

**AFFIDAVIT**

**Planning and Environment Court**

**David Manteit V Brisbane City Council 2916/24**

I, David Manteit of 82 Rowe Tce Darra, developer, under affirmation says:

1. Attached Exhibit "A" Civil Works Engineering, received on 28/3/25, dated 28/3/25.  
pages 1-14.

Signed:



Deponent David Manteit

Taken by:

  
  
Justice of the Peace

31 MAR 2025


Affirmed by David Manteit on

In the presence of

Signed:



Deponent – David Manteit


31 MAR 2025

**John James Stretch JP (Qual)**

**EXHIBIT "A"**

**Planning and Environment Court**

**David Manteit V Brisbane City Council 2916/24**

Civil Works Engineers Report dated 28/3/25, pages 1-14.

28 March 2025

Our Ref: CW24091

Brisbane City Council  
Planning and Environment Court

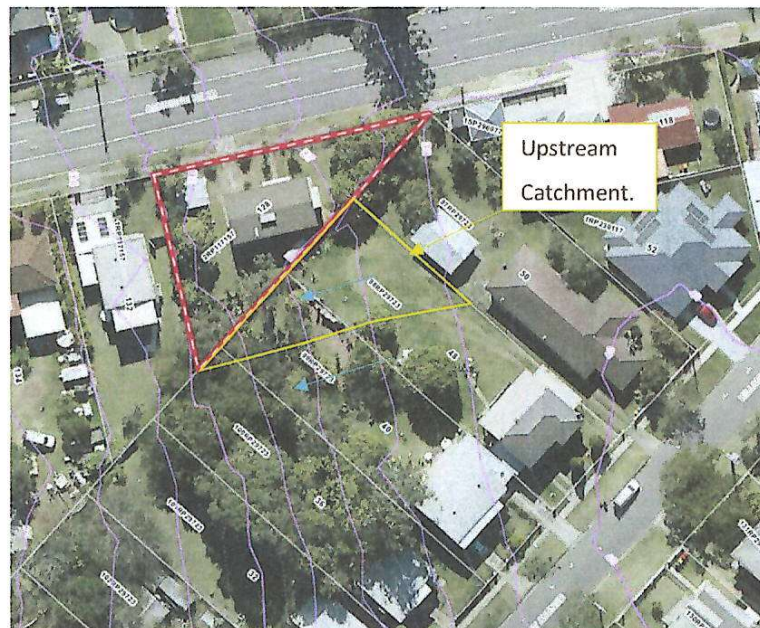
**128 Ashridge Road, Darra QLD 4076**  
**Stormwater Technical Assessment**  
**(Applications: A006565555)**

**1. Introduction**

This technical assessment has been prepared to support the formal request for the removal of Condition 18 and Condition 7, which requires the provision of upstream stormwater connections for Lots 98 and 99 RP29723 as well as the implications of easements associated as per the approved plans marked up by Council. Following a detailed review of the site conditions, natural drainage patterns, and Council's indicative sketch, it has been determined that this requirement is not feasible nor necessary. The following points outline the reasons why the upstream connection condition should be removed.

**2. Natural Drainage Patterns**

The existing topography of the upstream properties will first need to be considered prior to assessing the requirements for the provision of upstream property connections. Refer to diagram 1 for the existing contour plans obtained from Brisbane City Council's interactive mapping.





Based on the above diagram;

While it is understood that some portions of Lots 98 and 99 do drain towards the subject site, it is critical to note that all of Lot 2 naturally drains towards the downstream neighbouring property (Lot 1 RP117157). In addition to the above, over half of Lot 1 also drains towards the rear of the lot.

As such, any runoff from Lots 98 and 99 that does enter the subject site immediately continues to the downstream neighbouring properties rather than accumulating on site. This will imply that an upstream stormwater connection would serve no practical function as stormwater runoff already naturally drains downstream away from Ashridge Road.

With the above, it can be deduced that Council's request for upstream connections for Lots 98 and 99 are based on an assumed need rather than an assessment of the actual drainage patterns on site.

### 3. Physical Constraints of Site Topography

Based on the available survey plan, the topography along the western side of Lot 2 is characterized as undulating terrain, which prevents stormwater runoff to naturally be conveyed freely towards Ashridge Road.

As such, installing upstream stormwater connections would require significant modifications to the terrain, which would be impractical and disruptive.

### 4. Engineering Review of Council's Sketch

A detailed review of Council's indicative stormwater sketch, considering surface levels, invert levels, and grade constraints, confirms that installing an upstream stormwater pipe at the minimum grade would result in an exposed pipe for most of its length along the western side of Lot 2. Refer to diagram 2 obtained from attached concept sketch CW24091-SK01-REVA:

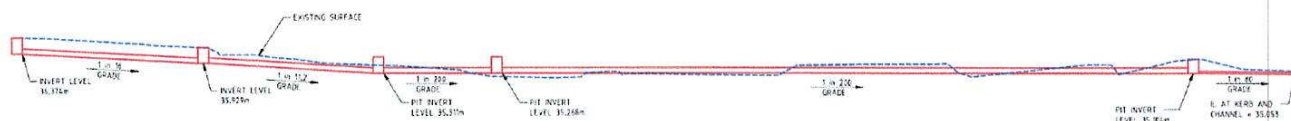


Diagram 2 : Stormwater schematic based on Council's approved sketch with minimum grades

This is a fundamental engineering issue as an exposed stormwater pipe:

- Is structurally vulnerable.
- Would pose maintenance and safety concerns.
- Is not a standard or practical stormwater solution.




Diagram illustrating the longitudinal section of the proposed road. The profile shows the existing surface (dashed line) and the proposed road grade (solid line). Key features include:

- EXISTING SURFACE** (dashed line)
- PROPOSED ROAD GRADE** (solid line)
- GRADES:** 1 in 16, 1 in 29.5, 1 in 8.9, 1 in 53.3, and 1 in 40.
- INVERT LEVELS:**
  - INVERT LEVEL 36.374m
  - INVERT LEVEL 35.276m
  - PIT INVERT LEVEL 34.938m
  - PIT INVERT LEVEL 34.872m
  - PIT INVERT LEVEL 34.640m
- AT KEYS AND CHANNEL:** 35.055

It will result in a charged system with an approximate drop of 1.181m between the internal network and the kerb outlet resulting in a charged system that would be inefficient.

## 5. Potential Downstream Flooding Issues

Understanding the lower elevations at the rear of the site due to the existing topography, any surcharging from the upstream drainage system could result in stormwater flowing toward adjacent downstream properties. This would in turn create nuisance flows, which contradicts the "No Worsening" principle of stormwater management upheld by Council.

In addition, an upstream connection does not prevent overland flow risks as the proposed infrastructure will only cater for minor flow storm events, as the primary issue for this development is the existing site terrain, which naturally directs water toward the rear.

## 6. Future Development Considerations

An assessment of post-development discharge for the upstream fully developed site conditions for Lots 98 and 99 have been undertaken using the Rational Method taking into consideration of the site in its entirety as well as a conservative potential total roof area of 600m<sup>2</sup>, in accordance with QUDM and BCC Infrastructure Design PSP – Chapter 7. The below calculations are only for 1 lot considering the lot sizes are the same.

### Post Development (Lot 98/99)

The following parameters have been adopted for using the Rational Method:

- **Catchment Area (ha)** – 0.1012 (Total Existing Site Area).
- **Runoff Coefficient ( $C_{10}$ )** – 0.870 (According to QUDM Section 4.5). This is based on the future lot being fully developed (LMR3).
- **Time of Concentration ( $t_c$ )** – 5 minutes (According to QUDM Section 4.6). This is based on 5 minutes travel times from roof to main system connection.
- **Rainfall Intensity (mm/hr)** – Data obtained from BCC Infrastructure Design PSP – Chapter



a

7 Table 7.2.2.2.A.

The peak flow rates have been outlined in Table 1 below. Refer to Appendix F for detailed calculation.

**Table 1 – Peak Flow Rates Using Rational Method (Post Development)**

Catchment	Q <sub>1</sub> (m <sup>3</sup> /s)	Q <sub>2</sub> (m <sup>3</sup> /s)	Q <sub>5</sub> (m <sup>3</sup> /s)	Q <sub>10</sub> (m <sup>3</sup> /s)	Q <sub>20</sub> (m <sup>3</sup> /s)	Q <sub>50</sub> (m <sup>3</sup> /s)	Q <sub>100</sub> (m <sup>3</sup> /s)
Existing Site	0.023	0.031	0.044	0.053	0.064	0.082	0.091

#### Post Development (Lot 98/99)

The following parameters have been adopted for using the Rational Method:

- **Catchment Area (ha)** – 0.060 (Future Roof Areas).
- **Runoff Coefficient (C<sub>10</sub>)** – 0.870 (According to QUDM Section 4.5). This is based on the future lot being fully developed (LMR3).
- **Time of Concentration (t<sub>c</sub>)** – 5 minutes (According to QUDM Section 4.6). This is based on 5 minutes travel times from roof to main system connection.
- **Rainfall Intensity (mm/hr)** – Data obtained from BCC Infrastructure Design PSP – Chapter 7 Table 7.2.2.2.A.


The peak flow rates have been outlined in Table 1 below. Refer to Appendix F for detailed calculation.

**Table 2 – Peak Flow Rates Using Rational Method (Post Development- Roof Areas Only)**

Catchment	Q <sub>1</sub> (m <sup>3</sup> /s)	Q <sub>2</sub> (m <sup>3</sup> /s)	Q <sub>5</sub> (m <sup>3</sup> /s)	Q <sub>10</sub> (m <sup>3</sup> /s)	Q <sub>20</sub> (m <sup>3</sup> /s)	Q <sub>50</sub> (m <sup>3</sup> /s)	Q <sub>100</sub> (m <sup>3</sup> /s)
Existing Site	0.014	0.019	0.026	0.031	0.038	0.049	0.054

Based on the above considering the lots are fully developed, it can be determined that the stormwater runoff will increase significantly, and the proposed upstream stormwater infrastructure will not be able support the additional flows based on QUDM Level III drainage.

Furthermore, Council's Planning Scheme Policy states that proposed kerb outlets should have a capacity which is limited to 30L/s for the 5% AEP event. However, runoff volume will exceed this capacity even with the conservative assumption of 600m<sup>2</sup> roof areas for each lot. Understanding Council's 30L/s limitation, even if stormwater infrastructure were to be modified, the proposed connection would still fail to meet compliance standards.



## 7. On-Site Detention (OSD) Feasibility for upstream developments

Understanding that OSD could be conditioned on Lots 98 and 99 to mitigate flows to 30L/s to allow for compliance. However this is viewed as an highly unfavourable outcome to be imposed upon the upstream lots as the provision of OSD for small freehold lots will be impractical, highly inefficient due to the relatively large OSD requirements to achieve the desired mitigation not to mention the costs involved.

Furthermore, under Council's ROL (Reconfiguration of a Lot) guidelines, freehold lots in infill subdivisions are not required to provide on-site detention, meaning there is no mechanism to mitigate upstream flows to 30L/s.

### Conclusion

It is our understanding that Condition 18 and associated Condition 7 should never have been imposed based on the below justifications:

- Natural drainage patterns already direct runoff downstream.
- The site's terrain prevents effective upstream drainage.
- A compliant connection would result in an exposed pipe, which is not feasible.
- A compliant connection based on providing minimum cover would result in an inefficient charged system.
- Forcing an upstream connection would lead to downstream nuisance flooding, violating the "No Worsening" principle.
- If the upstream properties are developed, they will generate flows exceeding the allowable kerb discharge limits.
- There is no viable OSD option to mitigate excess runoff, per Council's guidelines.

Given these points, Condition 18 and 7 should be formally removed, as the upstream connection is neither practical nor justifiable from an engineering perspective.

Should you require any further information, please do not hesitate to contact the undersigned.

Yours sincerely,

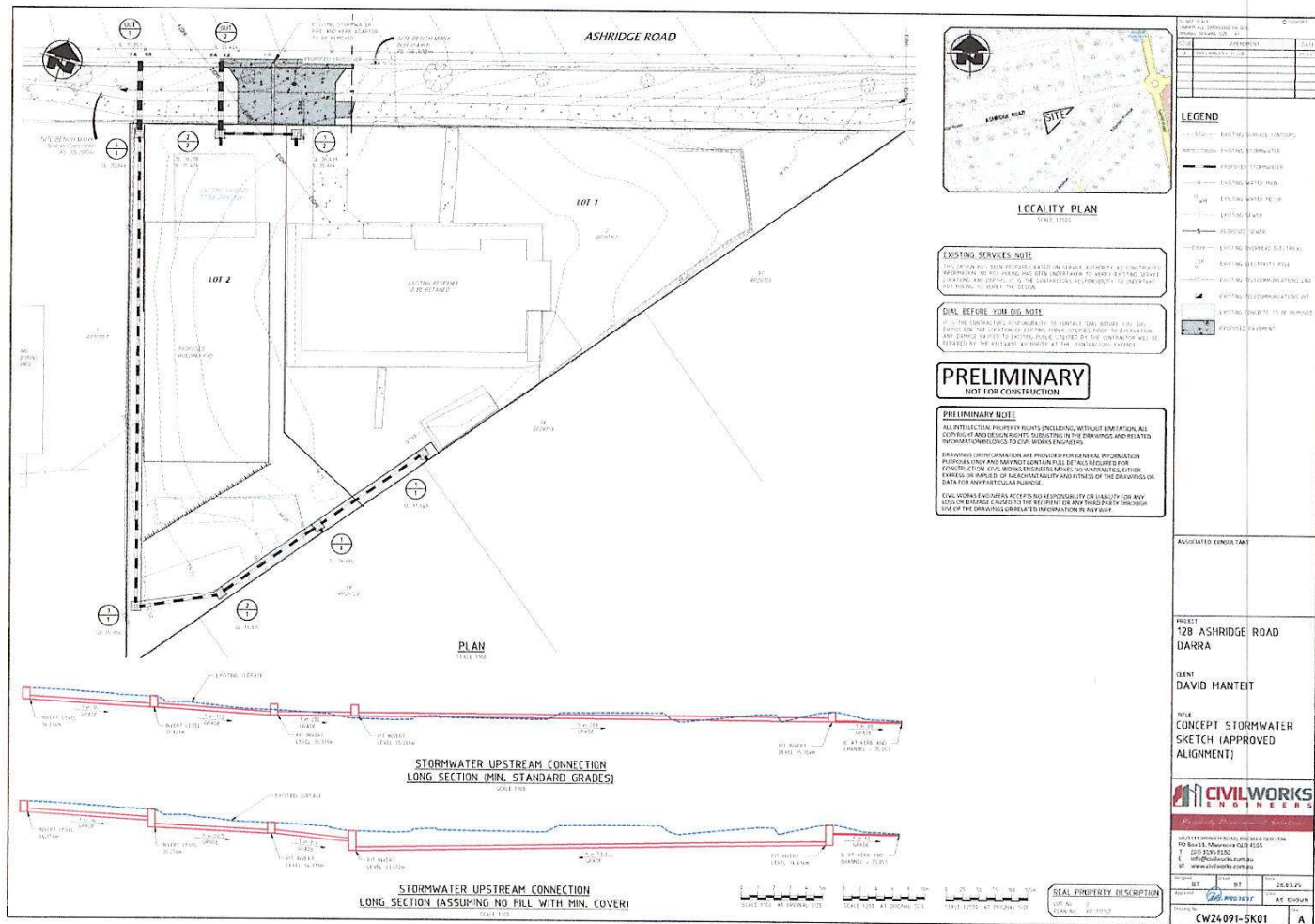
**CIVIL WORKS ENGINEERS**



Wai Yeow Lee  
Senior Civil Engineer



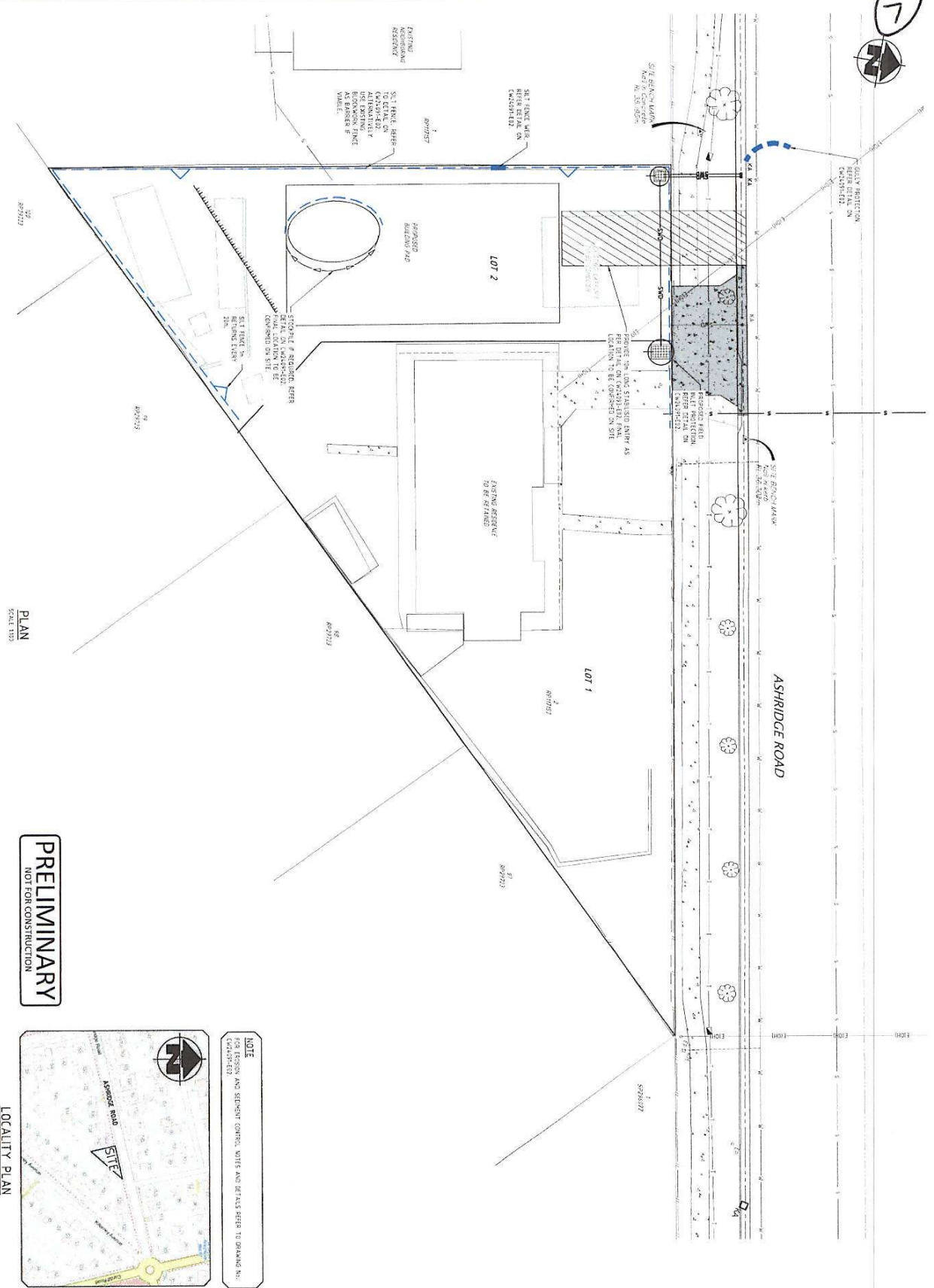
Roger Andrade RPEQ 7675  
Principal Civil Engineer



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*[Handwritten signature]*





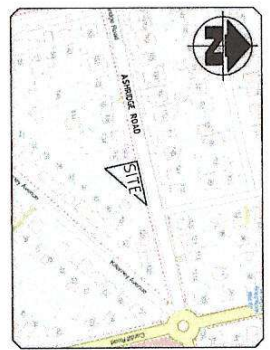
PLAN  
SCALE 1:100

**PRELIMINARY**  
NOT FOR CONSTRUCTION



REAL PROPERTY DESCRIPTION  
Lot 1 & 2, AS 10755

LOCALITY PLAN  
SCALE 1:2500



**NOTE**  
EROSION AND SEDIMENT CONTROL, NOTES AND DETAILS REFER TO DRAWING NO. CW24091-E02

PROJECT  
128 ASHRIDGE ROAD,  
DARRA

CLIENT  
DAVID MANTEIT

TITLE  
EROSION AND SEDIMENT  
CONTROL PLAN

ASSOCIATED CONSULTANT

LEGEND

- CUT FENCE
- STABILIZED DRIVEWAY
- EXISTING SURFACE CONTOURS
- EXISTING STORMWATER
- PROPOSED STORMWATER
- EXISTING WATER MAIN
- EXISTING WATER METER
- EXISTING SINKHOLE
- PROPOSED SINKHOLE
- EXISTING OVERHEAD ELECTRICAL
- EXISTING TELECOMMUNICATIONS LINE
- EXISTING TELECOMMUNICATIONS PIT
- EXISTING FIELD FLET PROTECTION
- EXISTING CONCRETE TO BE REPAIRED
- PROPOSED PAVEMENT

DATE  
12/12/20

SCALE  
1:100

PROJECT NO.  
CW24091-E01

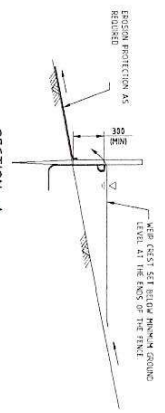
UNIVERSITY OF WATKINS  
1001 WATKINS ROAD  
T 807 3315 8180  
E info@uowatkins.com.au  
W www.uowatkins.com.au

ENGINEER  
DAVID MANTEIT  
AS 3600

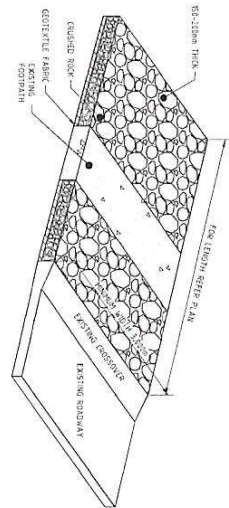
DATE  
12/12/20

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CW24091-E01



## SECTION A



STABILISED ENTRY/EXIT POINT  
TYPICAL DETAIL

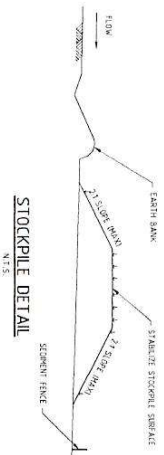


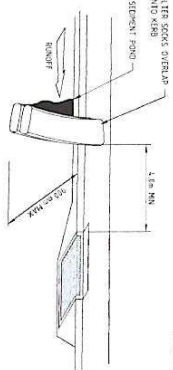
Table 1 - Soil Loss Rates (RUSLE)

Soil Loss Class	Estimated Soil Loss Rate (t/ha/yr)	Soil Loss Class	Estimated Soil Loss Rate (t/ha/yr)
1	< 0.05	5	0.45 - 0.55
2	0.05 - 0.10	6	0.55 - 0.65
3	0.10 - 0.15	7	0.65 - 0.75
4	0.15 - 0.20	8	0.75 - 0.85
5	0.20 - 0.25	9	0.85 - 0.95
6	0.25 - 0.30	10	0.95 - 1.05
7	0.30 - 0.35	11	1.05 - 1.15
8	0.35 - 0.40	12	1.15 - 1.25
9	0.40 - 0.45	13	1.25 - 1.35
10	0.45 - 0.50	14	1.35 - 1.45
11	0.50 - 0.55	15	1.45 - 1.55
12	0.55 - 0.60	16	1.55 - 1.65
13	0.60 - 0.65	17	1.65 - 1.75
14	0.65 - 0.70	18	1.75 - 1.85
15	0.70 - 0.75	19	1.85 - 1.95
16	0.75 - 0.80	20	1.95 - 2.05
17	0.80 - 0.85	21	2.05 - 2.15
18	0.85 - 0.90	22	2.15 - 2.25
19	0.90 - 0.95	23	2.25 - 2.35
20	0.95 - 1.00	24	2.35 - 2.45
21	1.00 - 1.05	25	2.45 - 2.55
22	1.05 - 1.10	26	2.55 - 2.65
23	1.10 - 1.15	27	2.65 - 2.75
24	1.15 - 1.20	28	2.75 - 2.85
25	1.20 - 1.25	29	2.85 - 2.95
26	1.25 - 1.30	30	2.95 - 3.05
27	1.30 - 1.35	31	3.05 - 3.15
28	1.35 - 1.40	32	3.15 - 3.25
29	1.40 - 1.45	33	3.25 - 3.35
30	1.45 - 1.50	34	3.35 - 3.45
31	1.50 - 1.55	35	3.45 - 3.55
32	1.55 - 1.60	36	3.55 - 3.65
33	1.60 - 1.65	37	3.65 - 3.75
34	1.65 - 1.70	38	3.75 - 3.85
35	1.70 - 1.75	39	3.85 - 3.95
36	1.75 - 1.80	40	3.95 - 4.05
37	1.80 - 1.85	41	4.05 - 4.15
38	1.85 - 1.90	42	4.15 - 4.25
39	1.90 - 1.95	43	4.25 - 4.35
40	1.95 - 2.00	44	4.35 - 4.45
41	2.00 - 2.05	45	4.45 - 4.55
42	2.05 - 2.10	46	4.55 - 4.65
43	2.10 - 2.15	47	4.65 - 4.75
44	2.15 - 2.20	48	4.75 - 4.85
45	2.20 - 2.25	49	4.85 - 4.95
46	2.25 - 2.30	50	4.95 - 5.05
47	2.30 - 2.35	51	5.05 - 5.15
48	2.35 - 2.40	52	5.15 - 5.25
49	2.40 - 2.45	53	5.25 - 5.35
50	2.45 - 2.50	54	5.35 - 5.45
51	2.50 - 2.55	55	5.45 - 5.55
52	2.55 - 2.60	56	5.55 - 5.65
53	2.60 - 2.65	57	5.65 - 5.75
54	2.65 - 2.70	58	5.75 - 5.85
55	2.70 - 2.75	59	5.85 - 5.95
56	2.75 - 2.80	60	5.95 - 6.05
57	2.80 - 2.85	61	6.05 - 6.15
58	2.85 - 2.90	62	6.15 - 6.25
59	2.90 - 2.95	63	6.25 - 6.35
60	2.95 - 3.00	64	6.35 - 6.45
61	3.00 - 3.05	65	6.45 - 6.55
62	3.05 - 3.10	66	6.55 - 6.65
63	3.10 - 3.15	67	6.65 - 6.75
64	3.15 - 3.20	68	6.75 - 6.85
65	3.20 - 3.25	69	6.85 - 6.95
66	3.25 - 3.30	70	6.95 - 7.05
67	3.30 - 3.35	71	7.05 - 7.15
68	3.35 - 3.40	72	7.15 - 7.25
69	3.40 - 3.45	73	7.25 - 7.35
70	3.45 - 3.50	74	7.35 - 7.45
71	3.50 - 3.55	75	7.45 - 7.55
72	3.55 - 3.60	76	7.55 - 7.65
73	3.60 - 3.65	77	7.65 - 7.75
74	3.65 - 3.70	78	7.75 - 7.85
75	3.70 - 3.75	79	7.85 - 7.95
76	3.75 - 3.80	80	7.95 - 8.05
77	3.80 - 3.85	81	8.05 - 8.15
78	3.85 - 3.90	82	8.15 - 8.25
79	3.90 - 3.95	83	8.25 - 8.35
80	3.95 - 4.00	84	8.35 - 8.45
81	4.00 - 4.05	85	8.45 - 8.55
82	4.05 - 4.10	86	8.55 - 8.65
83	4.10 - 4.15	87	8.65 - 8.75
84	4.15 - 4.20	88	8.75 - 8.85
85	4.20 - 4.25	89	8.85 - 8.95
86	4.25 - 4.30	90	8.95 - 9.05
87	4.30 - 4.35	91	9.05 - 9.15
88	4.35 - 4.40	92	9.15 - 9.25
89	4.40 - 4.45	93	9.25 - 9.35
90	4.45 - 4.50	94	9.35 - 9.45
91	4.50 - 4.55	95	9.45 - 9.55
92	4.55 - 4.60	96	9.55 - 9.65
93	4.60 - 4.65	97	9.65 - 9.75
94	4.65 - 4.70	98	9.75 - 9.85
95	4.70 - 4.75	99	9.85 - 9.95
96	4.75 - 4.80	100	9.95 - 10.05

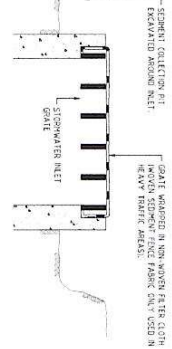
TABLE 3 - SEDIMENT CONTROL STANDARD

TABLE 3. SEDIMENT CONTROL STANDARD				
SEDIMENT CONTROL TYPE	SEDIMENTATION LOSS RATE (%)	TYPICAL CONTROL MEASURES	SITE CATCHMENT (ha/acre)	
1	0.2-1.0	3-50	SEDIMENT BASIN OR TRAP ACCUMULATED WITH 10% EFFICIENCY	
2	0.1-0.5	5-75	SEDIMENT TRAP BUILT ON DESIGN FLOOD PLANE WITH 10% SEDIMENT TRAP EFFICIENCY	
3	<0.10	ALL DICES	SEDIMENT DICES (10% EFFICIENCY) %ET PREVENTION BOXES (SEE TABLES)	

NOTE: TABLES 1 TO 3 ESTIMATE  
SOIL LOSS AND ASSOCIATED  
EROSION HAZARD CATEGORY  
DUE TO SHEET AND RILL  
EROSION.



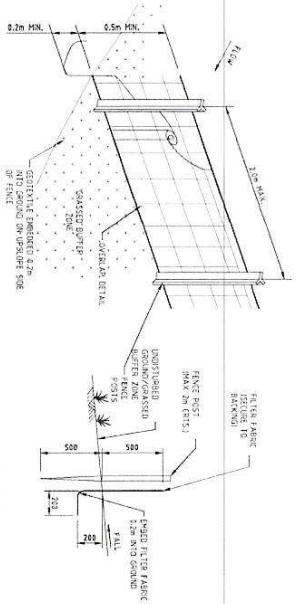
GRADED STORMWATER (FIELD)  
INLET SEDIMENT TRAP



ISOMETRIC SECTION

TEMPORARY SILT FENCE DETAIL

N.T.S.



**PRELIMINARY**  
NOT FOR CONSTRUCTION

SLOPE	MAXIMUM SLOPE LENGTH
1	50m
2	60m
3	70m
4	80m
5	90m
6	100m
7	110m
8	120m
9	130m
10	140m

GULLY INLET PROTECTION  
NOT TO SCALE

**PRELIMINARY**  
NOT FOR CONSTRUCTION

SLOPE	MAXIMUM SLOPE LENGTH
1	50m
2	60m
3	70m
4	80m
5	90m
6	100m
7	110m
8	120m
9	130m
10	140m

REAL PROPERTY DESCRIPTION	

LOT No. 2  
PLAN No. RFP 11757

**NOTE**  
FOR EROSION AND SEDIMENT CONTROL PLAN REFER TO DRAINING No. CW24091-E01.

**STOCKPILE - CONSTRUCTION NOTES**

2. CONSTRUCT ON THE CONTOUR AS LOW FLAT ELEVATION MINIMUM.
3. WHERE THERE IS SUFFICIENT AREA, POOL STORMWATER SHALL BE 15% FLAT; 10% IN "BEDDING".
4. WHERE THERE ARE NOT BE IN FLAT FOR MORE THAN 10 DAYS, STABILIZE FOLLOWING THE APPROVED SLOPE OR SHARP TO EXCEED THE C-FACTOR TO LESS THAN 1.0.
5. CONSTRUCT EARTH BANKS ON THE UP SLOPE SIDE TO OFFER WATER AROUND STORMWATER AND SEDIMENT TRAPLES; 1 TO 2 METERS DOWN SLOPE.

**PRE-START MEETING NOTE**

THE PLANS AND DESIGN CERTIFICATIONS MUST BE AVAILABLE ON SITE AT ALL TIMES FOR INSPECTION BY COUNTY OFFICIALS UNTIL ALL EXPOSED SOIL AREAS ARE PERMANENTLY STABILIZED AGAINST EROSION.

## MATERIALS

**FAIRLY LIGHT TYPING, AROUND  
HEAVY-DUTY METEOL, AIRCRAFT  
EQUIPMENT.**

### EROSION AND SEDIMENT CONTROL NOTES

- [illegible]

**EROSION AND SEDIMENT CONTROL NOTES - TRENCHING**

1. GET ONLY AS MUCH GROUND AS REQUIRED TO UPGRADE THE WORK AT HAND.
2. WORKER POSSIBLE TO CONSTRUCT TIE-ROPS FROM OUTRIGER TO DOWNSYSTEM.
3. PLACE BELLOW MATERIAL AS SOON AS PRACTICAL AFTER GETTING TRENCH.
4. CLOSE TRENCH AS SOON AS PRACTICABLE AFTER LAMING PIPE.
5. INITIAL STAY PILES IN TRENCHES AT THE CHAINS WHERE A NEW TRENCH BE PROPOSED TO BE LIFT UPON AND ANCHORED TO.

#### DEMOLITION NOTES

1. ALL DEMOLITION WORKS, INCLUDING THE PREPARATION OF ANY NECESSARY APPROVALS, IS THE RESPONSIBILITY OF THE DEMOLITION CONTRACTOR. ALL DEMOLITION WORKS TO BE UNDERTAKEN IN ACCORDANCE WITH CURRENT LEGISLATION.
2. THE ENTITY RESPONSIBLE FOR THE DEMOLITION WORKS SHALL ENSURE THAT ALL VEHICLES LEAVING THE SITE CARRYING DEMOLITION MATERIALS HAVE THEIR LOADS COVERED AND DO NOT TRUCK SOLID OR WASTE MATERIALS ONTO THE ROAD.

**PEDESTRIAN SAFETY NOTE**

CONSTRUCTION OF A SAFE PEDESTRIAN ACCESS IN ACCORDANCE WITH THE MANUAL FOR UNIFORM TRAFFIC CONTROL DEVICES.

**EXISTING SERVICES NOTE**

THIS DESIGN HAS BEEN PREPARED BASED ON SERVICE AUTHORITY AS CONSULTANT INFORMATION. NO POT HOLEING HAS BEEN UNDERTAKEN TO VERIFY EXISTING SERVICE LOCATIONS AND DEPTHS. IT IS THE CONTRACTORS RESPONSIBILITY TO UNDERTAKE POT HOLEING TO VERIFY THE DESIGN.

ASBESTOS NOTE

SHOULD ASBESTOS BE PRESENT, ITS REMOVAL SHALL BE CARRIED OUT IN ACCORDANCE WITH THE NATIONAL OHS COMMITTEE - CODE OF PRACTICE FOR SAFE REMOVAL OF ASBESTOS AND ITS CODE OF PRACTICE FOR THE MANAGEMENT AND CONTROL OF ASBESTOS IN THE WORKPLACE.

## SILT FENCE MAINTENANCE

- REGULAR INSPECTIONS AND MAINTENANCE ARE REQUIRED TO REDUCE DAMAGE CAUSED BY ON-SITE VEHICLES OR THE MOVEMENT OF STORAGE MATERIALS.
- INSPECT AT THE END OF EACH DAY WORK DUE TO THE POSSIBILITY OF OVERSIGHT, RAIN AND IMMEDIATELY PRIOR TO AND AFTER EACH STORM EVENT.
- REMOVE EXCESSIVE SEDIMENT DEPOSITS.
- INVESTIGATE THE SOURCE OF EXCESSIVE SEDIMENT.
- IF THE FENCE IS REGULARLY DAMAGED, INSTALL A SECOND FENCE AT LEAST 1 METRE DOWN-SLOPE OF THE EXISTING FENCE.











ACCORDANCE WITH CURRENT SOUTH  
PRACTICES AND STANDARDS.



**PRELIMINARY**  
NOT FOR CONSTRUCTION

010 4105  
 T (071 3195 8180  
 E info@trialwork.com.au  
 W www.trialwork.com.au

SEWERAGE LONGITUDINAL SECTION AND

SHEET 2 OF 2	DRAWING NO.	CW24091-S02	P	ES

WER TO  
AND

OF THE  
DONATE ITS  
HALL GIVE THE  
EVEY WORKS.

OF 2 SHEETS	2
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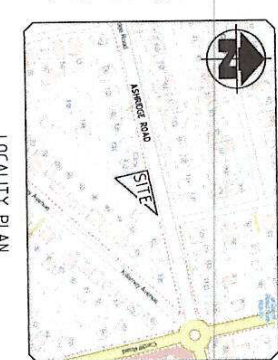
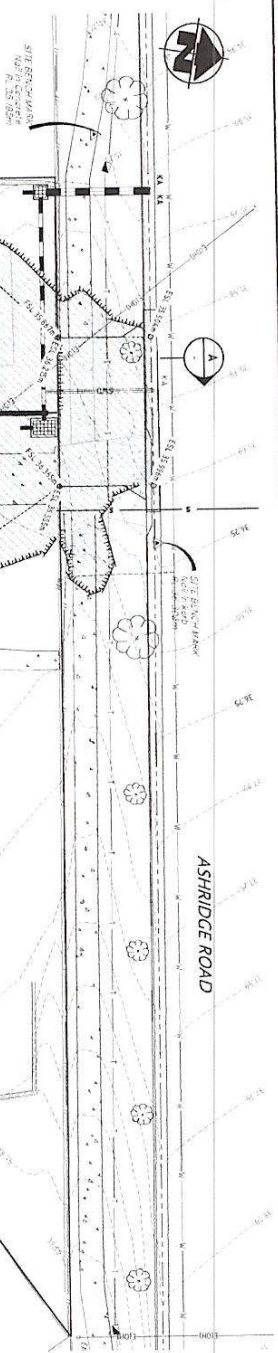
DRAWING STATUS	
PRELIMINARY	
SHEET 2 OF 2	SHEETS
DRAWING NO.	REV
CW24091-S02	A







[illegible]



**LEGEND**

SYMBOL	DESCRIPTION
[Line with cross-ticks]	EXISTING SURFACE CONTOUR
[Line with dots]	EXISTING STORMWATER
[Line with triangles]	PROPOSED STORMWATER
[Line with circles]	EXISTING WATER MAIN
[Line with squares]	PROPOSED WATER MAIN
[Line with diamonds]	EXISTING SEWER
[Line with crosses]	PROPOSED SEWER
[Line with asterisks]	EXISTING CEMETARY ELECTRICAL
[Line with hash marks]	PROPOSED CEMETARY ELECTRICAL
[Line with vertical bars]	EXISTING TELECOMMUNICATIONS
[Line with horizontal bars]	PROPOSED TELECOMMUNICATIONS
[Line with diagonal bars]	EXISTING CONCRETE TO BE REMOVED
[Line with wavy bars]	PROPOSED AREA OF CUT
[Line with zig-zag bars]	PROPOSED AREA OF FILL
[Line with solid bars]	PROPOSED RETAINING WALL
[Line with dashed bars]	EXISTING SURFACE LEVEL
[Line with dotted bars]	PROPOSED SURFACE LEVEL

**EXISTING SERVICES NOTE**

THE SERVICES SHOWN ON THIS PLAN HAVE BEEN OBTAINED FROM THE RECORD DRAWINGS AND FIELD SURVEY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE LOCATION AND DEPTH OF ALL SERVICES PRIOR TO CONSTRUCTION. ANY DAMAGE CAUSED TO EXISTING PUBLIC UTILITIES BY THE CONTRACTOR WILL BE REPAIRED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.

**DIAL BEFORE YOU DIG NOTE**

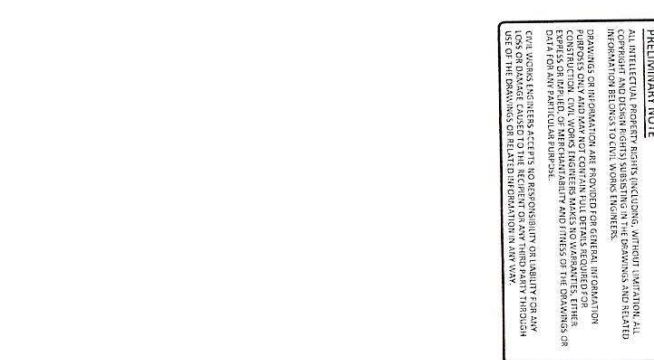
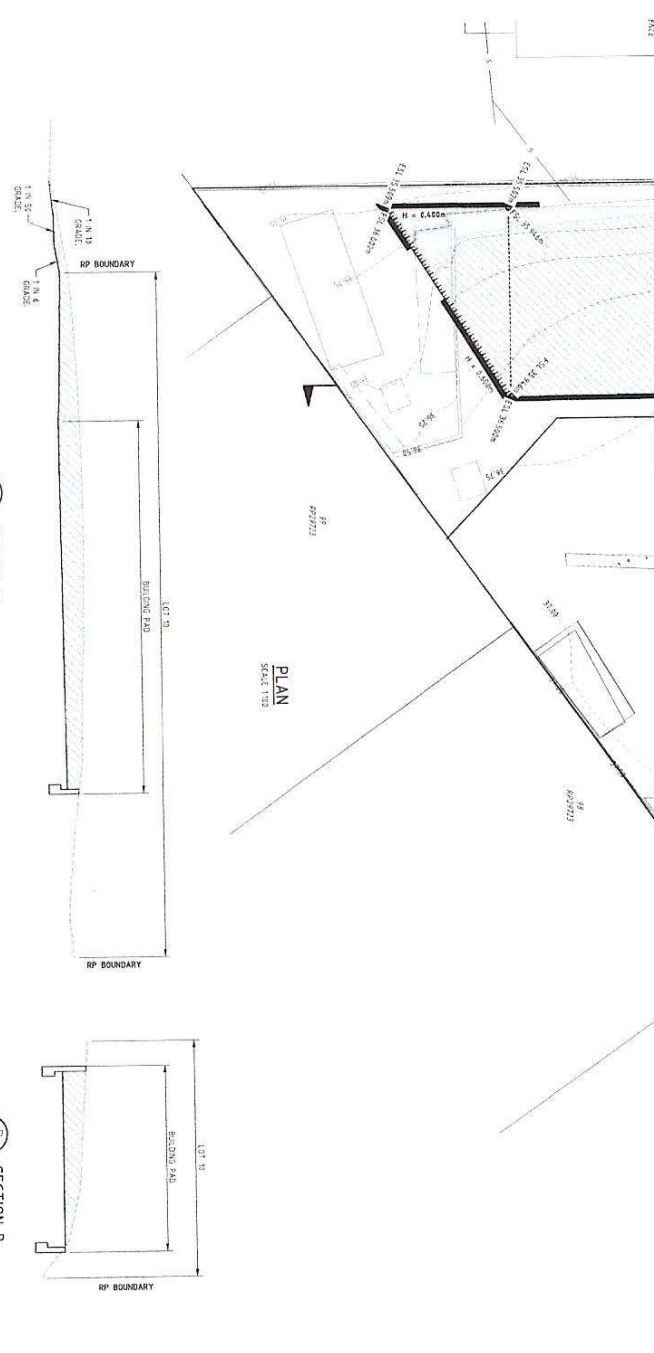
IT IS THE CONTRACTOR'S RESPONSIBILITY TO CONSULT WITH THE LOCAL COUNCIL AND THE UTILITIES COMPANIES PRIOR TO CONSTRUCTION. ANY DAMAGE CAUSED TO EXISTING PUBLIC UTILITIES BY THE CONTRACTOR WILL BE REPAIRED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.

**PRELIMINARY NOTE**

ALL INFORMATION ON THIS PLAN IS FOR INFORMATION ONLY. IT IS NOT TO BE USED FOR ANY OTHER PURPOSE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE LOCATION AND DEPTH OF ALL SERVICES PRIOR TO CONSTRUCTION. ANY DAMAGE CAUSED TO EXISTING PUBLIC UTILITIES BY THE CONTRACTOR WILL BE REPAIRED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.

**PRELIMINARY**

NOT FOR CONSTRUCTION



**CLIENT**

DAVID MANTEIT

**PROJECT**

728 ASHRIDGE ROAD

**CONCEPT EARTHWORKS SKETCH**

**ASSOCIATED CONSULTANT**

DAVID MANTEIT

**DATE**

2024-09-10

**SCALE**

1:500

**PROJECT DESCRIPTION**

CONCEPT EARTHWORKS SKETCH

**CLIENT**

DAVID MANTEIT

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728 ASHRIDGE ROAD

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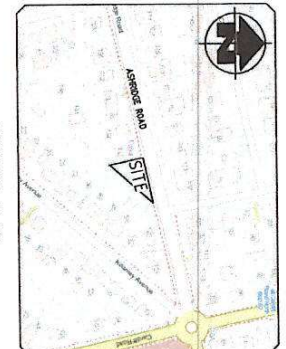
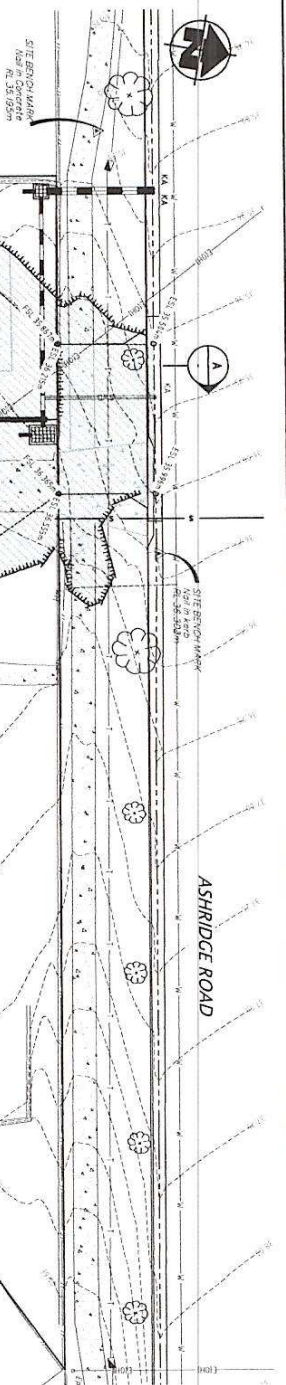
**SCALE**

1:500

**PROJECT DESCRIPTION**

CONCEPT EARTHWORKS SKETCH





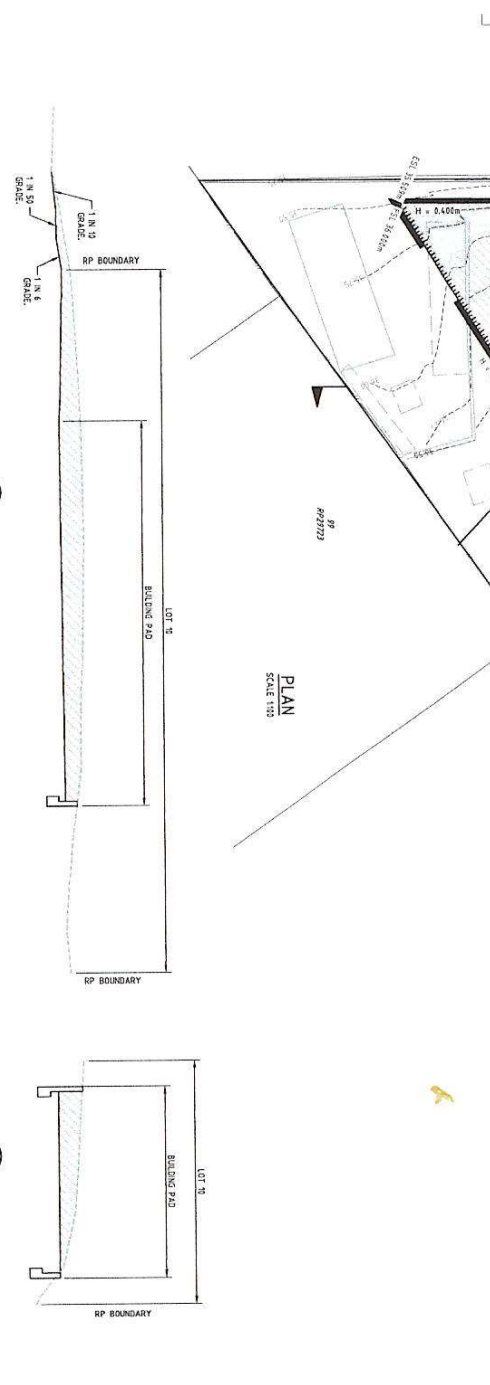
LOCALITY PLAN  
SCALE 1:250

**EXISTING SERVICES NOTE**  
THIS DESIGN HAS BEEN PREPARED BASED ON SERVICE AUTHORITY AS CONDUCTED INSPECTION. NO FURTHER INVESTIGATION HAS BEEN UNDERTAKEN TO VERIFY EXISTING SERVICE LOCATIONS OR DEPTHS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE LOCATION AND DEPTH OF ALL EXISTING SERVICES PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE CAUSED TO EXISTING SERVICES BY THE CONTRACTOR'S WORK.

**DEAL BEFORE YOU DIG NOTE**  
IT IS THE CONTRACTOR'S RESPONSIBILITY TO CONTACT THE RELEVANT AGENCIES AND OBTAIN ALL NECESSARY PERMITS PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE CAUSED TO EXISTING SERVICES BY THE CONTRACTOR'S WORK.

## PRELIMINARY NOT FOR CONSTRUCTION

**PRELIMINARY NOTE**  
THIS DESIGN IS A PRELIMINARY DESIGN AND IS NOT TO BE USED FOR CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE LOCATION AND DEPTH OF ALL EXISTING SERVICES PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE CAUSED TO EXISTING SERVICES BY THE CONTRACTOR'S WORK.



SECTION B  
SCALE 1:100

SECTION B  
SCALE 1:100

SCALE 1:100 AT ORIGINAL SIZE  
SCALE 1:250 AT ORIGINAL SIZE

**REAL PROPERTY DESCRIPTION**  
LOT 1, LOT 2, LOT 3, LOT 4, LOT 5, LOT 6, LOT 7, LOT 8, LOT 9, LOT 10, LOT 11, LOT 12, LOT 13, LOT 14, LOT 15, LOT 16, LOT 17, LOT 18, LOT 19, LOT 20, LOT 21, LOT 22, LOT 23, LOT 24, LOT 25, LOT 26, LOT 27, LOT 28, LOT 29, LOT 30, LOT 31, LOT 32, LOT 33, LOT 34, LOT 35, LOT 36, LOT 37, LOT 38, LOT 39, LOT 40, LOT 41, LOT 42, LOT 43, LOT 44, LOT 45, LOT 46, LOT 47, LOT 48, LOT 49, LOT 50, LOT 51, LOT 52, LOT 53, LOT 54, LOT 55, LOT 56, LOT 57, LOT 58, LOT 59, LOT 60, LOT 61, LOT 62, LOT 63, LOT 64, LOT 65, LOT 66, LOT 67, LOT 68, LOT 69, LOT 70, LOT 71, LOT 72, LOT 73, LOT 74, LOT 75, LOT 76, LOT 77, LOT 78, LOT 79, LOT 80, LOT 81, LOT 82, LOT 83, LOT 84, LOT 85, LOT 86, LOT 87, LOT 88, LOT 89, LOT 90, LOT 91, LOT 92, LOT 93, LOT 94, LOT 95, LOT 96, LOT 97, LOT 98, LOT 99, LOT 100.

Drawing No. **CW24091-SK10**  
Scale **AS SHOWN**  
Sheet **A**

**CIVILWORKS ENGINEERS**  
128 ASHRIDGE ROAD  
DARRA  
DAVID MANTEIT  
CONCEPT EARTHWORKS SKETCH

ASSOCIATED CONSULTANT  
128 ASHRIDGE ROAD  
DARRA  
DAVID MANTEIT  
CONCEPT EARTHWORKS SKETCH

128 ASHRIDGE ROAD  
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