

## GENERAL NOTES

GENERAL

1. CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONDITIONS, AND ELEVATIONS RELATING TO EXISTING CONSTRUCTION BY MAKING FIELD MEASUREMENTS PRIOR TO COMMENCING WORK.
2. CONTRACTOR SHALL PROTECT EXISTING STRUCTURES AND UTILITIES FROM DAMAGE.
3. TEMPORARY SHORING & BRACING FOR CONSTRUCTION IS THE RESPONSIBILITY OF THE CONTRACTOR
4. ALL EXCAVATED MATERIAL & CONSTRUCTION DEBRIS SHALL BE REMOVED FROM SITE & DISPOSED OF BY THE CONTRACTOR.

CODE

1. FLORIDA BUILDING CODE, 2023, 8th EDITION

## DESIGN LOADS

1. WIND LOADS PER ASCE 7-16 EDITION (CH.29 FOR SOLID FREESTANDING WALLS)

- |    |                                |                      |
|----|--------------------------------|----------------------|
| a) | BASIC WIND SPEED               | 130 MPH              |
| b) | RISK CATEGORY                  | I                    |
| c) | EXPOSURE CATEGORY              | C                    |
| d) | INTERNAL PRESSURE COEFFICIENTS | $\pm 0.0$ (OPEN)     |
| e) | WIND PRESSURES                 | $\pm 38$ PSF (WALLS) |

## FOUNDATIONS

1. NO SOILS REPORT HAS BEEN PERFORMED (AT OWNERS OPTION AND RISK). ENGINEER HAS ASSUMED 1500 PSF SOIL BEARING STRENGTH FOR PURPOSES OF FOUNDATION AND STRUCTURAL DESIGN IN ACCORDANCE WITH FBC 2017 SECTION 1806.2 "PRESUMPTIVE LOAD-BEARING" VALUES. IT IS THE CONTRACTORS FULL RESPONSIBILITY TO PERFORM EARTHWORK AND ADDITIONAL SOIL TESTS AS REQUIRED TO ENSURE THAT THE SUBSURFACE SOIL CONDITIONS ARE ADEQUATE TO PROVIDE THE MINIMUM ASSUMED BEARING SOIL STRENGTH STATED ABOVE. ENGINEER ASSUMES NO LIABILITY FOR VARYING SUBSURFACE CONDITIONS AND THEIR RESULTING IMPACT OR DETRIMENT TO BUDGET, STRUCTURE, OR DRAINAGE.
2. DEWATER, AS REQUIRED, SUCH THAT THE EXCAVATIONS ARE DRY AT THE TIME OF CONCRETE PLACEMENT.

## REINFORCING STEEL

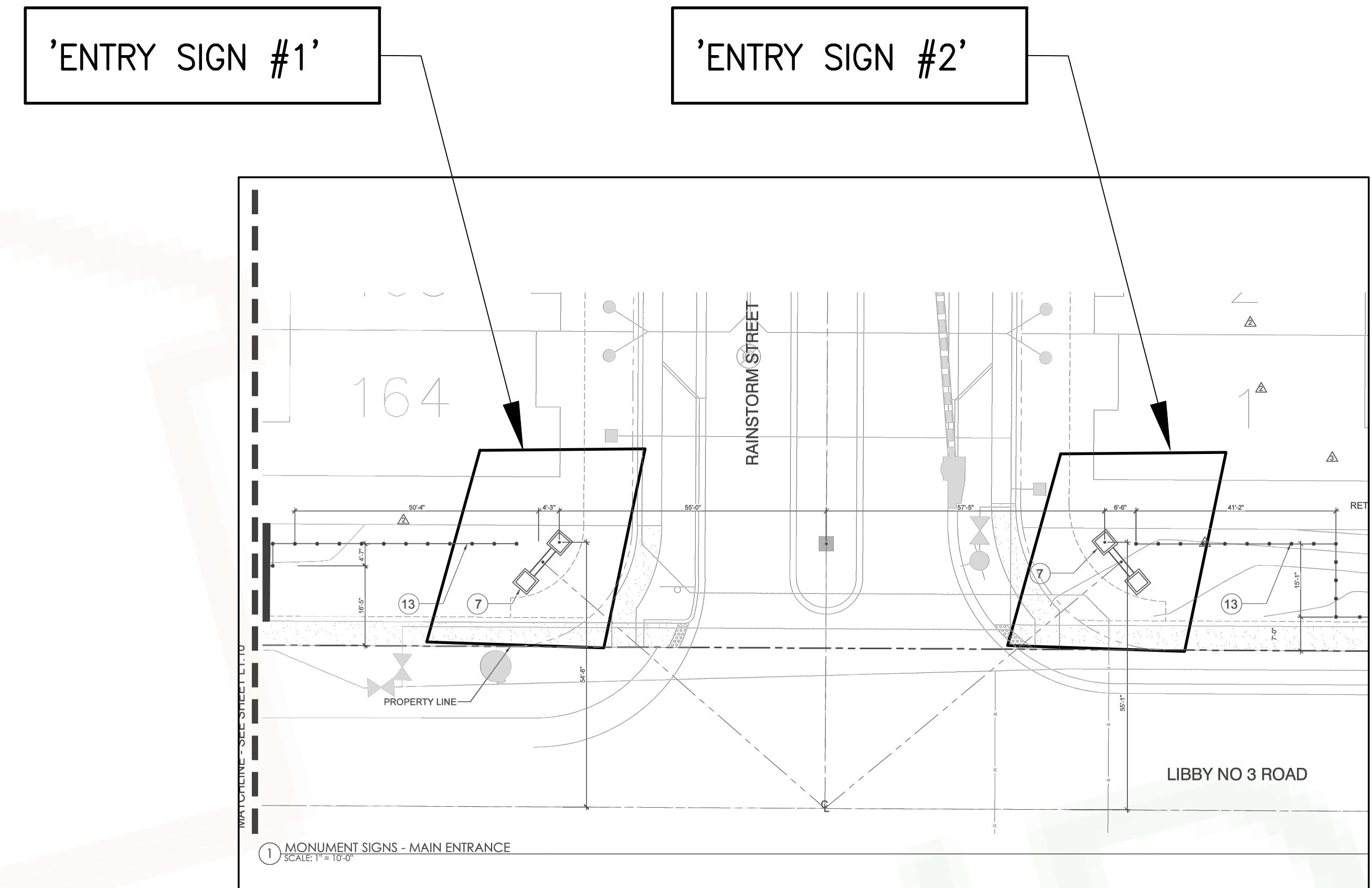
1. CRSI AND ACI STANDARDS APPLY.
2. ALL DEFORMED BARS SHALL CONFORM TO ASTM A615, GRADE 60.
3. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
4. SUBMIT FOR REVIEW SHOP DRAWINGS OF REINFORCING STEEL DETAILS PRIOR TO FABRICATING REINFORCING STEEL.
5. LAP ALL WELDED WIRE FABRIC A MINIMUM DISTANCE OF ONE CROSS WIRE SPACING PLUS TWO INCHES.
6. ALL REINFORCING STEEL SHALL BE SUPPORTED ON STANDARD ACCESSORIES, HELD RIGIDLY AND ACCURATELY IN PLACE, AND PROTECTED AGAINST DISPLACEMENT BEFORE AND DURING PLACEMENT OF CONCRETE.
7. REINFORCEMENT CHAIR LEGS THAT REST ON CONCRETE SURFACES THAT WILL BE EXPOSED IN THE FINISHED STRUCTURE SHALL BE FABRICATED OF STAINLESS STEEL OR SHALL BE PLASTIC COATED.
8. WHERE SPICE LENGTHS ARE NOT SPECIFIED, USE 50 BAR DIAMETERS.

### CONCRETE MASONRY:

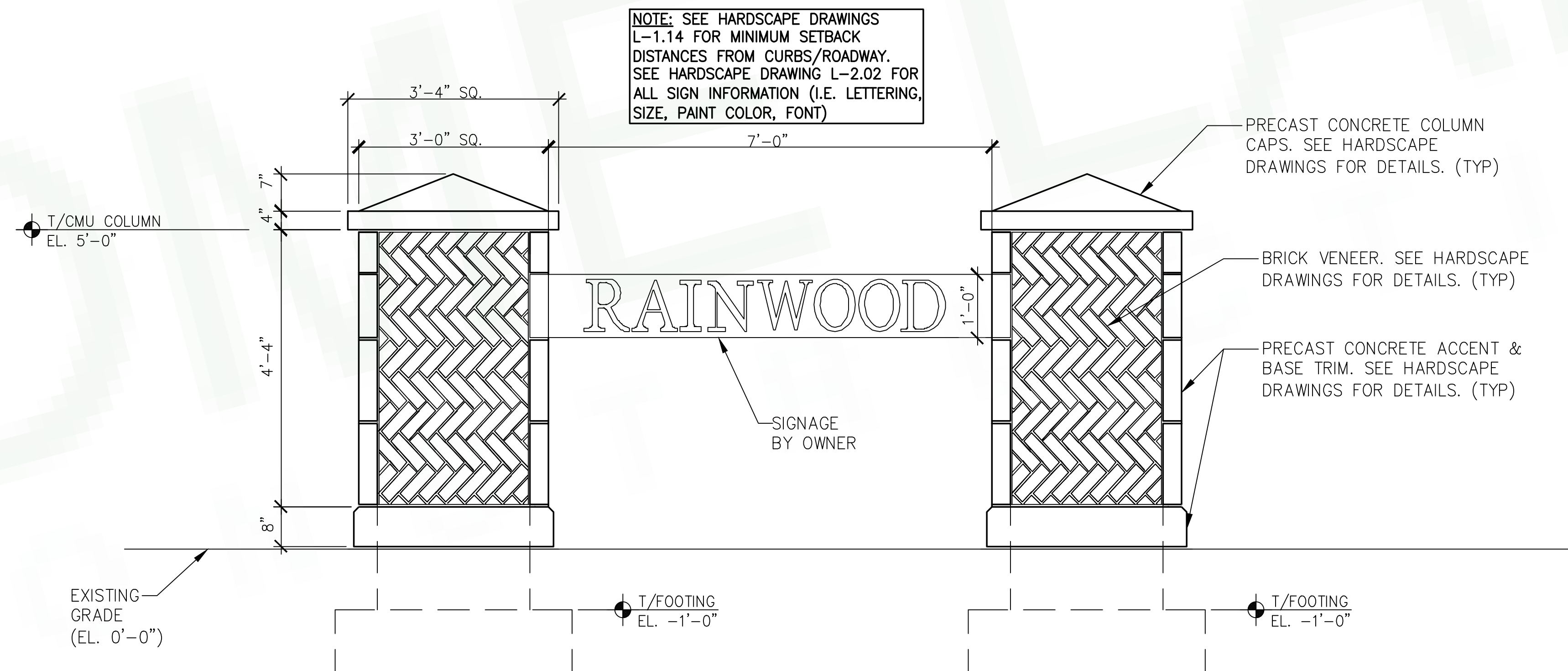
1. THE FOLLOWING CODES AND SPECIFICATIONS APPLY:
  - A) ACI 530/ ASCE 5 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.
  - B) ACI 530.1/ ASCE 6 SPECIFICATIONS FOR MASONRY STRUCTURES.
2. CONCRETE MASONRY UNITS SHALL BE LOAD BEARING TYPE CONFORMING TO ASTM C-90, TYPE 2, HAVING A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI (NET AREA). SEE ARCHITECTURAL DRAWINGS FOR FIRE RATING REQUIREMENTS. DESIGN MASONRY ASSEMBLY COMPRESSIVE STRENGTH:  
 $f'm=1500$  psi.
3. FILL CELLS AS NOTED ON DRAWINGS WITH COARSE GROUT CONFORMING TO ASTM C-476 HAVING A SLUMP RANGE OF 9" +/- 1" AND A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.
4. PROVIDE GALVANIZED WIRE LADDER TYPE HORIZONTAL JOINT REINFORCING AT 16" OC. (9 GAGE DURAWALL MINIMUM) UNLESS NOTED OTHERWISE. PROVIDE PREFABRICATED TEE, CORNER JOINT REINFORCING.
5. THE BEAMS CAST ON MASONRY WALLS SHALL BE CONSTRUCTED SO TO KEY AND BOND INTO BLOCK CELLS. THE USE OF BUILDING PAPER OR SHEET PLASTIC TO CLOSE VOIDS BELOW IS NOT ALLOWED.
6. ALL MASONRY WALLS SHOWN ON THE STRUCTURAL DRAWINGS HAVE BEEN DESIGNED TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES IN THE FINAL CONSTRUCTED CONFIGURATION ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO BRACE THE WALLS FOR VERTICAL AND LATERAL LOADS THAT COULD POSSIBLY BE APPLIED PRIOR TO COMPLETION OF CONNECTIONS AT FLOORS OR ROOF FRAMING LEVELS.
7. WALL REINFORCING IS CONTINUOUS FROM FOUNDATION TO TOP OF WALL.
8. USE ASTM C-270 TYPE M OR S MORTAR.

CONCRETE

1. THE LATEST EDITION OF THE FOLLOWING ACI STANDARDS APPLY:  
ACI 318 (CODE) ACI 304 (PLACING)  
ACI 306 (WINTER CONCRETING) ACI 315 (DETAILING)  
ACI 347 (FORM WORK) ACI 305 (HOT WEATHER CONCRETING)  
ACI 301 (SPECIFICATIONS) ACI 211.1 (MIX COMPRESIVE)
2. THE REQUIRED CONCRETE STRENGTH BASED ON AN AGE OF 28 DAYS FOR ELEMENTS IN THE STRUCTURE SHALL NOT BE LESS THAN:  
  - a.) 3000 PSI FOR FOOTINGS & PRECAST WALL CAPS
3. A CONCRETE MIX DESIGN FOR EACH STRENGTH AND AGGREGATE TYPE OR COARSE AGGREGATE SIZE SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL 15 DAYS PRIOR TO PLACING ANY CONCRETE.
4. MIX PROPORTIONS AND DESIGN: PROPORTION MIXES COMPLYING WITH MIX DESIGN PROCEDURES SPECIFIED IN ACI 301.
  - a. LIMIT GROUND GRANULATED BLAST FURNACE SLAG (ASTM C989) TO NOT EXCEED 50% OF CONTENT BY WEIGHT.
- b. LIMIT USE OF FLY ASH (ASTM C618 TYPE C OR F) TO NOT EXCEED 25% OF PORTLAND CEMENT (ASTM C150 TYPE 1 OR 2) CONTENT BY WEIGHT. DO NOT USE FLYASH IN CONJUNCTION WITH BLAST FURNACE SLAG.
- c. DESIGN MIXES TO PROVIDE NORMAL WEIGHT CONCRETE WITH THE FOLLOWING PROPERTIES:
  1. WATER-CEMENT RATIO: 0.50 MAX
  2. AIR ENTRAINMENT (ASTM C 260): 3% +/- 1%
- d. SLUMP LIMITS: PROPORTION AND DESIGN MIXES TO RESULT IN CONCRETE SLUMP AT POINT OF PLACEMENT AS FOLLOWS:
  1. RAMPS AND SLOPING SURFACES; NOT MORE THAN 3 INCHES.
  2. OTHER CONCRETE: 4 INCHES +/- 1"
  3. CONCRETE CONTAINING HRWR ADMIXTURE:  
NOT MORE THAN 8" AFTER ADDITION OF HRWR TO PLANT VERIFIED 2-4" SLUMP CONCRETE
- e. FINE & COURSE AGGREGATES SHALL BE PER ASTM C33.
5. SITE ADDED WATER IS PERMITTED WITHIN THE 1ST 15 MINUTES THAT THE TRUCK ARRIVES ON THE JOB SITE PROVIDED THAT THE WATER CEMENT RATIO AND THE SLUMP LIMITS ARE NOT EXCEEDED, HOWEVER NOT MORE THAN ONE GALLON OF WATER PER CUBIC YARD OF CONCRETE MAY BE USED TO ADJUST THE MIX AT THE JOBSITE.
6. CONSOLIDATE ALL CONCRETE, OTHER THAN SLABS ON GRADE USING MECHANICAL VIBRATING EQUIPMENT.
7. DO NOT PLACE CONCRETE DURING RAIN OR IF RAIN IS LIKELY TO OCCUR PRIOR TO CONCRETE HARDENING.
8. THE CONTRACTOR SHALL ENGAGE A TESTING AGENCY ACCEPTABLE TO THE ENGINEER TO PERFORM FOR EACH DAYS POUR OF EACH TYPE OF CONCRETE;
  - a. A SLUMP TEST (ASTM C143).
  - b. AN AIR ENTRAINMENT TEST (ASTM C173)
  - c. COMPRESSIVE STRENGTH TESTS (ASTM C39). PROVIDE ONE SET OF 4 CYLINDER SPECIMENS PER ASTM C31. ONE SPECIMEN IS TO BE TESTED AT SEVEN DAYS, TWO SPECIMENS AT 28 DAYS, AND ONE SPECIMEN RETAINED IF LATER TESTING IS REQUIRED. SUBMIT COPIES OF TEST RESULTS TO THE ARCHITECT.
9. EPOXY ADHESIVE FOR FASTENING BOLTS, REBAR, OR DOWELS SHALL BE HILTI RE500.



SITE PLAN:



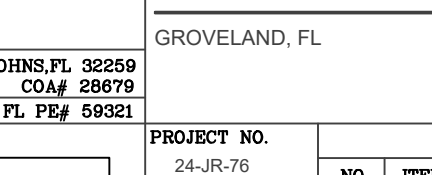
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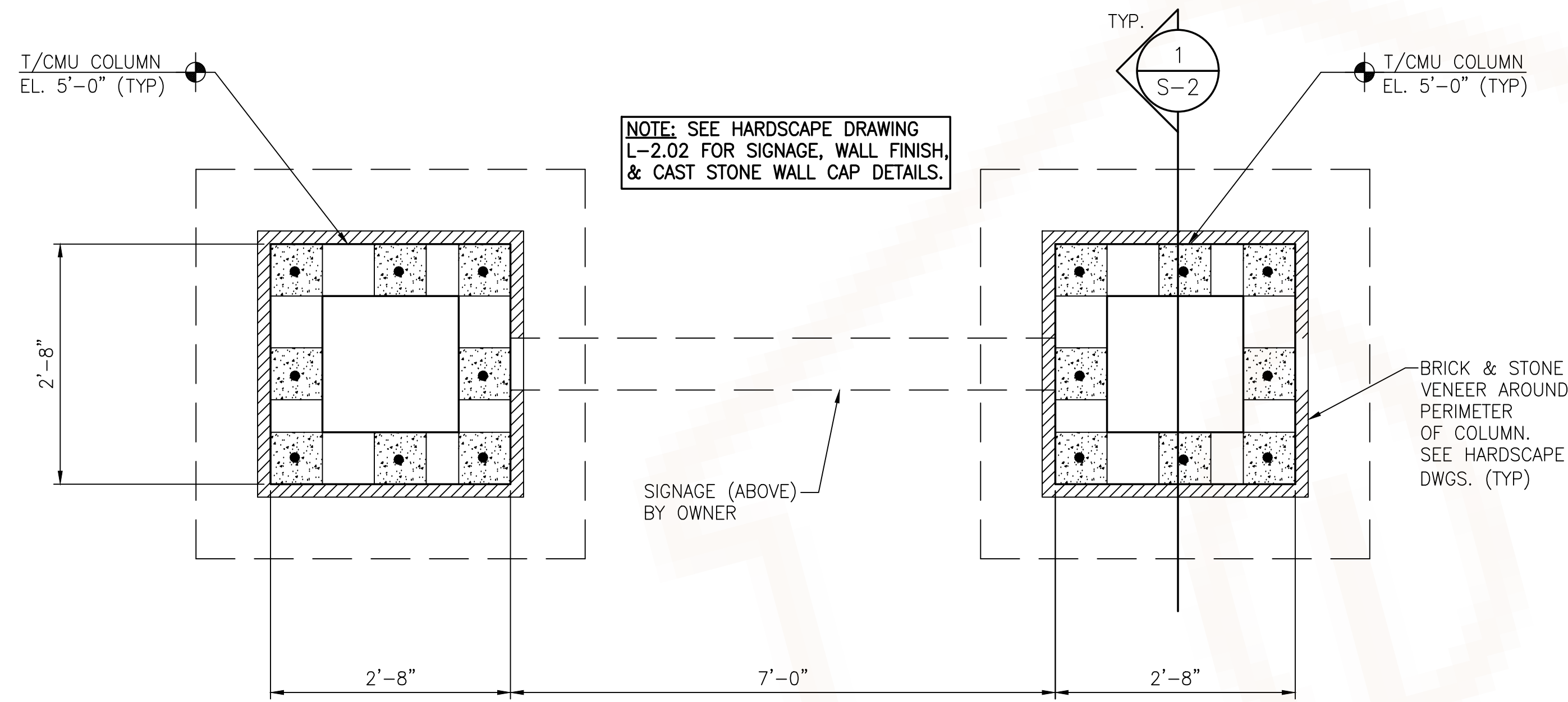
ENTRY MONUMENT SIGN ELEVATION (2 TOTAL)

SCALE  $3/4" = 1'-0"$

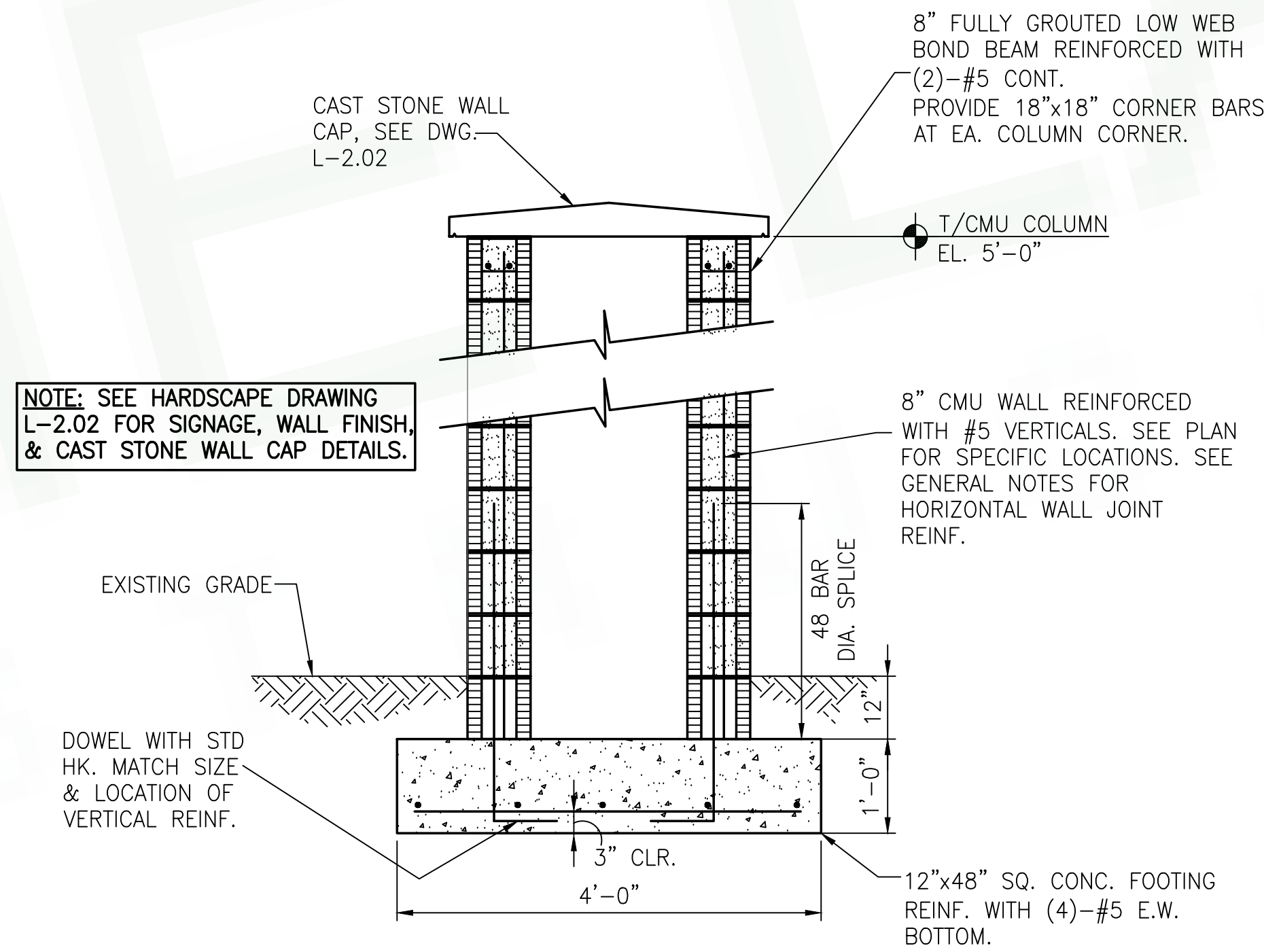
<b>SJR STRUCTURAL ENGINEERING, LLC</b>  1085 DUBBIN PARKS DRIVE ST. JOHNS, FL 32059 904.510.5605      CO.# 28679 <b>TIMOTHY G. FRAZIER II, P.E.</b> FL. PE. 59321	<b>PROJECT TITLE</b> <u><b>RAINWOOD ENTRY SIGNS</b></u>  GROVELAND, FL  <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th style="width: 15%;">PROJECT NO.</th> <th colspan="4" style="text-align: center;">REVISIONS</th> </tr> <tr> <td style="height: 40px; vertical-align: top;">24-JR-76</td> <th style="width: 10%;">NO.</th> <th style="width: 30%;">ITEM</th> <th style="width: 10%;">BY</th> <th style="width: 10%;">DATE</th> </tr> <tr> <td style="height: 40px; vertical-align: top;"> <b>DATE</b> 7/29/2024   <b>DESIGN PHASE</b> CD   <b>DRAWN BY</b> TGF II   <b>CHECKED BY</b> TGF II   <b>OWNER</b>    <b>APPROVED BY</b>   <b>DRAWING TITLE</b> GENERAL NOTES &amp; SITE PLAN   <b>DATE</b> </td> <td></td> <td></td> <td></td> <td></td> </tr> </table>	PROJECT NO.	REVISIONS				24-JR-76	NO.	ITEM	BY	DATE	<b>DATE</b> 7/29/2024  <b>DESIGN PHASE</b> CD  <b>DRAWN BY</b> TGF II  <b>CHECKED BY</b> TGF II  <b>OWNER</b>   <b>APPROVED BY</b>  <b>DRAWING TITLE</b> GENERAL NOTES & SITE PLAN  <b>DATE</b>				
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ENTRY MONUMENT SIGN PLAN (2 TOTAL)  
SCALE 3/4" = 1'-0"



1  
S-2 COLUMN SECTION  
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	CHECKED BY	TGF II	
OWNER			
APPROVED BY		DRAWING TITLE ENTRY SIGN PLAN & WALL DETAILS	
DATE		S-2	
DRAWING SCALE :		SHEET OF SHEETS	