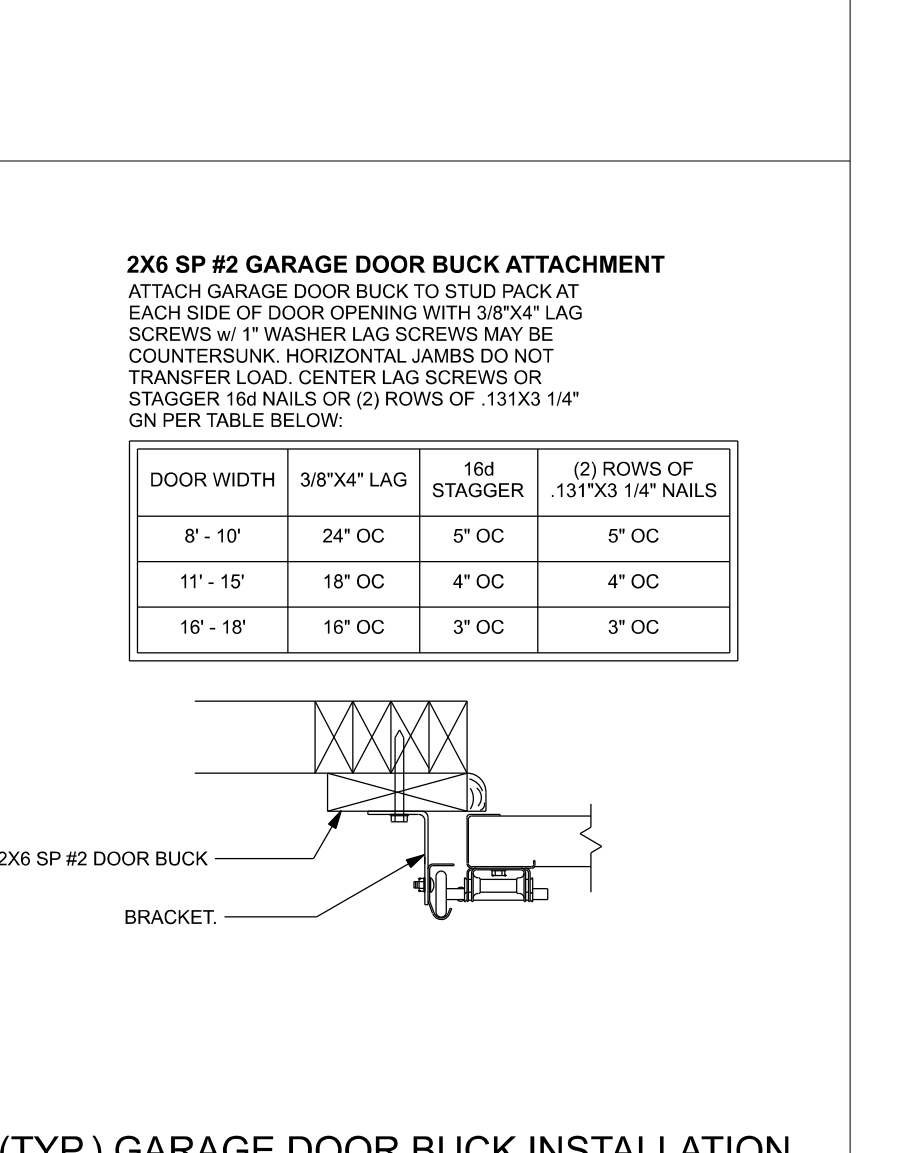
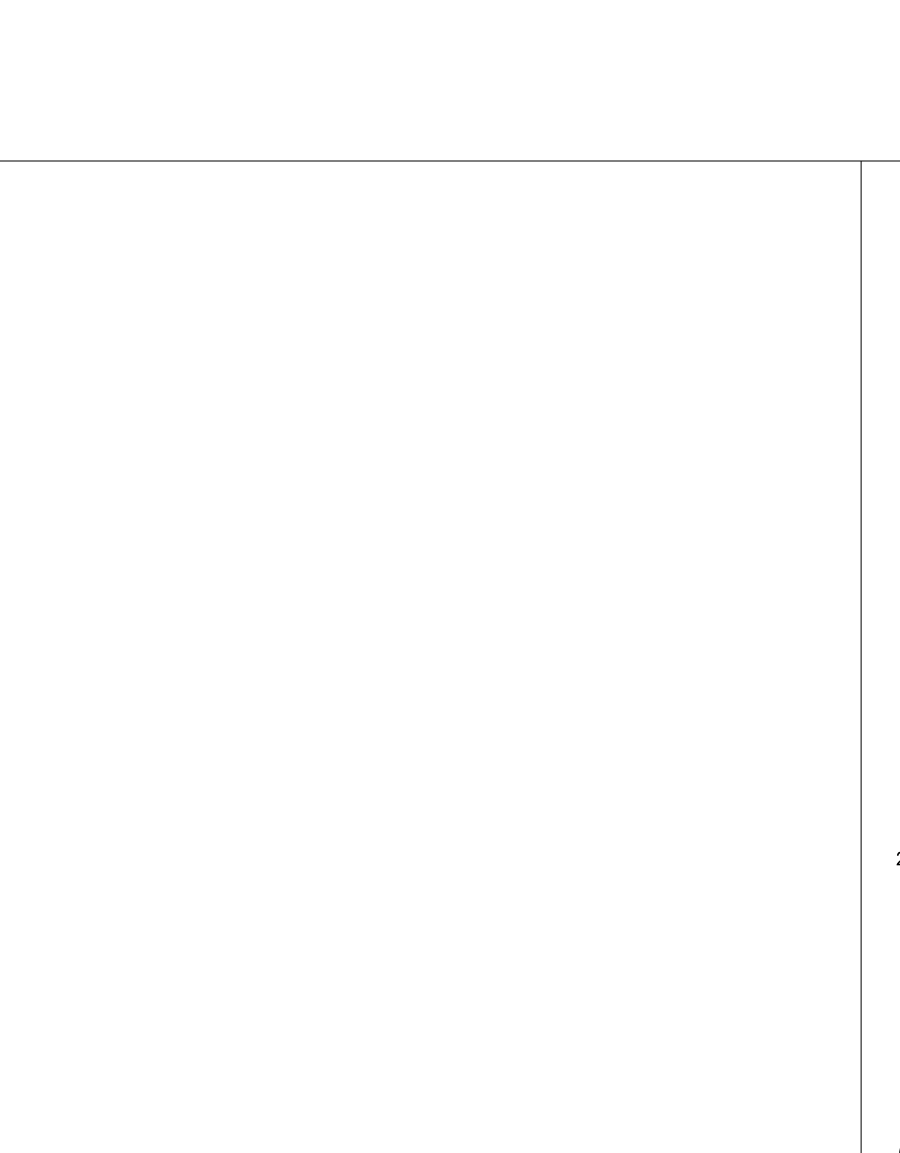
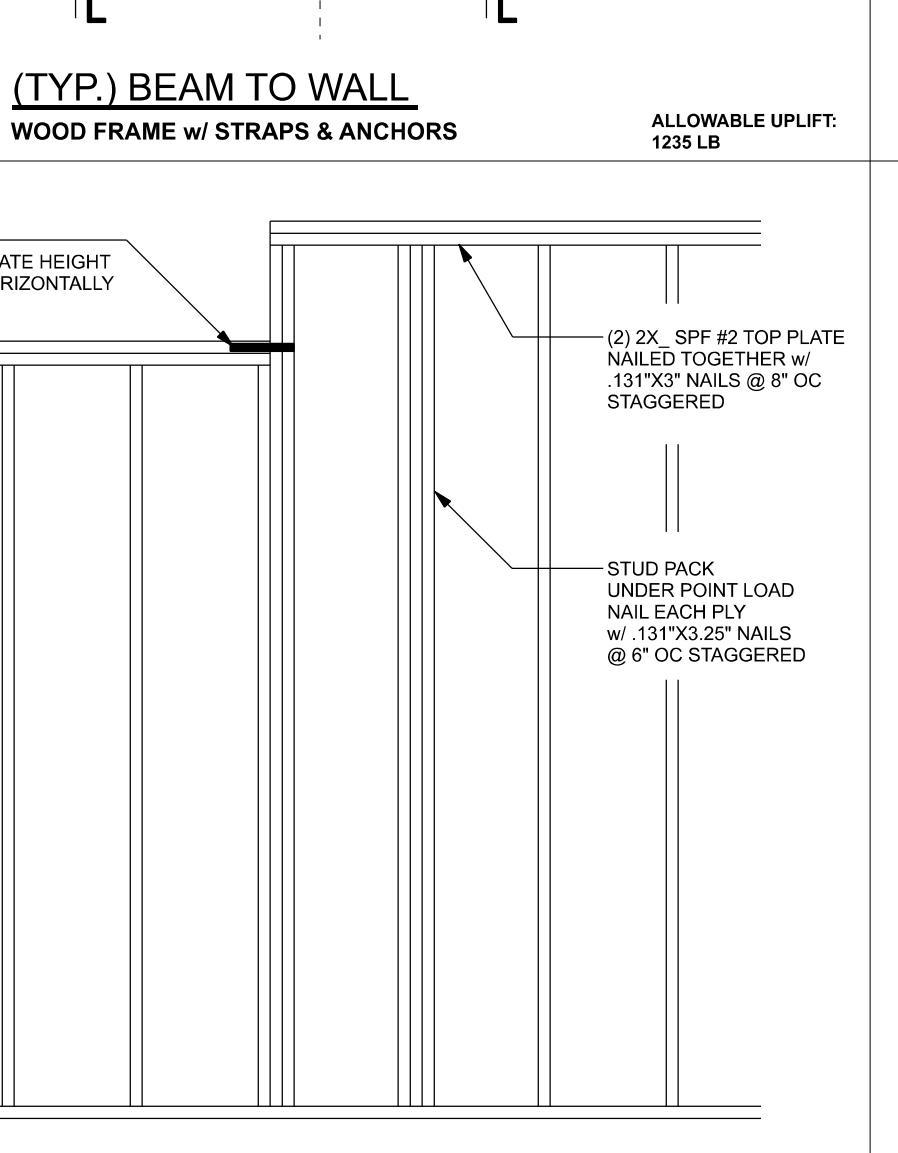
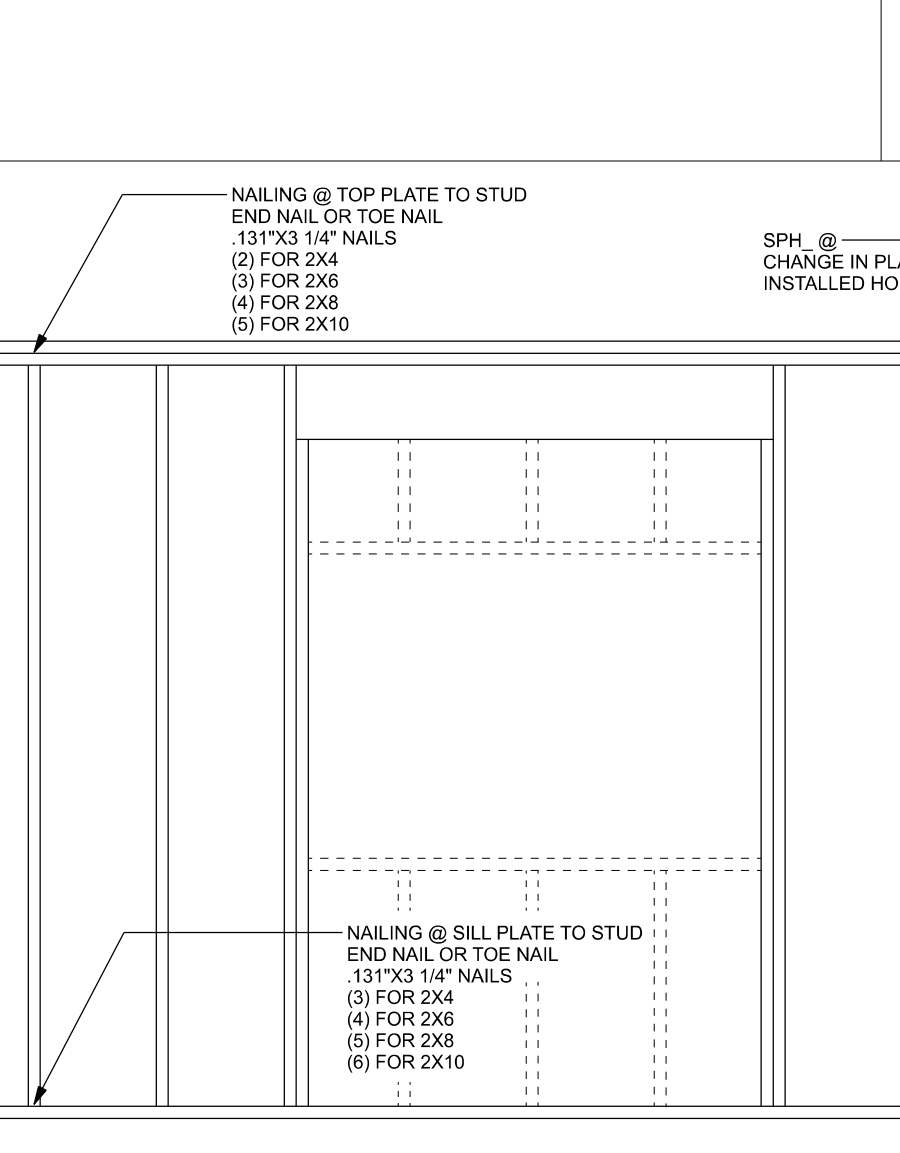
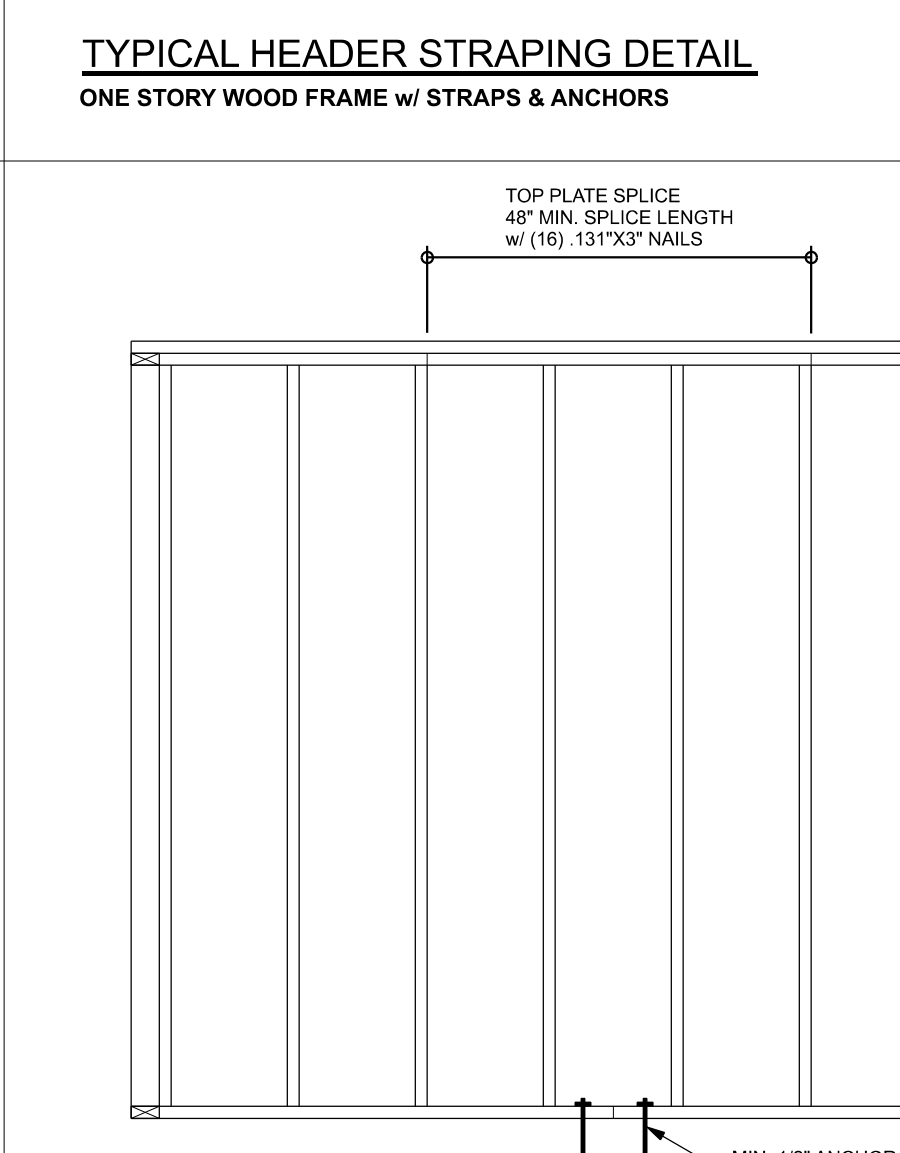
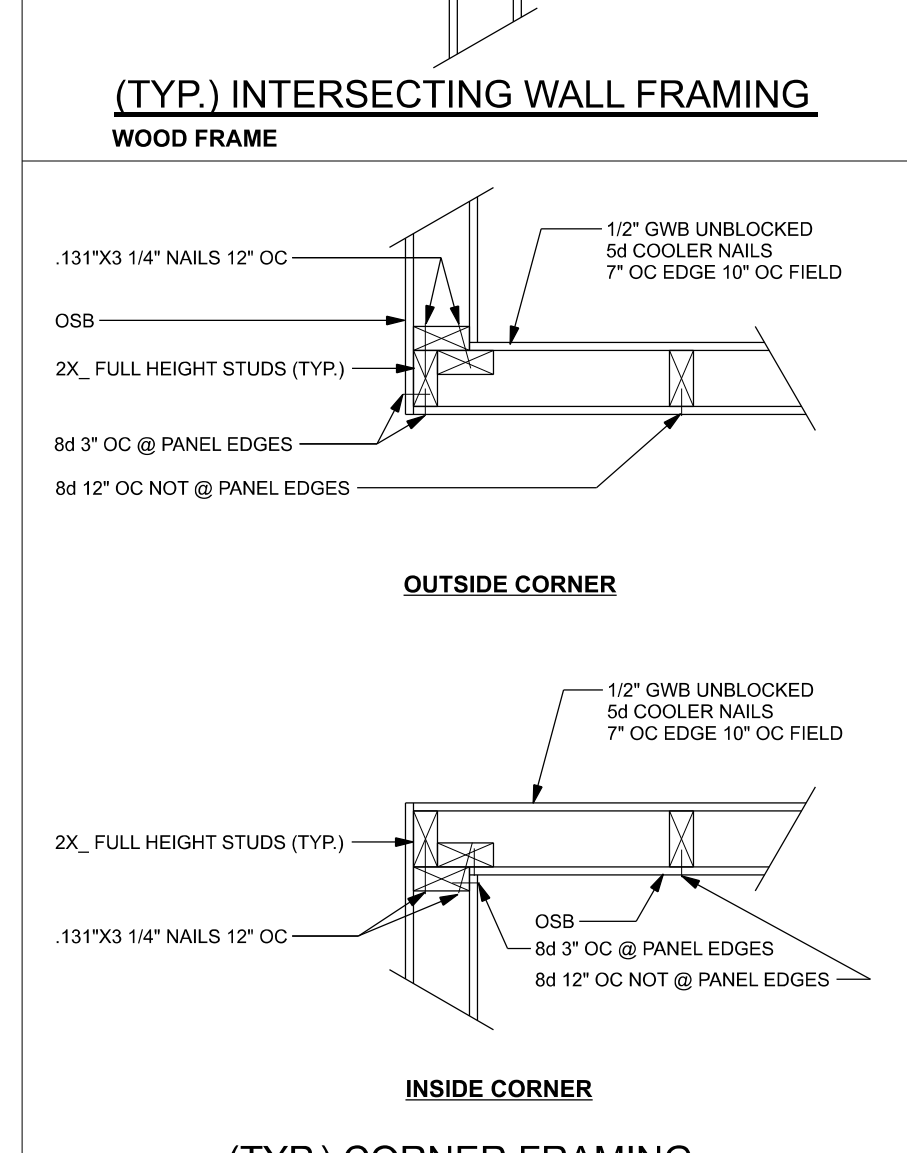
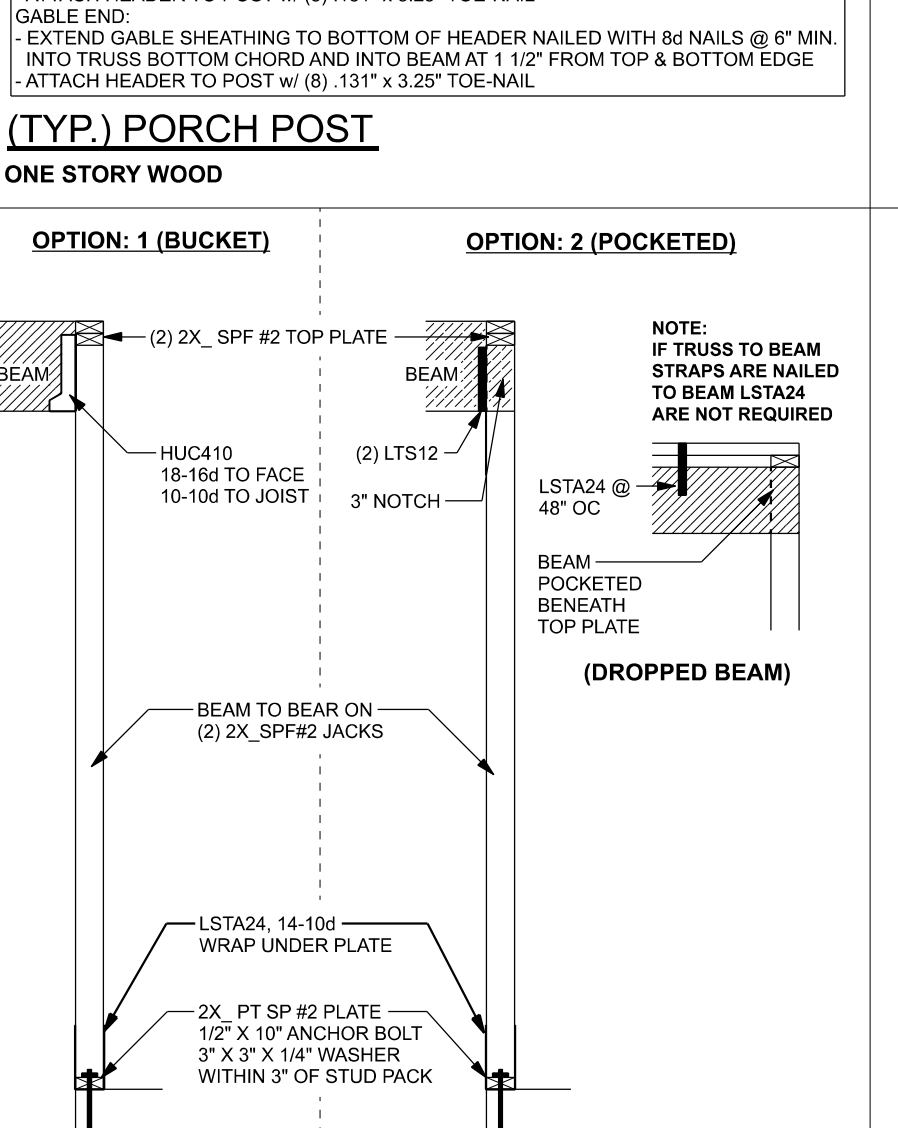


**HEADER STRAP TABLE**

Uplift	Top Connection	Bottom Connection
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate
< 1455	MSTA24, 18-10d header to jacks	DTT22
< 1800	(2) MST24, 18-10d header to jacks	DTT22
< 2910	(2) MST24, 18-10d header to jacks	HTT4

**SILL SPANS FOR 10'-0" WALL HEIGHT**

Design	Max. Spans for SPF #2	Based on WFCM
WIND SPEED	(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6	BASED ON WFCM TABLE A-3.2.8
130 MPH EXP. C	5'-2" 7'-9" 7'-7" 11'-3"	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)



**DESIGN CRITERIA & LOADS:**

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)	
CODE FOR DESIGN LOADS	ASCE 7-16	
<b>WINDLOADS</b>		
BASIC WIND SPEED (ASCE 7-10, SS (SUST))	130 MPH	
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C	
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	II	
RISK CATEGORY	II	
ENCLOSURE CLASSIFICATION	ENCLOSED	
INTERNAL PRESSURE COEFFICIENT	0.18	
ROOF ANGLE	7-45 DEGREES	
MEAN ROOF HEIGHT	30 FT	
<b>C&amp;C DESIGN PRESSURES</b>	SEE TABLE	
<b>FLOOR LOADING</b>		
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD	
SLEEPING ROOMS	30 PSF LIVE LOAD	
<b>ROOF LOADING</b>		
FLAT OR < 4:12	20 PSF LIVE LOAD	
4:12 TO < 12:12	16 PSF LIVE LOAD	
12:12 & GREATER	12 PSF LIVE LOAD	
SOIL BEARING CAPACITY	1500 PSF	
<b>FLOOD ZONE</b>	THIS BUILDING IS NOT IN THE FLOOD ZONE	
<b>COMPONENT &amp; CLADDING DESIGN PRESSURES 130 MPH (EXP C)</b>		
EFFECTIVE WIND AREA (FT <sup>2</sup> )	ZONE 4 INTERIOR	ZONE 5 END 4 FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(Vult) -46.2(Vult)	+42.6(Vult) -57(Vult)
<b>GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)</b>		
8x7 GARAGE DOOR	+22.8(Vasd) -25.5(Vasd)	
16x7 GARAGE DOOR	+21.7(Vasd) -24.1(Vasd)	

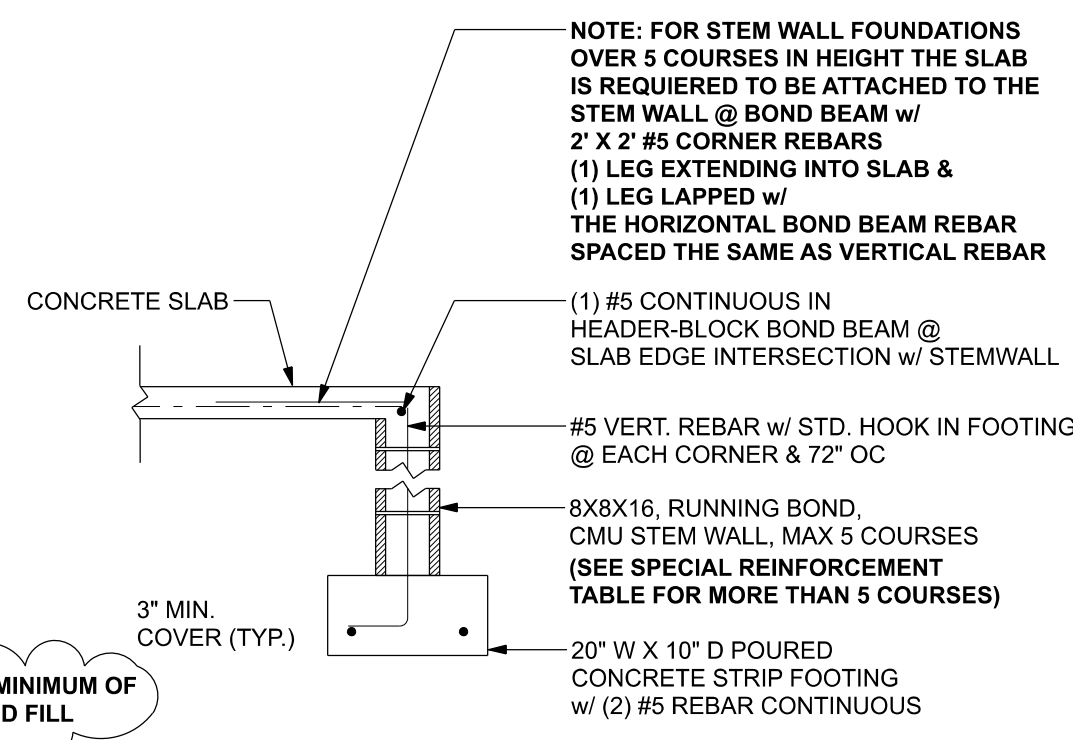
**Sample Engineering (Frame Construction)**

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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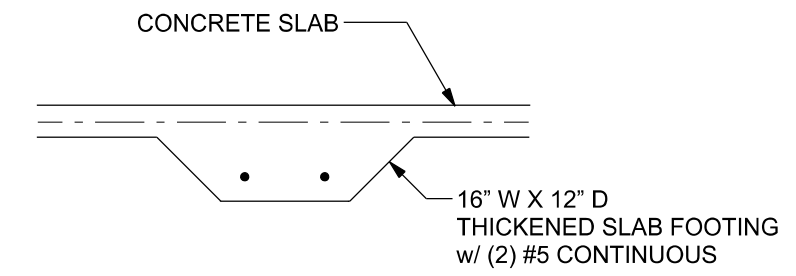
**Mark Disoway P.E.**  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disowaydesign@gmail.com

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201218  
**S-1**  
OF 3 SHEETS

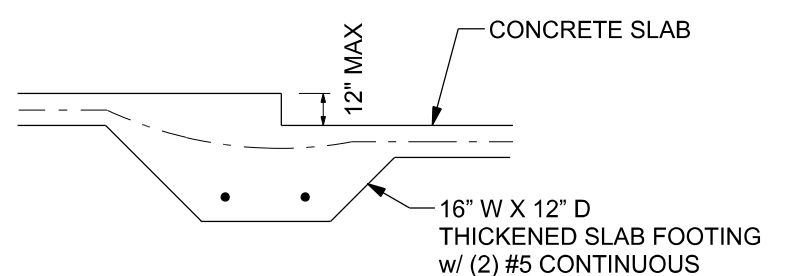


BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12\"/>

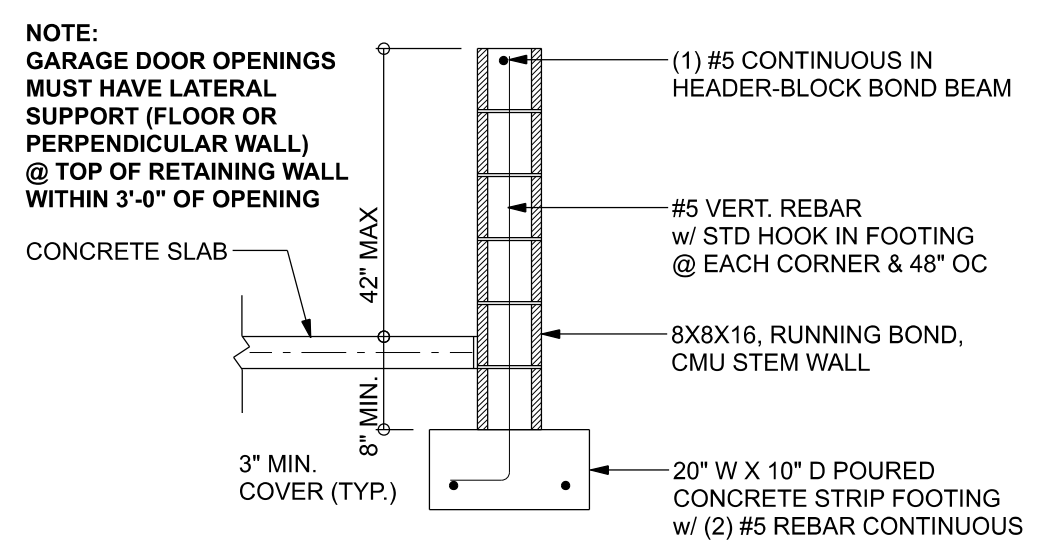
**F1 S-2** OPTIONAL STEM WALL FOOTING  
SCALE: 1/2\"/>



**F2 S-2** INTERIOR BEARING FOOTING  
SCALE: 1/2\"/>



**F3 S-2** INTERIOR BEARING STEP FOOTING  
SCALE: 1/2\"/>



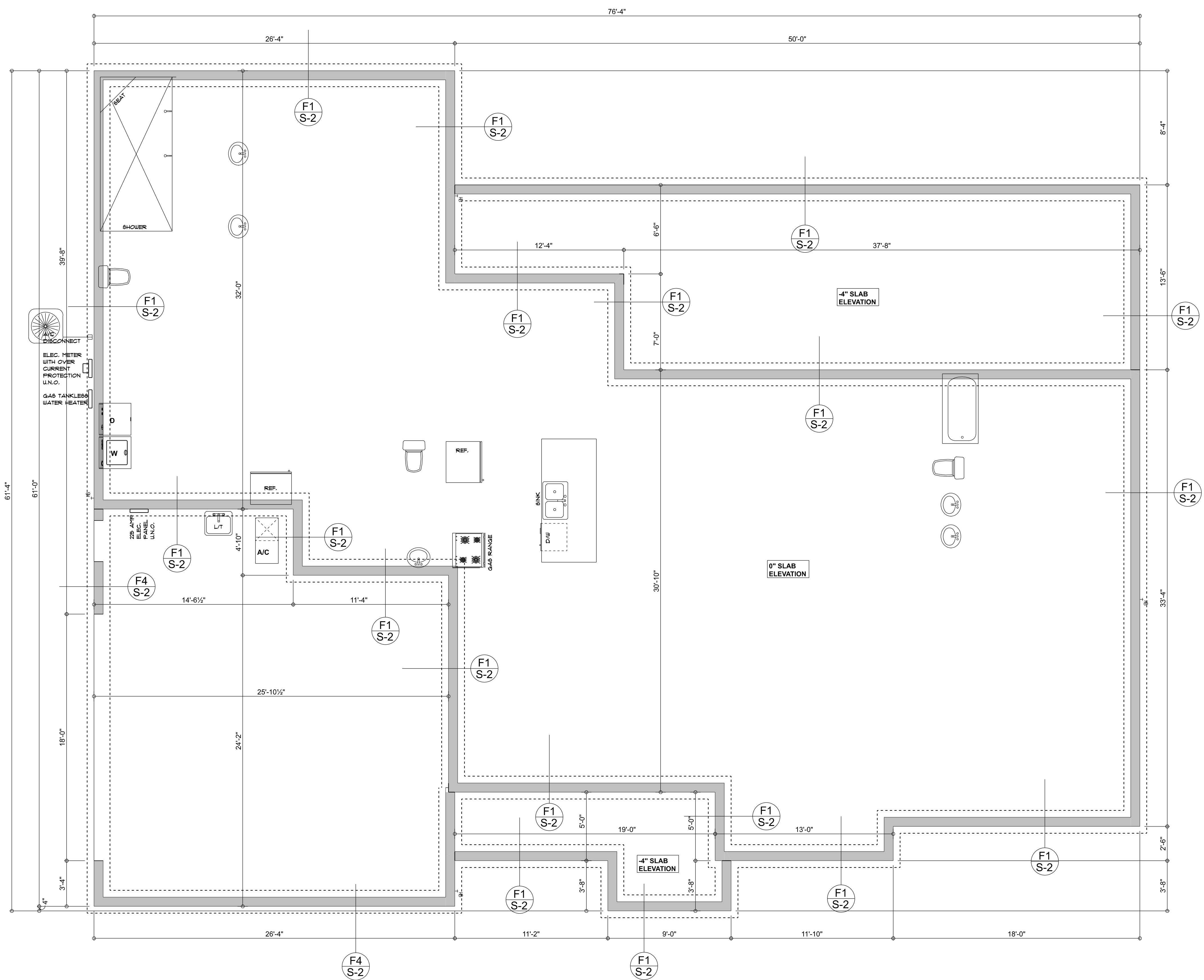
**F4 S-2** OPTIONAL STEM WALL CURB FOOTING  
SCALE: 1/2\"/>

**TALL STEM WALL TABLE:**  
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6\"/>

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8\"/>		
		#5	#7	#8
3.3	3.0	96	96	96
4.0	3.7	96	96	96
4.7	4.3	88	96	96
5.3	5.0	56	96	96
6.0	5.7	40	80	96
6.7	6.3	32	56	80
7.3	7.0	24	40	56
8.0	7.7	16	32	48
8.7	8.3	8	24	32
9.3	9.0	8	16	24

**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 8/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8\"/>
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, hollow, medium surface finish, 8\"/>
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5\"/>
2.4 Reinforcing bars, #3 - #11	ASTM A63, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25\"/>
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or air ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



**FOUNDATION PLAN**  
SCALE: 1/4\"/>

**FOUNDATION NOTES**

**FN - 1** DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

**FN - 2** IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN BY THE SUPPLIER BEFORE FINALIZING FOUNDATION PLAN.

**FN - 3** THE SLAB SHALL BE 4\"/>

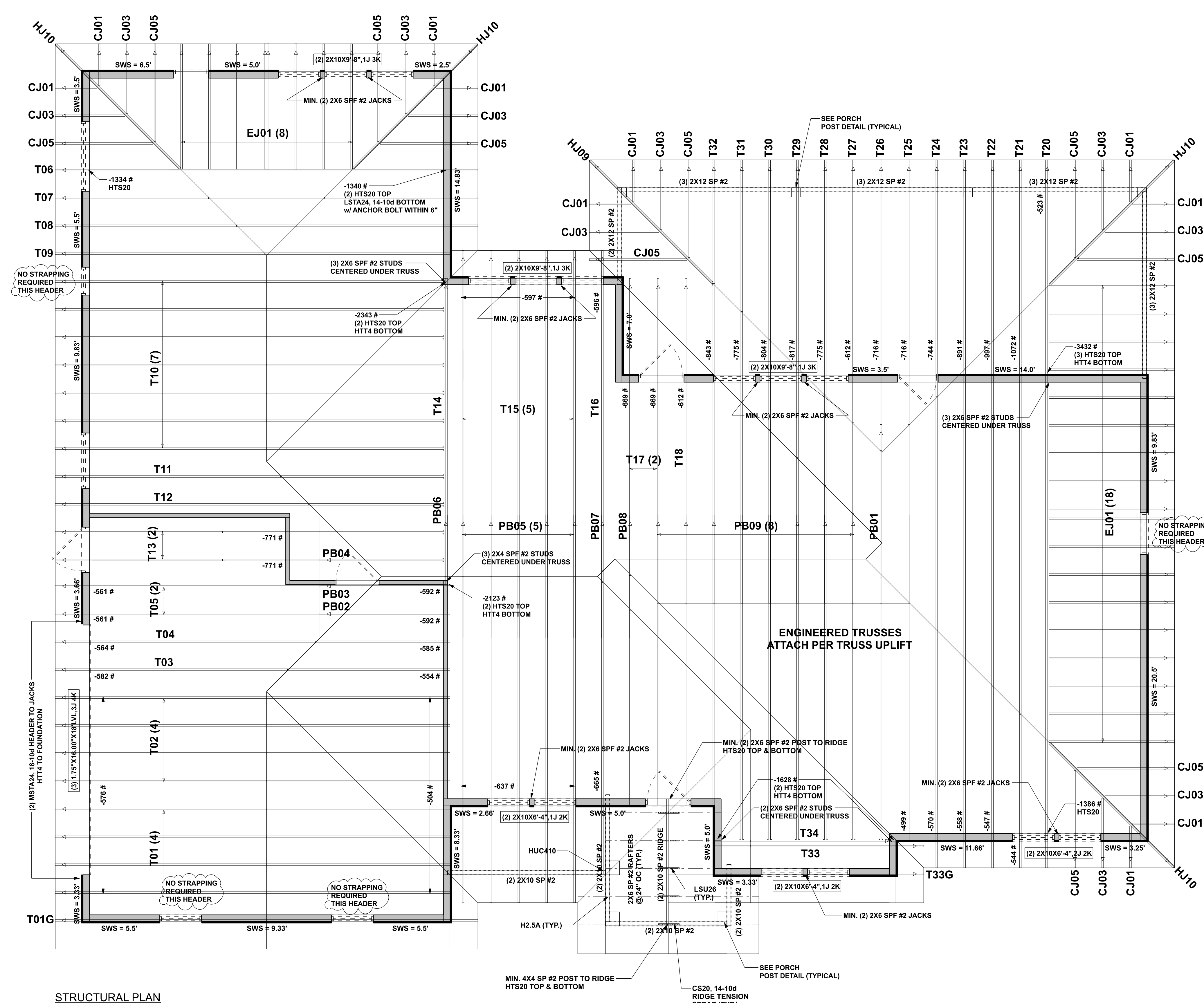
Sample Engineering (Frame Construction)

**DIMENSIONS:**  
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**Mark Disosway P.E.**  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disoswaydesign@gmail.com

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201218  
**S-2**  
OF 3 SHEETS

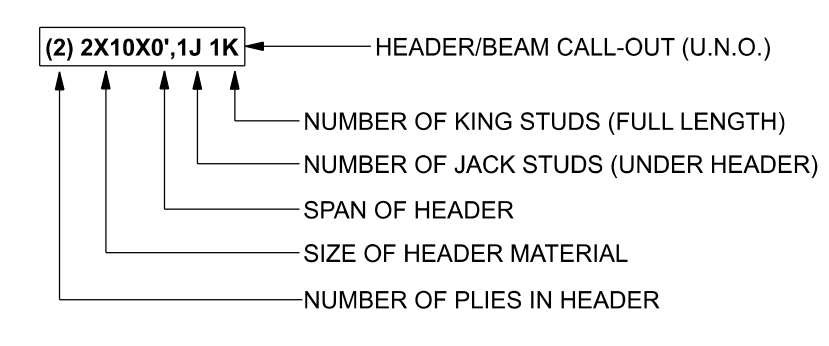


**STRUCTURAL PLAN**  
SCALE: 1/4" = 1'-0"

**STRUCTURAL PLAN NOTES**

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03. BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

**HEADER LEGEND**



**ACTUAL vs REQUIRED SHEARWALL**

	TRANSVERSE	LONGITUDINAL
ACTUAL	21914 LBF	18775 LBF
REQUIRED	18810 LBF	14467 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2552276

Sample Engineering (Frame Construction)

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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Mark Disoway P.E.  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disowaydesign@gmail.com

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**S-3**  
OF 3 SHEETS