# GABOR LORANT ARCHITECTS UPTOWN SEDONA PARKING FACILITY

PROJECT ADDRESS

GLA Project Number: PROJECT # Owner Project Number: 200806

KATHY KINSELLA

JON THOMPSON

HOLLI PLOOG,

#### PROJECT TEAM

#### OWNER

CITY COUNCIL: BILL CHISHOLM CITY OF SEDONA 102 ROADRUNNER DRIVE SEDONA, ARIZONA 86336 VICE MAYOR: SCOTT JABLOW CITY MANAGER: KAREN OSBURN CITY ENGINEER: J. ANDY DICKEY, PE

#### **ARCHITECT**

GABOR LORANT ARCHITECTS INC. 3326 N 3RD AVENUE SUITE 200 PHOENIX, ARIZONA 85013 TEL: 602.667.9090 | FAX: 602.667.9133 WWW.GABORLORANT.COM

#### CMAR

MCCARTHY BUILDING COMPANIES, INC. 6225 N 24TH ST - STE 200 PHOENIX, AZ 85016 PHONE 480-449-4700 EMAIL PHX@MCCARTHY.COM HTTPS://WWW.MCCARTHY.COM/

#### STRUCTURAL ENGINEER

ADVANCED STRUCTURAL ENGINEERING 9308 E. RAINTREE DR. SCOTTSDALE, AZ 85260 TEL: 602.264.1010

#### MPE ENGINEER

APPLIED ENGINEERING 2800 SOUTH RURAL ROAD SUITE 101 **TEMPE AZ 85282** TEL: 480.968.3070 WWW.APPLIEDENGINEERING.WS

#### CIVIL ENGINEERING LANDSCAPE ARCHITECTURE

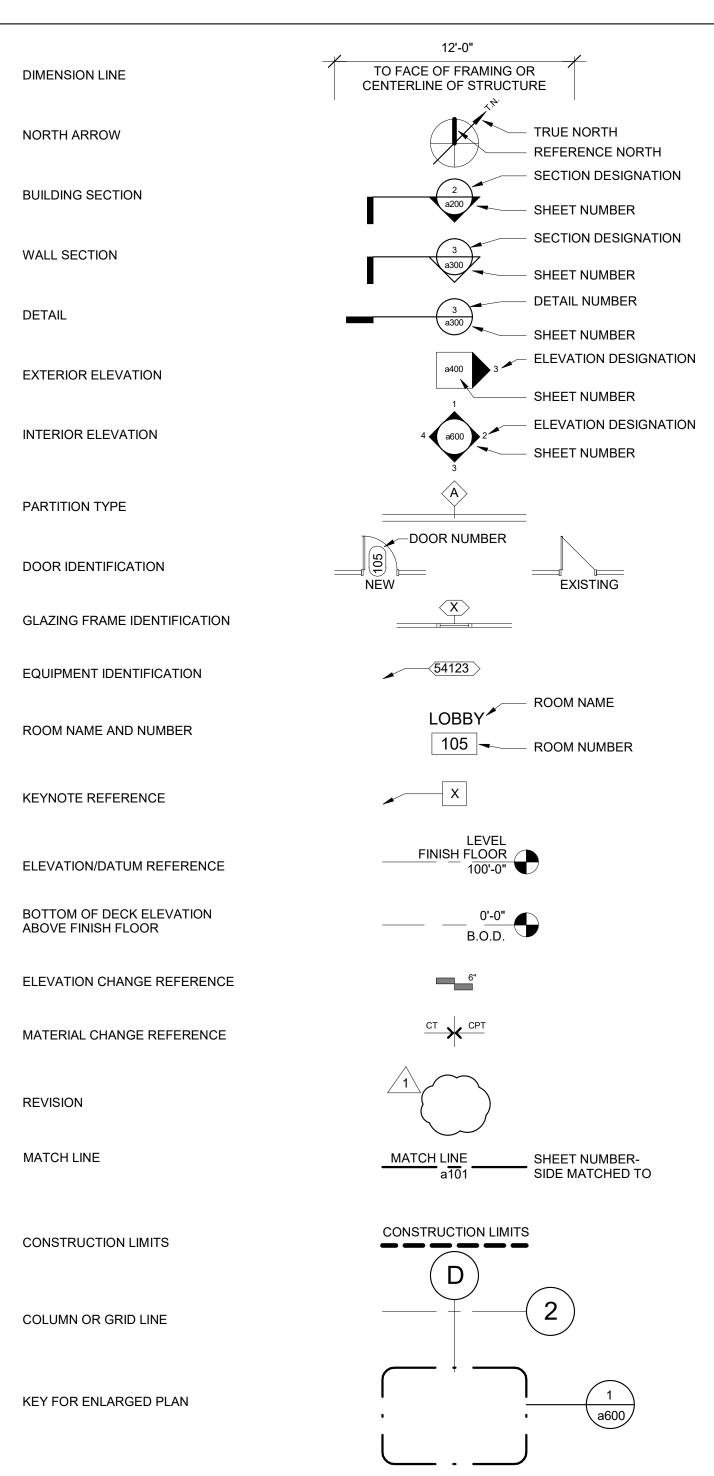
1001 WEST SOUTHERN AVENUE SUITE 131 MESA, AZ 85210 480-207-2666 HTTPS://WWW.KIMLEY-HORN.COM

# NOTICE OF EXTENDED PAYMENT PROVISION

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS
PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT.



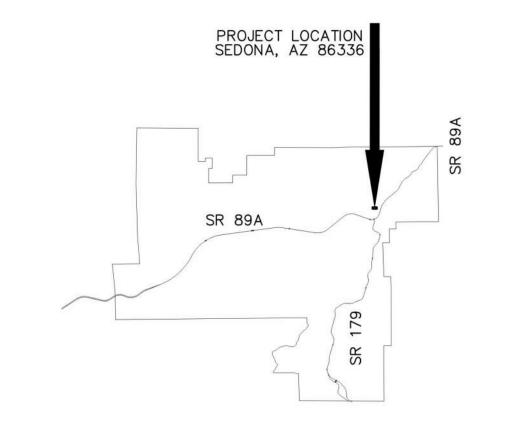
#### DRAFTING SYMBOLS





01 Architectu	ral	
a000	Cover Sheet	
a002	Occupancy & Code Analysis	
a050	Architectural Site Plan	
a100	Basement Floor Plan	
a101	Ground Level Floor Plan	
a102	Upper Deck Floor Plan	
a400	Exterior Elevations	
a500	Sections	
a850	Accessibility Diagrams	
02 Structural S000	COVER SHEET	
S001	GSN	
S002	GSN	
S003	GSN	
S100	BASEMENT FOUNDATION PLAN	
S101	GROUND LEVEL FRAMING PLAN	
S102	UPPER DECK FRAMING PLAN	
S200	PT BEAM PROFILES	
S300	TYP. CONCRETE DETAILS	
S301	TYP. CONCRETE DETAILS	
S302	TYP. MASONRY DETAILS	
S400	FOUNDATION DETAILS	
S500	FRAMING DETAILS	
S501	FRAMING DETAILS	
06 Civil		
CV01	COVER SHEET	
GD01	GRADING AND DRAINAGE PLAN	
GN01	GENERAL NOTES AND TYPICAL SECTIONS	
RD01	ROADWAY PLAN	
RD02	ROADWAY PLAN	
UT01	UTILITY PLAN	
07 Landscap	e	
LS01	LANDSCAPE PLAN	

SHEET INDEX





# SCHEMATIC DESIGN

**VERIFY SCALES** BAR IS ONE INCH ON ORIGINAL DRAWING

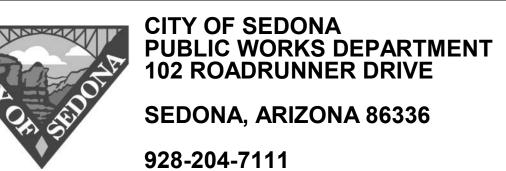
IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

Symbol	Revisions	Date	Appr.	Designed by:	Date:
				JL	07/30/2021
				Drawn by:	Scale:
				RGH	1/8" = 1'-0"
				Checked by:	Project Code:
					GLA PROJECT NO. 2010

#### gabor lorant architects

3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com





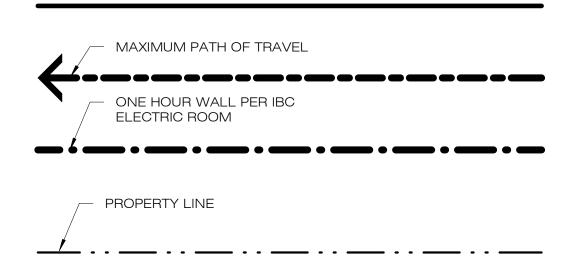
Uptown Sedona Parking Facility Schematic Design

Cover Sheet

a000 SHEET NO.

SHEET ID

#### SYMBOL LEGEND



#### PARKING COUNTS

BASEMENT LEVEL 97 SPACES

GROUND LEVEL
74 SPACES
7 ACC. SPACES

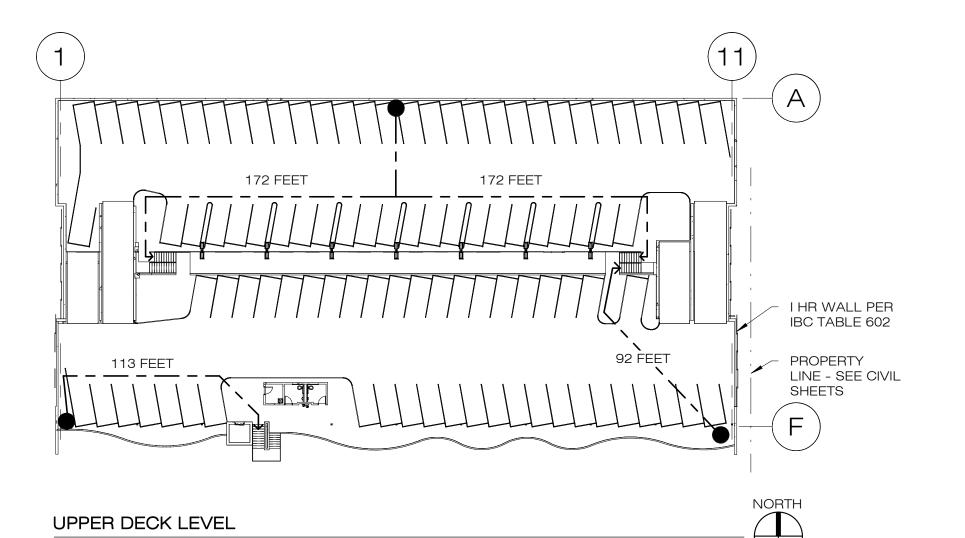
UPPER LEVEL 97 SPACES

TOTAL SPACES: 275

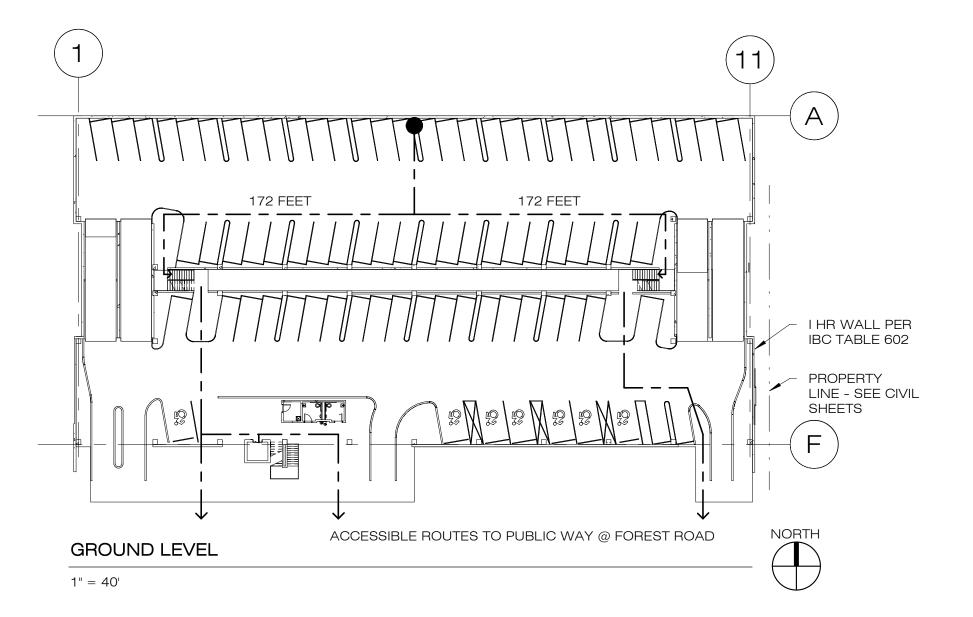
#### PARKING NOTES:

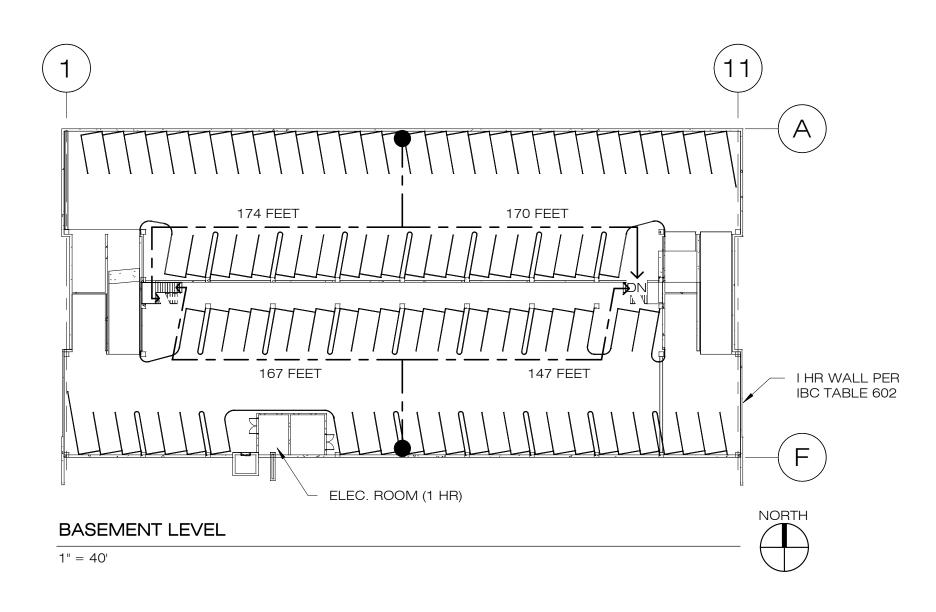
EACH ACCESSIBLE PARKING SPACE SHALL COMPRISE
RECTANGULAR AREAS NOT LESS THAN 11 FEET WIDE BY 18 FEET
LONG WITH A 5-FOOT MINIMUM WIDTH ACCESS AISLE ON ITS
RIGHT SIDE (EXCEPT IN LAYOUTS WHEREIN 2 ACCESSIBLE PARKING
SPACES SHARE A SINGLE 5-FOOT-WIDE ACCESS AISLE.

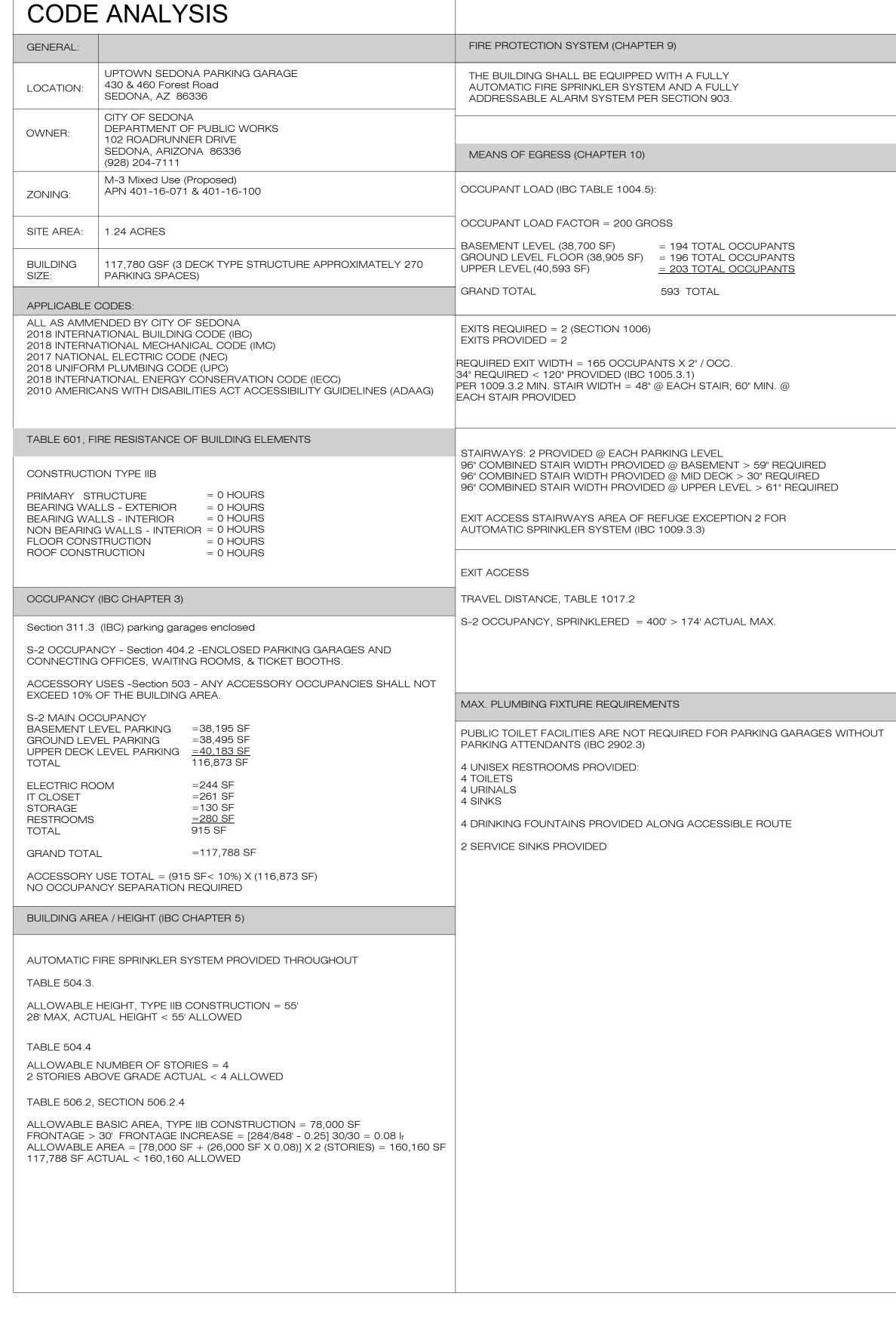
EACH ACCESS AISLE SHALL LEAD DIRECTLY TO AN ACCESSIBLE ROUTE OF TRAVEL.



1" = 40'







#### NOTICE OF EXTENDED PAYMENT PROVISION

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS
THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS
AFTER CERTIFICATION AND APPROVALS OF
BILLINGS AND ESTIMATES FOR PROGRESS
PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION
AND APPROVALS OF BILLINGS AND ESTIMATES
FOR RELEASE OF RETENTION AND WITHIN 15
DAYS AFTER CERTIFICATION AND APPROVAL OF
BILLINGS AND ESTIMATES FOR FINAL PAYMENT.



# SCHEMATIC DESIGN

VERIFY SCALES

BAR IS ONE INCH ON ORIGINAL DRAWING

IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

Symbol	Revisions	Date	Appr.	Designed by:	Date:
-			' '	JL	07/30/2021
				Drawn by:	Scale:
				RGH	1/8" = 1'-0"
				Checked by:	Project Code:
				JL/JP	GLA PROJECT NO. 20109

#### gabor lorant architects

3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com



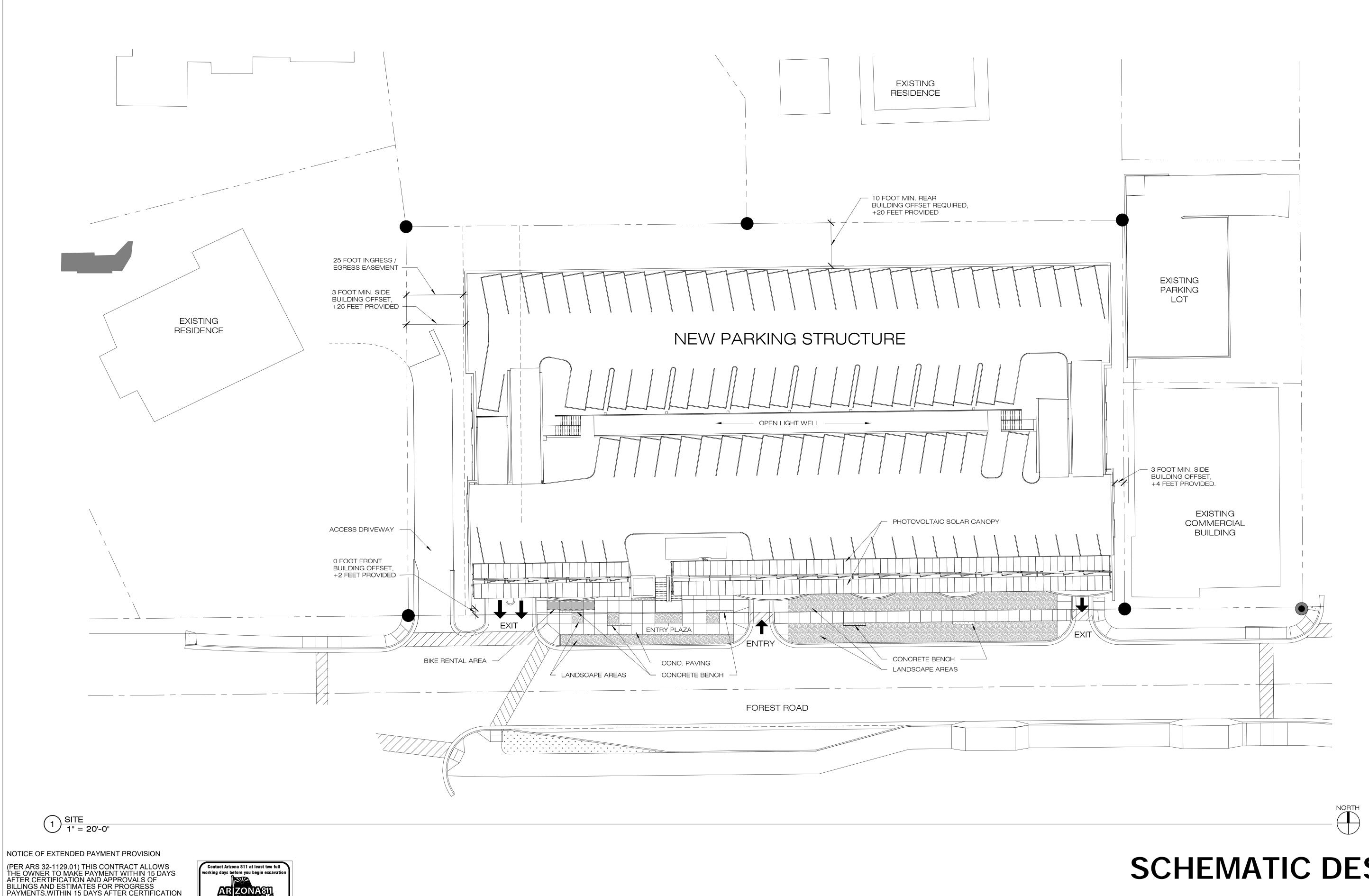


SEDONA, ARIZONA 86336 928-204-7111

Uptown Sedona Parking Facili Schematic Design
Occupancy & Code Analysis

sheet id a002

SHEET NO. OF



(PER ARS 32-1129.01) THIS CONTRACT ALLOWS
THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS
AFTER CERTIFICATION AND APPROVALS OF
BILLINGS AND ESTIMATES FOR PROGRESS
PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION
AND APPROVALS OF BILLINGS AND ESTIMATES
FOR RELEASE OF RETENTION AND WITHIN 15
DAYS AFTER CERTIFICATION AND APPROVAL OF
BILLINGS AND ESTIMATES FOR FINAL PAYMENT.



# SCHEMATIC DESIGN

VERIFY SCALES BAR IS ONE INCH ON ORIGINAL DRAWING

IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

Symbol	Revisions	Date	Appr.	Designed by:	Date:	
				JL	07/30/2021	
				Drawn by:	Scale:	
				RGH	1/8" = 1'-0"	
				Checked by:	Project Code:	
				JL/JP	GLA PROJECT NO. 20109	

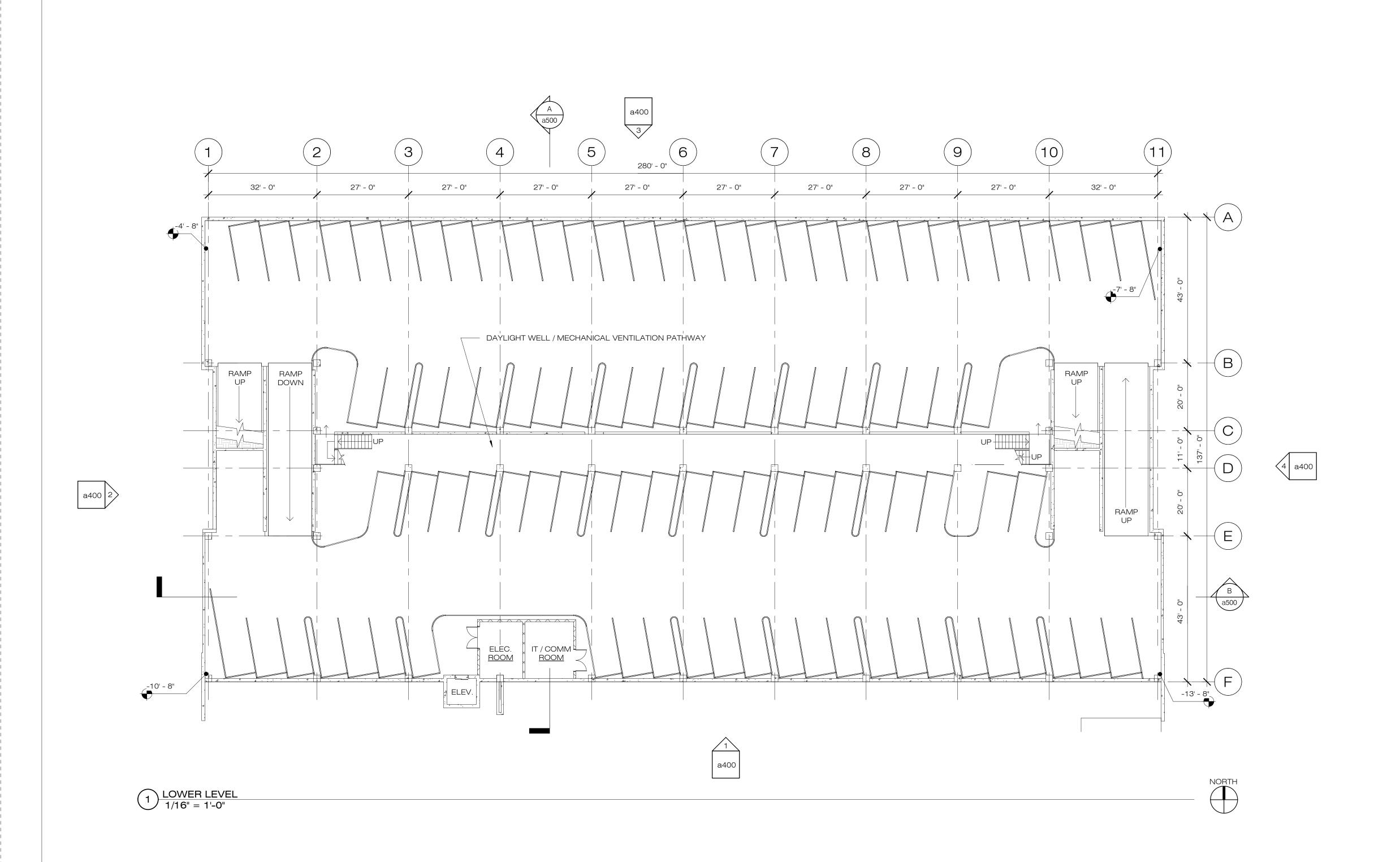
gabor lorant architects	
3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com	gla



CITY OF SEDONA PUBLIC WORKS DEPARTMENT 102 ROADRUNNER DRIVE
SEDONA, ARIZONA 86336
928-204-7111

Uptown Sedona Parking Facility Schematic Design Architectural Site Plan

SHEET ID a050



ROOM AREA TABULATION AREA Number Name ELEC. RM. 210 SF IT ROOM 240 SF 59 SF JAN UNISEX RR 61 SF UNISEX RR 61 SF 59 SF 61 SF UNISEX RR UNISEX RR 61 SF

PARKING COUNTS BASEMENT LEVEL 97 SPACES <u>GROUND LEVEL</u> 74 SPACES 7 ACC. SPACES

<u>UPPER LEVEL</u> 97 SPACES

TOTAL SPACES: 275

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS
THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS
AFTER CERTIFICATION AND APPROVALS OF
BILLINGS AND ESTIMATES FOR PROGRESS
PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION
AND APPROVALS OF BILLINGS AND ESTIMATES
FOR RELEASE OF PETENTION AND WITHIN 15

NOTICE OF EXTENDED PAYMENT PROVISION

FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT.



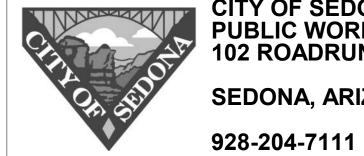
# SCHEMATIC DESIGN

VERIFY SCALES BAR IS ONE INCH ON ORIGINAL DRAWING

IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

Designed by: Date: Date Symbol Revisions Appr. 07/30/2021 Scale: Drawn by: 1/8" = 1'-0" Project Code: Checked by: GLA PROJECT NO. 20109

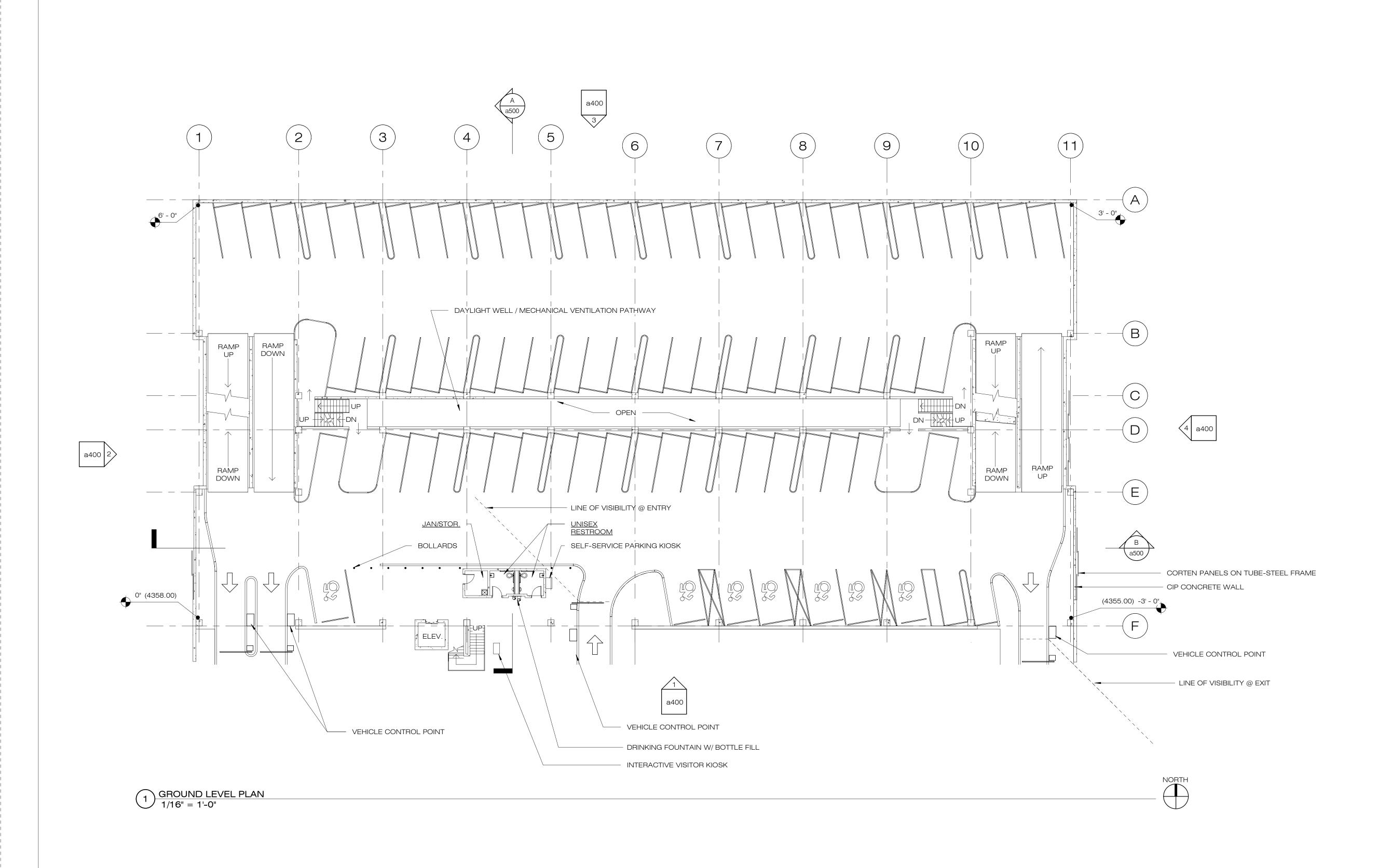
gabor lorant architects 3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com



CITY OF SEDONA
PUBLIC WORKS DEPARTMENT
102 ROADRUNNER DRIVE **SEDONA, ARIZONA 86336** 

Uptown Sedona Parking Facility Schematic Design Basement Floor Plan

SHEET ID a100



NOTICE OF EXTENDED PAYMENT PROVISION

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS
THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS
AFTER CERTIFICATION AND APPROVALS OF
BILLINGS AND ESTIMATES FOR PROGRESS
PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION
AND APPROVALS OF BILLINGS AND ESTIMATES
FOR RELEASE OF PETENTION AND WITHIN 15 FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT.



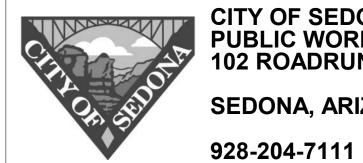
# SCHEMATIC DESIGN

VERIFY SCALES BAR IS ONE INCH ON ORIGINAL DRAWING

IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

Designed by: Date: Date Symbol Revisions Appr. 07/30/2021 Scale: Drawn by: 1/8" = 1'-0" Project Code: Checked by: GLA PROJECT NO. 20109

gabor lorant architects 3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com

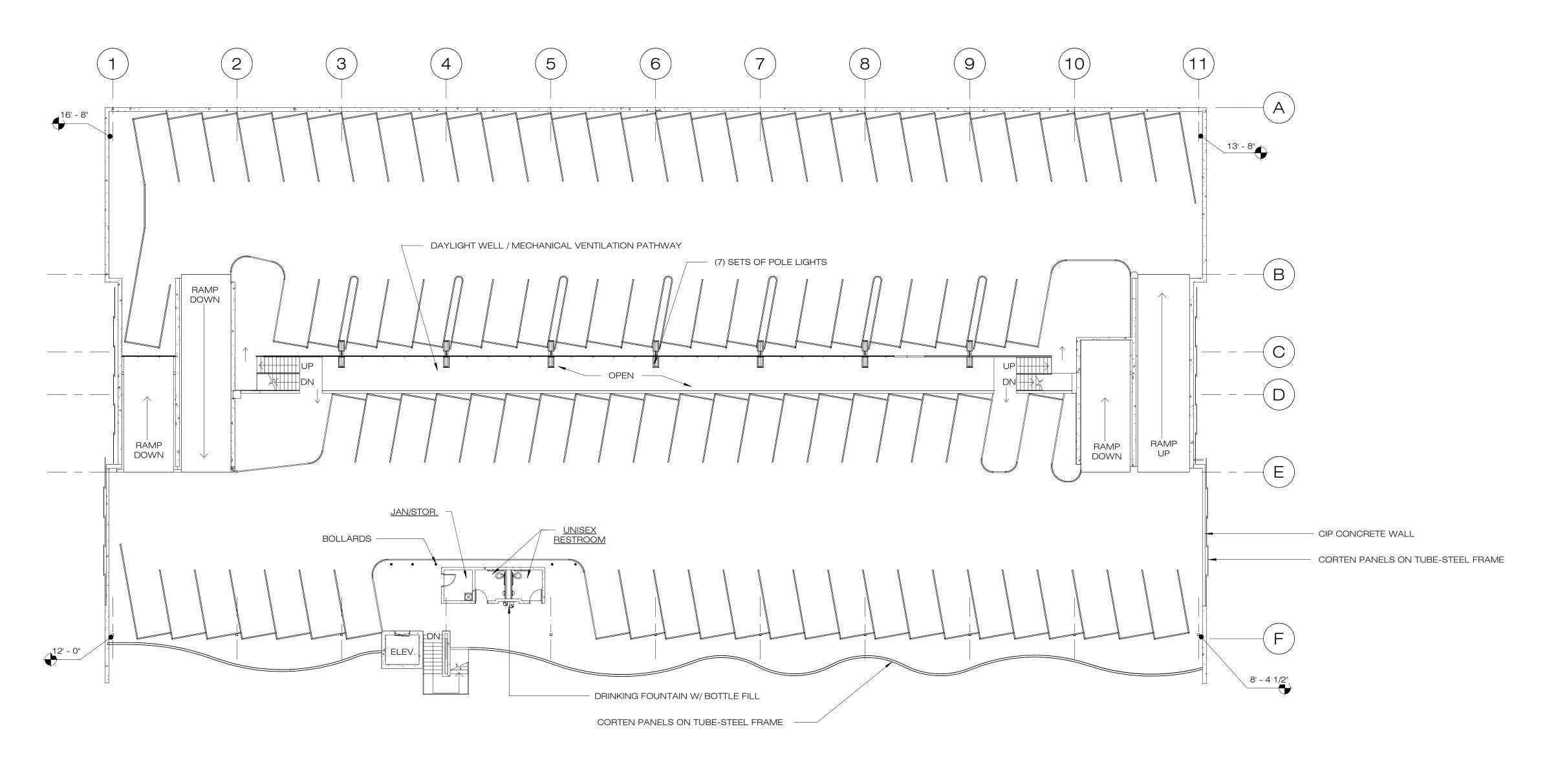


CITY OF SEDONA
PUBLIC WORKS DEPARTMENT
102 ROADRUNNER DRIVE **SEDONA, ARIZONA 86336** 

Uptown Sedona Parking Facility Schematic Design

Ground Level Floor Plan

SHEET ID a101



1 UPPER DECK 1/16" = 1'-0"



NOTICE OF EXTENDED PAYMENT PROVISION

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS
THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS
AFTER CERTIFICATION AND APPROVALS OF
BILLINGS AND ESTIMATES FOR PROGRESS
PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION
AND APPROVALS OF BILLINGS AND ESTIMATES
FOR RELEASE OF RETENTION AND WITHIN 15
DAYS AFTER CERTIFICATION AND APPROVAL OF
BILLINGS AND ESTIMATES FOR FINAL PAYMENT.



# SCHEMATIC DESIGN

VERIFY SCALES

BAR IS ONE INCH ON ORIGINAL DRAWING

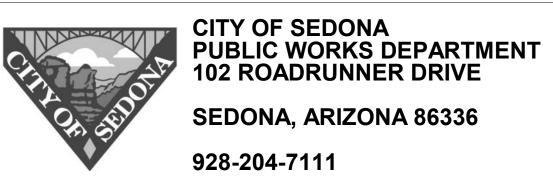
0 \_\_\_\_\_\_\_1"

IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

Sedona Parking Facility SHEET ID

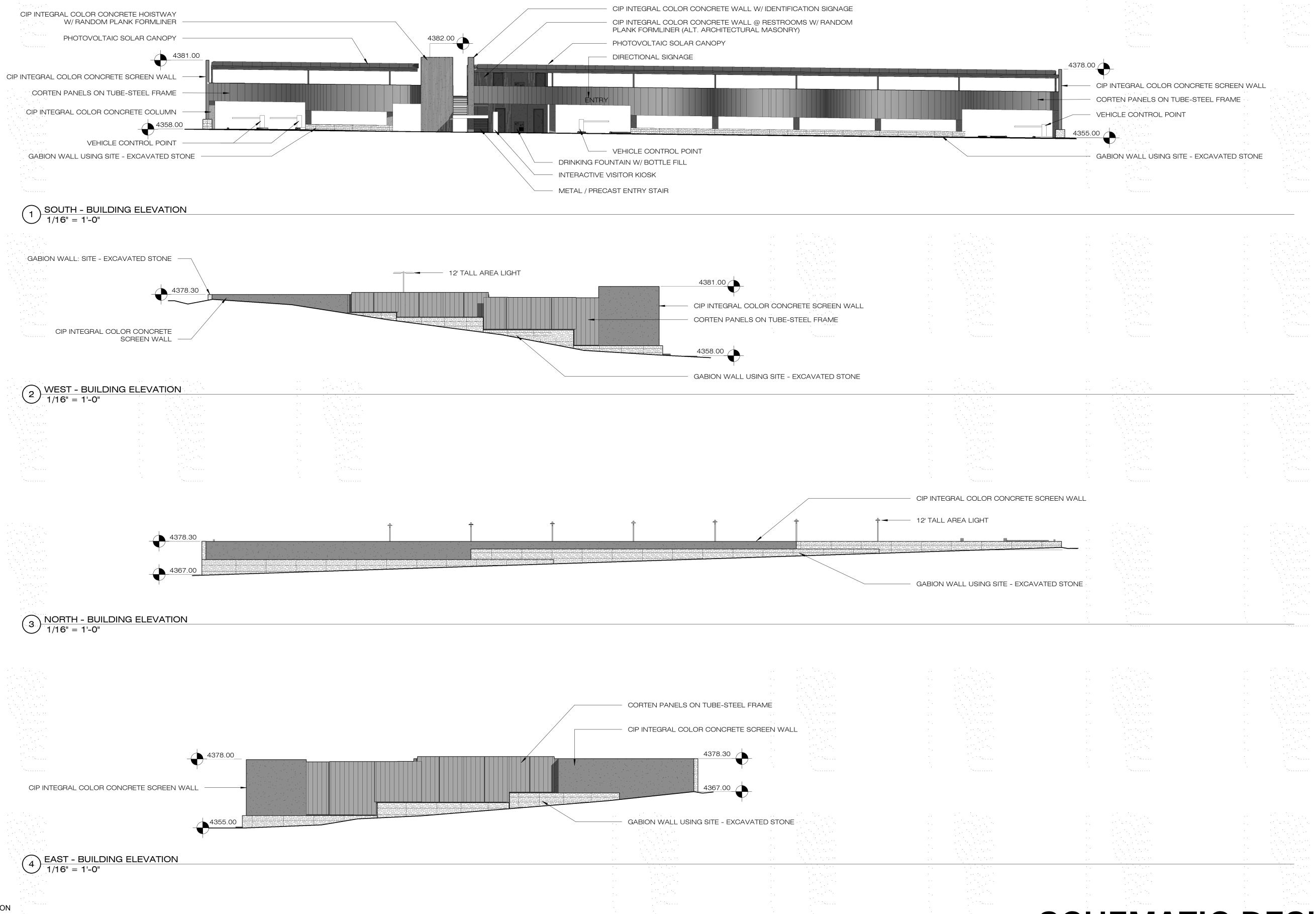
Symbol	Revisions	Date	Appr.	Designed by:	Date:
				JL	07/30/2021
				Drawn by:	Scale:
				RGH	1/8" = 1'-0"
				Checked by:	Project Code:
				JL/JP	GLA PROJECT NO. 20109

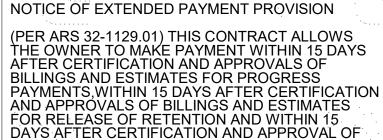




Schematic Design
Upper Deck Floor Plan

a102 SHEET NO.





BILLINGS AND ESTIMATES FOR FINAL PAYMENT.



# SCHEMATIC DESIGN

VERIFY SCALES

BAR IS ONE INCH ON ORIGINAL DRAWING

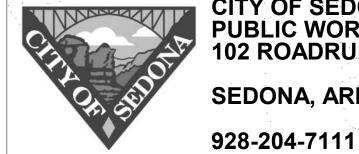
0 \_\_\_\_\_\_\_1"

IF NOT ONE INCH ON
THIS SHEET, ADJUST
SCALES ACCORDINGLY

Symbol	. *•	Revisions		Date	Appr.	Designed by.	Date.
	1, 11	. •				<u> </u>	07/30/2021
	. * *	٠.				JL	07/30/2021
* *.						Drawn by:	Scale:
		• •			· · ·	Diawii by.	Goale.
1	<u> </u>				·	RGH	1/8" = 1'-0"
	· · · .	• •				KGH	1/0 - 1-0
		•				Olaria alla alla alla alla alla alla alla	Duning to On the
						Checked by:	Project Code:
*****			**********		***	<u>.</u>	· · · · · · · · · · · · · · · · · · ·
						JL/JP	GLA PROJECT NO. 20109
						02/01	

gabor lorant architects

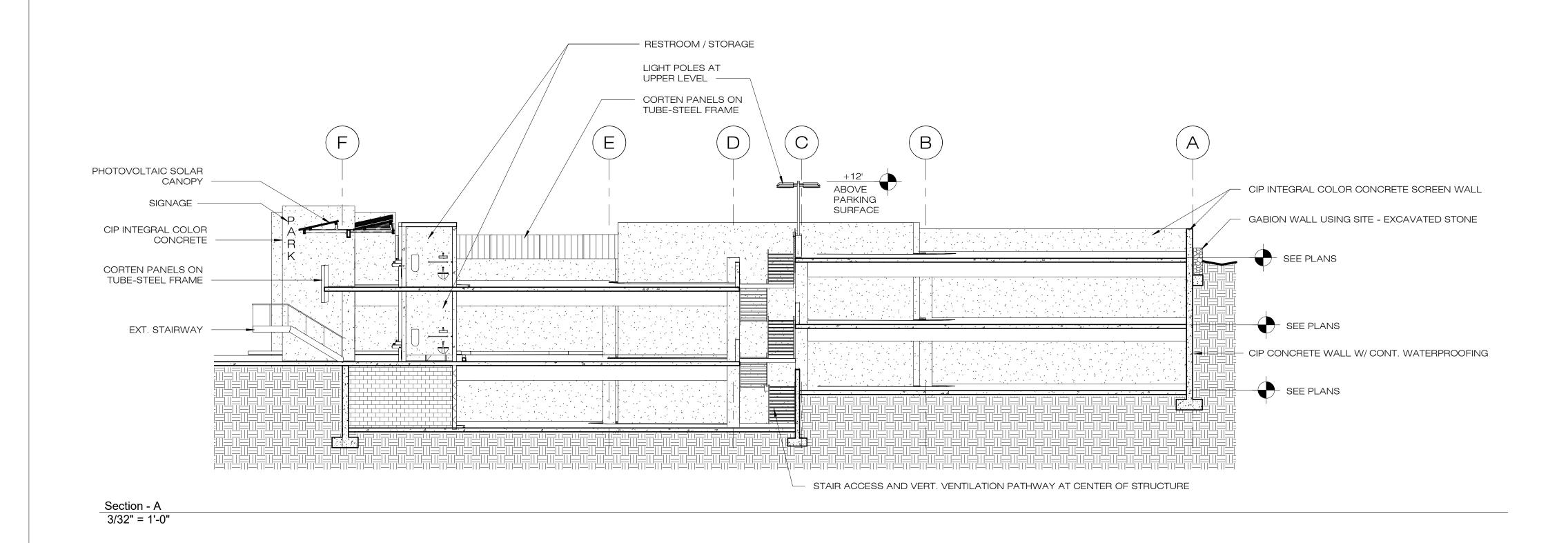
3326 n. 3rd avenue suite 200
phoenix, arizona 85013
tel: 602.667.9090
fax: 602.667.9133

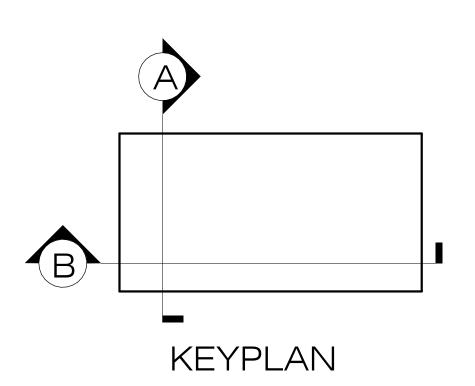


P	CITY OF SEDONA PUBLIC WORKS DEPARTMENT 102 ROADRUNNER DRIVE
	SEDONA, ARIZONA 86336

Uptown Sedona	Park	ing Facility
Schematic Design	· .	Northern Settler
Exterior Elevation	S	

ty	SHEET ID
	a400





(11)(9)LIGHT POLES AT UPPER LEVEL +12'
ABOVE ARCHITECTURAL PARKING GUARD RAILS AT TOP SURFACE LEVEL CORTEN PANELS ON TUBE-STEEL FRAME CORTEN PANELS ON RAMP TUBE-STEEL FRAME GABION WALL USING SLOPE -RAMP SITE - EXCAVATED GABION WALL USING SITE - EXCAVATED STONE STONE RAMP CIP CONCRETE WALL W/ WATERPROOFING BELOW SLOPE ( RAMP GRADE - CIP CONCRETE WALL W/ WATERPROOFING BELOW RAMP GRADE SLOPE. RAMP

Section - B 3/32" = 1'-0"

NOTICE OF EXTENDED PAYMENT PROVISION

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS
THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS
AFTER CERTIFICATION AND APPROVALS OF
BILLINGS AND ESTIMATES FOR PROGRESS
PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION
AND APPROVALS OF BILLINGS AND ESTIMATES
FOR RELEASE OF RETENTION AND WITHIN 15 FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT.



# SCHEMATIC DESIGN

VERIFY SCALES BAR IS ONE INCH ON ORIGINAL DRAWING IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

Uptown Sedona Parking Facility Schematic Design	
Sections	

SHEET ID
a500
SHEET NO.

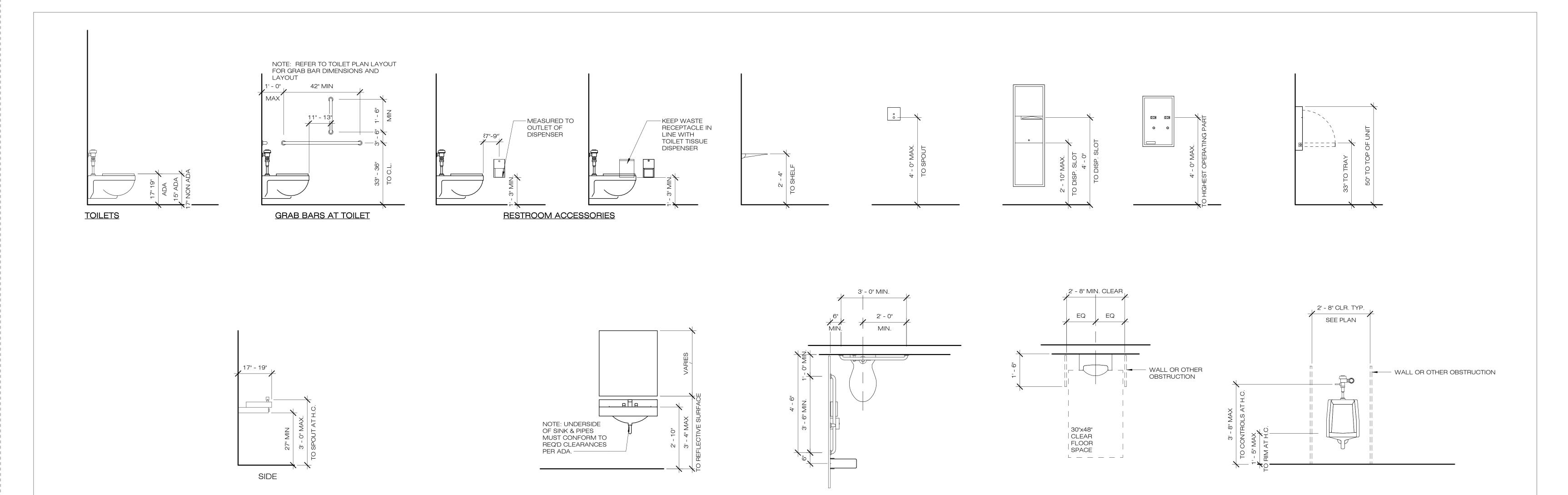
Symbol	Revisions	Date	Appr.	Designed by:	Date:
				JL	07/30/2021
				Drawn by:	Scale:
				RGH	1/8" = 1'-0"
				Checked by:	Project Code:
				JL/JP	GLA PROJECT NO. 20109

Date:

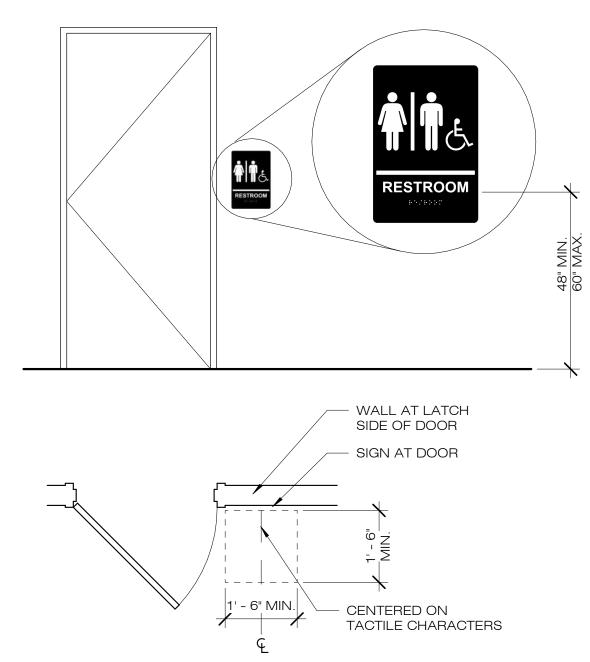
gabor lorant architects 3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com



CITY OF SEDONA PUBLIC WORKS DEPARTMEN 102 ROADRUNNER DRIVE
SEDONA, ARIZONA 86336
928-204-7111



LAVATORY SINKS





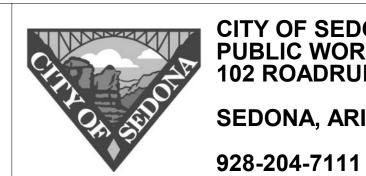
**TOILETS** 

THE OV AFTER BILLING PAYMEI	RS 32-1129.01) THIS CONTRACT ALLOWS WHER TO MAKE PAYMENT WITHIN 15 DAYS CERTIFICATION AND APPROVALS OF SS AND ESTIMATES FOR PROGRESS NTS, WITHIN 15 DAYS AFTER CERTIFICATION	Contact Arizona 811 at leas working days before you begin		SCH	<b>EMATIC</b>
FOR RE	PPROVALS OF BILLINGS AND ESTIMATES ELEASE OF RETENTION AND WITHIN 15 FTER CERTIFICATION AND APPROVAL OF GS AND ESTIMATES FOR FINAL PAYMENT.  Revisions	Call 811 or click Arizona  Date		Designed by:	Date:
Суппол	IVENSIOUS	Date	Appr.	JL	07/30/2021
				Drawn by:	Scale:
				RGH	1/8" = 1'-0"
				Checked by:	Project Code:
				JL/JP	GLA PROJECT NO. 20109

NOTICE OF EXTENDED PAYMENT PROVISION

**DRINKING FOUNTAINS** 

gabor lorant architects 3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com



CITY OF SEDONA PUBLIC WORKS DEPARTMENT 102 ROADRUNNER DRIVE
SEDONA, ARIZONA 86336

<u>URINALS</u>

Uptown Sedona Parking Facility
Schematic Design

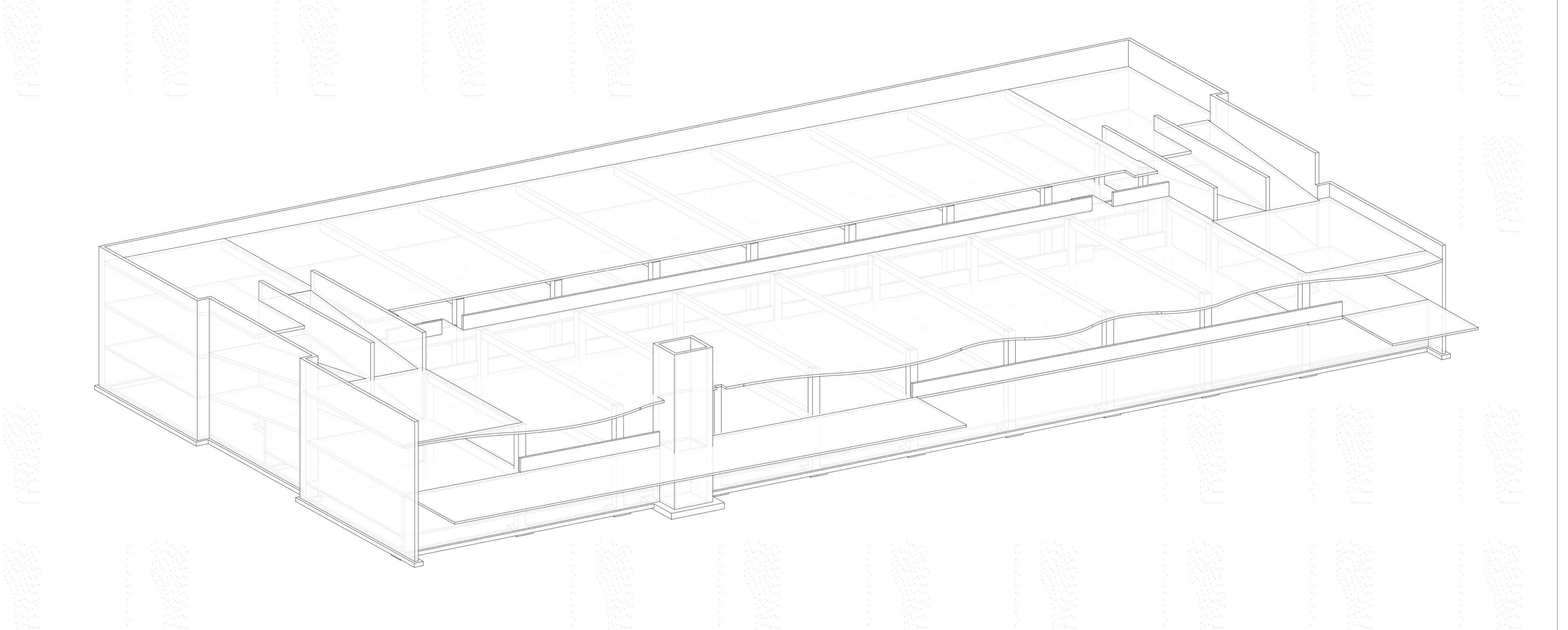
SHEET ID
a850

VERIFY SCALES

BAR IS ONE INCH ON ORIGINAL DRAWING

IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

Accessibility Diagrams SHEET NO.



NOTICE OF EXTENDED PAYMENT PROVISION (PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION

AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT.





EHales

Checked by:

# SCHEMATIC DESIGN

**REVIT MODEL NOTES:** 

LEVEL OF DETAIL IS NOT SUITABLE TO BE USED AS A FABRICATION TOOL BY THE GENERAL CONTRACTOR OR SUBCONTRACTORS. SEPARATE FABRICATION MODELS VERIFY SCALES BAR IS ONE INCH ON ORIGINAL DRAWING

IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

Designed by: Date: Revisions Date Appr. 07/30/2021 JAppleyard PROGRESS PRINT NOT FOR Scale: Drawn by:

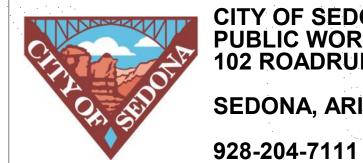
AS NOTED

GLA PROJECT NO. 20109

Project Code:

**CONSTRUCTION** 





CITY OF SEDONA
PUBLIC WORKS DEPARTMENT
102 ROADRUNNER DRIVE **SEDONA, ARIZONA 86336** 

Uptown Sedona Parking Facility Schematic Design **COVER SHEET** 

SHEET ID S000

#### GENERAL STRUCTURAL NOTES

#### BUILDING CODE:

2018 EDITION OF THE INTERNATIONAL BUILDING CODE, WITH CITY OF SEDONA AMENDMENTS.

#### LOADS:

GROUND SNOW LOAD  $(Pg) = \_\_$  PSF (NON-REDUCIBLE). FLAT ROOF SNOW LOAD  $(ff) = \_\_\_$  PSF (NON-REDUCIBLE). Ct = \_\_\_\_, Ce = \_\_\_\_ ROOF LIVE LOAD = 20 PSF (REDUCIBLE).

TOTAL ROOF DEAD LOAD = 20 PSF. FLOOR LIVE LOAD = 40 PSF (80% REDUCIBLE). SUPERIMPOSED DEAD LOAD ON FLOOR SLABS = 5 PSF.

STAIR LIVE LOAD = 100 PSF.

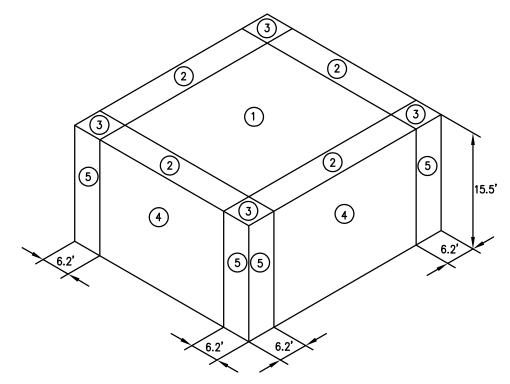
#### WIND:

THE STRUCTURE AND ITS COMPONENTS ARE DESIGNED FOR WIND LOADS AS DEFINED BY THE BUILDING CODE, WHERE:

BASIC WIND SPEED \_\_\_ MPH. RISK CATEGORY: \_\_\_\_. WIND EXPOSURE: \_\_\_\_

INTERNAL PRESSURE COEFFICIENT (GCpi) = \_\_\_\_.

VELOCITY PRESSURE, qz = \_\_\_ PSF AT h = \_\_ FT. (MEAN ROOF HEIGHT). DESIGN WIND PRESSURES COMPONENTS AND CLADDING:



LOCATION	ZONE	EFFECTIVE WIND AREA (SF)	DESIGN WIND PRESSURE	
		10	+	
	1	20	+	
		50	+	
		100	+	
		10	+	
ROOF	2	20	+	
		50	+	
		100	+	
		10	+	
	3	20	+	
		50	+	
		100	+	
		10	+	
WALLS	4	20	+	
		50	+	
		100	+	
		500	+	
		10	+	
		20	+	
	5	50	+	
		100	+	
		500	+	

ZONE1: INTERIOR AREA OF ROOF AWAY FROM BUILDING EXTERIOR WALLS.

ZONE2: ROOF AREAS WITHIN \_'-\_" OF EXTERIOR WALLS. ZONE3: ROOF AREAS WITHIN \_'-\_" OF BUILDING CORNERS.

ZONE4: EXTERIOR WALLS AWAY FROM BUILDING CORNERS.

ZONE5: EXTERIOR WALLS WITHIN\_'-\_" OF BUILDING CORNERS. -DESIGN WIND PRESSURES - PLUS AND MINUS SIGNS SIGNIFY PRESSURE ACTING TOWWARD

AND AWAY FROM EXTERIOR SURFACE. -LINEAR INTERPOLATION BETWEEN VALUES OF TRIBUTARY AREA IS PERMISSIBLE.

#### SEISMIC:

RISK CATEGORY =  $\_\_$ Seismic Importance Factor: le = \_\_\_\_. DESIGN CATEGORY = \_\_\_\_. SITE CLASS \_\_\_\_.  $Sds = ___, Sd1 = ___$ BASIC SEISMIC-FORCE-RESISTING SYSTEM (\_\_\_\_\_ ANALYSIS PROCEDURE USED (\_\_\_\_\_) Cs = \_\_\_\_ (FACTORED/WORKING STRESS). Vbase = \_\_\_\_ (FACTORED/WORKING STRESS).

NOTICE OF EXTENDED PAYMENT PROVISION

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT

Symbol





Designed by:

Drawn by:

Checked by:

JAppleyard

**EHales** 

# PK Associates, L.L.C. Website: www.pkastructural.com

Date:

Scale:

Project Code

07/30/2021

**AS NOTED** 

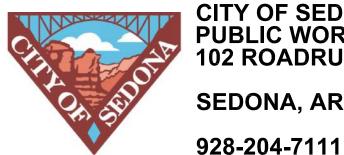
GLA PROJECT NO. 20109

# SCHEMATIC DESIGN

NOT FOR

# gabor lorant architects

3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com





**SEDONA, ARIZONA 86336** 

NOTES

Schematic Design

#### LAP SPLICES IN CONCRETE (CONT.):

ALL SPLICE LOCATIONS SUBJECT TO APPROVAL BY THE STRUCTURAL ENGINEER. PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZONTAL BARS AT ALL CORNERS AND INTERSECTIONS PER TYPICAL DETAILS. REINFORCING BAR SPACING GIVEN ARE MAXIMUM ON CENTERS. ALL BARS PER CRSI SPECIFICATIONS AND HANDBOOK. DOWEL ALL VERTICAL REINFORCING TO FOUNDATION WITH STANDARD 90-DEGREE HOOKS UNLESS NOTED OTHERWISE. SECURELY TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE. CONCRETE COLUMN DOWEL EMBEDMENT SHALL BE A STANDARD COMPRESSION DOWEL WITH EMBEDMENT LENGTH ACCORDING TO THE LATEST EDITION OF THE ACI 318 FOR SHEARWALL LATERAL SYSTEMS AND TENSION DOWEL LENGTHS FOR MOMENT FRAME LATERAL SYSTEMS. (UNLESS NOTED OTHERWISE ON PLANS OR DETAILS).

ALL ANCHOR BOLTS SHALL BE ASTM F1554 GRADE 36 (WELDABLE), U.N.O., WHEN SPECIFIED ON PLAN, ALL ASTM F1554 GRADE ANCHOR BOLTS SHALL BE 511 (WELDABLE) ANCHOR BOLTS.

MAY BE BENT IN FIELD PER AISC REQUIREMENTS TO FIXED DAMAGED ANCHOR BOLTS; FIELD BENT IS LIMITED TO 45° OR LESS. ANCHOR BOLTS CAN HAVE HEADED OR HOOKED BOLTS SHAPES. USE HEADED ANCHOR BOLTS WHERE ANCHOR BOLT SHAPE IS NOT SPECIFIED. ANCHOR BOLTS SHALL BE INSPECTED PER SPECIAL INSPECTION TABLE REQUIREMENTS.

#### DRYPACK:

DRYPACK SHALL BE 5,000 PSI NON-SHRINK GROUT, FIVE STAR OR EQUIVALENT. INSTALL DRYPACK UNDER BEARING PLATES BEFORE FRAMING MEMBER IS INSTALLED. AT COLUMNS, INSTALL DRYPACK UNDER BASEPLATES AFTER COLUMN HAS BEEN PLUMBED BUT PRIOR TO FLOOR OR ROOF INSTALLATION.

#### STRUCTURAL STEEL:

ALL STRUCTURAL STEEL SHALL BE ASTM A992 (Fy = 50 KSI). ALL CHANNELS, ANGLES, AND PLATES SHALL BE ASTM A36 (Fy = 36 KSI). ALL PIPE STEEL SHALL BE ASTM A501 (Fy = 36 KSI) OR ASTM A53, TYPE E OR S, GRADE B (Fy = 35 KSI). (NOTE: ALL PLATES IN MOMENT CONNECTIONS, BRACED FRAMES AND WHERE OTHERWISE NOTED SHALL BE Fy = 50 KSI STEEL). ALL TUBE STEEL SHALL BE ASTM A500 (Fy = 46 KSI RECT. AND FV = 42 KSI ROUND). ALL CONSTRUCTION PER LATEST AISC HANDBOOK. ALL REFERENCE TO HEADED STUDS SHALL BE HIGH STRENGTH HEADED STUDS. ATTACHMENT OF HEADED STUDS SHALL CONFORM TO ALL REQUIREMENTS OF THE LATEST EDITION OF THE "RECOMMENDED PRACTICES FOR STUD WEIDING" AND THE "STRUCTURAL WELDING CODE" PUBLISHED BY AWS. ALL BOLTS, ANCHOR BOLTS, EXPANSION BOLTS, ETC. SHALL BE INSTALLED WITH STEEL WASHERS AT FACE OF WOOD OR AT SLOTTED HOLES IN STEEL SECTIONS. ALL HIGH STRENGTH BOLTING SHALL BE INSPECTED BY AN INDEPENDENT TESTING LABORATORY. ALL WELDING SHALL BE PERFORMED BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN THE TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES. CERTIFICATES SHALL BE THOSE ISSUED BY AN ACCEPTED TESTING AGENCY. PER AISC, CHARPY V-NOTCH (CVN) IMPACT TEST RESULTS IN ACCORDANCE WITH ASTM A6/A6M ARE REQUIRED FOR HOT-ROLLED SHAPES WITH A FLANGE THICKNESS EXCEEDING 2" AND BUILT-UP MEMBERS CONSISTING OF PLATES WITH A THICKNESS EXCEEDING 2" THAT ARE SPLICED USING COMPLETE JOINT PENETRATION GROOVE WELDS. IN ADDITION, WHEN USED IN THE LATERAL FORCE RESISTING SYSTEM, HOT-ROLLED SHAPES WITH FLANGES 1 1/2" AND GREATER THICKNESS, AND PLATES 2" AND GREATER THICKNESS, SHALL ALSO MEET (CVN) TOUGHNESS REQUIREMENTS PER AISC 341. ALL WELDING DONE BY E70 SERIES LOW HYDROGEN RODS UNLESS NOTED OTHERWISE. FOR GRADE 60 REINFORCING BARS, USE E90 SERIES. AT MOMENT CONNECTIONS, REMOVE ALL WELD BACKING AND RUN-OFF TABS AND BACKGOUGE TO SOUND WELD METAL. BACKWELD WITH A MINIMUM 5/16" FILLET. ALL WELDING PER LATEST AMERICAN WELDING SOCIETY STANDARDS, (EXCEPT STEEL JOISTS AND JOIST GIRDERS SHALL COMPLY WITH SJI STANDARDS). THESE DRAWINGS DO NOT DISTINGUISH BETWEEN SHOP AND FIELD WELDS; THE CONTRACTOR MAY SHOP WELD OR FIELD WELD AT HIS DISCRETION. SHOP WELDS AND FIELD WELDS SHALL BE SHOWN ON THE SHOP DRAWINGS SUBMITTED FOR REVIEW. ALL FULL (COMPLETE) PENETRATION WELDS SHALL BE TESTED AND CERTIFIED BY AN INDEPENDENT TESTING LABORATORY. WHEN STRUCTURAL STEEL IS FURNISHED TO A SPECIFIED MINIMUM YIELD POINT GREATER THAN 36 KSI, THE ASTM OR OTHER SPECIFICATION DESIGNATION SHALL BE INCLUDED NEAR THE FRECTION MARK ON EACH SHIPPING ASSEMBLY OR IMPORTANT CONSTRUCTION COMPONENT, OVER ANY SHOP COAT OF PAINT, PRIOR TO SHIPMENT FROM THE FABRICATOR'S PLANT.

#### STRUCTURAL BOLTS:

ALL STRUCTURAL BOLTS SHALL BE ASTM F3125 GRADE A325 TYPE 1 AND SHALL BE INSTALLED AS BEARING-TYPE CONNECTIONS WITH THREADS INCLUDED IN SHEAR PLANE (I.E. A TYPE "N" CONNECTION) UNLESS NOTED OTHERWISE. USE A325 TYPE 3 BOLTS AT ALL CONNECTIONS EXPOSED TO WEATHERING. USE SC (SLIP CRITICAL) AT ALL MOMENT FRAMES AND BRACED FRAMES. USE ASTM A354 BOLTS WHERE THE BOLT LENGTH REQUIRED FOR THE CONNECTION EXCEEDS THE MINIMUM LENGTH OF AN A325 BOLT. BOLTS MAY BE TIGHTENED USING ANY AISC APPROVED METHOD. ALL STRUCTURAL BOLTING SHALL BE INSPECTED BY AN INDEPENDENT TESTING LABORATORY TO ENSURE BOLT TENSION.

#### STEEL DECKING:

#### ROOF DECK:

DECK SHALL BE \_\_\_\_ DEEP, \_\_ WIDE, \_\_ GAGE PAINTED (GALVANIZED) STEEL, WITH MINIMUM YIELD STRESS OF 50 KSI, WITH MINIMUM S = \_\_\_\_\_ IN3 AND I = \_\_\_\_\_ IN4 PER FOOT OF WIDTH. DECK SHALL BE ERECTED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AS \_ SPAN MINIMUM AND SHALL BE ATTACHED FOR A MINIMUM DIAPHRAGM SHEAR CAPACITY OF \_\_\_ PLF USING THE FOLLOWING MINIMUM

WELD DECK TO SUPPORTING MEMBERS WITH  $_-$  1/2" DIAMETER OR 3/8" X 1" PUDDLE WELDS PER SHEET AT ENDS, END LAPS AND AT INTERMEDIATE SUPPORTS, AND AT \_\_\_" O.C. AT PERIMETER BEAMS AND OPENING EDGES RUNNING PARALLEL TO THE DECK. SIDE SEAM ATTACHMENT SHALL BE BUTTON PUNCHES AT \_\_\_ O.C. (1 1/2" SIDE SEAM WELDS AT \_\_\_" O.C.).

SHEET INDEX		
SHEET		
S000	COVER SHEET	
S001	GENERAL STRUCTURAL NOTES	
S002	GENERAL STRUCTURAL NOTES	
S003	GENERAL STRUCTURAL NOTES	
S100	BASEMENT FOUNDATION	
S101	GROUND LEVEL FRAMING PLAN	
S102	UPPER DECK FRAMING PLAN	
S200	PT BEAM PROFILES	
S300	TYPICAL CONCRETE DETAILS	
S301	TYPICAL CONCRETE DETAILS	
S302	TYPICAL MASONRY DETAILS	
S400	FOUNDATION DETAILS	
S500	FRAMING DETAILS	
S501	FRAMING DETAILS	
CHECKED BY: _ CHECKED BY: _ CHECKED BY: _		

#### APPLY UNLESS NOTED OTHERWISE MASONRY:

2000

2500

3000

\_; DATED \_ SOIL REPORT BY \_\_; JOB NO.\_ SHALL BEAR ON FIRM, UNDISTURBED SOIL (CONTROLLED COMPACTED FILL). IN ACCORDANCE WITH THE ABOVE REPORT. BOTTOM OF FOOTING TO BE \_\_\_\_\_ MINIMUM BELOW FINISHED GRADE. (THESE FOOTING DEPTHS ARE MINIMUMS AND THE CONTRACTOR SHALL COORDINATE WITH SOILS REPORT AND OTHER TRADES TO ENSURE THESE MINIMUMS ARE SUFFICIENT FOR THE WORK.) (COMPACTED FILL SHALL EXTEND \_\_\_\_ BEYOND EACH EDGE OF FOOTING.) FINISHED GRADE IS DEFINED AS TOP OF SLAB FOR INTERIOR FOOTINGS AND LOWEST ADJACENT GRADE WITHIN 5 FEET FOR PERIMETER FOOTINGS. DESIGN SOIL BEARING VALUE = \_\_\_\_\_ PSF.

PLACE CAISSON CONCRETE ON CLEAN, INSPECTED BEARING A MINIMUM OF \_\_\_\_\_ INTO BEARING STRATA. FOR BID PURPOSES ONLY, ASSUME TOP OF BEARING STRATA \_\_\_\_\_\_ BELOW FINISHED FLOOR (AT ELEVATION \_\_\_\_\_\_). MINIMUM CAISSON DEPTH SHALL NOT BE LESS THAN \_\_\_\_\_ B.F.F. SEE SPECIFICATIONS FOR ADD OR DEDUCT PROVISIONS FROM THE BASE BID. EXACT BEARING ELEVATION SHALL BE FIELD VERIFIED BY SOILS ENGINEER AT THE TIME OF DRILLING. DESIGN SOIL BEARING VALUE = \_\_\_\_\_ PSF.

FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY SOILS ENGINEER PRIOR TO PLACEMENT OF CONCRETE.

#### **CONCRETE:**

FOUNDATIONS:

MINIMUM 28 DAY STRENGTH 3,000 PSI EXCEPT AS FOLLOWS: (TYPE II, U.N.O.)

COLUMNS ----5,000 PSI MIN(SEE SCHEDULE) TOPPING FOR STEFL DECK -----3.500 PSI TOPPING FOR PRECAST UNITS -----3.000 PSI BEAMS. WALLS AND GRADE BEAMS -----4.000 PSI MILD AND POST-TENSIONED SLABS ----- 5.000 PSI AT 28 DAYS. 6.000 PSI AT 56 DAYS SLABS ON GRADE ------3.000 PSI SHEAR WALLS -----SEE SCHEDULE FOUNDATIONS AND CAISSONS -----------------3,000 PSI (SEE SCHEDULE)

MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED, EXCEPT THAT SLABS ON GRADE NEED BE VIBRATED ONLY AROUND UNDER-FLOOR DUCTS, ETC. MAXIMUM SLUMP 4 1/2" FOR CONCRETE WITHOUT PLASTICIZER. IF PLASTICIZER IS USED. A HIGHER FINAL SLUMP MAY BE ALLOWED UPON STRUCTURAL ENGINEER'S APPROVAL. CAST CLOSURE POUR AROUND COLUMNS AFTER COLUMN DEAD LOAD IS APPLIED. UNLESS APPROVED OTHERWISE IN WRITING BY THE ARCHITECT, ALL CONCRETE SLABS ON GRADE SHALL BE BOUND BY CONTROL JOINTS (KEYED OR SAW CUT). SUCH THAT THE ENCLOSED AREA DOES NOT EXCEED 225 SQUARE FEET. KEYED CONTROL JOINTS NEED ONLY OCCUR AT EXPOSED EDGES DURING POURING, ALL OTHER JOINTS MAY BE SAW CUT. CONTRACTOR SHALL SUBMIT PROPOSED LOCATIONS FOR APPROVAL PRIOR TO CONSTRUCTION.

ALL CONCRETE SLABS OVER STEEL DECK SHALL BE BOUND BY CONTROL JOINTS (KEYED OR SAW CUT) SUCH THAT THE ENCLOSED AREA DOES NOT EXCEED 900 SQUARE FEET. CONTRACTOR SHALL SUBMIT PROPOSED LOCATIONS FOR APPROVAL PRIOR TO CONSTRUCTION.

VERIFY LOCATION OF ALL CONTROL JOINTS IN CONCRETE TOPPING SLABS OVER PRECAST ELEMENTS (HAMBRO JOISTS) WITH MANUFACTURER. TOOLED JOINTS SHALL OCCUR AT ENDS AND SIDES OF ALL INDIVIDUAL PRECAST ELEMENTS.

CONTRACTOR SHALL REVIEW ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR SPECIAL SLAB TREATMENTS AND VAPOR BARRIERS REQUIRED FOR FINISH FLOORING.

SUMMARY OF COVER REQUIRED FOR FIRE RATINGS

MIN. BOTTOM BAR

COVERAGE

3/4" TO PT

3/4" TO REBAR

3/4" TO REBAR

3/4" TO PT

1 3/4" TO PT

1" TO REBAR

3/4" TO PT

3/4" TO REBAR

2 3/8" TO PT

1 1/4" TO REBAR

COVERAGE

1" TO LONGITUDINAL

\* PROVIDE 3/4" MINIMUM TOP COVERAGE FOR ALL STRUCTURAL ELEMENTS

(1) INTERIOR BAYS ARE CONSIDERED RESTRAINED BY SLAB TO COLUMN

BOTTOM REINFORCING ONLY. FIRST COLUMN ALONG EDGE IS

CONSIDERED UNRESTRAINED FOR THE TOP REINFORCING ONLY.

(2) MINIMUM THICKNESS OF CONCRETE COVER FOR BOTH PRESTRESSING

TENDONS AND MILD REINFORCEMENT U.N.O.

ALL OTHER PER LATEST EDITION OF ACI 318.

CONNECTIONS. END BAYS ARE CONSIDERED UNRESTRAINED FOR

STRUCTURAL

ELEMENT

FLOOR/ROOF

MILD OR PT SLAB

FLOOR/ROOF

MILD OR PT SLAB

FLOOR /ROOF

MILD OR PT SLAB

FLOOR/ROOF

MILD OR PT SLAB

FLOOR/ROOF

MILD OR PT SLAB

FLOOR/ROOF

MILD OR PT SLAB

CONCRETE WALL

CONCRETE

COLUMN

CONCRETE

CONCRETE

CONCRETE

CONCRETE

**RATING** 

/3HR.

**FOOT NOTES:** 

RESTRAINT

CONDITION (1)

RESTRAINED

UNRESTRAINED

RESTRAINED

UNRESTRAINED

RESTRAINED

UNRESTRAINED

RESTRAINED

UNRESTRAINED

UNRESTRAINED

THE PLANS, PLACE CONTROL JOINTS IN MASONRY WALLS SUCH THAT NO STRAIGHT RUN OF WALL EXCEEDS 24'-0". CONTROL JOINTS SHALL NOT OCCUR AT WALL CORNERS, INTERSECTIONS, ENDS, WITHIN 24" OF CONCENTRATED POINTS OF BEARING OR JAMBS, OR OVER OPENINGS UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DRAWINGS. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL BE GROUTED SOLID. LAP SPLICES SHALL BE AS FOLLOWS: #3 #4 #5 #6 #7 #8 #9 27 36 45 54 63 72 81 BAR LAPS (inches) SOLID BRICK MASONRY UNITS SHALL CONFORM TO ASTM C216, GRADE MW, TYPE FBS, F'm - 1,500 PSI,

HOLLOW BRICK MASONRY SHALL CONFORM TO ASTM C652, GRADE MW, TYPE HBS, F'm 2,300 PSI, RUNNING BOND, MORTAR TYPE S. 1.800 PSI, GROUT 3.000 PSI, MECHANICALLY VIBRATE GROUT IN VERTICAL SPACES

IMMEDIATELY AFTER POURING AND AGAIN ABOUT 5 MINUTES LATER. PROVIDE CLEANOUTS IF GROUT LIFT

EXCEEDS 4'-0" IN BLOCK WALLS. MAXIMUM GROUT LIFT SHALL BE 8'-0". UNLESS NOTED OTHERWISE ON

HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, NORMAL WEIGHT, RUNNING BOND, MORTAR

GROUT (PSI)

2000

2500

3200

3800

LOCATION

TYPICAL U.N.O.

WHERE NOTED

WHERE NOTED

WHERE NOTED

TYPE S (1800 PSI), COMPRESSIVE STRENGTHS AS FOLLOWS:

UNIT STRENGTH, NET (PSI)

2800

3750

RUNNING BOND, MORTAR TYPE S, 1,800 PSI. GROUT 2,000 PSI. LAY UP TWO-WYTHE WALL WITH FULL HEAD AND BED MORTAR JOINTS. ALL LONGITUDINAL VERTICAL JOINTS SHALL BE GROUTED SOLID. ONE TIER MAY BE CARRIED UP 16" BEFORE GROUTING, BUT THE OTHER TIER SHALL BE LAID UP AND GROUTED IN LIFTS NOT TO EXCEED SIX TIMES THE WIDTH OF THE GROUT SPACE OR 8" MAXIMUM. AT THE CONTRACTORS OPTION. TWO-WYTHE WALLS MAY BE GROUTED IN FOUR-FOOT LIFTS PROVIDING IT MEETS ALL THE REQUIREMENTS OF I.C.B.O. REPORT 3038. ROD GROUT IN VERTICAL SPACES IMMEDIATELY AFTER POURING AND AGAIN ABOUT 5 MINUTES LATER. UNLESS NOTED OTHERWISE ON THE PLANS, PLACE CONTROL JOINTS IN MASONRY WALLS SUCH THAT NO STRAIGHT RUN OF WALL EXCEEDS 24'-0". CONTROL JOINTS SHALL NOT OCCUR AT WALL CORNERS, INTERSECTIONS, ENDS, WITHIN 24" OF CONCENTRATED POINTS OF BEARING OR JAMBS. OR OVER OPENINGS UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DRAWINGS. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL BE GROUTED SOLID.

GLASS BLOCK MASONRY UNITS SHALL CONFORM TO ASTM C240 PART 18. MORTAR SHALL BE TYPE S. 1.800 PSI CONFORMING TO ASTM C270. HORIZONTAL REINFORCING SHALL CONSIST OF TWO PARALLEL #9 GAGE WIRES 1 5/8" TO 2" O.C. WITH CROSS WIRES AT REGULAR INTERVALS. LOCATE REINFORCING AT 16" O.C. MAXIMUM. PANEL ANCHORS SHALL BE #20 GAGE GALVANIZED PERFORATED STEEL STRIPS 24" LONG X 1 3/4" WIDE LOCATED AT 24" O.C. AT ALL JAMBS AND INTERMEDIATE SUPPORTS. 1/2" THICK EXPANSION MATERIAL SHALL BE PROVIDED AT SIDES AND TOP.

#### VERTICAL REINFORCING:

1 #5 IN CENTER OF GROUT AT CENTER OF WALL, CONTINUOUS FULL HEIGHT OF WALL AT ALL CORNERS, INTERSECTIONS, WALL ENDS, BEAM BEARINGS, JAMBS, EACH SIDE OF CONTROL JOINTS AND AT INTERVALS NOT TO EXCEED 48" O.C. UNLESS NOTED OTHERWISE. TIE AT 8'-0" VERTICALLY, WITH SINGLE WIRE LOOP TIE BY A.A. WIRE PRODUCTS COMPANY. LAP SPLICES SHALL BE AS PER SCHEDULE UNDER "MASONRY" SECTION. DOWEL ALL VERTICAL REINFORCING TO FOUNDATION WITH DOWELS TO MATCH VERTICAL REINFORCING.

#### HORIZONTAL REINFORCING:

2 #5 IN MINIMUM 8" DEEP GROUTED CONTINUOUS BOND BEAM AT (ELEVATED FLOORS AND) ROOFLINE. 1 \_\_\_ IN MINIMUM 8" DEEP GROUTED CONTINUOUS BOND BEAM AT TOP OF PARAPET OR TOP OF A FREESTANDING WALL. USE 1 #4 HORIZONTAL BOND BEAM AT 48" O.C. VERTICALLY SPACED FOR ALL WALLS WITHOUT RUNNING BOND (STACKED BOND), PLACE THESE BARS CONTINUOUS THRU CONTROL JOINTS PER TYPICAL DETAIL. PROVIDE BENT BARS PER TYPICAL DETAILS, TO MATCH HORIZONTAL BOND BEAM REINFORCING, AT CORNERS AND WALL INTERSECTION TO MAINTAIN BOND BEAM CONTINUITY. LAP SPLICES SHALL BE AS PER SCHEDULE UNDER "MASONRY" SECTION. DO NOT SPLICE WITHIN 8'-0" OF CONTROL JOINTS. USE STANDARD WEIGHT (NO. 9 GAGE WIRE) DUR-O-WAL OR DUR-O-WIRE (OR EQUIVALENT) LADDER TYPE JOINT REINFORCEMENT AT 16" O.C. IN MASONRY WALLS.

#### DOUBLE ANGLE LINTELS:

UNLESS NOTED OTHERWISE OR SHOWN, PROVIDE THE FOLLOWING LINTELS IN 8" NON-BEARING MASONRY WALLS. USE THESE LINTELS FOR OPENINGS REQUIRED BY OTHER DISCIPLINES (MECHANICAL, ELECTRICAL, PLUMBING, ETC.). PROVIDE MINIMUM 5" BEARING OF ANGLES ON JAMBS. FOR BEARING WALLS SEE SKETCH WHERE THESE ANGLES MAY BE USED. (NOTE: WHERE THE REQUIREMENTS OF THIS SKETCH ARE NOT POSSIBLE, NOTIFY THE STRUCTURAL ENGINEER PRIOR TO START OF MASONRY CONSTRUCTION).

		LINE OF BEARING
OPENING WIDTH	LINTEL ANGLES	OPENING WIDTH OF
0' 3'-4" 3'-5" - 4'-8"	2 - 3 1/2" X 2 1/2" X 1/4" (SLV) 2 - 3 1/2 X 3" X 1/4" (SLV)	GREATER
4'-9" - 6'-0" 6'-1" OR LARGER	2 – 3 1/2" X 3 1/2" X 1/4" ( NOTIFY STRUCTURAL ENGINEER	OPENING WIDTH

THESE LINTELS, OR THE OPENING THEY SPAN, SHALL NOT BE PLACED SO AS TO INTERFERE WITH THE REQUIREMENTS OF OTHER STRUCTURAL ELEMENTS (I.E. BOND BEAMS, LINTELS, CONTROL JOINTS, CONCENTRATED POINTS OF BEARING, ETC.) WITHOUT THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.

SOLID GROUT SHALL BE PROVIDED BETWEEN WEBS AND MASONRY FACE SHELLS FOR FULL LENGTH OF ALL STEEL LINTELS. MORTAR MAY BE USED FOR GROUT FOR THIS PURPOSE ONLY. FACE UNITS, SOAPS, ROMANS, ETC., SHALL BE LAID WITH FULL HEAD AND BED JOINTS.

FOR ADDITIONAL INFORMATION AT OPENINGS IN MASONRY WALLS, SEE TYPICAL DETAILS.

#### **REINFORCING:**

ASTM A615 (Fy = 60 KSI) DEFORMED BARS FOR ALL BARS. ALL GRADE 60 REINFORCING TO BE WELDED SHALL BE ASTM A706. WELDED WIRE FABRIC PER ASTM A185, WIRE PER ASTM A82. NO TACK WELDING OF REINFORCING BARS ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE WITH THE STRUCTURAL ENGINEER. LATEST ACI CODE AND DETAILING MANUAL APPLY. CLEAR CONCRETE COVERAGES AS FOLLOWS:

CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH ---- 3" EXPOSED TO EARTH OR WEATHER #6 OR LARGER ----- 2" #5 AND SMALLER ----- 1 1/2" COLUMNS (TO TIES) ----- 1 1/2" BEAMS (TO STIRRUPS) ----- 1 1/2" FLAT SLAB ----- 3/4" ALL OTHER PER LATEST EDITION OF ACI 318.

#### LAP SPLICES IN CONCRETE:

LAP SPLICES, UNLESS NOTED OTHERWISE, SHALL BE CLASS "B" TENSION LAP SPLICES PER LATEST EDITION OF ACI 318. LAP SPLICES IN CONCRETE COLUMNS SHALL BE STANDARD COMPRESSION LAP SPLICES FOR SHEARWALL BUILDINGS AND TENSION LAP SPLICES FOR MOMENT FRAME LATERAL SYSTEMS. SEE GSN SEISMIC SECTION FOR LATERAL SYSTEM TYPE. STAGGER SPLICES A MINIMUM OF ONE LAP LENGTH. LAPS IN WELDED WIRE FABRIC SHALL BE MADE SO THAT THE OVERLAP, MEASURED BETWEEN OUTERMOST CROSS WIRES OF EACH FABRIC SHEET, IS NOT LESS THAN THE SPACING OF CROSS WIRES PLUS 2 INCHES. ALL WELDED WIRE FABRIC SHALL BE CHAIRED TO ENSURE PROPER CLEARANCES.

Revisions

Appr.

PROGRESS PRINT CONSTRUCTION



GENERAL STRUCTURAL

Uptown Sedona Parking Facility

SHEET ID S001

#### PERFORMANCE SPECIFICATION FOR ALTERNATIVE STAIR DESIGN:

STAIRS SHALL BE DESIGNED FOR SELF WEIGHT PLUS A LIVE LOAD EQUAL TO 100 PSF. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS WITH DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER FOR REVIEW PRIOR TO MANUFACTURE.

#### STEEL STAIRS:

REFER TO PROJECT SPECIFICATIONS. ARCHITECTURAL DRAWINGS AND STRUCTURAL DRAWINGS TO DETERMINE EXTENT OF STAIRS AND WHAT SHALL BE PROVIDED BY THE STAIR MANUFACTURER. STAIRS ARE TO FRAME INTO FLOOR BEAMS AND BUILDING COLUMNS ONLY. DO NOT CONNECT STAIRS TO ELEVATOR GUIDE RAIL SUPPORTS OR DIAGONAL BRACING. VERIFY ALL OTHER CONDITIONS WITH ARCHITECT PRIOR TO PROCEEDING. IF STAIR FRAMING RESULTS IN ECCENTRIC LOADING OF THE STRUCTURAL MEMBERS, STAIR MANUFACTURER SHALL PROVIDE BRACING OF STRUCTURAL MEMBERS. CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION.

LANDING PANS SHALL BE 12 GAGE MINIMUM. TREAD PANS SHALL BE 14 GAGE MINIMUM. CONCRETE FILL FOR TREADS UNDER 4'-0" IN LENGTH SHALL BE REINFORCED WITH 6 X 6 - W1.4 X W1.4 W.W.F CONCRETE FILL FOR TREADES GREATER THAN 4'-0" IN LENGTHS SHALL BE REINFORCED WITH #3 AT 12" o.c. FOR ACTUAL LANDING AND STAIR PAN CONFIGURATIONS, SEE ARCHITECTURAL DRAWINGS.

#### **ELEVATED SLAB POST TENSIONING:**

#### AT TRANSFER OF POST-TENSIONING. CONCRETE STRENGTH SHALL BE 3.000 PSI MINIMUM.

THE INSTALLATION AND OPERATION OF ALL POST-TENSIONING SHALL BE IN ACCORDANCE WITH THE LATEST VERSION OF THE INTERNATIONAL BUILDING CODE, ACI 318 LATEST EDITION, AND POST-TENSIONING MANUAL

#### POST-TENSIONING REINFORCING SHALL CONFORM TO THE FOLLOWING:

POST-TENSIONING TENDON MATERIAL	LOW RELAXATION
NOMINAL DIAMETER	1/2"
CROSS SECTION AREA	0.153 SQ.IN.
MODULUS OF ELASTICITY	
ASTM DESIGNATION	
ULTIMATE STRENGTH	
TEMPORARY STRESS TO OVERCOME FRICTION	216 KSI (.8 ULT.)
ANCHOR STRESS	
EFFECTIVE STRESS	
ELONGATION	0825 IN./FT.
CONCRETE COVER	3/4" MIN. TO STRAND
CONCRETE COVER	2 1/4" MIN. TO WEDGES

POST-TENSIONING DESIGN WAS PREFORMED USING LOW RELAXATION STRAND. SUPPLIER MAY SUBSTITUTE WITH STRESS RELIEVED STRAND PROVIDED THEY PREFORM AND SUBMIT THE NECESSARY CALCULATIONS.

SUPPLIER SHALL SUBMIT CALCULATIONS FOR ALL LOSSES FOR SPECIFIED STRESSING LENGTHS TO ENSURE MINIMUM FINAL EFFECTIVE FORCE IS MAINTAINED.

#### TENDON QUANTITY (SEE ENTIRE PARAGRAPH ON TYPICAL DETAIL).

THE SUPPLIER SHALL BASE ALL ELONGATION CALCULATIONS UPON THE MODULUS OF ELASTICITY SHOWN ON THE MILL CERTICFICATES FOR THE TENDONS BEING FURNISHED TO THE SITE. ALL TENDONS SHALL HAVE THEIR HEAT NUMBER MARKED ON THE TAG ATTACHED TO THE TENDON.

ONE (1) SAMPLE OF EACH REEL SHALL BE TESTED BY AN APPROVED LABORATORY. TEST RESULTS SHALL BE SUBMITTED TO THE ARCHITECT, STRUCTURAL ENGINEER AND BUILDING DEPARTMENT BEFORE STRESSING.

SUPPLIER SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR ELONGATIONS AND FRICTION LOSSES AND SHALL SHOW FINAL TENDON LAYOUTS AND DRAPES FOR ALL TENDONS.

ANCHORAGE HARDWARE SHALL BE IN ACCORDANCE WITH ICC-ES EVALUATION REPORT, ESR-2515 OR ESR-2381 NOTE: ANCHOR CASTING, GTI SC1 AND HSM ARE NOT PERMITTED. ANCHORAGES EXPOSED TO WEATHER, OR UTILIZED IN EXTERIOR APPLICATIONS, SHALL USE AN ENCAPSULATED SYSTEM.

DRAPES SHALL CONFORM TO CONTROLLING POINTS SHOWN ON DRAWINGS AND SHOULD BE IN AN APPROXIMATELY PARABOLIC DRAPE BETWEEN SUPPORTS. DIMENSIONS LOCATE THE CENTER OF GRAVITY OF THE TENDON OR GROUP OF TENDONS. LOW POINTS ARE AT MIDSPAN, UNLESS NOTED OTHERWISE.

TENDONS SHALL BE SECURED TO A SUFFICIENT NUMBER OF POSITIONING DEVICES TO ENSURE CORRECT LOCATION DURING PLACEMENT OF CONCRETE, AND SHALL BE SPACED AT NOT MORE THAN 4'-O" O.C. ALL CHAIRS TO BE PROPERLY SECURED TO #4 SUPPORT BARS. LAP BARS 1'-6" MIN.

#### PLACE 2- #5 CONTINUOUS BARS EDGE OF SLAB ANCHORS.

ALL POCKETS REQUIRED FOR ANCHORAGE SHALL BE REINFORCED SO AS NOT TO DECREASE THE STRENGTH OF THE STRUCTURE. ALL POCKETS SHALL BE WATERPROOF SO AS TO ELIMINATE WATER LEAKAGE THRU THE POCKET. ALL DAMAGE TO MASTIC SHEATHING AROUND TENDONS SHALL BE REPAIRED.

TWISTING OR ENTWINING OF INDIVIDUAL WIRES OR STRANDS WITHIN A BUNDLE OR A BEAM SHALL NOT BE PERMITTED.

GROUT OR CONCRETE CONTAINING CHLORIDES SHALL NOT BE USED IN THE VICINITY OF THE TENDONS OR

#### CONTINUOUS INSPECTION IS REQUIRED FOR ALL PRESTRESS WORK.

RECORDS OF ALL JACKING FORCES AND ELONGATIONS SHALL BE KEPT BY A CERTIFIED PRESTRESS INSPECTOR AND SHALL BE PROMPTLY SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER.

WHILE STRESSING, FIELD TESTS SHALL BE MADE WITH JACKS OR OTHER INSTRUMENTS ON TENDONS TO DETERMINE BEHAVIOR OF THE TENDONS. FIELD READINGS OF ELONGATIONS AND/OR STRESSING FORCES SHALL NOT VARY MORE THAN 5% FROM CALCULATIONS REQUIRED VALUES.

ALL INSERTS AND SLEEVES SHALL BE CAST-IN-PLACE WHENEVER FEASIBLE. DRILLED OR POWER DRIVEN FASTENERS WILL BE PERMITTED WHEN IT CAN BE SHOWN THAT THE INSERTS WILL NOT SPALL THE CONCRETE AND LOCATED SO AS TO AVOID THE TENDONS AND ANCHORAGES.

#### STRESSING SEQUENCE:

#### TWO-WAY SLAB (BANDED)

1. STRESS CONTINUOUS DISTRIBUTED (UNIFORM) TENDONS. 2. STRESS CONTINUOUS BANDED TENDONS.

Revisions

- 3. STRESS ADDED DISTRIBUTED (UNIFORM) TENDONS.
- 4. STRESS ADDED BANDED TENDONS. 5. STRESS TEMPERATURE TENDONS (IF APPLICABLE).

#### NOTICE OF EXTENDED PAYMENT PROVISION

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS.WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT

Symbol



Appr.



JAppleyard

EHales

Designed by:

Checked by:

Drawn by:

7434 E. McDonald Drive Scottsdale, Arizona 85250 Phone: (480) 922-8854 Fax: (480) 922-3739 Email: cadd@pkastructural.com Website: www.pkastructural.com

Date:

PK Associates, L.L.C.

ONE-WAY SLAB AND BEAM

STRESSING SEQUENCE (CONT.):

- 1. STRESS TEMPERATURE TENDONS. 2. STRESS CONTINUOUS DISTRIBUTED (UNIFORM) SLAB TENDONS. 3. STRESS ADDED DISTRIBUTED (UNIFORM) SLAB TENDONS.
- 4. STRESS CONTINUOUS BEAM TENDONS. 5. STRESS ADDED BEAM TENDONS.

SLABS OR BEAMS MAY BE DE-SHORED WHEN ALL TENDONS HAVE BEEN STRESSED EXCEPT WHEN SHORING IS REQUIRED TO CARRY FLOORS ABOVE OR WHERE SLAB REQUIRES FUTURE CONTINUITY, OR VERTICAL SUPPORT OR WHERE NOTED ON PLANS. CONTINUOUS SHORING SHALL BE PROVIDED IN BAYS WITH CLOSURE STRIPS. RESHORING SHALL BE DESIGNED BY CONTRACTOR TO SUPPORT FULL LOAD AND SHALL EXTEND TO FOUNDATION SLAB UNLESS OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER.

THE CONTRACTOR RESPONSIBLE FOR THE PLACEMENT OF ALL POST-TENSIONING SHALL HAVE A MINIMUM EXPERIENCE LEVEL OF 3 YEARS OR 5 PROJECTS FOR THIS TYPE OF CONSTRUCTION.

CALCULATIONS REQUIRED BY THIS SECTION SHALL BE SEALED BY A REGISTERED ENGINEER AND SUBMITTED TO THE PROJECT STRUCTURAL ENGINEER FOR REVIEW PRIOR TO PROCEEDING WITH THE WORK.

#### POST-TENSIONING REINFORCING SHALL CONFORM TO THE FOLLOWING:

POST-TENSIONING TENDON MATERIAL	STRESS RELIEVED LOW	RELAXATION
ASTM DESIGNATION	A416	- A416
ULTIMATE STRENGTH	270 KSI	270 KSI
TEMPORARY STRESS TO OVERCOME FRICTION	216 KSI	- 216 KSI
ANCHOR STRESS	189 KSI	- 189 KSI
EFFECTIVE STRESS		
ELONGATION	0825 IN/FT	0825 IN/FT.
CONCRETE COVER	- 3/4" TO STRAND 3	/4" TO STRAND
CONCRETE COVER	- 2 1/4" TO WEDGES 2	1/4" TO WEDGES

POST-TENSIONING DESIGN WAS PERFORMED USING LOW RELAXATION STRAND. SUPPLIER MAY SUBSTITUTE WITH STRESS RELIEVED STRAND PROVIDED THEY PERFORM AND SUBMIT THE NECESSARY CALCULATIONS.

SUPPLIER SHALL SUBMIT CALCULATIONS FOR ALL LOSSES FOR SPECIFIED STRESSING LENGTHS TO ENSURE MINIMUM FINAL EFFECTIVE FORCE IS MAINTAINED.

THE SUPPLIER SHALL BASE ALL ELONGATION CALCULATIONS UPON THE MODULES OF ELASTICITY SHOWN ON THE MILL CERTIFICATES FOR THE TENDONS BEING FURNISHED TO THE SITE. ALL TENDONS SHALL HAVE THEIR HEAT NUMBER MARKED ON THE TAG ATTACHED TO THE TENDON.

ONE (1) SAMPLE OF EACH REEL SHALL BE TESTED BY AN APPROVED LABORATORY. TEST RESULTS SHALL BE SUBMITTED TO THE ARCHITECT, STRUCTURAL ENGINEER AND BUILDING DEPARTMENT BEFORE STRESSING.

SUPPLIER SHALL SUBMIT SHOP DRAWINGS AS PER SPECIFICATION.

ANCHORAGE HARDWARE SHALL BE IN ACCORDANCE WITH I.C.C. ESR-2515 AS SUPPLIED BY GENERAL TECHNOLOGIES, INC. OR I.C.C. ESR-2381 AS SUPPLIED BY PRECISION-HAYES INTERNATIONAL OR OTHER MANUFACTURER WITH CURRENT AND EQUIVALENT I.C.C. APPROVAL. ANCHORAGES EXPOSED TO WEATHER, OR UTILIZED IN EXTERIOR APPLICATIONS, SHALL USE AN ENCAPSULATED SYSTEM.

DRAPES SHALL CONFORM TO CONTROLLING POINTS SHOWN ON DRAWINGS AND SHOULD BE IN AN APPROXIMATELY PARABOLIC DRAPE BETWEEN SUPPORTS. DIMENSIONS LOCATE THE CENTER OF GRAVITY OF THE TENDON OR GROUP OF TENDONS. LOW POINTS ARE AT MIDSPAN, UNLESS NOTED OTHERWISE.

TENDONS SHALL BE SECURED TO A SUFFICIENT NUMBER OF POSITIONING DEVICES TO ENSURE CORRECT LOCATION DURING PLACEMENT OF CONCRETE, AND SHALL BE SPACED AT NOT MORE THAN 4'-0" O.C. ALL CHAIRS TO BE STAPLED UNLESS NOTED OTHERWISE BY ARCHITECT.

ALL #4 SUPPORT BARS SHALL BE LAPPED 1'-6".

PLACE 2 #4 CONTINUOUS BARS EDGE OF SLAB ANCHORS.

ALL POCKETS REQUIRED FOR ANCHORAGE SHALL BE REINFORCED SO AS NOT TO DECREASE THE STRENGTH OF THE STRUCTURE. ALL POCKETS SHALL BE WATERPROOF SO AS TO ELIMINATE WATER LEAKAGE THRU THE POCKET. ALL DAMAGE TO MASTIC SHEATHING AROUND TENDONS SHALL BE REPAIRED.

TWISTING OR ENTWINING OF INDIVIDUAL WIRES OR STRANDS WITHIN A BUNDLE OR A BEAM SHALL NOT BE PERMITTED.

GROUT OR CONCRETE CONTAINING CHLORIDES SHALL NOT BE USED IN THE VICINITY OF THE TENDONS OR ANCHORS.

CONTINUOUS INSPECTION IS REQUIRED FOR ALL PRESTRESS WORK.

RECORDS OF ALL JACKING FORCES AND ELONGATIONS SHALL BE KEPT BY A CERTIFIED PRESTRESS INSPECTOR AND SHALL BE PROMPTLY SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER.

WHILE STRESSING, FIELD TESTS SHALL BE MADE WITH JACKS OR OTHER INSTRUMENTS ON TENDONS TO DETERMINE BEHAVIOR OF THE TENDONS. FIELD READINGS OF ELONGATIONS AND/OR STRESSING FORCES SHALL NOT VARY MORE THAN 5% FROM CALCULATED REQUIRED VALUES.

ALL INSERTS AND SLEEVES SHALL BE CAST-IN-PLACE WHENEVER FEASIBLE. DRILLED OR POWDER DRIVEN FASTENERS WILL BE PERMITTED WHEN IT CAN BE SHOWN THAT THE INSERTS WILL NOT SPALL THE CONCRETE AND LOCATED SO AS TO AVOID THE TENDONS AND ANCHORAGES.

THE CONTRACTOR RESPONSIBLE FOR THE PLACEMENT OF ALL POST-TENSIONING SHALL HAVE A MINIMUM EXPERIENCE LEVEL OF 3 YEARS OR 5 PROJECTS FOR THIS TYPE OF CONSTRUCTION.

CALCULATIONS REQUIRED BY THIS SECTION SHALL BE SEALED BY A REGISTERED ENGINEER AND SUBMITTED

#### TO THE PROJECT STRUCTURAL ENGINEER FOR REVIEW PRIOR TO PROCEEDING WITH THE WORK. CONSTRUCTION JOINTS:

#### ALL CONSTRUCTION JOINTS IN WALLS SHALL BE KEYED IN ACCORDANCE WITH THE TYPICAL CONSTRUCTION JOINT DETAILS SHOWN ON THE STRUCTURAL DRAWINGS OR. AT THE CONTRACTOR'S OPTION. SHALL BE INTENTIONALLY ROUGHENED IN ACCORDANCE WITH THE FOLLOWING: THE SURFACE OF ROUGHENED JOINTS SHALL BE SAND BLASTED OR ROUGHENED WITH A CHIPPING HAMMER TO EXPOSE THE AGGREGATE EMBEDDED IN THE PREVIOUS POUR. THE EXPOSED AGGREGATE SHALL PROTRUDE A MINIMUM OF 1/4 INCH. ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED AND LAITANCE REMOVED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, ALL CONSTRUCTION JOINTS SHALL BE WETTED AND STANDING WATER REMOVED. VERTICAL CONSTRUCTION JOINTS IN WALLS SHALL BE HELD TO A MAXIMUM SPACING OF 30'-0". ALL CONSTRUCTION JOINTS IN SLABS, JOISTS, BEAMS, AND GIRDERS SHALL BE OFFSET A DISTANCE EQUAL TO TWICE THE

ACCORDANCE WITH THE TYPICAL SLAB ON DECK CONSTRUCTION JOINT DETAIL SHOWN ON THE STRUCTURAL DRAWINGS. BEAMS AND GIRDERS HAVE BEEN DESIGNED ASSUMING THE CONSTRUCTION JOINTS TO BE LOCATED IN THE MIDDLE THIRD OF THE BEAM, GIRDER, OR SLAB SPAN. ALL CONSTRUCTION, CONTROL, AND ISOLATION JOINTS FOR SLABS ON GRADE SHALL BE IN ACCORDANCE WITH THE TYPICAL SLAB ON GRADE DETAILS. THE CONTRACTOR SHALL SUBMIT THE PROPOSED LOCATIONS OF CONSTRUCTION JOINTS TO THE ENGINEER FOR ACCEPTANCE BEFORE STARTING CONSTRUCTION.

APPLY UNLESS NOTED OTHERWISE FORM CAMBER FOR CONCRETE SLABS:

> CONCRETE FORMWORK SHALL HAVE BUILT-IN CAMBER TO COMPENSATE FOR FORM SAG UNDER WET CONCRETE LOAD, IN ADDITION TO STRUCTURAL CAMBER NOTED. ONE-WAY POST-TENSIONED SLABS SHALL BE CAMBERED 1/1000 OF THE SPAN DISTANCE, ONE-WAY MILD REINFORCED SLABS SHALL BE CAMBERED 1/500 OF THE SPAN DISTANCE, TWO-WAY POST-TENSIONED SLABS SHALL BE CAMBERED 1/1000 OF THE ACROSS DIAGONAL SPAN DISTANCE, TWO-WAY MILD REINFORCED SLABS SHALL BE CAMBERED 1/500 OF THE ACROSS DIAGONAL SPAN DISTANCE, UNLESS NOTED OTHERWISE ON THE DRAWINGS. BEAM AND JOIST STRUCTURAL CAMBERS ARE NOTED ON THE DRAWINGS. CAMBERS OF LESS THAN 1/8 INCH MAY BE NEGLECTED.

CONTRACTOR IS REQUIRED TO PROVIDE SLABS WITH FLATNESS PROPERTIES IN CONFORMANCE WITH SPECIFICATIONS AS PART OF THE BASE BID. WHERE SLAB FLATNESS DOES NOT MEET SPECIFICATIONS, CONTRACTOR SHALL LEVEL THE SLAB USING AN APPROVED LEVELING AGENT.

#### ELECTRICAL CONDUIT IN CONCRETE SLABS:

ELECTRICAL CONDUIT SHALL BE RIGID STEEL CONDUIT OR FLEXIBLE PLASTIC CONDUIT. ALUMINUM CONDUIT IS PROHIBITED. CONDUIT WITH A MAXIMUM OUTSIDE DIAMETER OF 1/6 TIMES THE SLAB THICKNESS MAY BE EMBEDDED IN ONE LAYER AT THE MID-DEPTH OF SLABS. MINIMUM CLEAR DISTANCE BETWEEN CONDUITS SHALL BE 3 TIMES CONDUIT DIAMETER. CONDUIT SHALL BE FIRMLY CHAIRED AND TIED TO PREVENT DISPLACEMENT DURING POURING. PLACE #3 AT 12 INCHES O.C. ADDITIONAL REINFORCING ABOVE AND BELOW. PERPENDICULAR TO THE CONDUIT. THE ADDED REINFORCING SHALL EXTEND 1'-0" PAST THE CONDUIT ON BOTH SIDES. FOR CONDUIT PLACED IN SLABS ON METAL DECKING, CONDUIT SHALL RUN IN THE METAL DECK FLUTES PER THE TYPICAL CONDUIT IN SLAB ON METAL DECK DETAIL.

#### **CONCRETE INSERTS:**

THIN SLAB TYPE INSERTS SHALL BE GALVANIZED AND HAVE THE FOLLOWING MINIMUM WORKING LOADS(A 4:1 RATIO OF ULTIMATE TO WORKING VALUES IS ASSUMED):

DOLT DIAMETER	WORKING LOAD			
BOLT DIAMETER	SHEAR	TENSION		
1/2" 5/8" 3/4"	1,000 LBS. 1,250 LBS. 1,600 LBS.	650 LBS. 700 LBS. 850 LBS.		

COIL LOOP INSERTS SHALL BE GALVANIZED AND HAVE THE FOLLOWING MINIMUM WORKING LOADS(A 3:1 RATIO OF ULTIMATE TO WORKING VALUES IS ASSUMED):

DOLT DIAMETED	WORKING LOAD			
BOLT DIAMETER	SHEAR	TENSIO		
1/2"	2,200 LBS.	2,820 LB		
5/8"	3,000 LBS.	3,620 LE		
3/4"	3,100 LBS.	3,660 LB		

THREADED COIL RODS, COIL NUTS, ETC., USED IN CONJUNCTION WITH CONCRETE INSERTS, SHALL HAVE A WORKING LOAD EQUAL TO OR GREATER THAN THE CORRESPONDING CONCRETE INSERT. CONTRACTOR SHALL SUBMIT MANUFACTURER'S SIZE AND STRENGTH DATA PRIOR TO CONSTRUCTION TO THE STRUCTURAL ENGINEER THRU THE ARCHITECT. VALUES LISTED ABOVE ARE FOR RICHMOND STRUCTURAL CONNECTION INSERTS.

#### EXPANSION AND SCREW ANCHORS:

USE STUD TYPE EXPANSION ANCHOR WITH A SINGLE PIECE WEDGE ONLY WHERE NOTED ON PLANS. IF USE IS REQUESTED FOR OTHER THAN WHERE NOTED CONTACT STRUCTURAL ENGINEER THROUGH ARCHITECT FOR APPROVAL.

CONTRACTOR SHALL SUBMIT MANUFACTURER'S SIZE AND STRENGTH DATA TO ENGINEER THROUGH ARCHITECT PRIOR TO CONSTRUCTION. INSTALL ALL BOLTS AS OUTLINED IN MANUFACTURER'S SPECIFICATIONS, UTILIZING PROPER SIZE AND TYPE OF DRILL, CLEANING HOLE, DRIVING AND TIGHTENING BOLT.

#### IN CONCRETE:

ANCHORS USED MUST HAVE ICC APPROVAL AND INCLUDE HILTI KWIK BOLT TZ (ESR-1917), AND SIMPSON STRONG BOLT-2 (ESR-3037), AND SIMPSON TITEN HD (ESR-2713), HILTI KWIK HUS-EZ (ESR-3027), OR APPROVED EQUAL.

#### IN MASONRY:

ANCHORS USED MUST HAVE ICC APPROVAL AND INCLUDE HILTI KWIK BOLT 3 (ESR-1385), AND SIMPSON STRONG BOLT-2 (ES-240), HILTI KWIK HUS-EZ (ESR-3056) AND SIMPSON TITEN HD (ESR-1056), OR APPROVED EQUAL.

#### EPOXY ANCHORS IN CONCRETE AND MASONRY:

INJECTABLE ADHESIVE SHALL BE USED FOR INSTALLATION OF REINFORCING STEEL DOWELS OR THREADED ANCHOR RODS AND INSERTS INTO NEW OR EXISTING CONCRETE OR SOLID GROUTED CONCRETE MASONRY UNITS ONLY WHERE SPECIFIED ON PLANS. IF USE IS REQUESTED FOR OTHER THAN WHERE NOTED CONTACT STRUCTURAL ENGINEER THROUGH ARCHITECT FOR APPROVAL. ADHESIVE SHALL BE FURNISHED IN SIDE BY SIDE PACKS WHICH KEEP COMPONENT A AND COMPONENT B SEPARATE. USE ONLY INJECTION TOOLS AND STATIC MIXING NOZZLES RECOMMENDED BY MANUFACTURER, MANUFACTURER'S INSTRUCTIONS SHALL BE FOLLOWED.

#### IN CONCRETE:

ANCHORS USED MUST HAVE I.C.C. APPROVAL IN CRACKED CONCRETE AND INCLUDE SIMPSON SET-XP (ESR-2508), HILTI HIT-RE500-V3 (ESR-3814), OR APPROVED EQUIVALENT. THE USE OF ANY EPOXY ANCHOR MUST BE APPROVED BY THE ENGINEER OF RECORD PRIOR TO INSTALLATION.

#### IN MASONRY:

ANCHORS USED MUST HAVE ICC APPROVAL AND INCLUDE SIMPSON SET (ESR-1772) AND HILTI HIT-HY 270 MAX (ESR-4143) OR APPROVED EQUIVALENT. THE USE OF ANY EPOXY ANCHOR MUST BE APPROVED BY THE ENGINEER OR RÉCORD PRIOR TO INSTALLATION.

#### STRUCTURAL FILL:

ALL FILL PLACED TO SUPPORT SLABS ON GRADE, BEHIND PERMANENT WALLS, AND AROUND ALL DRAINS SHALL CONSIST OF WELL GRADED, GRANULAR MATERIAL PER THE SPECIFICATIONS. SOILS FOR STRUCTURAL FILL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. STRUCTURAL FILL SHALL BE PLACED ON SOUND NATIVE MATERIAL. PROOF-ROLL CUT AREAS WHICH PROVIDE SUPPORT FOR PERMANENT STRUCTURES. AREAS WHICH ARE EXCESSIVELY YIELDING, AS DETERMINED BY THE CONTINUOUS OBSERVATION OF THE GEOTECHNICAL ENGINEER, SHALL BE OVEREXCAVATED AND REPLACED WITH STRUCTURAL FILL. STRUCTURAL FILL SHALL BE PLACED PER THE SPECIFICATION. USE LEAN CONCRETE FILL BELOW ALL PIPES EXISTING OUT OF BASEMENT WALL, FULL HEIGHT OF WALL. CONCRETE SHALL CONTAIN 2 SACKS OF CEMENT PER YARD.

#### LATERAL PRESSURE ON SUBGRADE WALLS:

THE DESIGN PRESSURES FOR SUBGRADE WALLS ARE BASED ON A "DRAINED" CONDITION. SEE CIVIL AND MECHANICAL DRAWINGS FOR SUBGRADE DRAINAGE SYSTEM. SEE GEOTECHNICAL REPORT FOR COMPACTION REQUIREMENTS AT SUBGRADE WALLS. SUBGRADE WALLS AND SUPPORTING SLABS SHALL HAVE ATTAINED THEIR FULL CONCRETE STRENGTH BEFORE PLACING ANY BACKFILL U.N.O. THE CONTRACTOR SHALL PROVIDE TEMPORARY BRACES FOR WALLS IF BACKFILL IS PLACED BEFORE WALLS AND SLABS ACHIEVE FULL CONCRETE

#### NOTES ON CRACKING OF CONCRETE STRUCTURES:

CRACKING IS INHERENT TO THE MATERIAL PROPERTIES OF CONCRETE CONSTRUCTION (INCLUDING POST-TENSIONED CONCRETE STRUCTURES). WHILE EVERY EFFORT HAS BEEN MADE TO MINIMIZE THE EFFECTS OF UNSIGHTLY CRACKING, THE PRESENCE OF CRACKS ARE NORMAL AND UNAVOIDABLE. THE DESIGN OF THE CONCRETE STRUCTURAL ITEMS HAVE BEEN ANALYZED USING A "CRACKING SECTION." THE PRESENCE OF THE CRACKING SHOULD NOT BE CONSIDERED DETRIMENTAL TO THE STRUCTURE. CRACKS LARGER THAN 10 MILS SHALL BE FILLED AND SEALED WITH AN APPROVED CRACK FILLER TO PREVENT FUTURE DETERIORATION. ALLOWANCE SHALL BE MADE IN THE CONSTRUCTION BUDGET FOR SEALING OF SUCH CRACKS. IN SOME CASE, CRACKS DO NOT APPEAR UNTIL WELL AFTER CONSTRUCTION HAS BEEN COMPLETED. IT IS THE RESPONSIBILTY OF THE OWNER TO MAINTAIN THE STRUCTURE PROPERLY OVER THE LIFE OF THE STRUCTURE. CONCRETE CRACKS, SHOULD THEY OCCUR, SHALL BE FILLED AND SEALED TO PREVENT PREMATURE DETERIORATION OF THE STRUCTURE.

#### SHOP DRAWINGS:

SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS IN ADDITION TO ITEMS REQUIRED BY ARCHITECTURAL SPECIFICATIONS.

THE CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMITTAL. ITEMS NOT IN ACCORDANCE WITH CONTRACT DOCUMENTS SHALL BE FLAGGED UPON HIS REVIEW.

VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND ALL FINISHED GRADE WITH CIVIL DRAWINGS.

ANY CHANGES, SUBSTITUTIONS, OR DEVIATIONS FROM CONTRACT DOCUMENTS SHALL BE CLOUDED BY MANUFACTURER OR FABRICATOR. ANY OF THE AFOREMENTIONED WHICH ARE NOT CLOUDED OR FLAGGED BY SUBMITTING PARTIES, SHALL NOT BE CONSIDERED APPROVED AFTER ENGINEER'S REVIEW, UNLESS NOTED

THE ENGINEER HAS THE RIGHT TO APPROVE OR DISAPPROVE ANY CHANGES TO CONTRACT DOCUMENTS AT ANYTIME BEFORE OR AFTER SHOP DRAWING REVIEW.

THE SHOP DRAWINGS DO NOT REPLACE THE CONTRACT DOCUMENTS. ITEMS OMITTED OR SHOWN INCORRECTLY AND ARE NOT FLAGGED BY THE STRUCTURAL ENGINEER OR ARCHITECT ARE NOT TO BE CONSIDERED CHANGES TO CONTRACT DOCUMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO MAKE SURE ITEMS ARE CONSTRUCTED PER THE CONTRACT DOCUMENTS.

THE ADEQUACY OF ENGINEERING DESIGNS AND LAYOUT PERFORMED BY OTHERS RESTS WITH THE DESIGNING OR SUBMITTING AUTHORITY.

REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP DRAWINGS.

#### DEFERRED SUBMITTALS: (PER 2018 IBC 107.3.4.1)

RESPONSIBILITY FOR CORRECTNESS SHALL REST WITH THE CONTRACTOR.

FOR THE PURPOSES OF THIS SECTION. DEFERRED SUBMITTALS ARE DEFINED AS THOSE PORTIONS OF THE DESIGN WHICH ARE NOT SUBMITTED AT THE TIME OF THE APPLICATION AND THAT ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITHIN A SPECIFIED PERIOD.

DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE PROFESSIONAL IN RESPONSIBLE CHARGE FOR REVIEW. THE CONTRACTOR SHALL FORWARD THE REVIEWED DOCUMENTS TO BUILDING OFFICAL WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

#### DEFERRED SUBMITTAL ITEMS:

STEEL STAIRS EXTERIOR CLADDING AND STEEL STUD FRAMING NON-BEARING INTERIOR STUD PARTITION WALLS EXCAVATION SHORING SYSTEMS GUARDRAILS/HANDRAILS

#### **GENERAL:**

ENTIRE CONTRACT DOCUMENTS SHALL BE USED TO CONSTRUCT BUILDING. SOME CRITICAL ITEMS REQUIRED BY OTHER DISCIPLINES MAY NOT BE SHOWN ON STRUCTURAL DRAWING (i.e. WALL, FLOOR AND ROOF OPENINGS, ARCHITECTURAL, MECHANICAL AND PLUMBING LOADS, SUPPORT PLATES ETC.)

ITEMS SHOWN BY OTHER DISCIPLINES WITH REFERENCE TO STRUCTURAL DRAWING BUT NOT SHOWN ON THESE STRUCTURAL DOCUMENT SHALL BE CONSIDERED DESIGN BUILD ITEMS. CONTRACTOR SHALL SUBMIT DESIGN BY OTHERS FOR REVIEW.

THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE. BUT NOT BE LIMITED TO. BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO (NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS).

CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED CONSTRUCTION. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT.

WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDA.

ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL WITH APPROPRIATE TRADES, DRAWINGS AND SUBCONTRACTORS PRIOR TO CONSTRUCTION.

OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. IF THE CONTRACTOR CHOOSES AN OPTION, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL NECESSARY CHANGES AND SHALL COORDINATE ALL DETAILS.

NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL

DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE ALL DIMENSIONS SHOWN (INCLUDING ELEVATIONS) ON STRUCTURAL DRAWINGS ARE TO ASSIST CONTRACTOR IN VERIFICATION. SCALING DIMENSIONS FROM DRAWINGS IS NOT PERMITTED. LOCATION OF ALL ITEMS SHALL

CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ARCHITECTURAL AND FINISHED GRADE WITH CIVIL DRAWINGS PRIOR TO START OF CONSTRUCTION. RESOLVE ANY DISCREPANCY

BE DETERMINED BY DIMENSIONS OR NOTES ONLY; DO NOT USE GRAPHIC APPEARANCE TO ASSUME SPECIFIC

WITH THE ARCHITECT. TYPICAL DETAILS MAY NOT NECESSARILY BE CUT ON PLANS, BUT APPLY UNLESS NOTED OTHERWISE.

WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.

ANY ENGINEERING DESIGN, PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF JURISDICTION.

SUPPLIER OF ENGINEERED STRUCTURAL COMPONENTS (i.e. STEEL JOISTS, STAIRS, PRECAST ITEMS) SHALL BE RESPONSIBLE FOR COMPLETE DESIGN AND SHALL USE ENTIRE CONTRACT DOCUMENTS TO INCLUDE ALL LOADS AND DETAIL REQUIREMENTS FROM ALL DISCIPLINES. SUPPLIER SHALL PROVIDE ADDITIONAL MATERIAL REQUIRED TO MEET ALL THEIR REQUIREMENTS FOR INSTALLATION (i.e. WIDER BEARING PLATES, SHIMS, ERECTION BOLTS ETC.).

STRUCTURAL STEEL SUPPLIER SHALL FURNISH BOLTS FOR OSHA CONNECTIONS (SEE DRAWINGS FOR DETAILS).

# SCHEMATIC DESIGN

Scale: **AS NOTED** Project Code GLA PROJECT NO. 20109

07/30/2021

WIDTH OF THE BEAM.

PROGRESS PRINT NOT FOR **CONSTRUCTION** 

gabor lorant architects

3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com





CITY OF SEDONA PUBLIC WORKS DEPARTMENT **102 ROADRUNNER DRIVE SEDONA, ARIZONA 86336** 

Uptown Sedona Parking Facility Schematic Design GENERAL STRUCTURAL

SHEET NO.

SHEET ID

S002

928-204-7111

NOTES

#### GENERAL STRUCTURAL NOTES

APPLY UNLESS NOTED OTHERWISE

#### GENERAL (CONT.):

WALL SHORING SHALL BE INSTALLED PRIOR TO BACKFILLING BEHIND ALL BUILDING RETAINING WALLS, UNLESS ALL RESTRAINING SLABS ARE INSTALLED. USE HANDTAPPING ONLY WHEN WITHIN 8'-0", OR WITHIN HALF THE WALL HEIGHT OF BACKFILLED WALL.

CONTINUOUS FOUNDATION DRAIN PIPES (FRENCH DRAINS) OR WEEP HOLES SHALL BE PROVIDED BEHIND ALL BASEMENT WALLS AND ALL EXTERIOR RETAINING WALLS THAT RETAIN MORE THAN 3'-0" OF SOIL. WEEP HOLES WHERE USED SHALL BE 2" IN DIAMETER AT 6'-0" O.C. MAXIMUM.

#### **BUILDING TOLERANCES:**

STANDARD TOLERANCES SHALL BE BASED ON THE REQUIREMENTS OF THE AISC CODE OF STANDARD PRACTICE AND ACI 117, STANDARD SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS.

#### SEQUENCING CONSTRUCTION AND LATERAL STABILITY:

THE STRUCTURAL COMPONENTS BY THEMSELVES ARE A NON-SELF-SUPPORTING STRUCTURE. LATERAL FORCES DUE TO WIND, EARTHQUAKE, OR SOIL ARE CARRIED BY THE ROOF AND FLOOR DIAPHRAGMS TO THE LATERAL SYSTEM. CERTAIN ELEMENTS SHOWN ON OR LOCAL STABILITY OF OTHER ELEMENTS (SUCH AS BEAMS, COLUMNS, AND WALLS). IF, DUE TO SEQUENCING OF CONSTRUCTION, THESE STABILITY ELEMENTS ARE NOT IN PLACE, THE CONTRACTOR SHALL RETAIN A LICENSED STRUCTURAL ENGINEER WHO SHALL INVESTIGATE WHERE TEMPORARY SHORING/BRACING IS REQUIRED, AND SHALL DESIGN THIS TEMPORARY SHORING/BRACING. THE CONTRACTOR SHALL PROVIDE THIS SHORING/BRACING UNTIL THE REQUIRED STRUCTURAL ELEMENTS AND THEIR CONNECTIONS HAVE BEEN INSTALLED AND REACH THEIR FINAL DESIGN STRENGTHS.

#### MISCELLANEOUS:

REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL, ELEVATOR, OR OTHER SPECIALTY ENGINEERING DRAWINGS FOR DIMENSIONS NOT SHOWN, INCLUDING BUT NOT LIMITED TO: SIZE AND LOCATION OF CURBS, EQUIPMENT HOUSEKEEPING PADS, WALL AND FLOOR OPENINGS, BLOCKOUTS, FLOOR DEPRESSIONS, SUMPS, DRAINS, ANCHOR BOLTS, EMBEDDED ITEMS, ARCHITECTURAL TREATMENT, ETC. CONTRACTOR SHALL VERIFY DIMENSIONS AND RESOLVE DISCREPANCIES OR CONFLICTS PRIOR TO CONSTRUCTION. WHERE SECTIONS ARE INDICATED ON THE PLAN BY A NUMBER AND A DRAWING NUMBER THUS, 1/S5.01, THE INDICATED SECTION (1) IS SHOWN ON STRUCTURAL DRAWING S5.01.

FLOOR FLATNESS/LEVELNESS SHALL MEET ARCHITECTURAL SPECIFICATIONS (1/4" IN 10 FOOT MINIMUM LEVELNESS UNLESS TIGHTER REQUIREMENT IN SPECIFICATIONS) IN HEIGHT FOR ALL STRUCTURAL SYSTEMS. CONTRACTOR SHALL INCLUDE COST FOR LEVELING ALL FLOORS. FOR ESTIMATING PURPOSES ONLY, ASSUME 1/2" THICK LEVELING AGENT OVER 15% OF FLOOR AREA.

#### SPECIAL INSPECTION:

SEE "SPECIAL INSPECTIONS TABLE" THIS SHEET (ON SHEET S003).

#### FABRICATOR APPROVAL:

SPECIAL INSPECTIONS NOTED ABOVE APPLY TO SHOP FABRICATED ASSEMBLIES IN ADDITION TO ON-SITE WORK UNLESS THE FABRICATOR IS AN "APPROVED FABRICATOR" AS DEFINED IN \_\_\_\_ IBC.

#### STRUCTURAL OBSERVATION:

ENGINEER OF RECORD SHALL PROVIDE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM, FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS, AT SIGNIFICANT CONSTRUCTION STAGES AND AT THE COMPLETION OF THE STRUCTURAL SYSTEM. CONTRACTOR IS REQUIRED FOR COORDINATING THE TIMING OF SITE VISITS AND SHALL INCLUDE A MINIMUM OF 3 VISITS. THESE VISITS ARE TO BE PAID BY THE CONTRACTOR. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED BY THE BUILDING CODE (NOTED ABOVE).

#### NOTE TO CONTRACTOR REGARDING PROGRESS CONSTRUCTION DRAWINGS:

THESE DRAWINGS HAVE BEEN PREPARED FOR PROGRESS REVIEW AND COORDINATION ONLY. AND ARE NOT INTENDED FOR FINAL PRICING. BID. OR GMP. ANY BUDGET PRICING BASED ON THESE PROGRESS DRAWINGS SHALL INCLUDE A DESIGN ALLOWANCE SUFFICIENT TO COVER THE COMPLETION OF THE STRUCTURAL DESIGN AND CONSTRUCTION DRAWINGS. ALL STRUCTURAL ITEMS SHOWN HEREIN ARE SUBJECT TO CHANGE AS THE DESIGN PROGRESSES. AND SHALL BE ALLOWED FOR WITHIN THE BUDGET. IN ADDITION, MISCELLANEOUS STEEL ITEMS MAY NOT BE SHOWN UNTIL FINAL CONSTRUCTION DOCUMENTS ARE ISSUED. THESE ITEMS INCLUDE, BUT ARE NOT LIMITED TO, ARCHITECTURAL ACCENTS AND CANOPIES, EDGE ANGLES, BRACING, CLADDING, CLADDING SUPPORT STEEL, EQUIPMENT SUPPORTS, BLOCKOUTS, ELEVATOR STEEL, STAIRS, EMBED PLATES, SPECIAL CONNECTIONS, ECT. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND OTHER SPECIALTY DRAWINGS FOR ADDITIONAL INFORMATION. IF ANY OF THE ABOVE IS NOT CLEAR, PLEASE CONTACT OUR OFFICE FOR FURTHER CLARIFICATION OF THE DESIGN INTENT.

#### Note to Contractor Regarding Pricing/Bidding of Progress Construction Drawings:

THESE DRAWINGS HAVE BEEN PREPARED FOR PROGRESS REVIEW AND COORDINATION, AND ARE NOT 100% CONSTRUCTION DOCUMENTS. IF THESE DOCUMENTS ARE TO BE USED FOR PRICING, BID, OR STEEL MILL ORDER, THE CONTRACTOR SHALL PROVIDE IN THE PROJECT BUDGET AN ALLOWANCE FOR THE COMPLETION OF THE STRUCTURAL DESIGN. THIS ALLOWANCE SHALL BE CARRIED UNTIL ALL SHELL BUILDING PERMITS ARE OBTAINED, AND FINAL CONSTRUCTION DOCUMENTS ARE ISSUED. AS THESE DRAWINGS ARE NOT ISSUED FOR CONSTRUCTION, CHANGES TO STRUCTURAL COMPONENTS SHOWN ARE POSSIBLE. ADDITIONALLY, MISCELLANEOUS ITEMS MAY NOT BE SHOWN ON THESE DRAWINGS. THESE ITEMS INCLUDE, BUT ARE NOT LIMITED TO, ARCHITECTURAL ACCENTS AND CANOPIES, EDGE ANGLES, BRACING, CLADDING SUPPORT STEEL, EQUIPMENT SUPPORTS, BLOCKOUTS, ETC. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND OTHER SPECIALTY DRAWINGS FOR ADDITIONAL INFORMATION. THE BUDGET ALLOWANCE SHALL INCLUDE THESE ITEMS. THE STRUCTURAL ENGINEER WILL NOT BE RESPONSIBLE FOR ANY CHANGE ORDER COSTS INCURRED (INCLUDING DISCARDED MATERIAL COSTS) DUE TO BIDDING OR STEEL MILL ORDER FROM PROGRESS DRAWINGS. CONTACT STRUCTURAL ENGINEER FOR CLARIFICATION IF THE SCOPE AND QUANTITY OF ALLOWANCE TO BE CARRIED IS

#### CONTRACTOR RESPONSIBILITY:

EACH CONTRACTOR OR SUB-CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF THE WIND AND/OR SEISMIC RESISTING SYSTEM THAT IS LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK REQUIRING SPECIAL INSPECTION. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS.

SPECIAL INSPECTION:					STEEL:			REFERENCED	IBC
PER IBC CHAPTER 17, SPECIAL INSPECTION IS REQUIRED	FOR THE FOL	LOWING ITEMS:			VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	STANDARD (NOTE1)	REFERENCE
CONCRETE: VERIFICATION AND INSPECTION	CONTINU	OUS PERIODIO	REFERRENCED STANDARD (NOTE 1)	IBC REFERENCE	Material verification of high—strength     bolts, nuts and washers:     a. Identification markings to conform to ASTM     standards specified in the approved			APPLICABLE ASTM MATERIAL SPECS	
Inspection of reinforcing steel, including prestressing tendons, and placement.	_	×	ACI 318: Ch. 20, 25.2 25.3, 26.6.1-26.6.3	1908.4	construction documents.  b. Manufacturer's certificate of compliance required.	_	X X	AISC 360, SEC. A3.3	
Reinforcing bar welding:     a. Verify weldability of reinforcing bars other than ASTM A706;     b. Inspect single—pass fillet welds, maximum	-	х	AWS D1.4 ACI 318: 26.6.4		2. Inspection of high-strength bolting: a. Bearing-type connections. AISC 360, Section M2.5 1704.3.3  3. Material verification of structural steel:	_	X		
5/16"; and c. Inspect all other welds.	X	X			a. Identification markings to conform to ASTM standards specified in the approved construction documents.	_	_	AISC 360, SEC. M2.5 ASTM A 6 OR	
3. Inspect anchors to be installed in concrete prior to and during placement of concrete where allowable loads have been increased or where strength design is used.	-	Х	ACI 318: 17.8.2		b. Manufacturers' certified mill test reports.  4. Material verification of weld filler materials: a. Identification markings to conform to AWS specification in the approved construction			ASTM A 568	
4. Inspection of anchors post—installed in hardened concrete members.  a. Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads.	X	_	ACI 318: 17.8.2.4		documents. b. Manufacturer's certificate of compliance required.  5. Inspection of welding: a. Structural steel:	- -	- - -	AISC 360, SEC. A3.5	
<ul> <li>b. Mechanical anchors and adhesive anchors not defined in 4.a.</li> </ul>	_	X	ACI 318: 17.8.2		Complete and partial penetration groove welds.     Wultipass fillet welds.	X		AWS D1.1  AWS D1.1	
5. Verifying use of required design mix.	_	X	ACI318: Ch. 19, 26.4.3, 26.4.4	1904.1, 1904.2 1908.2, 1908.3	3) Single-pass fillet welds > 5/16" 4) Single-pass fillet welds = 5/16"	x -	- X	AWS D1.1 AWS D1.1	
6. Prior to concrete placement, fabricate specimens for strength tests, per form slump and air content tests, and determine the temperature of the concrete.	x	-	ASTM C172 ASTM C31 ACI 318: 26.4, 26.12	1908.10	5) Floor and roof deck welds.  b. Reinforcing steel:  1) Verification of weld ability of reinforcing steel other than ASTM A 706.  2) Reinforcing steel—resisting flexural and Axial forces in intermediate and special	- -	x x	AWS D1.3  AWS D1.4 ACI 318: 3.5.2	
7. Inspection of concrete and shotcrete placement for proper application techniques.	X	-	ACI 318: 26.5	1908.6, 1908.7, 1908.8	Moment frames, and boundary elements of Special reinforced concrete shear walls and shear reinforcement. 3) Shear reinforcement. 4) Other reinforcing steel.	X X	- - x		
8. Verify maintenance of specified curing temperature and techniques.  9. Inspect prestressed concrete for:	_	X	ACI 318: 26.5.3-26.5.5	1908.9	6. Inspection of steel frame joint details for compliance with approved construction documents:	_	X		
a. Application of prestressing forces; and b. Grouting of bonded prestressing tendons	X		ACI 318: 26.10		a. Details such as bracing and stiffening. b. Member locations. c. Application of joint details at each connection.	<u>-</u> -			
10. Inspect erection of precast concrete members.	_	X	ACI 318: Ch. 26.8		NOTES:	_	_		
11. Verification of in—situ concrete strength, prior to stressing of tendons in post—tensioned concrete and prior to removal of shores and forms from beams and structural slab.	_	х	ACI 318: 26.11.2		WHERE APPLICABLE SEE ALSO SECTION 1701.1, SPECIAL I     TABLES TAKEN DIRECTLY FROM IBC FOR REFERENCE.  STEEL: CONSTRUCTION OTHER	INSPECTION FOR SEIS	MIC RESISTANCE.	REFERENCED	IBC
12. Inspect formwork for shape, location and dimensions of the concrete member being formed.	_	X	ACI 318: 26.11.1.2(b)		THAN STRUCTURAL STEEL: VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	STANDARD (NOTE1)	REFERENCE
NOTES:  1. WHERE APPLICABLE, SEE ALSO SECTION 1707.1, SPECI 2. TABLES TAKEN DIRECTLY FROM IBC FOR REFERENCE.	IAL INSPECTION	N FOR SEISMIC	RESISTANCE.		Material verification of cold—formed steel deck:     a. Identification markings to conform to ASTM standards specified in the approved construction documents.    Manufactures   Confident   Conf	<u>-</u>	x	APPLICABLE ASTM MATERIAL STANDARDS	
MASONRY INSPECTION TASK	FREQUENC		DE FOR CRITERIA  02 TMS 602	IBC REFERENCE	b. Manufacturer's certified test reports.  2. Inspection of welding: a. Cold formed steel deck:	_			
As masonry construction begins, verify that the following are in compliance:     a. Proportions of site—prepared mortar.  b. Grade, type and size of reinforcement	Р	P	ART. 2.1, 2.6A, & 2.6C ART. 3.4		1) Floor and roof deck welds.  b. Reinforcing steel: 1) Verification of weldability of reinforcing steel other than ASTM A 706. 2) Reinforcing steel resisting flexural and axial forces in intermediate and special	_ _ X	x x -	AWS D1.3 ACI 318: 3.5.2	
connectors and anchor bolts. c. Sample panel construction.	P	С	ART. 1.6D		moment frames, and boundary elements of special structural walls of concrete and shear reinforcement.  3) Shear reinforcement.	_	×	AWS D1.4	
<ol> <li>Prior to grouting, verify that the following are in compliance:         <ul> <li>a. Grout space.</li> </ul> </li> </ol>	P	С	ART. 3.2D &		4) Other reinforcing steel.	X	-		
<ul><li>b. Placement of reinforcement, connectors,</li><li>&amp; anchor bolts.</li><li>c. Proportions of site—prepared grout.</li></ul>		SEC. 6.1, 6.3.1, 6.3.6 & 6.3.7	ART. 2.6B &		Not the responsibility of the structural engineer. Special inspection certificate to be completed by geotechnical engineer.  WOOD:				
Verify compliance of the following during construction:     a. Materials and procedures with the approved	P	P		1705.04, 2104 & 2105	Exposure B where wind velocity > 120 mph Exposure C & D where wind velocity > 110 mph Refer to GSN for wind exposure category and wind velocity	CONTINUOUS DURING TASK LISTED	PERIODIC DURING TASK LISTED	REFERENCED STANDARD	IBC REFERENCE
submittals.  b. Placement of masonry units and mortar joint construction.  c. Size and location of structural members.  d. Type, size, and location of anchors, including other details of anchorage of masonry to structural members, frames, or other construction.	P P	P SEC. 1.2.1(4 6.2.1, & 6.	3.1		1. Where the specified fasteners spacing at panel edges is less than or equal to 4", special inspection is required for nailing, bolting, anchoring, and othe fastening of elements of the main wind force resisting system, including wood shearwalls, wood diaphragms, drag struts, braces and holdowns.	-	х		1705–11.01
e. Welding of reinforcement. f. Preparation, construction, and protection of masonry during cold weather (temperature below 40°F (4.4°C) or hot weather (temperature above 90°F (32.2°C). g. Placement of grout is in compliance.	P	SEC. 6.1.6.	ART. 1.8C & 1.8D  ART. 3.5		SOILS:  1. Verify materials below shallow foundations are adequate to achieve the design bearing apacity.  2. Verify excavations are extended to proper depth and have reached proper material.  3. Perform classification and testing of compacted	- -	X X		
4. Observe preparation of grout specimens, mortar specimens, and/or prisms.	P	С	ART. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 b.3, & 1.4 B.4		fill materials.  4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.  5. Prior to placement of compacted fill, inspect subgrade and verify that site has been properly prepared.	x _	x - x		
NOTES: (a) Frequency refers to the frequency of inspection, which during the listed task, as defined in the table. NR=Not Required, P=Periodic, C=Continuous (b) Required for the first 5000 sq. ft. (465 sq. m.) of A. (c) Required after the first 5000 sq. ft. (465 sq. m.) of	AC masonry.		the listed task or periodical	lly	EXPANSION, SCREW, AND EPOXY BOLTS:  1. During placemet of all expansion, screw, & epoxy bolts, for visual verification of hole diameter and depth and placement of bolt and/or epoxy.	_	х		

DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:

- A) THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED TO BE CERTAIN IT CONFORMS WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATION.
- B) THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND
- C) UPON COMPLETION OF THE ASSIGNED WORK THE ENGINEER OR ARCHITECT SHALL COMPLETE AND SIGN THE APPROPRIATE FORMS CERTIFYING THAT TO THE BEST OF HIS KNOWLEDGE THE WORK IS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.

#### INTERPRETATION OF DRAWINGS

	LOCATION OF INFORMATION						
ITEM	INFORMATION	LOCATION	SHEET				
FOOTINGS	SIZE, REINFORCING	SCHEDULE (F) (WF)	S200				
FOOTINGS	DEPTH OF FOOTING	GENERAL STRUCTURAL NOTES (G.S.N.)	S100				
FRAMING MEMBERS	TYPE, SIZE, CONNECTION CAMBER, BEARING PLATES	SCHEDULE (L)	S300				
COLUMNS	TYPE, SIZE, BASE PLATES, REINFORCING	SCHEDULE (C)	S200				
MASONRY WALLS	TYPICAL REINFORCING SPECIAL REINFORCING	GENERAL STRUCTURAL NOTES (G.S.N.) SEE PLAN(S) AND/OR DETAILS	S101 S400				
CONCRETE WALLS	REINFORCING	DETAILS AND SCHEDULE (W)	S200				
DLANLECEND							

	!	•						
	PLAN LEGEND							
SYMBOL	DESCRIPTION	REMARKS						
101 PLAN VIEW	DETAIL CUTS SHOWN ON PLANS	TYPICAL DETAILS ARE TWO DIGIT SERIES NUMBERS FOUNDATION DETAILS ARE 100 SERIES DETAILS FRAMING DETAILS ARE 200 SERIES NUMBERS						
4/////	8" MASONRY WALL U.N.O.	OTHER SIZES ARE DIMENSIONED ON PLANS						
4	PRECAST CONCRETE WALL U.N.O.	SEE PLANS & SCHEDULES FOR REINFORCING						
	MECHANICAL EQUIPMENT	SEE PLANS FOR UNIT WEIGHTS						
	OPENING IN FRAMING	SEE NOTE #4						

#### NOTES

- 1. FOR MATERIAL STRENGTHS, SEE GENERAL STRUCTURAL NOTES
- 2. VERIFY ALL DIMENSION WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION - RESOLVE ANY DISCREPANCIES WITH ARCHITECT.
- 3. FOR CLARITY, ALL EXTERIOR SLABS AND SIDEWALKS MAY NOT BE SHOWN. FOR EXACT DIMENSIONS, LOCATIONS, JOINT AND SCORE LINES, SEE ARCHITECTURAL DRAWINGS
- 4. FOR CLARITY. ALL OPENINGS MAY NOT BE SHOWN ON FRAMING PLANS. FOR EXACT SIZE, NUMBER, AND LOCATION OF OPENINGS, SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS. FOR FRAMING AT OPENINGS, SEE TYPICAL STRUCTURAL DETAILS, VERIFY ALL SIZES, WEIGHTS AND LOCATIONS OF MECHANICAL EQUIPMENT WITH
- 5. DETAILS MARKED "TYPICAL" MAY NOT BE CUT ON PLANS. 6. CONC C.J. - AS SHOWN ON PLAN INDICATES LOCATION OF EITHER KEYED OR SAW CUT CONTROL JOINT IN SLAB ON GRADE AT CONTRACTOR'S OPTION, SEE GENERAL STRUCTURAL
- 7. MAS C.J. AS SHOWN ON PLAN INDICATES MASONRY CONTROL JOINT IN MASONRY WALL, SEE G.S.N. AND TYPICAL DETAIL.
- 8. FOR CLARITY, DETAILS MAY SHOW ONLY ONE SIDE OF FRAMING CONDITION.
- 9. CONTRACTOR TO VERIFY, AND BE RESPONSIBLE FOR VARIATIONS IN CONCRETE QUANTITY DUE TO CAMBER, CONSTRUCTION DEAD LOAD DEFLECTIONS AND/OR TOLERANCES OF STRUCTURAL STEEL ELEMENTS (i.e. BEAMS, STEEL DECK, ETC.) AND PRECAST CONCRETE
- 10. ALL SCHEDULE MARK DESIGNATIONS MAY NOT NECESSARILY BE FOUND ON THE PLANS WHERE THE SCHEDULES OCCUR. SCHEDULES ARE TYPICAL TO THE PROJECT.

	ABBREVIATIONS						
ľ	A B C	- AGGREGATE BASE COURSE	HORIZ- — — — —	- HORIZONTAL			
١		- AIR CONDITIONER	K(KIP)				
ı		- ABOVE FINISHED FLOOR	L.L				
ı							
ı	ALT. ————		LBS (#) — — — —	— POUNDS			
ı		- ANCHOR BOLT		- LONG LEG HORIZONTAL			
ı	<u> </u>	- AT (MEASUREMENT)					
ı	BM		MFR('S) — — — —	- MANUFACTURER('S)			
ı		-BELOW FINISHED FLOOR		- MASONRY CONTROL JOINT			
ı	B.O.B. — — — —		MECH'L — — — — -				
ı	B.O.D. — — — —		N/A				
ı	B.O.F. — — — —	-BOTTOM OF FOOTING	N.T.S				
ı	BRG — — — — —	- BEARING	o.c				
ı	C.I.P. — — — — —	-CAST IN PLACE		- OUTSIDE FACE OF WALL			
ı	C.L. — — — — —		OPP				
ı	C.L.B. — — — — —	-CENTERLINE OF BEAM	P.C————	- PRECAST CONCRETE			
ı	c.L.c. — — — — —	- CENTERLINE OF COLUMN	P.J————	- PANEL JOINT			
ı	C.L.F. — — — — —	- CENTERLINE OF FOOTING	PLF	- POUNDS PER LINEAR FOOT			
ı	c.L.w. — — — —	- CENTERLINE OF WALL	PLYWD — — — — -				
ı	CLR — — — —		PREFAB — — — —	- PREFABRICATED			
ı	CONC — — — — —		PSF	- POUNDS PER SQUARE FOOT			
ı		- CONCRETE CONTROL JOINT	PSI	- POUNDS PER SQUARE INCH			
ı		- CONCRETE SAWCUT JOINT	REINF				
ı		- CONCRETE MASONRY UNIT		- SHORT LEG HORIZONTAL			
ı	CONN — — — —		SIV	- SHORT LEG VERTICAL			
ı	CONT — — — —		SIM	- SIMIL AR			
ı	D.L. — — — —		S.O.G — — — —				
ı	ø OR DIA. — — —		SQ	- SOLIARE			
ı	DN ————		STD				
ı	DWG(S) — — — —		T.L————				
ı	E.O.S. — — — —		T.O.B				
ı	EQ ————		T.O.D. — — — — —				
ı	EQUIP — — — —		T.O.F. – – – – –				
ı	EXP. BOLT — — —		T.O.L				
ı							
ı	EXP. JT (E.J.)— — -	- EXPANSION JUINT	T.O.M — — — —				
ı	E.W. — — — —	- EACH WAY	T.O.P				
ı	F.F. ————	-FINISHED FLOOR	T.O.S				
ı	F.O.M. — — — —	-FACE OF MEMBER	T.O.W — — — —				
ı	F.O.S. — — — —		TYP				
ı	F.O.W. — — — —		U.N.O — — — — —	- UNLESS NOTED OTHERWISE			
١	GA — — — —		VERT				
١	GALV — — — —	– GALVANIZED	V.I.F — — — —	- VERIFY IN FIELD			
١	G.S.N. — — — — —	-GENERAL STRUCTURAL NOTES	W.W.F — — — — —	- WELDED WIRE FABRIC			
١		-GLUED-LAMINATED BEAM	w/				
١	I.F.W. — — — —	-INSIDE FACE OF WALL	w/o	- WITHOUT			

#### NOTICE OF EXTENDED PAYMENT PROVISION

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION AND APPRÓVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT





PK Associates, L.L.C. 7434 E. McDonald Drive Scottsdale, Arizona 85250 Phone: (480) 922-8854 Fax: (480) 922-3739 Email: cadd@pkastructural.com Website: www.pkastructural.com

### SCHEMATIC DESIGN

Symbol	Revisions	Date	Appr.	Designed by:	Date:
				JAppleyard	07/30/2021
				Drawn by:	Scale:
				EHales	AS NOTED
				Checked by:	Project Code:
				JKoehler	GLA PROJECT NO. 20109

PROGRESS PRINT NOT FOR **CONSTRUCTION** 

# gabor lorant architects

3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com





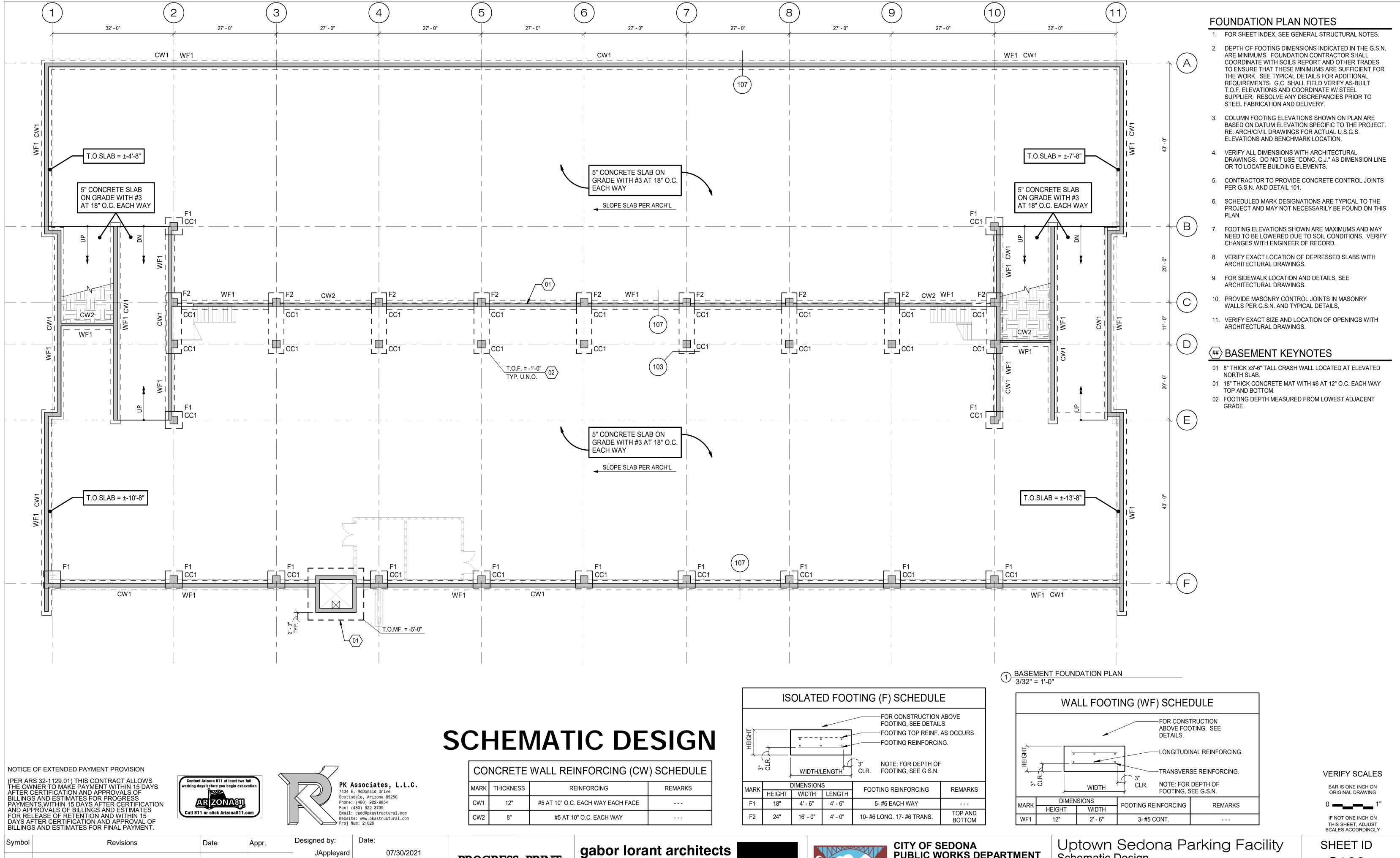
CITY OF SEDONA PUBLIC WORKS DEPARTMENT **102 ROADRUNNER DRIVE** 

**SEDONA, ARIZONA 86336** 928-204-7111

Schematic Design GENERAL STRUCTURAL NOTES

Uptown Sedona Parking Facility

SHEET ID S003



3326 n. 3rd avenue suite 200

phoenix, arizona 85013

tel: 602.667.9090

fax: 602.667.9133

gaborlorant.com

07/30/2021

AS NOTED

GLA PROJECT NO. 20109

PROGRESS PRINT

NOT FOR

**CONSTRUCTION** 

JAppleyard

**EHales** 

Drawn by:

Checked by:

Scale:

Project Code:

S100

BASEMENT FOUNDATION SHEET NO.

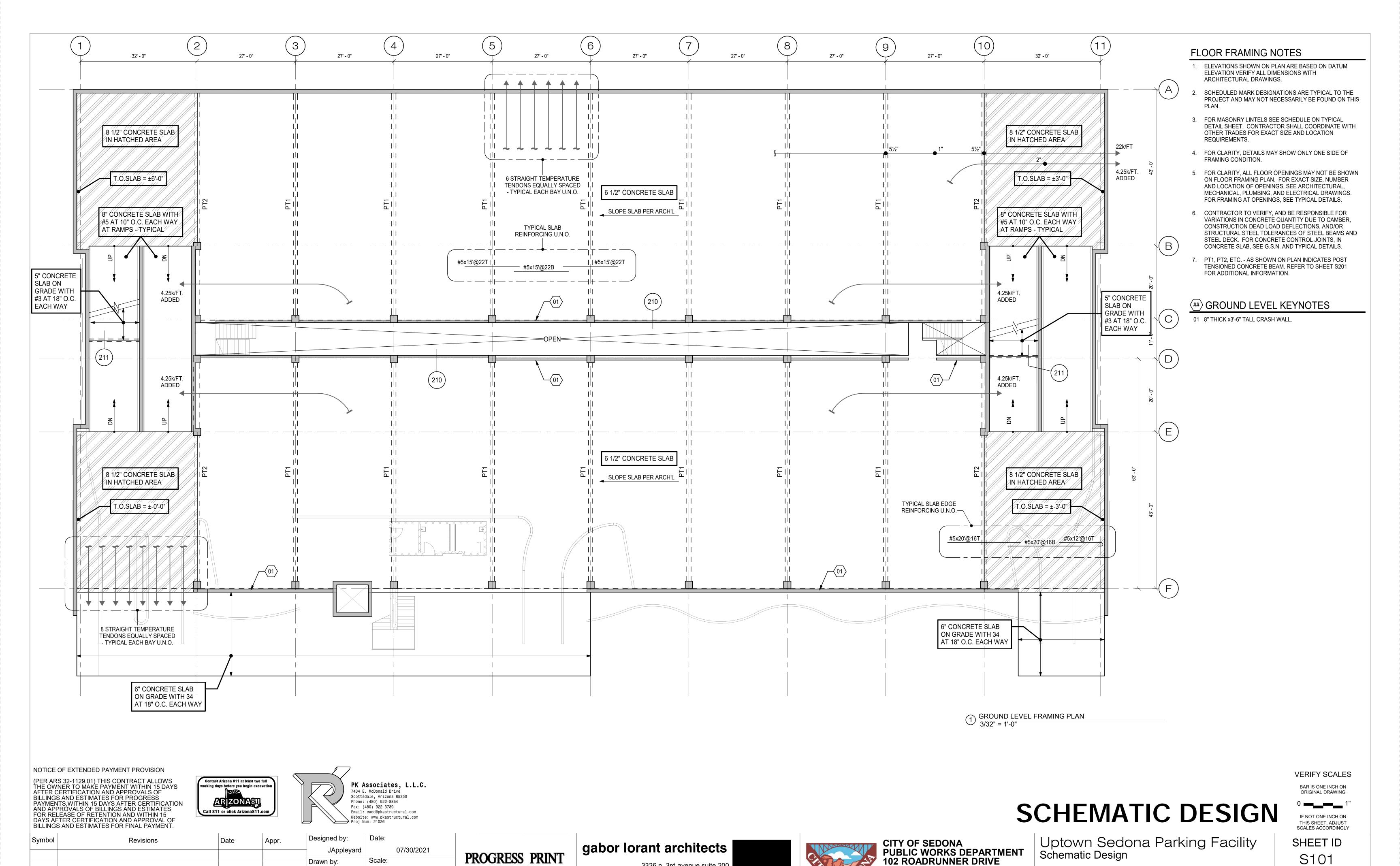
PUBLIC WORKS DEPARTMENT 102 ROADRUNNER DRIVE

**SEDONA, ARIZONA 86336** 

928-204-7111

Schematic Design

PLAN



3326 n. 3rd avenue suite 200

phoenix, arizona 85013

tel: 602.667.9090

fax: 602.667.9133

gaborlorant.com

Scale:

Project Code:

AS NOTED

GLA PROJECT NO. 20109

NOT FOR

**CONSTRUCTION** 

Drawn by:

Checked by:

EHales

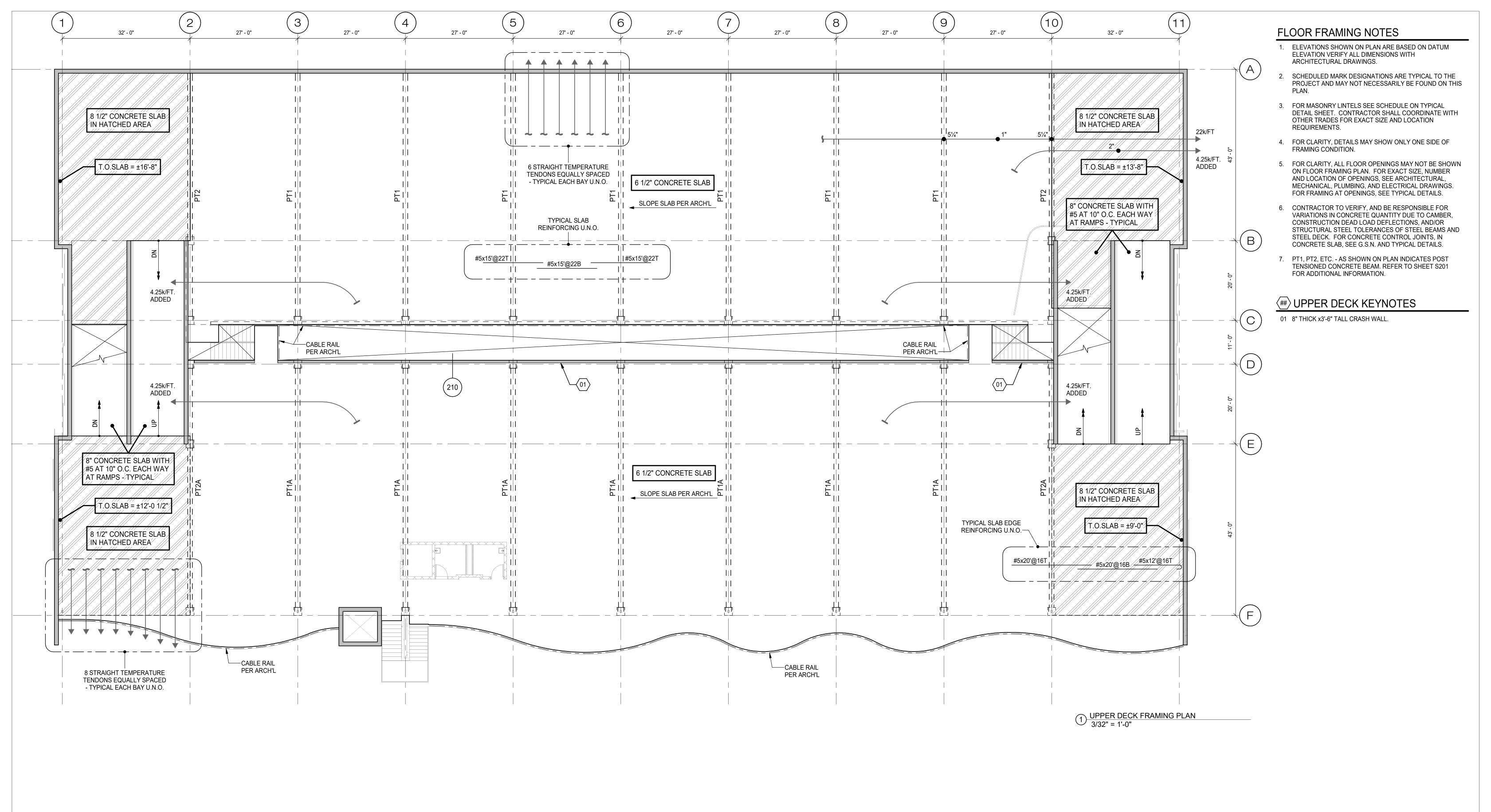
S101 SHEET NO.

GROUND LEVEL FRAMING

PLAN

**SEDONA, ARIZONA 86336** 

928-204-7111



NOTICE OF EXTENDED PAYMENT PROVISION (PER ARS 32-1129.01) THIS CONTRACT ALLOWS
THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS
AFTER CERTIFICATION AND APPROVALS OF
BILLINGS AND ESTIMATES FOR PROGRESS
PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION
AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT.



PK Associates, L.L.C. Scottsdale, Arizona 85250 Phone: (480) 922-8854 Fax: (480) 922-3739 Email: cadd@pkastructural.com Website: www.pkastructural.com

# SCHEMATIC DESIGN

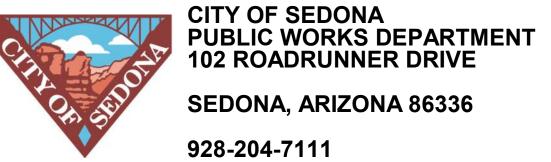
Uptown Sedona Parking Facility Schematic Design

IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY SHEET ID

Symbol	Revisions	Date	Appr.	Designed by:	Date:
				JAppleyard	07/30/2021
				Drawn by:	Scale:
				EHales	AS NOTED
				Checked by:	Project Code:
				JKoehler	GLA PROJECT NO. 20109

PROGRESS PRINT NOT FOR **CONSTRUCTION** 





UPPER DECK FRAMING PLAN

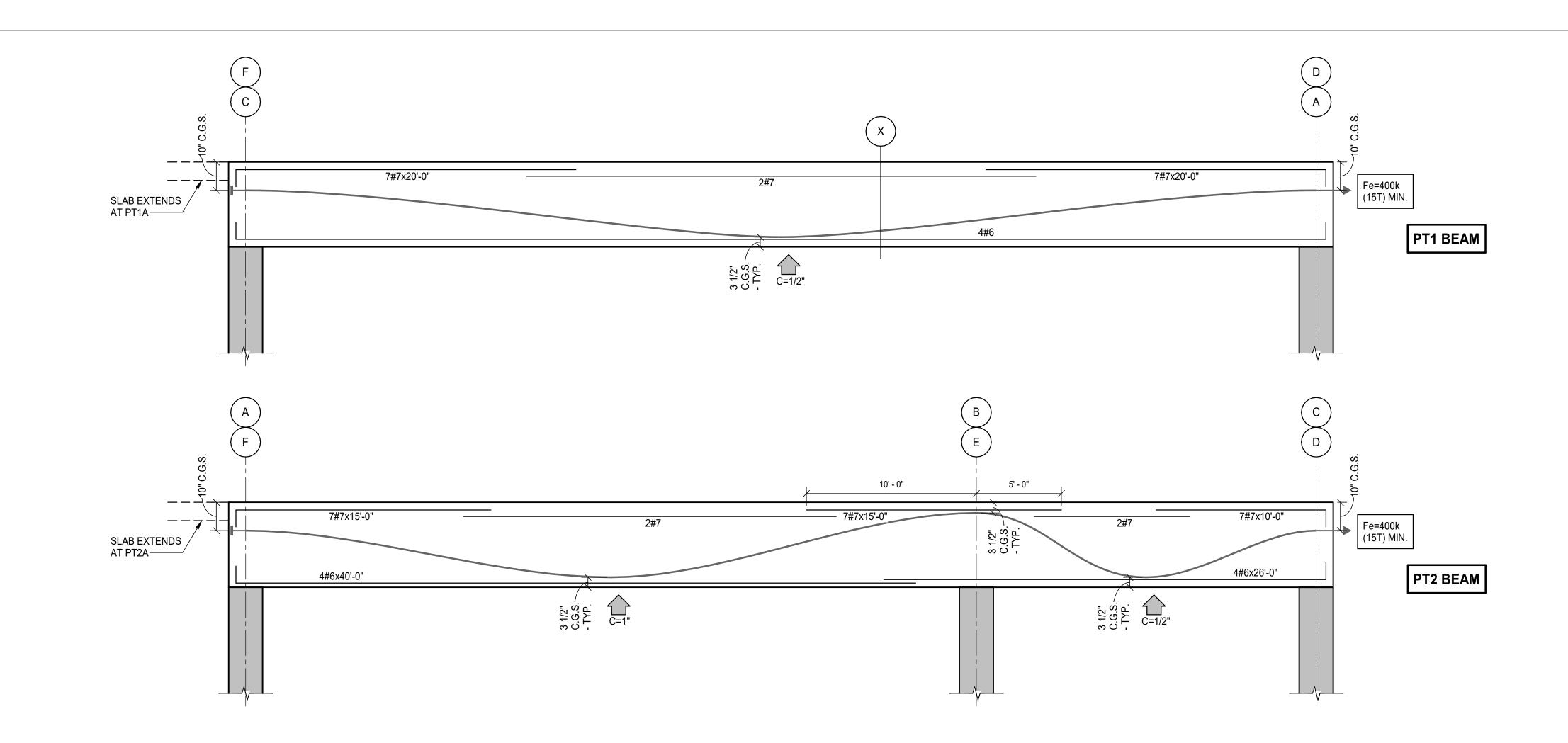
SHEET NO.

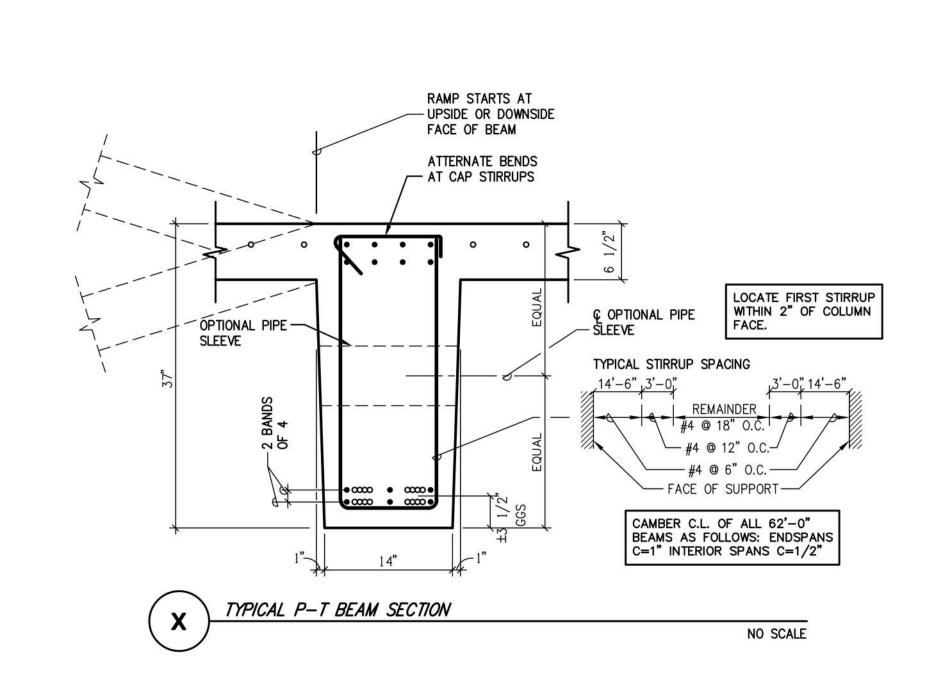
S102

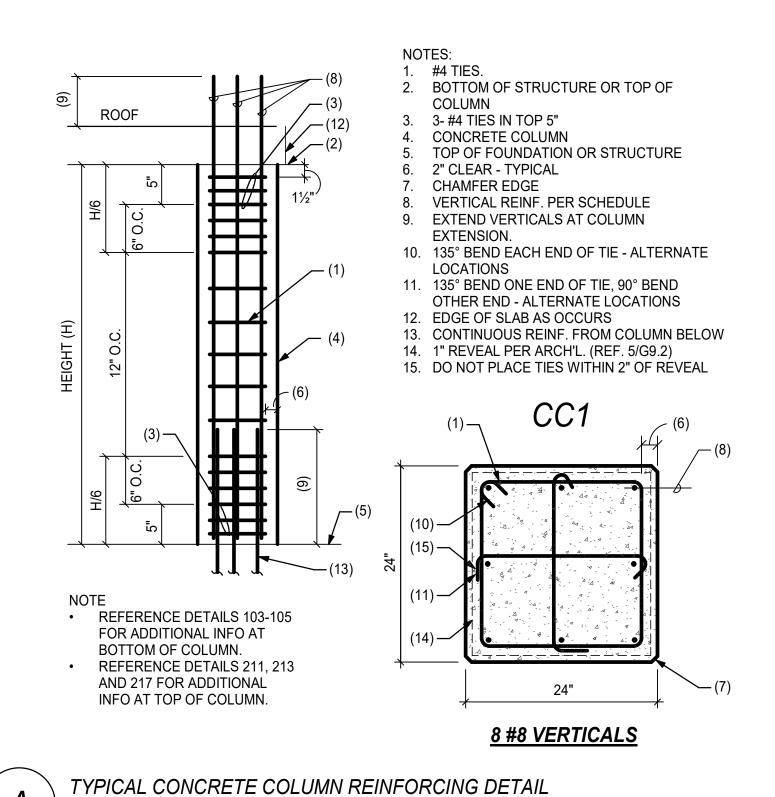
**VERIFY SCALES** 

BAR IS ONE INCH ON ORIGINAL DRAWING

0 \_\_\_\_1







# SCHEMATIC DESIGN

NOTICE OF EXTENDED PAYMENT PROVISION (PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS
PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION
AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT.

Revisions

Symbol



Appr.

Date



Date:

Scale:

Project Code:

07/30/2021

AS NOTED

GLA PROJECT NO. 20109

Designed by:

Drawn by:

Checked by:

JAppleyard

**EHales** 

gabor lorant architects

3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com



**CITY OF SEDONA** PUBLIC WORKS DEPARTMENT 102 ROADRUNNER DRIVE **SEDONA, ARIZONA 86336** 

Uptown Sedona Parking Facility Schematic Design

THIS SHEET, ADJUST SCALES ACCORDINGLY SHEET ID

NO SCALE

SHEET NO. OF

**VERIFY SCALES** 

BAR IS ONE INCH ON

ORIGINAL DRAWING

IF NOT ONE INCH ON

PROGRESS PRINT

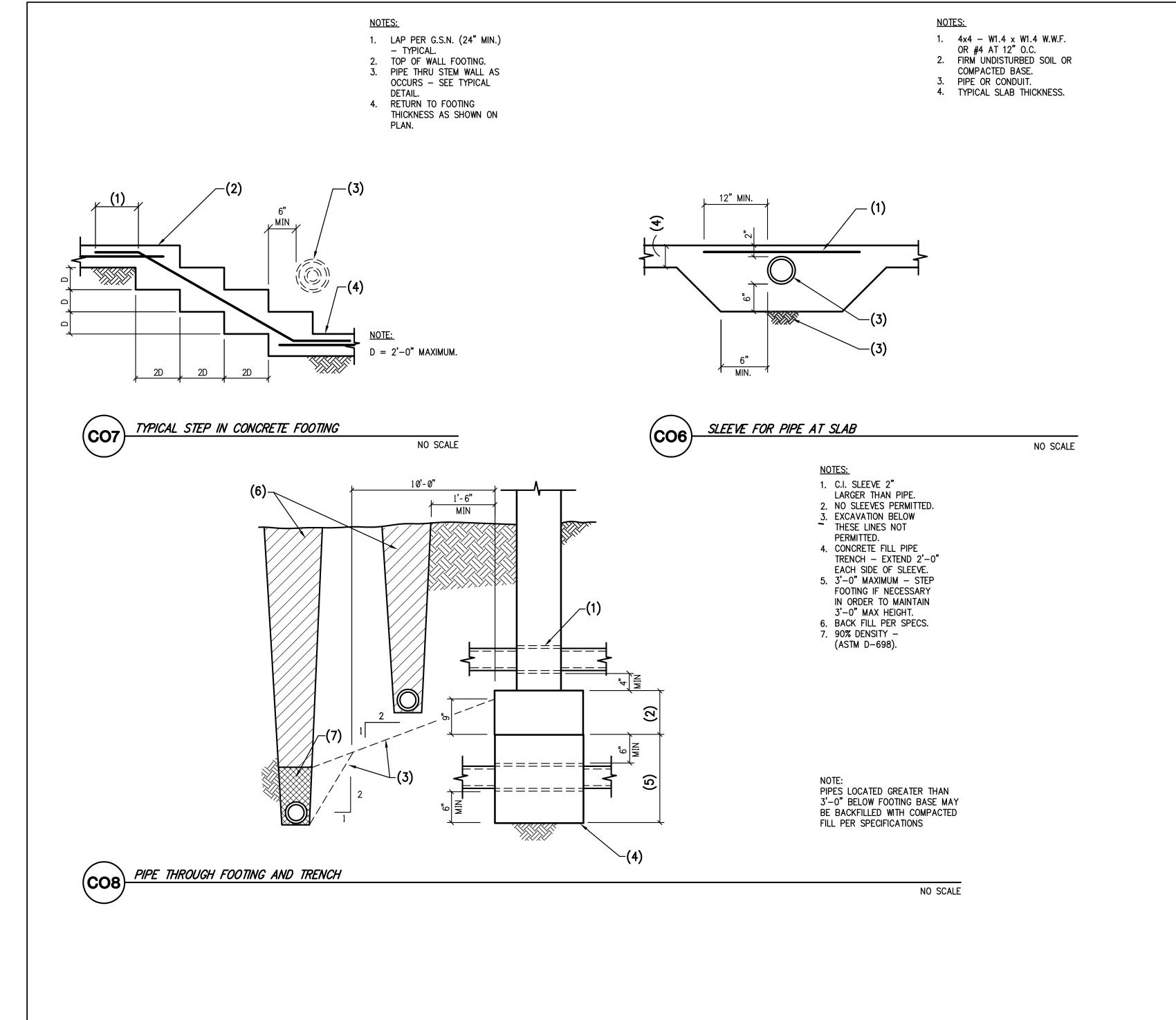
NOT FOR

CONSTRUCTION

928-204-7111

PT BEAM PROFILES

S200





# NON-CONTACT SPLICE

CONTACT SPLICE

SPLICE DETAIL

BEND & HOOK DETAILS

COLUMN TIES



BAR CLEARANCE

BEAM STIRRUPS

TYPICAL CONCRETE REINFORCING BAR DETAILS

<u>PATTERN "A"</u> TYP – U.N.O.

PATTERN "B"

4. 1d (1" MINIMUM) 5. RADIUS=3d FOR BARS NOT OVER #8; 4d FOR #9, #10 AND #11 BARS; 5d FOR #14 AND #18 BARS. 6. 4d ((2 1/2" MINIMUM). 7. 12d (90 DEGREE HOOK). 8. 6d (3" MINIMUM). 9. 135 DEGREE BEND.

LAP - SEE G.S.N.

WIRE TIES.

2. MAXIMUM 1/5 LAP LENGTH

BUT NOT MORE THAN 6".

10. BEND AROUND 1 1/2" Ø PIN FOR #3 BARS. BEND AROUND 2"ø PIN FOR #4 BARS. BEND AROUND 2 1/2"ø PIN FOR #5

- 11. PRIOR APPROVAL MUST BE GIVEN BY OUR OFFICE TO ALLOW NON-CONTACT SPLICES.
- 12. LAP TIE MIN. 6"
- 13. LONGITUDINAL REINFORCEMENT. 14. LONGITUDINAL BAR AS OCCURS.
- 15. PROVIDE 135° HOOK AT LONGITUDINAL REINFORCEMENT.
- 16. ROTATE AND ALTERNATE TIE LAP AT DIFFERENT VERTICAL REBAR LOCATION AT EACH TIE.

#### CONC. PSI | fc = 2,500 PSI / | fc = 4,000 PSI | fc = 5,000 PSIf'c = 3,000 PSIand higher REGULAR TOP REGULAR TOP REGULAR TOP STD ENCLOSED LOCATION CLASS CLASS CLASS CLASS CLASS LAP W/ SPIRAL 17**"** 12**"** 19" | 24" 22**"** 29**"** 28**" 36**" 43" 23" 63**"** 72**"** 92**"** 100**" 42**" 113**"** 78**"** 102"

CLASS B TENSION SPLICE LENGTHS 60 KSI REBAR

- 1. TOP BARS ARE ANY HORIZONTAL BARS PLACED SO THAT MORE THAN 12" OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE REINFORCEMENT.
- 2. UNLESS NOTED OTHERWISE, LAP SPLICES IN CONCRETE BEAMS, SLABS, AND COLUMNS WILL BE
- CLASS B TENSION SPLICE LENGTHS. 3. CONTACT STRUCTURAL ENGINEER IF CENTER TO CENTER SPACING OF REINFORCING IS LESS THAN OR
- EQUAL TO 3 BAR DIAMETERS <3db OR 2db QLEAR SPACING BETWEEN BARS. 4. ALL SPLICES MUST BE FULL CONTACT.

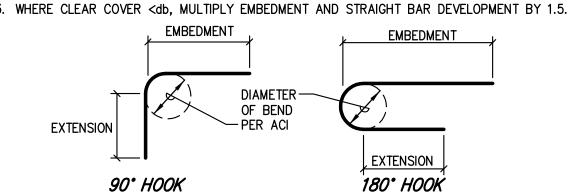
#### 5. SPLICES WITH #14 OR #18 BARS SHALL USE MECHANICAL COUPLERS. (THIS INCLUDES #14 OR #18 BARS TO SMALLER BARS SHOWN IN SCHEDULE).

LAP SCHEDULE FOR REINFORCING STEEL — 60KSI NO SCALE

COMP. BARS

BAR	HOOKED EMBEDMENT			EXTENSION		STRAIGHT BAR		
SIZE	3000 PSI CONCRETE	4000 PSI CONCRETE	5000 PSI CONCRETE	90. HOOK	180° HOOK	5000 PSI	MBEDMEI 4000 PSI	<b>V/</b> 3000 PSI
#3	6	6	6	4.5	2.5	13	14	16
#4	8	7	6	6.0	2.5	17	19	22
<b>#</b> 5	10	8	7	7.5	2.5	21	24	27
<b>#</b> 6	12	10	9	9.0	3.0	26	28	33
<b>#</b> 7	13	12	10	10.5	3.5	37	42	48
#8	15	13	12	12.0	4.0	43	47	55
#9	17	15	13	13.5	4.5	48	54	62
<b>#</b> 10	19	17	15	15.2	5.1	54	60	70
<b>#</b> 11	22	19	17	16.9	5.6	60	67	77
NOTEO								•

- EMBEDMENT LENGTH IS BASED ON 2 1/2" MINIMUM SIDE COVER AND 2" MINIMUM END COVER.
- 2. CONTACT STRUCTURAL ENGINEER IF CÉNTER TO CENTER SPACING OF REINFORCING IS LESS THAN OR EQUAL TO 3 BAR DIAMETERS <3db OR 2db CLEAR SPACING BETWEEN BARS.





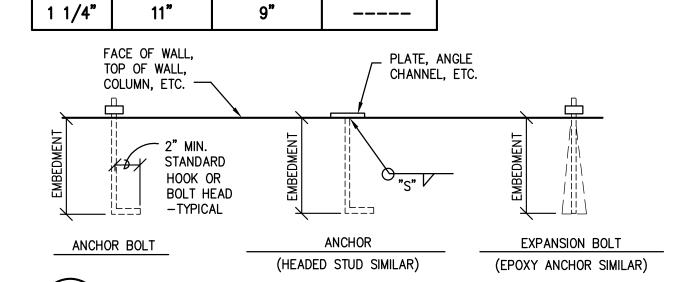
NO SCALE

BOLT DIAMETER	VERT BOLT EMBEDMENT LENGTH	HORIZ BOLT EMBEDMENT LENGTH	HEADED STUD FILLET WELD SIZE, "S".
1/2"	6"	4"	1/4"
5/8"	6"	4"	5/16"
3/4"	7"	5 <b>"</b>	5/16"
7/8"	8"	6"	5/16"
1"	9"	7"	3/8"
1 1/8"	10"	8"	

PROVIDE ANCHORS, ANCHOR BOLTS AND EXPANSION BOLTS PER THIS SCHEDULE UNLESS NOTED OTHERWISE ON PLANS OR DETAILS. AT "ANCHORS" USE 3/16" FILLET WELD("S").

THICKNESS OF DRYPACK DOES NOT APPLY TOWARDS EMBEDMENT.

SEE GENERAL STRUCTURAL NOTES FOR SPECIFICATIONS ON EXPANSION BOLTS AND EPOXY ANCHORS.



TYPICAL ANCHOR, ANCHOR BOLT, AND EXPANSION BOLT SCHEDULE Uptown Sedona Parking Facility

SHEET ID S300

NO SCALE

SHEET NO.

# SCHEMATIC DESIGN

NOTICE OF EXTENDED PAYMENT PROVISION (PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT

Revisions

Symbol



Appr.



Date:

Scale:

Project Code

07/30/2021

AS NOTED

GLA PROJECT NO. 20109

Designed by:

Drawn by:

Checked by:

JAppleyard

**EHales** 



PROGRESS PRINT NOT FOR **CONSTRUCTION** 

## gabor lorant architects

3326 n. 3rd avenue suite 200 phoenix, arizona 85013 tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com





PLAN VIEW - STEEL COLUMN AT STEEL BASE PLATE

**CITY OF SEDONA** PUBLIC WORKS DEPARTMENT **102 ROADRUNNER DRIVE** 

CENTERLINE OF COLUMN AND

STEEL BASE PLATE.

STEEL BASE PLATE.

4. DIMENSION PER AISC TABLE

5. EMBED MENT 12" FOR 1"ø

8. STEEL PLATE WASHER -

J3.5 (1 1/4" MIN). TYPICAL

BOLTS, 18" FOR 1 1/4" BOLTS.

4" X 4" X 1/2" W/ DOUBLE

9. ADDITIONAL BOLT HOLE WHERE

SHOWN ON SCHEDULE.

FOR ADDITIONAL INFORMATION,

SEE COLUMN SCHEDULE.

10. FULL PENETRATION AT EACH FLANGE AT PATTERN B ONLY.

STEEL COLUMN.

6. DOUBLE NUTS.

11. A325 BOLTS.

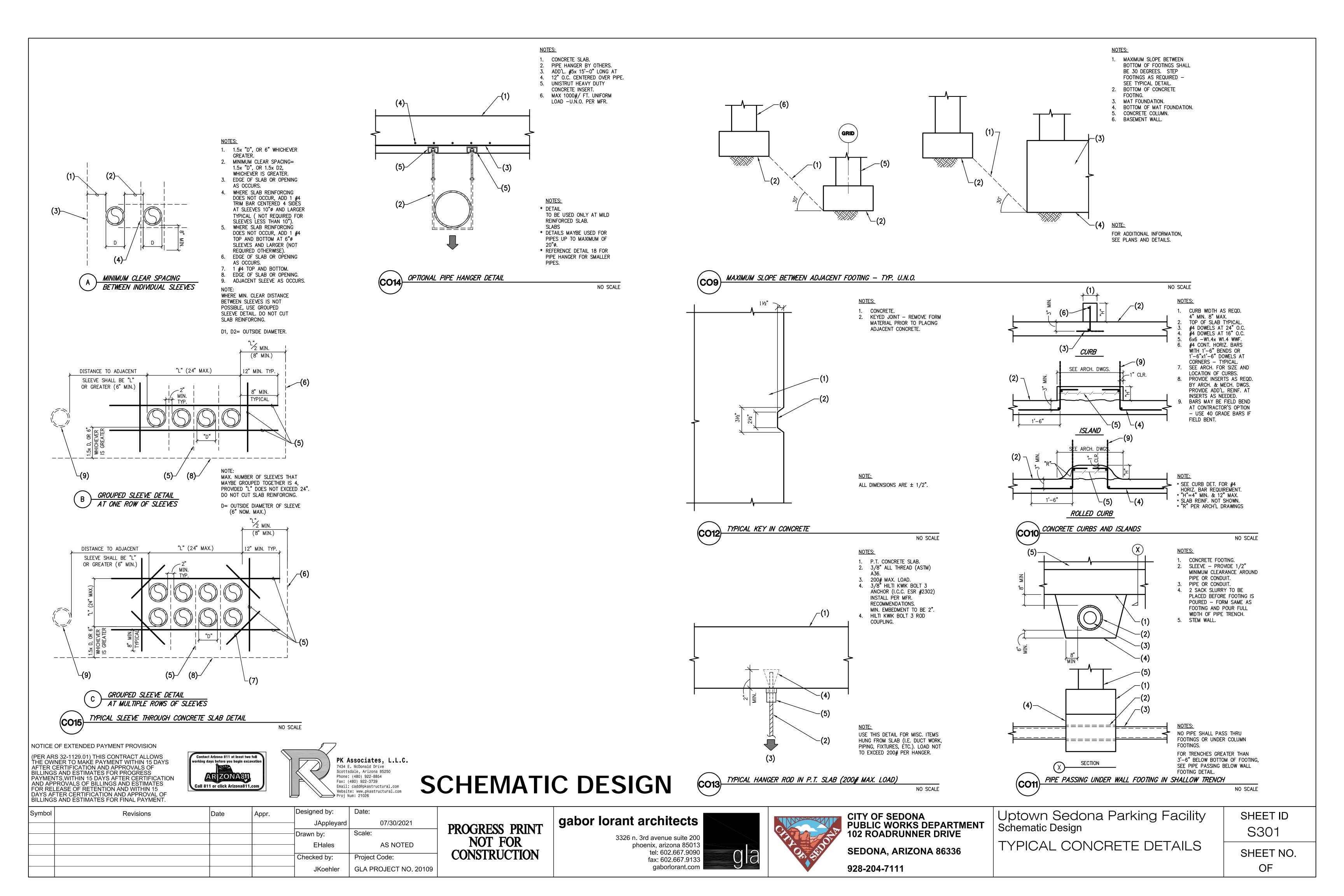
7. 1 1/2" DRYPACK.

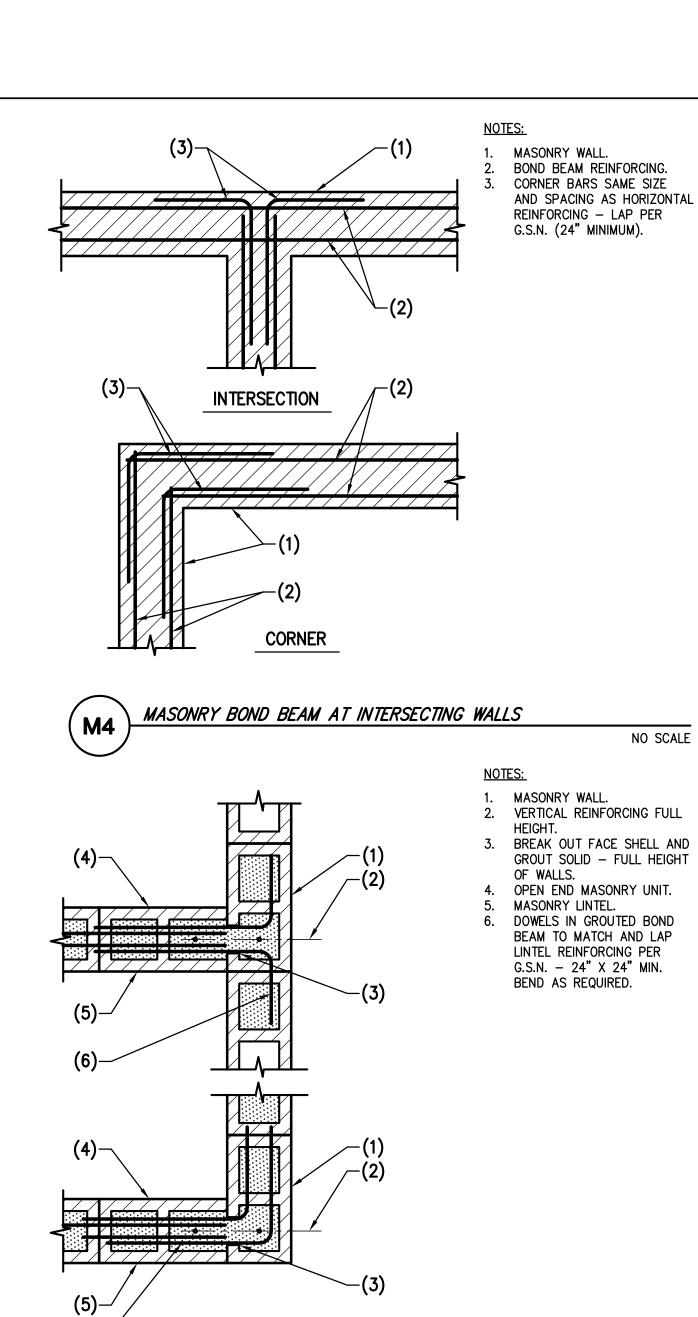
**SEDONA, ARIZONA 86336** 928-204-7111

NO SCALE

TYPICAL CONCRETE DETAILS

Schematic Design





PLAN VIEW - MASONRY WALL INTERSECTION AT MASONRY LINTEL

BAR AT EDGES

PLAN VIEW - TYPICAL MASONRY WALL REINFORCING PLACEMENT NO SCALE MASONRY WALL. CONTROL JOINT. CONTROL JOINT MATERIAL PER ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. WHERE A FIRE RATED CONTROL JOINT OCCURS SEE WALL CONTROL JOINT SCHEDULE FRO REQUIRED C.J. BLANKET. 4. 1 VERTICAL BAR EACH SIDE IN SOLID GROUTED CELLS TO MATCH VERTICAL WALL REINFORCING. 5. CONTINUOUS BOND BEAM BARS - WRAP BARS WITH MASTIC FOR BOND BREAK. AREA SEPARATION WALL C.J. 1 HOUR 1 1/2" CERE BLANKET DEPTH
2 HOUR 3" CERE BLANKET DEPTH
3 HOUR 4" CERE BLANKET DEPTH 4 HOUR 4 1/2" CERE BLANKET DEPTH • BOND BEAM BARS SHALL NOT BE LAPPED WITHIN 8'-0" OF CONTROL JOINT.
• MAX C.J. SPACING = 24'-0" 1'-6"

NOTES:

MASONRY WALL.
 VERTICAL REINFORCING AS

SHOWN ON PLANS.

(4) (3) (3) (3)

CONTROL JOINT IN MASONRY WALL

BAR AT 48" O.C.

3. 1 VERTICAL BAR IN GROUTED CELLS AT END OR AT INTERSECTION OF WALLS.

4. MASONRY WALL WHICH ABUTS CONTINUOUS MASONRY WALL AND IT NOT "TOOTHED" INTO IT.

1. MASONRY WALL.
2. 1/8" THICK x1 1/4" WIDE x2"x8"x2" BENT STEEL "Z"

NO SCALE

NOTICE OF EXTENDED PAYMENT PROVISION

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS
THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS
AFTER CERTIFICATION AND APPROVALS OF
BILLINGS AND ESTIMATES FOR PROGRESS
PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION
AND APPROVALS OF BILLINGS AND ESTIMATES
FOR RELEASE OF RETENTION AND WITHIN 15
DAYS AFTER CERTIFICATION AND APPROVAL OF
BILLINGS AND ESTIMATES FOR FINAL PAYMENT.





# SCHEMATIC DESIGN

Symbol	Revisions	Date	Appr.	Designed by:	Date:
				JAppleyard	07/30/2021
				Drawn by:	Scale:
				EHales	AS NOTED
				Checked by:	Project Code:
				JKoehler	GLA PROJECT NO. 20109

PROGRESS PRINT NOT FOR CONSTRUCTION





CITY OF SEDONA
PUBLIC WORKS DEPARTMENT
102 ROADRUNNER DRIVE
SEDONA, ARIZONA 86336

928-204-7111

NO SCALE

Uptown Sedona Parking Facility Schematic Design

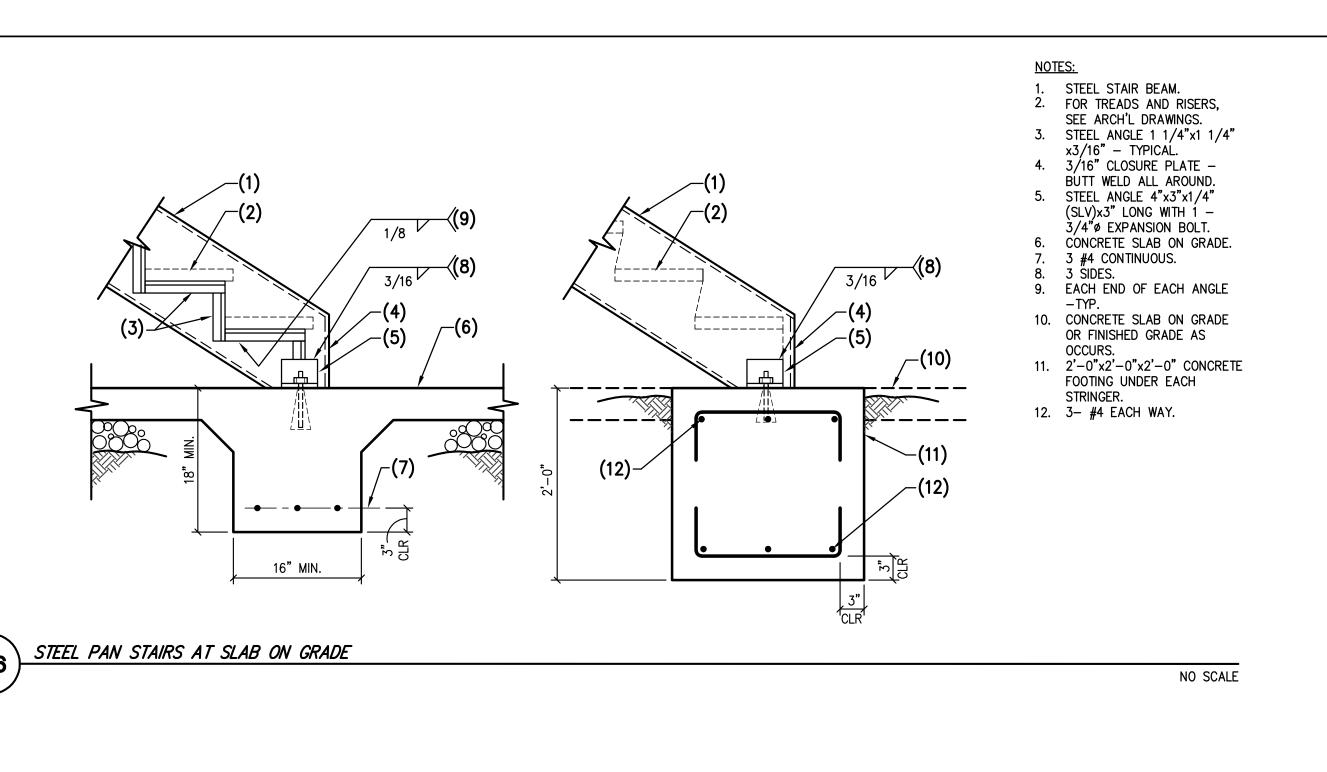
TYPICAL MASONRY DETAILS

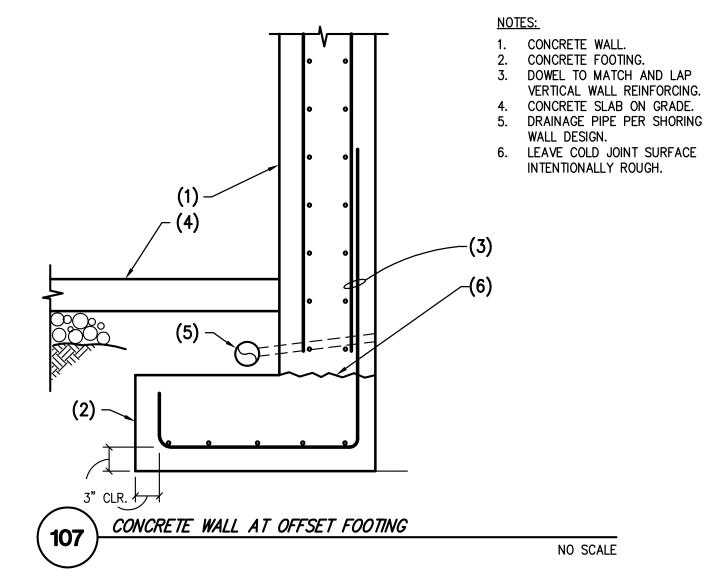
TYPICAL CONNECTION FOR NON-TOOTHED ABUTTING MASONRY WALLS

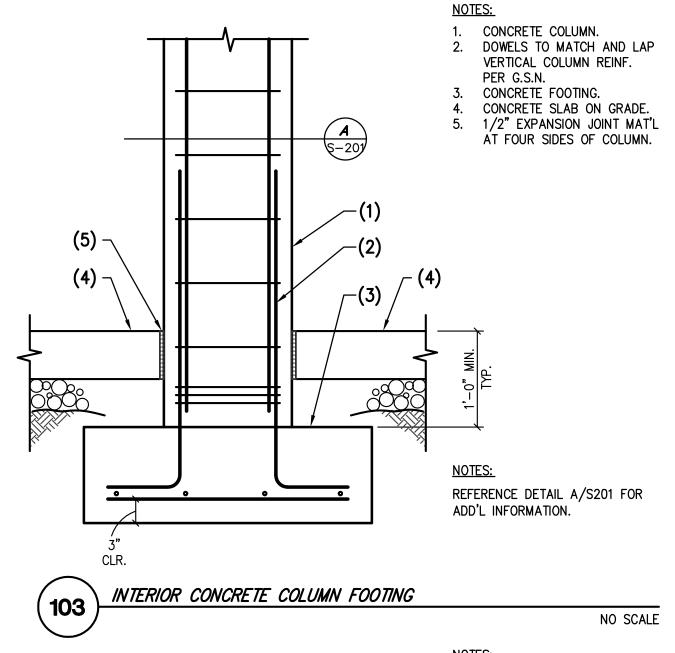
SHEET ID S302

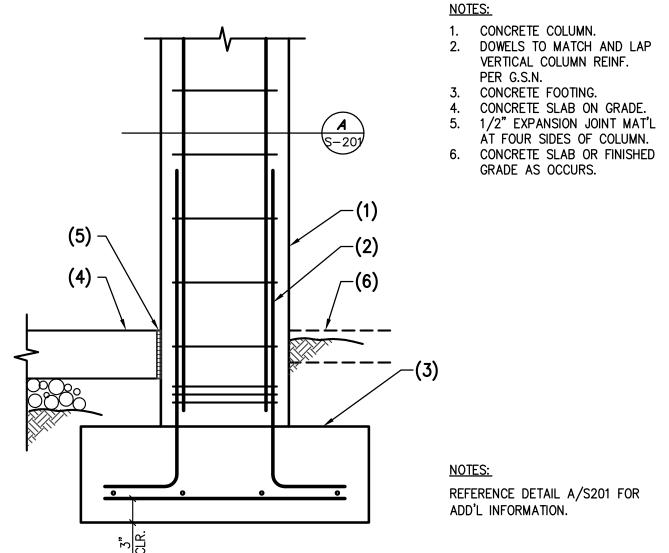
NO SCALE

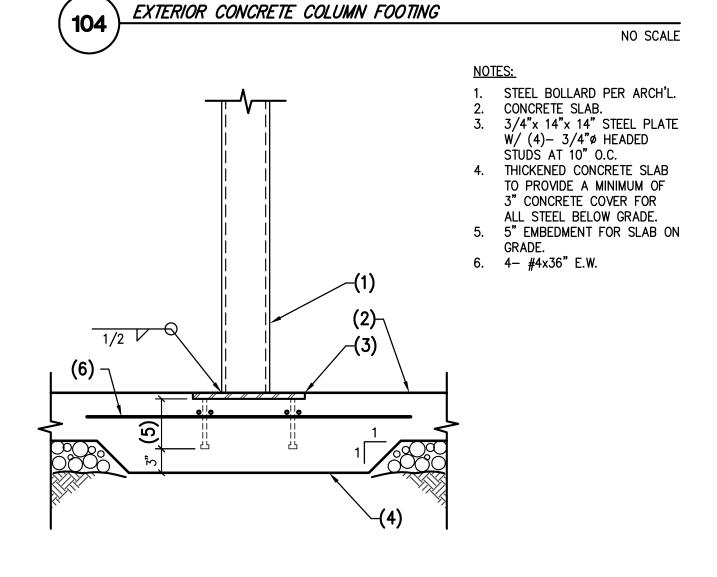
SHEET NO. OF

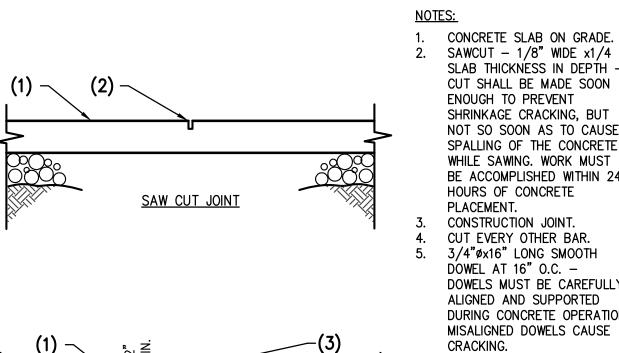


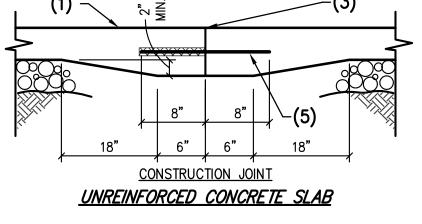


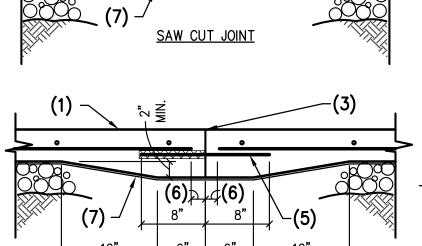












**CONSTRUCTION JOINT** 

REINFORCED CONCRETE SLAB

- CONSTRUCTION JOINTS NEED ONLY OCCUR AT EXPOSED EDGES DURING PLACEMENT UNLESS SPECIFICALLY NOTED ON THE PLANS.

SLAB THICKNESS IN DEPTH -

CUT SHALL BE MADE SOON

NOT SO SOON AS TO CAUSE

SPALLING OF THE CONCRETE

WHILE SAWING. WORK MUST

BE ACCOMPLISHED WITHIN 24

ENOUGH TO PREVENT SHRINKAGE CRACKING, BUT

HOURS OF CONCRETE

CONSTRUCTION JOINT. CUT EVERY OTHER BAR. 3/4"øx16" LONG SMOOTH DOWEL AT 16" O.C. -DOWELS MUST BE CAREFULLY

ALIGNED AND SUPPORTED

DURING CONCRETE OPERATION.

MISALIGNED DOWELS CAUSE

STOP SLAB REINF. AT 1 1/2"

VAPOR BARRIER WHERE SPECIFIED ON PLAN.

PLACEMENT.

CRACKING.

FROM JOINT.

- SLAB'S REINFORCEMENT TO BE LOCATED AT THE TOP 1/3 OF SLAB THICKNESS WHERE VAPOR BARRIER SPECIFIED ON

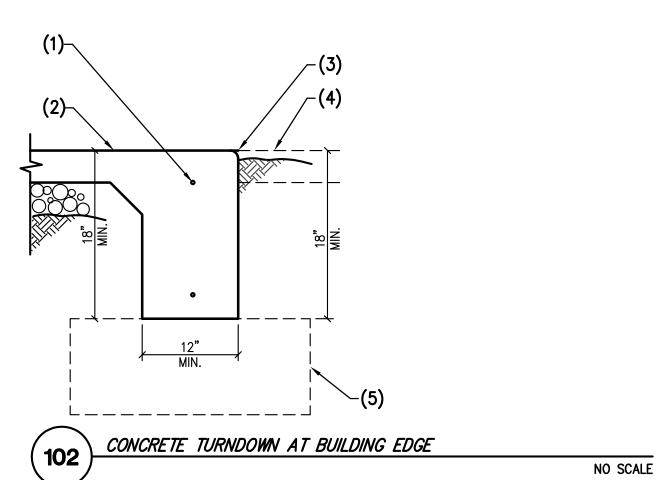
NO SCALE

#### CONTROL JOINTS IN CONCRETE SLAB ON GRADE

1. 1 — #4 CONT. TOP AND BOTTOM.

CONCRETE SLAB ON GRADE. TOOLED EDGE. 4. CONCRETE SLAB ON GRADE OR FINISHED GRADE AS OCCURS.

5. FOOTING AS OCCURS.

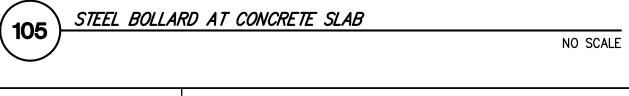


NOTICE OF EXTENDED PAYMENT PROVISION (PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT.





# SCHEMATIC DESIGN



Symbol	Revisions	Date	Appr.	Designed by:	Date:
				JAppleyard	07/30/2021
				Drawn by:	Scale:
				EHales	AS NOTED
				Checked by:	Project Code:
				JKoehler	GLA PROJECT NO. 20109

PROGRESS PRINT NOT FOR CONSTRUCTION





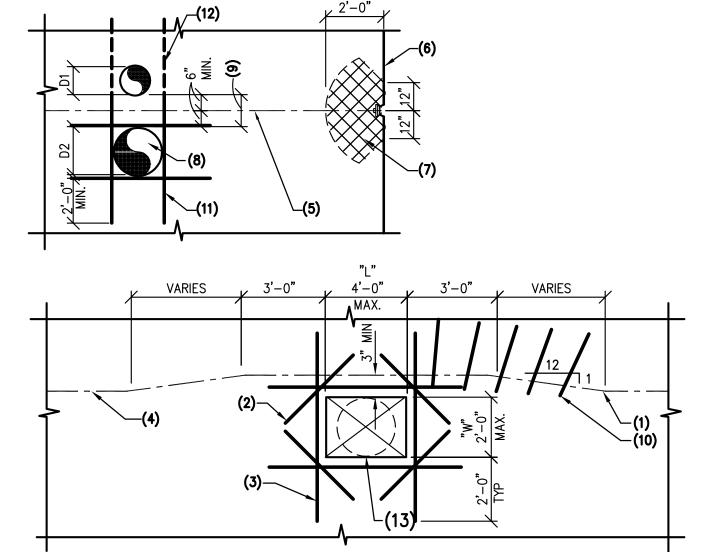
CITY OF SEDONA
PUBLIC WORKS DEPARTMENT **102 ROADRUNNER DRIVE SEDONA, ARIZONA 86336** 

928-204-7111

FOUNDATION DETAILS

Uptown Sedona Parking Facility Schematic Design

SHEET ID S400



1. TENDONS MUST MAKE SMOOTH

TRANSITION-NO KINKS PERMITTED. 2. 1 #5 X 4'-0" EACH CORNER AT

MINIMUM DEPTH. 3. 1 #5 TOP AND BOTTOM. DISPLACED TENDON.

TENDON. EDGE OF SLAB. HOLES NOT PERMITTED IN

HATCHED AREA. 8. HOLES IN SLAB - REINFORCE PER PLAN DETAIL FOR MORE THAN TWO

SLEEVES. 9. (D1 + D2)/3 OR 60" AT BANDED TENDONS. 3'-0"10. USE 5 #3 HAIRPINS AT 8" O.C. CENTERED AT FIRST BEND WHERE

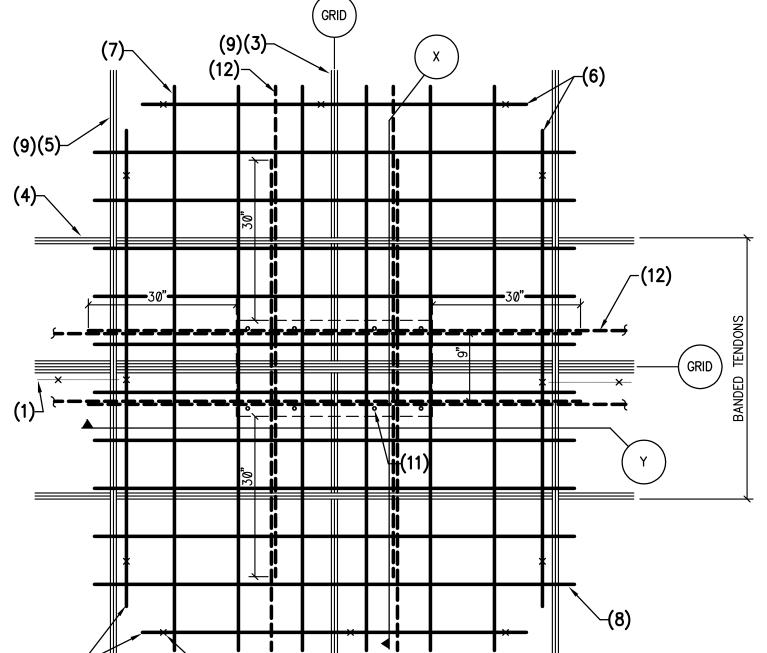
SLOPE EXCEED 12 TO 1, (SLOPE NOT TO EXCEED 6 TO 1 IN ANY CASE). 11. 1 #4 TOP AND BOTTOM 4 SIDES FOR SLËEVES LARGER THAN 4" DIA.

12. RUN TRIM BARS CONT. WHERE SLEEVES

SPACED CLOSER THAN 2 TIMES SLEEVE

13. ROUND HOLE AS OCCURS - USE SAME REINF. AS SHOWN.

NO SCALE



1. TERMINATE UNIFORM TENDON SUPPORT STEEL. HIGH CHAIR.

UNIFORM TENDON AT COLUMN. 4. BANDED TENDON.

UNIFORM TENDONS. MILD REINF. SUPPORT (BY REBAR SUPPLIER). 7. LOWER LAYER OF MILD

8. UPPER LAYER OF MILD REINFORCING. 9. TIE TENDONS TO UPPER

LAYER MAT REINFORCING. 10. MIN. 2 TENDON EACH DIRECTION THRU COLUMN

> 11. COLUMN VERTICALS. 12. SPLICE 2 #5 X 8'-0"

BOTTOM ACROSS TOP OF COLUMN. EXTEND 30" PAST EDGE OF COLUMN. 13. UNIFORMLY DISTRIBUTED

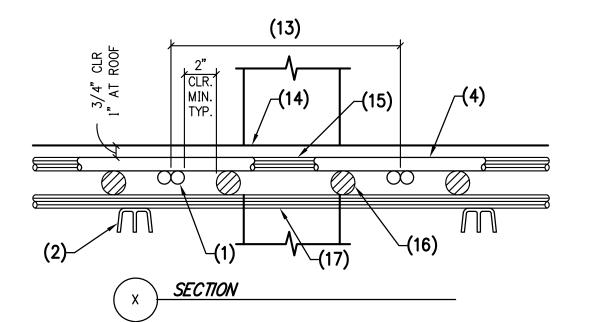
POST-TENSIONED TENDONS. 14. TOP OF CONCRETE SLAB. 15. TOP UPPER REINFORCING

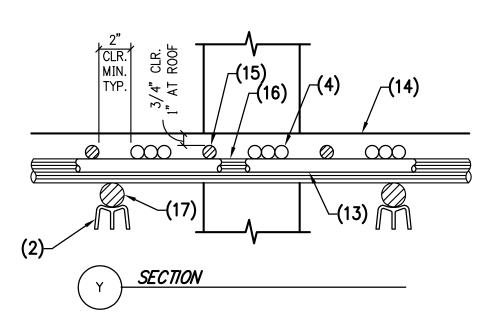
 SEE FRAMING PLAN. TOP LOWER REINFORCING SEE FRAMING PLAN.

17. SUPPORT REINFORCING BAR.

NOTE:

FOR REINFORCING STAGGER, SEE TYPICAL STAGGER DETAILS. FOR NUMBER OF REINFORCING BARS, SEE FRAMING PLAN. ALTERNATE TENDON AND REINFORCING GROUPS





TYPICAL TENDONS AROUND COLUMN

Revisions

NOTICE OF EXTENDED PAYMENT PROVISION (PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT

Symbol



Appr.



Date:

Scale:

Project Code

GLA PROJECT NO. 20109

Designed by:

Drawn by:

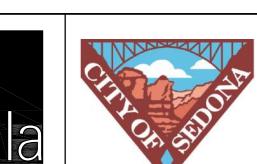
Checked by:

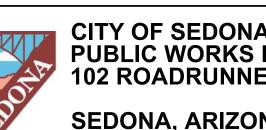
JAppleyard

**EHales** 

# SCHEMATIC DESIGN

NO SCALE





CITY OF SEDONA PUBLIC WORKS DEPARTMENT **102 ROADRUNNER DRIVE SEDONA, ARIZONA 86336** 

Uptown Sedona Parking Facility Schematic Design

SHEET ID S500

gabor lorant architects 3326 n. 3rd avenue suite 200 phoenix, arizona 85013

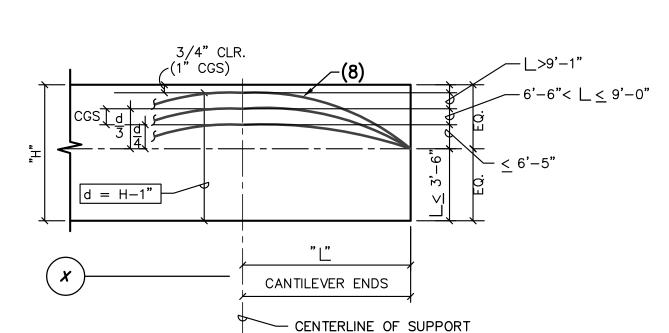
tel: 602.667.9090 fax: 602.667.9133 gaborlorant.com

-TYPICAL TENDON PROFILE FOR POST TENSIONED CONCRETE SLABS

FRAMING DETAILS

SHEET NO.

INTERIOR SPAN END SPAN CANTILEVER SEE BELOW RESTRAINED UNRESTRAINED CENTERLINE~



SECOND POUR

— ALL ANCHORS SHALL BE INSTALLED IN THE CENTER OF SLAB UNLESS OTHERWISE NOTED.

— ANCHORS MUST BE OF A TYPE APPROVED BY THE I.C.B.O. AND THE GOVERNING AGENCIES.

— SEE TENDON PROFILE DETAIL AND PLANS FOR TENDON LOCATION AND REQUIREMENTS.

STRESSING ANCHOR

<del>╸╸╡╶╞╚┋┋╣</del>╟<del>┥╸╺╪╸╸</del>

— AFTER STRESSING, TENDON HARDWARE SHALL BE COATED WITH A RUST PREVENTIVE MASTIC BEFORE CONCRETING OR DRYPACKING.

— A MAXIMUM OF 3" OF EXPOSED STRAND (WITHOUT PLASTIC SHEATH) IS PERMITTED AT ENDS, REPAIR ALL TORN SHEATHS.

<u>TYPICAL TENDON ANCHORAGE DETAIL FOR POST TENSIONED CONCRETE</u>

INTERIOR STRESSING ANCHOR

(ANCHOR AT CONSTRUCTION JOINT)

3. SEE PLANS FOR TENDON REQUIREMENTS. 4. POST TENSIONED CONCRETE

NOTES:

1. COATED TENDON.

3. POCKET FORMER (GROMMET) -

GROUT AFTER STRESSING.

4. SEE TYPICAL REINFORCED EDGE

5. REPAIR ALL EXPOSED TENDONS

6. 2 #4 CONT. BEHIND ALL

EDGES.

NOT USED.

8. ANCHOR CASTING.

BANDED AND UNIFORM

7. NAIL CASTING TIGHT AGAINST

10. BURN OFF 1/2" BEHIND FACE

11. #3 HAIRPINS AT BANDED

OF CONCRETE AFTER STRESS.

TENDONS PER DETAIL 205.

AT 6" O.C. BEHIND BANDED

TENDONS. EXTEND 12" PAST LAST ANCHOR EACH END.

NO SCALE

12. ADD 2 #4 TOP AND BOTTOM

ANCHORS LAP 1'-0" MIN.

CONTINUOUS AT ALL TENDON

W/ SHEATHING WHERE REQUIRED.

5. TOP AND BOTTOM REINFORCING, SEE PLAN FOR SIZE, SPACING,

1. 1/2"ø POST TENSION TENDON. ADDED TENDON MID-HEIGHT.

- AND LENGTH. 6. #4 SUPPORT BAR CONT. W/ MIN. 1'−6" LAP.
- 7. 2 #5 CONT. BEHIND ANCHORS CONT. ALONG ALL EDGES TYPICAL. TENDON DRAPE.
- STRAIGHT TENDON OVER BEAM. 10. SEE FIRE RATED SLAB AREAS ON PLAN AND FIRE RATING COVER REQUIREMENTS ON GSN.

NO SCALE

- TENDONS SHALL FORM A SMOOTH PARABOLIC CURVE BETWEEN CONTROL. POINTS INDICATED. AFTER PLACING HIGH AND LOW POINTS, SUPPORTS AND CHAIRS SHALL BE ADEQUATE TO MAINTAIN TENDON PROFILE.

— ALL TENDONS SHALL BE 1/2" WITH PLASTIC SHEATHS.

FIXED ANCHOR

MAX SPACING OF CHAIRS TO BE 3'-0" - USE SLAB BOLSTERS WHERE C.G.T. IS 1" OR LESS TO BOTTOM OF SLAB. — ADDED TENDONS SHALL MAINTAIN THE SAME PROFILE AS THROUGH TENDONS.

— SEE ANCHOR DETAILS FOR ANCHOR REQUIREMENTS. --- MAXIMUM TENDON SPACING SHALL NOT EXCEED 7 TIMES THE SLAB THICKNESS OR 42" O.C.

— ALL TENDONS ARE TO BE SET FOR MAXIMUM DRAPE, 1" FROM CENTERLINE OF TENDON TO BOTTOM OF SLAB AT MIDSPAN, AND DEPTH MINUS 1" FROM CENTERLINE OF TENDON TO BOTTOM OF SLAB AT SUPPORT TYPICAL UNLESS NOTED OTHERWISE ON FRAMING PLANS. AT END SPANS AND CANTILEVERS, IN OPPOSITE DIRECTION SHOWN, MAXIMUM DRAPE SHALL BE 1" FROM CENTERLINE OF TENDON TO BOTTOM OR TOP OF SLAB, UNLESS AT END

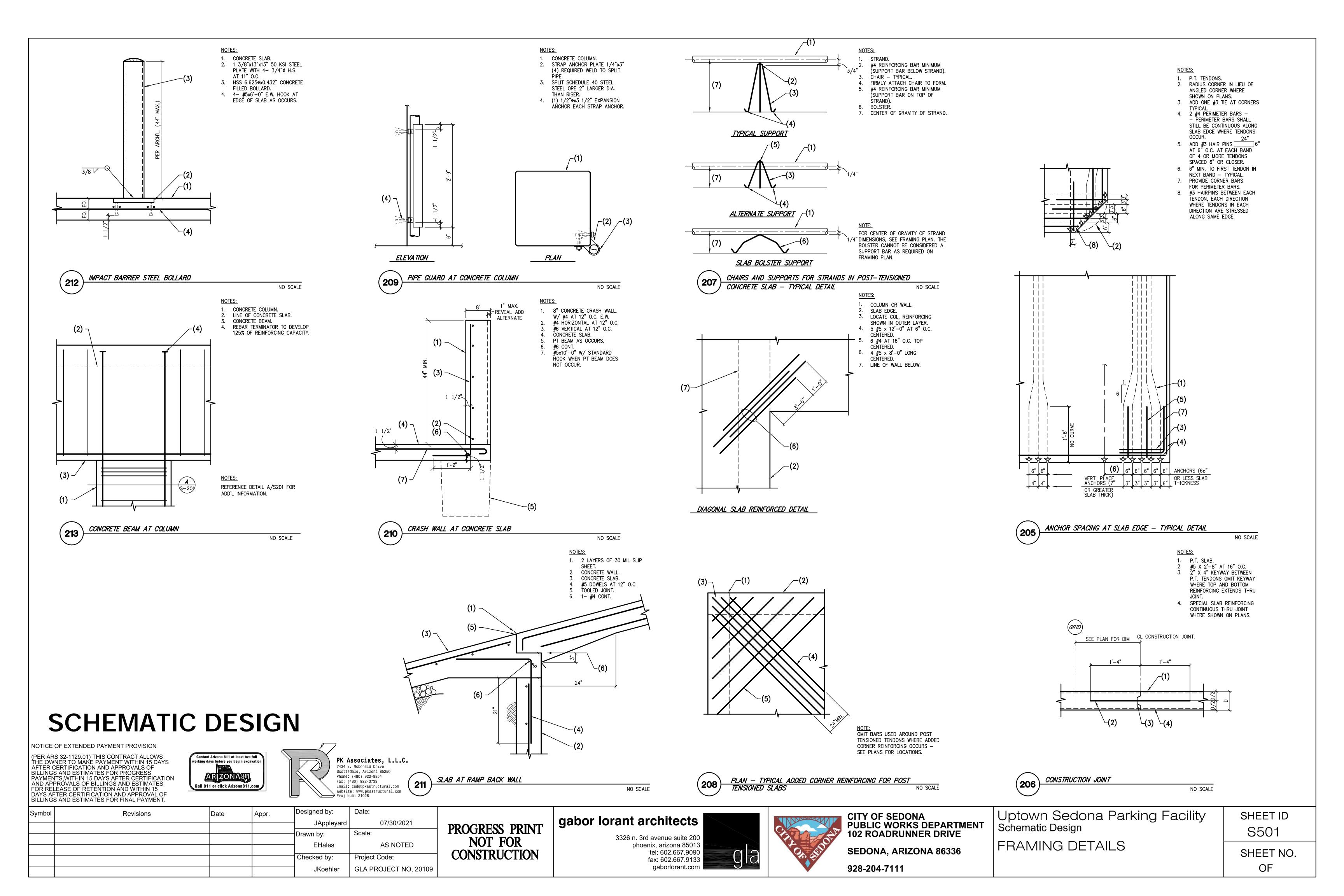
SPAN IN DIRECTION PARALLEL TO TENDON. —FIRST TENDON IN UNIFORM DIRECTION SHALL BE 6" FROM EDGE.

07/30/2021 PROGRESS PRINT NOT FOR AS NOTED

**CONSTRUCTION** 

928-204-7111

# PLAN - TYPICAL TENDON DISPLACEMENT AT SLAB OPENINGS REINFORCING.



# UPTOWN SEDONA PARKING GARAGE

LOCATED IN A PORTION OF SECTION 07, TOWNSHIP 17 NORTH, RANGE 6 EAST, GILA AND SALT RIVER MERIDIAN, COCONINO COUNTY, ARIZONA.

# SR 89A

#### PROJECT SURVEY POINTS

POINT NO.	ELEVATION	NORTHING	EASTING	DESCRIPTION
10216258	4364.07	49521.9120	61030.5320	RB FD. 1/2 IN. W/ PL. CAP MKD. LS 16536
1002	4375.01	49520.3590	60865.5290	RB FD. 1/2 IN. W/ WIRED TAG MKD. FOUND MON. RLS 32224
1001	4379.62	49518.9830	60715.5460	RB FD. 1/2 IN. W/ WIRED TAG MKD. FOUND MON. RLS 32224
10703	4358.45	49347.9800	60716.5360	RB FD 1/2 IN. UP 5 INS. W/ TAG WIRED ON MKD. FNDMON 32224
10701	4355.42	49350.8710	61031.5920	RB FD. 1/2 IN UP 6 INS. NO CAP
10216164	4346.61	49350.9940	61109.3980	OP FD. 3/4 IN. W/ NO MARKS

#### SHEET SET LIST

SHEET NO.	<u>DESCRIPTION</u>	SHEET TITLE
01	CV01	COVER SHEET
02	GN01	GENERAL NOTES AND TYPICAL SECTIONS
03	GD01	GRADING AND DRAINAGE PLAN
04	UT01	UTILITY PLAN
05	RD01	ROADWAY PLAN
06	RD02	ROADWAY PLAN
07	LS01	LANDSCAPE PLAN

#### UTILITY COORDINATION BLOCK

	☐ ARIZONA PUBLIC SERVICE	MATT HERRERA	RECEIVED:	
		COMPANY REPRESENTATIVE CONTACTED		
	☐ ARIZONA WATER COMPANY	CASEY GOFF	RECEIVED:	
		COMPANY REPRESENTATIVE CONTACTED		
	☐ SUDDEN LINK	JASON QUINLAN	RECEIVED:	
		COMPANY REPRESENTATIVE CONTACTED		_
	☐ UNISOURCE	ROB JUSTUS	RECEIVED:	
		COMPANY REPRESENTATIVE CONTACTED		_
	☐ CENTURY LINK	ANDRE HATCHER	RECEIVED:	
		COMPANY REPRESENTATIVE CONTACTED		_
	☐ SEDONA SEWER	ANDY DICKEY	RECEIVED:	
		COMPANY REPRESENTATIVE CONTACTED	-	_
-				

# FOREST ROAD

#### CONTACT INFORMATION

OWNER CITY OF SEDONA

MAYOR

ENGINEER

SANDY MORIARTY VICE MAYOR SCOTT JABLOW

HOLLI PLOOG JON THOMPSON TOM LAMKIN JESSICA WILLIAMSON

CITY MANAGER KAREN OSBURN

KIMLEY-HORN & ASSOCIATES, INC. REBECCA FIELDS, P.L.A. 333 E WETMORE RD, SUITE 280 TUCSON, AZ 85705 (520)-352-8657 REBECA.FIELD@KIMLEY-HORN.COM

KIMLEY-HORN & ASSOCIATES, INC. ANDREW, BAIRD, P.E 201 W MONTEZUMA ST, SUITE 206 PRESCOTT, AZ 86301 (928) 458-7121 ANDREW.BAIRD@KIMLEY-HORN.COM

#### RECORD DRAWING STATEMENT

HEREBY STATE, BASED ON MY FIELD OBSERVATION AND INFORMATION PROVIDED BY THE GENERAL CONTRACTOR AND OTHERS, THAT THE WORK ON SHEETS 1 THROUGH 05, MARKED AS "RECORD DRAWING" HAS BEEN CONSTRUCTED IN SUBSTANTIAL CONFORMANCE WITH THESE CONSTRUCTION PLANS, SPECIFICATIONS, INCLUDING CHANGES AND REVISIONS.

REGISTERED LAND SURVEYOR/ENGINEER

DATE

REGISTRATION NUMBER

EXPIRATION DATE



NOTICE OF EXTENDED PAYMENT PROVISION

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT.

# SCHEMATIC DESIGN

Symbol	Revisions	Date	Appr.	Designed by: DLP	Date: 07/28/2021
				Drawn by:	Scale:
				ALD	NA
				Checked by:	Project Code:
				JRW	091885001

PRELIMINARY 30% FOR REVIEW ONLY NOT FOR CONSTRUCTION **Kimley** »Horn PE NO.<u>48841</u> DATE<u>07/28</u>



PHONE: 928-458-7121



CITY OF SEDONA
PUBLIC WORKS DEPARTMENT **102 ROADRUNNER DRIVE SEDONA, ARIZONA 86336** 

928-204-7111

**UPTOWN SEDONA** PARKING GARAGE

**COVER SHEET** 

SHEET ID CV01

SHEET NO. 01 OF 07

#### GENERAL NOTES:

- 1. ALL STRUCTURES ARE DESIGNED TO ACT AS A STRUCTURAL UNIT UPON COMPLETION. CONTRACTOR SHALL DESIGN AND PROVIDE NECESSARY BRACING, TEMPORARY SUPPORTS, AND SHORING TO RESIST FORCES ON THE STRUCTURE DURING CONSTRUCTION.
- 2. VERIFY ALL DIMENSIONS AND ELEVATIONS PRIOR TO STARTING WORK. NOTIFY THE ENGINEER OF ANY DISCREPANCIES.
- 3. VERIFY LOCATIONS OF ALL EXISTING UTILITIES PRIOR TO STARTING WORK.
- 4. CONTRACTOR SHALL EXERCISE EXTREME CARE DURING THE EXCAVATION AND CONSTRUCTION FOR NEW STRUCTURE TO AVOID DAMAGE TO EXISTING STRUCTURES AND EXISTING UTILITIES. CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL MEANS AND METHODS REQUIRED TO FACILITATE CONSTRUCTION OF THE WORK AND ENSURING THE SAFETY, STABILITY AND INTEGRITY OF ADJACENT STRUCTURES AND FACILITIES.
- 5. THE ENGINEER SHALL BE NOTIFIED A MINIMUM OF 24 HOURS PRIOR TO BEGINNING ANY CONSTRUCTION.
- 6. ANY WORK PERFORMED WITHOUT THE KNOWLEDGE AND APPROVAL BY THE ENGINEER AND/OR ALL WORK MATERIAL NOT IN CONFORMANCE WITH THE PLANS AND SPECIFICATIONS IS SUBJECT TO REMOVAL AND REPLACEMENT AT THE CONTRACTOR'S EXPENSE.
- 7. NO JOB WILL BE CONSIDERED COMPLETE UNTIL ALL CURBS, PAVEMENT AND SIDEWALKS (NEW AND EXISTING) HAVE BEEN SWEPT CLEAN OF ALL DIRT AND DEBRIS.
- 8. ALL QUANTITIES SHOWN ON PLANS ARE APPROXIMATE, ARE NOT VERIFIED BY THE ENGINEER, AND ARE FURNISHED SOLELY FOR THE CONTRACTOR'S CONVENIENCE. THEY DO NOT NECESSARILY CORRESPOND TO BID SCHEDULE ITEMS. PAYMENT WILL BE BASED ON BID SCHEDULE ITEMS. THE CONTRACTOR SHALL NOT BE RELIEVED OF HIS RESPONSIBILITY FOR INDEPENDENTLY ESTIMATING WORK QUANTITIES PRIOR TO BIDDING.
- 9. BACKFILL COMPACTION SHALL BE PER MAG 301, UNLESS OTHERWISE NOTED. SUBGRADE PREPARATION SHALL MEET THE PROJECT STANDARD SPECIFICATION SECTION 2600.
- 10. REMOVAL OF STRUCTURES AND OBSTRUCTIONS AS NECESSARY TO COMPLETE THE WORK, OTHER THAN SPECIALLY SCHEDULED IN THE BID, IS INCIDENTAL TO THE CONTRACT. NO SEPARATE MEASUREMENT OF PAYMENT FOR UNSCHEDULED REMOVAL ITEMS WILL BE MADE.
- 11. CONSTRUCTION STAKING SHALL BE BY THE CONTRACTOR'S SURVEYOR WITH CONTROL PROVIDED BY THE DESIGN ENGINEER WHO STAMPED THE PLANS.
- 12. THE CITY OF SEDONA MAY ORDER ANY OR ALL WORKMANSHIP AND MATERIALS TO BE TESTED ACCORDING TO APPLICABLE STANDARDS.
- 13. THE CONTRACTOR WILL BE RESPONSIBLE FOR ALL REWORK AND/OR REMOVAL AND REPLACEMENT OF ALL MATERIALS AND/OR WORKMANSHIP REPRESENTED BY A FAILING TEST.
- 14. THE CONTRACTOR WILL BE RESPONSIBLE FOR ALL COSTS OF TESTING AND QUALITY ASSURANCE/QUALITY CONTROL AS DELINEATED IN THE CITY'S PROJECT SPECIFICATIONS. THE COST OF TESTING IS INCIDENTAL TO EACH ITEM OF WORK. THE CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR THE COST OF ANY CITY INSPECTION AND CITY INSPECTION TIME IF THE CONTRACTOR'S WORK IS BEING PERFORMED IN OVERTIME, AT NIGHT, OR ON WEEKENDS.
- 15. APPROVAL OF A PORTION OF THE WORK IN PROGRESS DOES NOT GUARANTEE ITS FINAL ACCEPTANCE. TESTING AND EVALUATION MAY CONTINUE UNTIL WRITTEN FINAL ACCEPTANCE OF A COMPLETE AND WORKABLE UNIT.
- 16. THE CITY OF SEDONA MAY SUSPEND THE WORK BY WRITTEN NOTICE WHEN, IN ITS JUDGEMENT, PROGRESS IS UNSATISFACTORY, WORK BEING DONE IS UNAUTHORIZED OR DEFECTIVE, WEATHER CONDITIONS ARE UNSUITABLE, OR THERE IS A DANGER TO THE PUBLIC HEALTH OR SAFETY.
- 17. CLEARING AND GRUBBING IS CONSIDERED INCIDENTAL TO THE WORK UNLESS SEPARATELY IDENTIFIED IN THE BID SCHEDULE. NO SEPARATE MEASUREMENT OF OR PAYMENT FOR CLEARING, GRUBBING, AND TREE REMOVAL WILL BE MADE. THE SITE SHALL BE REMOVED AND DISPOSED OF ALL EXCAVATION AND EMBANKMENTS. FILLS SHALL FIRST BE CLEARED OF STUMPS, TRASH, WEEDS, RUBBISH, AND LOOSE BOULDERS. THE CONTRACTOR MUST SATISFY HIMSELF REGARDING THE CHARACTER AND AMOUNT OF LOAM, CLAY, SAND, QUICKSAND, HARDPAN, GRAVEL, ROCK, WATER, AND ALL OTHER MATERIAL TO BE ENCOUNTERED AND WORK TO BE PERFORMED.
- 18. THE LOCATIONS OF ALL EXISTING UTILITIES SHOWN ARE APPROXIMATE AND ARE BASED ON FIELD DATA AND MAP RECORDS. THE CONTRACTOR SHALL CONTACT 1—800—STAKE—IT PRIOR TO ANY CONSTRUCTION ACTIVITY TO VERIFY THE ACTUAL LOCATION OF ALL UTILITIES. THE CONTRACTOR SHALL DETERMINE WHICH UTILITIES DO NOT PARTICIPATE IN 1—800—STAKE—IT AND CONTACT THEM DIRECTLY TO VERIFY THE LOCATION OF THOSE UTILITIES. ANY DAMAGE TO EXISTING UTILITIES CAUSED BY CONTRACTOR'S OPERATION SHALL BE REPORTED TO THE UTILITY OWNER IMMEDIATELY AND REPAIRED OR REPLACED AT NO COST TO THE CITY. IN CASES WHEN THE EXISTING UTILITIES ARE NOT AS DEPICTED ON THE PLANS SOME MINOR DEVIATION TO THE PROPOSED ALIGNMENT MAY BE ALLOWED TO MAINTAIN MINIMUM SEPARATION DISTANCES BETWEEN UTILITIES. ANY PROPOSED TO MAINTAIN MINIMUM SEPARATION DISTANCES BETWEEN UTILITIES. ANY PROPOSED TO CHANGES TO THE ALIGNMENT MUST BE SUBMITTED TO THE CITY'S REPRESENTATIVE FOR REVIEW. NO CHANGES WILL BE ALLOWED WITHOUT PRIOR APPROVAL.
- 19. CONTRACTOR SHALL OBTAIN ANY ADDITIONAL TEMPORARY EASEMENTS OR USE AGREEMENTS THAT ARE DEEMED NECESSARY FOR CONSTRUCTION AT NO ADDITIONAL COST TO THE CITY. COPIES OF ALL CONTRACTOR OBTAINED EASEMENTS AND USE AGREEMENTS SHALL BE PROVIDED TO THE CITY'S REPRESENTATIVE PRIOR TO THE UTILIZATION OF THE SITE.
- 20. THE CONTRACTOR SHALL GRADE AND RESURFACE ALL AREAS DISTURBED BY CONSTRUCTION, INCLUDING LANDSCAPE ROCK, IN ACCORDANCE WITH THE SPECIFICATIONS AND TO A CONDITION EQUAL TO, OR BETTER THAN, THE PRE—CONSTRUCTION CONDITION.
- 21. THE CONTRACTOR SHALL PROTECT ALL CONCRETE STRUCTURES TO REMAIN. ALL CONCRETE REPLACEMENT SHALL BE FROM JOINT TO JOINT (WALLS, SIDEWALK) AND SHALL BE REPLACED WITH 4000 PSI CONCRETE. ALL DAMAGED CONCRETE PANELS MUST BE REPLACED AND SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

#### GENERAL NOTES CONTINUED:

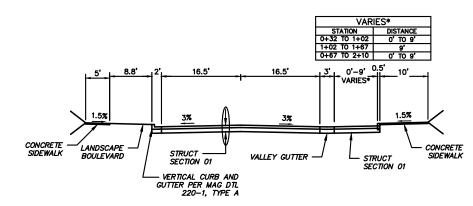
- 22. THE CONTRACTOR SHALL PROVIDE PROTECTION TO PREVENT UNDERMINING OR DAMAGING THE STRUCTURAL INTEGRITY OF ALL FENCES, RETAINING WALLS, STREET SIGNS, OTHER UTILITY POLES, OR OTHER PRIVATE OR PUBLIC IMPROVEMENTS WITHIN THE PROJECT AREA. THE CONTRACTOR SHALL MAKE ARRANGEMENTS WITH THE OWNING UTILITY AS NECESSARY TO PROVIDE TEMPORARY SUPPORT, OR PROTECTION DURING CONSTRUCTION WORK, AND SHALL NEATLY REMOVE AND PROMPTLY REPLACE NON UTILITY IMPROVEMENTS WITHOUT UNDUE DISRUPTION. THE COST OF ALL SUCH PROTECTION, REMOVAL, AND REPLACEMENT REQUIRED TO COMPLETE THE PROJECT SHALL BE SUBSIDIARY TO OTHER BID ITEMS.
- 23. THE CONTRACTOR SHALL REMOVE ALL FENCING, ASPHALT AND CONCRETE ROADS AND DRIVEWAYS, CURB AND GUTTER, RIP—RAP, DRAINAGE CULVERTS AND ASSOCIATED APPURTENANCES AS REQUIRED FOR CONSTRUCTION PURPOSES. ALL ITEMS DAMAGED OR REMOVED SHALL BE RESTORED IN ACCORDANCE WITH THE SPECIFICATION TO A CONDITION EQUAL TO, OR BETTER THAN, THEIR CONDITION PRIOR TO THE START OF THE PROJECT. ITEMS OF WORK NOT SPECIFICALLY INCLUDED IN THE MEASUREMENTS AND PAYMENT SECTION OF THE SPECIFICATIONS SHALL BE CONSIDERED SUBSIDIARY TO OTHER BID ITEMS AND SHALL NOT BE PAID FOR SEPARATELY.
- 24. IT IS NOT THE INTENTION OF THE NOTES TO SUPERSEDE ANY FEDERAL, STATE OR LOCAL LAWS, REGULATIONS AND/OR ORDINANCES; THEY SHALL GOVERN IN ALL INSTANCES. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SHOW A GOOD FAITH EFFORT AND TO PROTECT ALL EXISTING UTILITY TIES AND STRUCTURES AND TO ABIDE BY ALL FEDERAL, STATE AND LOCAL LAWS AND ORDINANCES IN THIS RESPECT.
- 25. THE CONTRACTOR SHALL PROTECT ALL EXISTING IMPROVEMENTS ON PRIVATE PROPERTY. ALL ITEMS DAMAGED OR REMOVED SHALL BE RESTORED IN ACCORDANCE WITH THE SPECIFICATION TO A CONDITION EQUAL TO. OR BETTER THAN. THEIR CONDITION PRIOR TO THE START OF THE PROJECT.
- 26. PROPERTY LINES SHOWN ON DRAWINGS ARE APPROXIMATE
- 27. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT PLANS AND SPECIFICATIONS AS WELL AS THE LATEST ADDITION OF MAG STANDARDS AND DETAILS AND THE PROJECT SPECIFICATIONS.
- 28. ALL WORK SHALL BE PREFORMED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT PREPARED BY XXXXXXXXXXXXX DATED XXXX XX. XXXX.

#### ACRONYM/ABBREVIATION

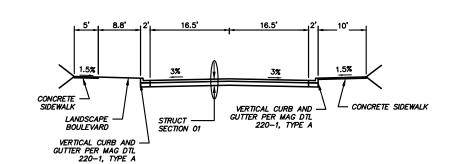
AGGREGATE BASE	Н	HEIGHT	RT	RIGHT
AGGREGATE BASE COARSE	INV	INVERT	R	RADIUS
ASPHALTIC CONCRETE	LI	LANDSCAPE ISLAND	R/W	RIGHT-OF-WAY
BEGIN PROJECT	LT	LEFT	, RDWY	ROADWAY
BOTTOM OF WALL	LF	LINEAR FEET	SHT	SHEET
CONCRETE	M.A.G.	MARICOPA ASSOCIATION	SM	SURVEY MARKER
CENTERLINE		OF GOVERNMENTS	SS	SANITARY SEWER
CONSTRUCTION	MH	MANHOLE	ST	STREET
COMMUNICATION	MIN	MINIMUM	STA	STATION
DETAIL	Ν	NORTHING	STD	STANDARD
DRIVE PATH	NO	NUMBER	STRUCT	STRUCTURAL
EASTING	Р	PAVEMENT	SW	SIDEWALK
EACH	PC	POINT OF CURBE	TBC	TOP OF CURB
ELEVATION	PCC	PORTLAND CEMENT CONCRETE	TELE	TELEPHONE
END PROJECT	PI	POINT OF INTERSECTION	TEMP	TEMPORARY
EXISTING	POB	POINT OF BEGINNING	TOP	TOP OF PIPE
FACE OF CURB	POE	POINT OF ENDING	TW	TOP OF WALL
FIRE HYDRANT	PT	POINT OF TANGET	TSW	TOP OF SEAT WALL
FLOW LINE	QC	QUAD CITY	TYP	TYPICAL
FEET				
	AGGREGATE BASE COARSE ASPHALTIC CONCRETE BEGIN PROJECT BOTTOM OF WALL CONCRETE CENTERLINE CONSTRUCTION COMMUNICATION DETAIL DRIVE PATH EASTING EACH ELEVATION END PROJECT EXISTING FACE OF CURB FIRE HYDRANT FLOW LINE	AGGREGATE BASE COARSE  ASPHALTIC CONCRETE  BEGIN PROJECT  BOTTOM OF WALL  CONCRETE  CONCRETE  CONSTRUCTION  COMMUNICATION  DETAIL  DRIVE PATH  EASTING  EACH  ELEVATION  END PROJECT  EXISTING  FACE OF CURB  FIRE HYDRANT  FLOW LINE  INV  ASPHALTINE  LI  BRIVE  LI  BRIVE  M.A.G.  MH  M.A.G.  MH  NO  PC  MH  NO  PC  PC  PC  PC  PC  PC  PC  POB  POB	AGGREGATE BASE COARSE  ASPHALTIC CONCRETE  BEGIN PROJECT  BOTTOM OF WALL  CONCRETE  CONCRETE  CONSTRUCTION  COMMUNICATION  DETAIL  DRIVE PATH  EASTING  EACH  ELEVATION  EXISTING  FACE OF CURB  FIRE HYDRANT  FLOW LINE  ASPHALTIC CONCRETE  LI  LANDSCAPE ISLAND  INVERT  LANDSCAPE ISLAND  ASSOCIATION  OF GOVERNMENTS  OF GOVERNME	AGGREGATE BASE COARSE  ASPHALTIC CONCRETE  LI  LANDSCAPE ISLAND  R/W  BEGIN PROJECT  LT  LEFT  RDWY  BOTTOM OF WALL  CONCRETE  M.A.G.  MARICOPA ASSOCIATION  SM  CENTERLINE  CONSTRUCTION  MH  MANHOLE  ST  COMMUNICATION  MIN  MINIMUM  STA  DETAIL  N  NORTHING  STD  DRIVE PATH  NO  NUMBER  EACH  PC  POINT OF CURBE  TBC  ELEVATION  PCC  PORTLAND CEMENT CONCRETE  END PROJECT  END PROJECT  END POB  POB  POINT OF INTERSECTION  TEMP  EXISTING  POB  POB  POINT OF ENDING  TW  FIRE HYDRANT  PT  POINT OF TANGET  TSW  FLOW LINE

#### LEGEND:

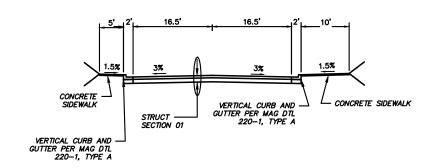
Ем	EXISTING ELECTRIC METER	5520	EXISTING CONTOUR
ER	EXISTING ELECTRIC RISER	<del></del> 5520 <del></del>	PROPOSED CONTOUR
IV	EXISTING IRRIGATION VALVE		EXISTING STORM DRAIN
	EXISTING WATER METER		
WR	EXISTING WATER RISER		PROPOSED STORM DRAIN
$\otimes$	EXISTING WATER VALVE		LIMITS OF DISTURBANCE
	EXISTING FIRE HYDRANT	s	FLOW LINE EXISTING SANITARY SEWER LINE
SS	EXISTING SANITARY SEWER MANHOLE		EXISTING COMMUNICATION LINE
	EXISTING STORM DRAIN INLET	G	EXISTING GAS LINE
SD	EXISTING STORM DRAIN MANHOLE	——— FO ———	EXISTING FIBER OPTIC LINE
ø	EXISTING ELECTRIC POLE		EXISTING BUILDING OVERHANG
*	EXISTING LIGHT POST		PROPERTY LINE, R.O.W.  EXISTING WATER LINE
4	EXISTING SIGN		EXISTING CONCRETE
TR	EXISTING TELEPHONE RISER	•	
	SURVEY BENCHMARK	√	REMOVE CONCRETE SIDEWALK
-		√ √ √ √ √ √ √ √ √ √ √ √ √ √ √ √ √ √ √	PROPOSED CONCRETE



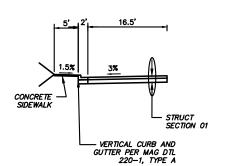
FOREST RD STA 0+32 TO STA 2+10



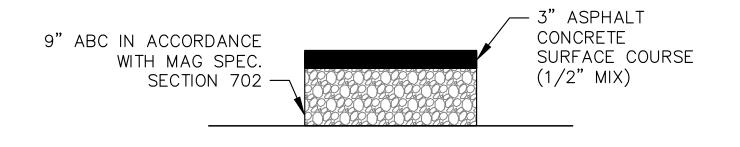
FOREST RD STA 2+10 TO STA 2+70



FOREST RD STA 3+10 TO STA 4+47



FOREST RD STA 4+47 TO STA 8+73



STRUCTURAL SECTION 01

TOTAL THICKNESS = 12"

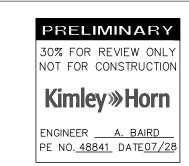


#### **SCHEMATIC DESIGN**

NOTICE OF EXTENDED PAYMENT PROVISION

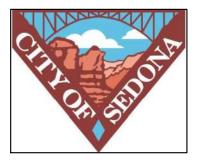
(PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT.

e: 07/28/2021
07/28/2021
le:
NA
ject Code:
091885001
le: jed





© 2020 KIMLEY-HORN AND ASSOCIATES, INC.
201 NORTH MONTEZUMA SUITE 206 PRESCOTT ARIZONA, 86301
PHONE: 928-458-7121
WWW.KIMLEY-HORN.COM



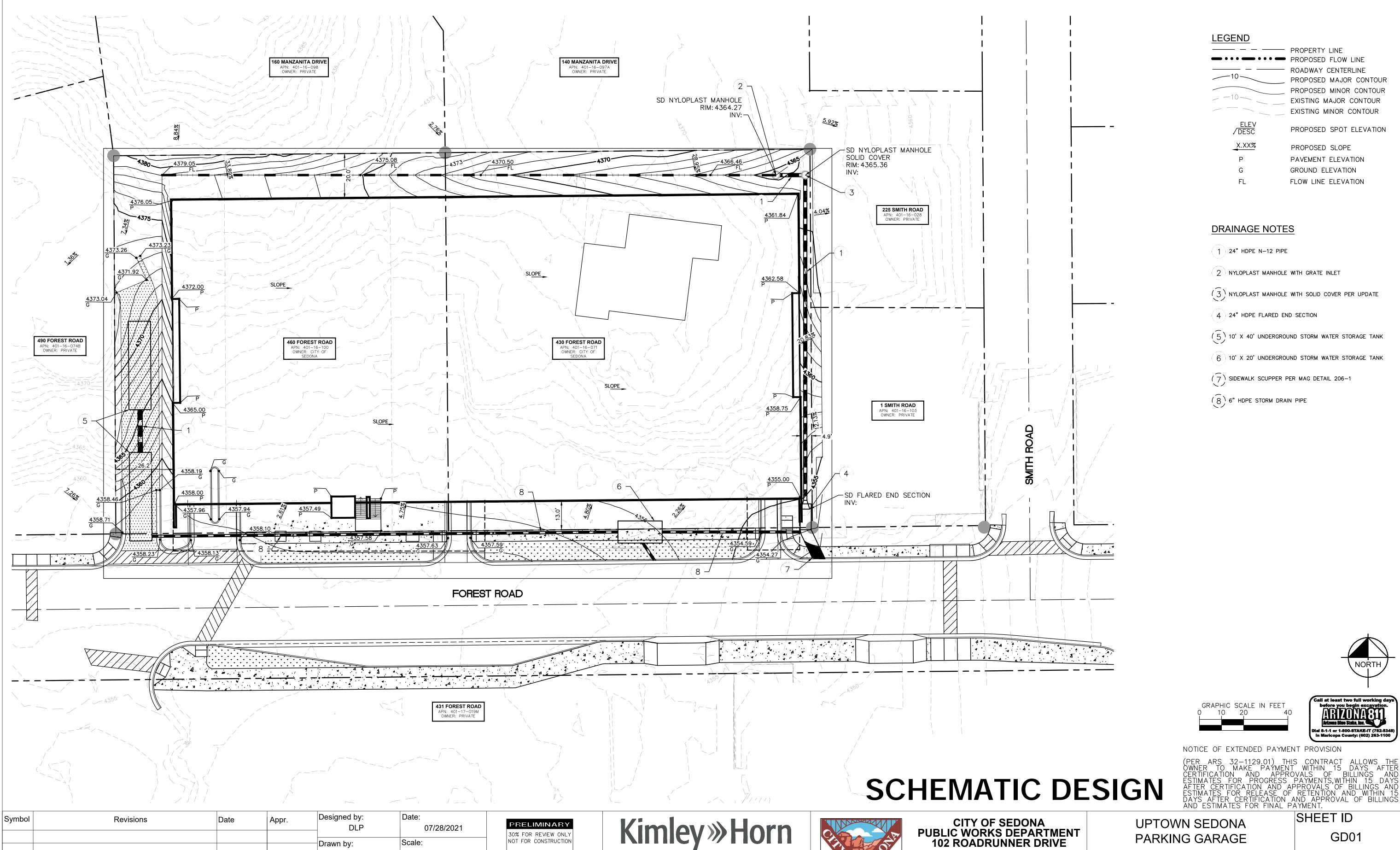
CITY OF SEDONA
PUBLIC WORKS DEPARTMENT
102 ROADRUNNER DRIVE

SEDONA, ARIZONA 86336 928-204-7111 UPTOWN SEDONA PARKING GARAGE GENERAL NOTES AND

TYPICAL SECTIONS

SHEET ID GN01 SHEET NO.

02 OF 07



© 2020 KIMLEY-HORN AND ASSOCIATES, INC.

201 NORTH MONTEZUMA SUITE 206 PRESCOTT ARIZONA, 86301

PHONE: 928-458-7121

WWW.KIMLEY-HORN.COM

SHEET NO.

03 OF 07

GRADING AND DRAINAGE

**PLAN** 

**SEDONA, ARIZONA 86336** 

928-204-7111

NOT FOR CONSTRUCTION

**Kimley** »Horn

ENGINEER A. BAIRD

PE NO. 48841 DATE 07/28

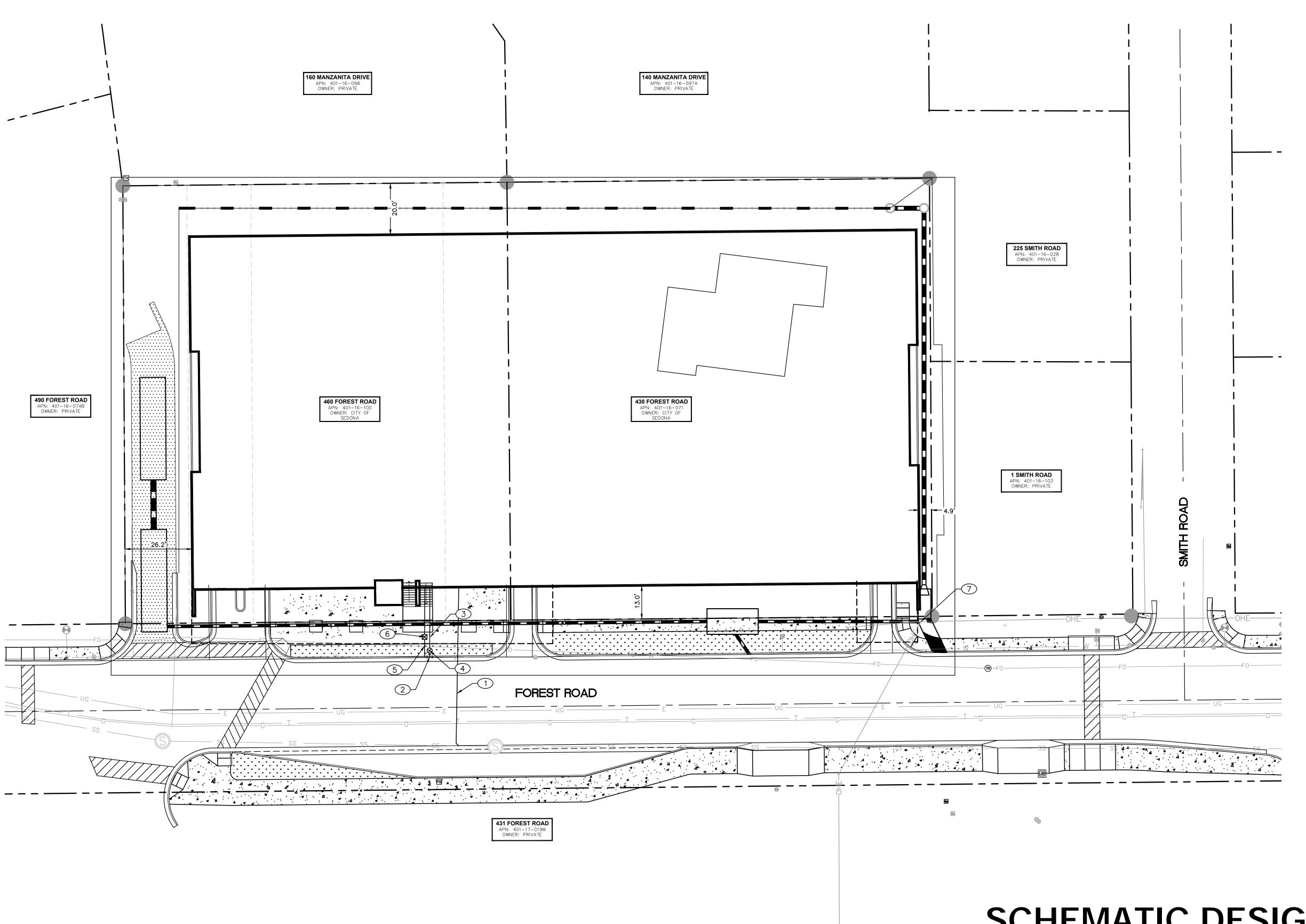
HORIZ. 1"=20'

091885001

**Project Code:** 

Drawn by:

Checked by:



#### **UTILITY NOTES**

- 4" SANITARY SEWER SERVICE CONNECTION PER MAG STD DTL 440-1.
- 2 CONNECT TO EXISTING WATER MAIN.
- 6" CLASS 350 DIP FIRE LINE PER ARIZONA WATER COMPANY REQUIREMENTS.
- 6" VALVE BOX AND COVER PER ARIZONA WATER COMPANY REQUIREMENTS.
- 1" WATER SERVICE CONNECTION PER ARIZONA WATER COMPANY REQUIREMENTS.
- 6 1" WATER METER AND BOX PER ARIZONA WATER COMPANY REQUIREMENTS.
- RELOCATE EXISTING POWER POLE. COORDINATE WITH UTILITY OWNER.







NOTICE OF EXTENDED PAYMENT PROVISION (PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT.

# SCHEMATIC DESIGN

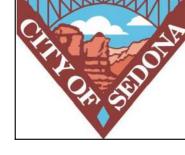
Symbol	Revisions	Date	Appr.	Designed by: DLP	Date: 07/28/2021
					3172872821
				Drawn by:	Scale:
				ALD	HORIZ. 1"=20'
				ALD	1101(12. 1 -20
				Checked by:	Project Code:
				ID)A/	004005004
				JRW	091885001

PRELIMINARY 30% FOR REVIEW ONLY NOT FOR CONSTRUCTION **Kimley** »Horn ENGINEER <u>A. BAIRD</u> PE NO. 48841 DATE 07/28



PHONE: 928-458-7121

WWW.KIMLEY-HORN.COM



CITY OF SEDONA
PUBLIC WORKS DEPARTMENT
102 ROADRUNNER DRIVE

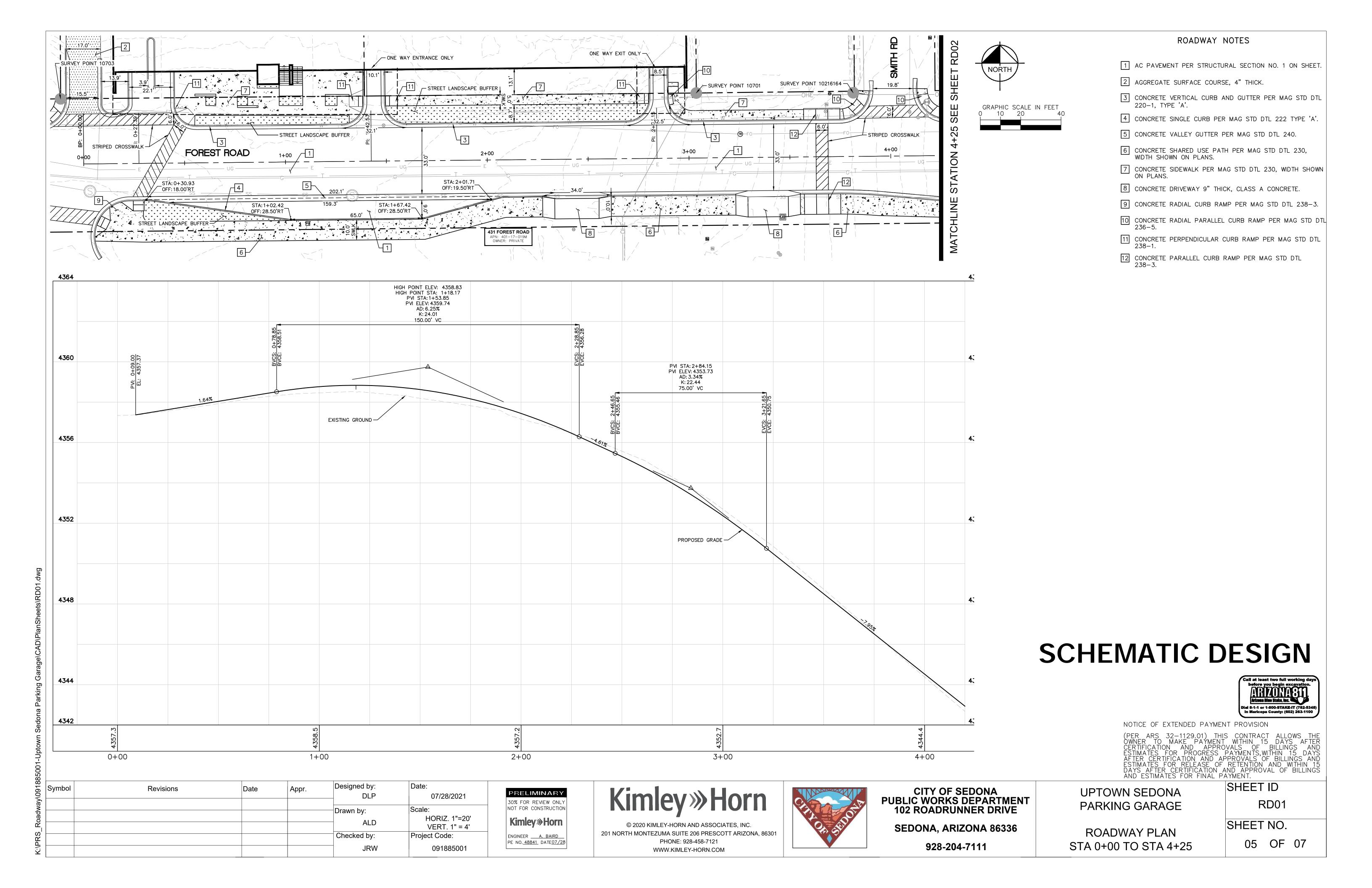
**SEDONA, ARIZONA 86336** 928-204-7111

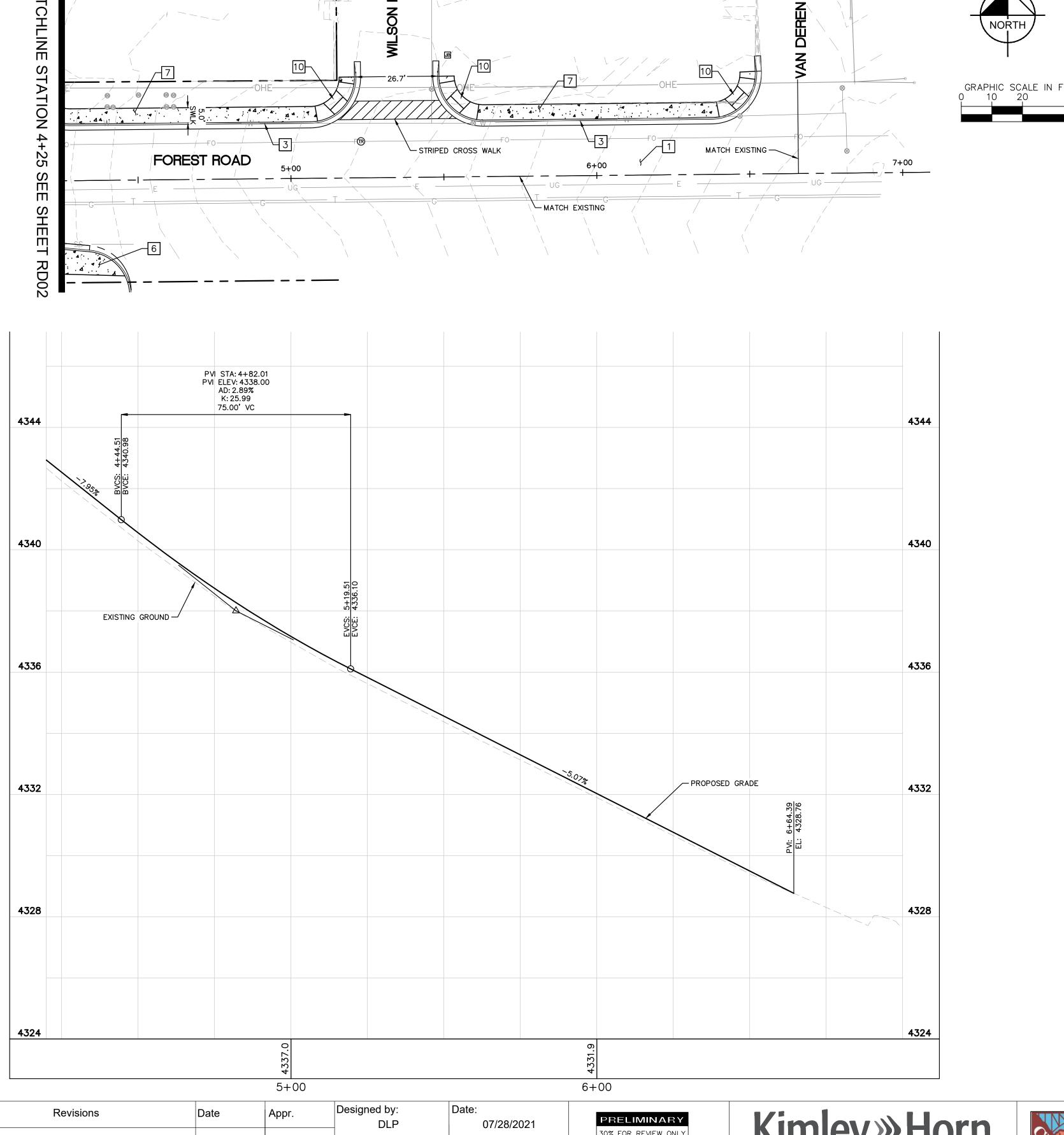
**UPTOWN SEDONA** PARKING GARAGE

**UTILITY PLAN** 

SHEET ID UT01

SHEET NO. 04 OF 07





#### ROADWAY NOTES

- 1 AC PAVEMENT PER STRUCTURAL SECTION NO. 1 ON SHEET.
- 3 CONCRETE VERTICAL CURB AND GUTTER PER MAG STD DTL 220-1, TYPE 'A'.
- 6 CONCRETE SHARED USE PATH PER MAG STD DTL 230, WIDTH SHOWN ON PLANS.
- 7 CONCRETE SIDEWALK PER MAG STD DTL 230, WIDTH SHOWN ON PLANS.
- 10 CONCRETE RADIAL PARALLEL CURB RAMP PER MAG STD DTL 236-5.



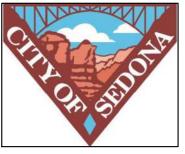
(PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT. SCHEMATIC DESIGN

$\geq$	Symbol	Revisions	Date	Appr.	Designed by: DLP	Date: 07/28/2021
_коадwау					Drawn by:	Scale: HORIZ. 1"=20' VERT. 1"=4'
ひといろ					Checked by: JRW	Project Code: 091885001

PRELIN	MINARY
30% FOR R NOT FOR CO	EVIEW ONLY DNSTRUCTION
Kimlev	»Horn
,	,
ENGINEER	A. BAIRD 1 DATE <u>07/28</u>
PE NO. 4884	<u>1</u> DATE <u>07/28</u>

# © 2020 KIMLEY-HORN AND ASSOCIATES, INC.

201 NORTH MONTEZUMA SUITE 206 PRESCOTT ARIZONA, 86301 PHONE: 928-458-7121 WWW.KIMLEY-HORN.COM

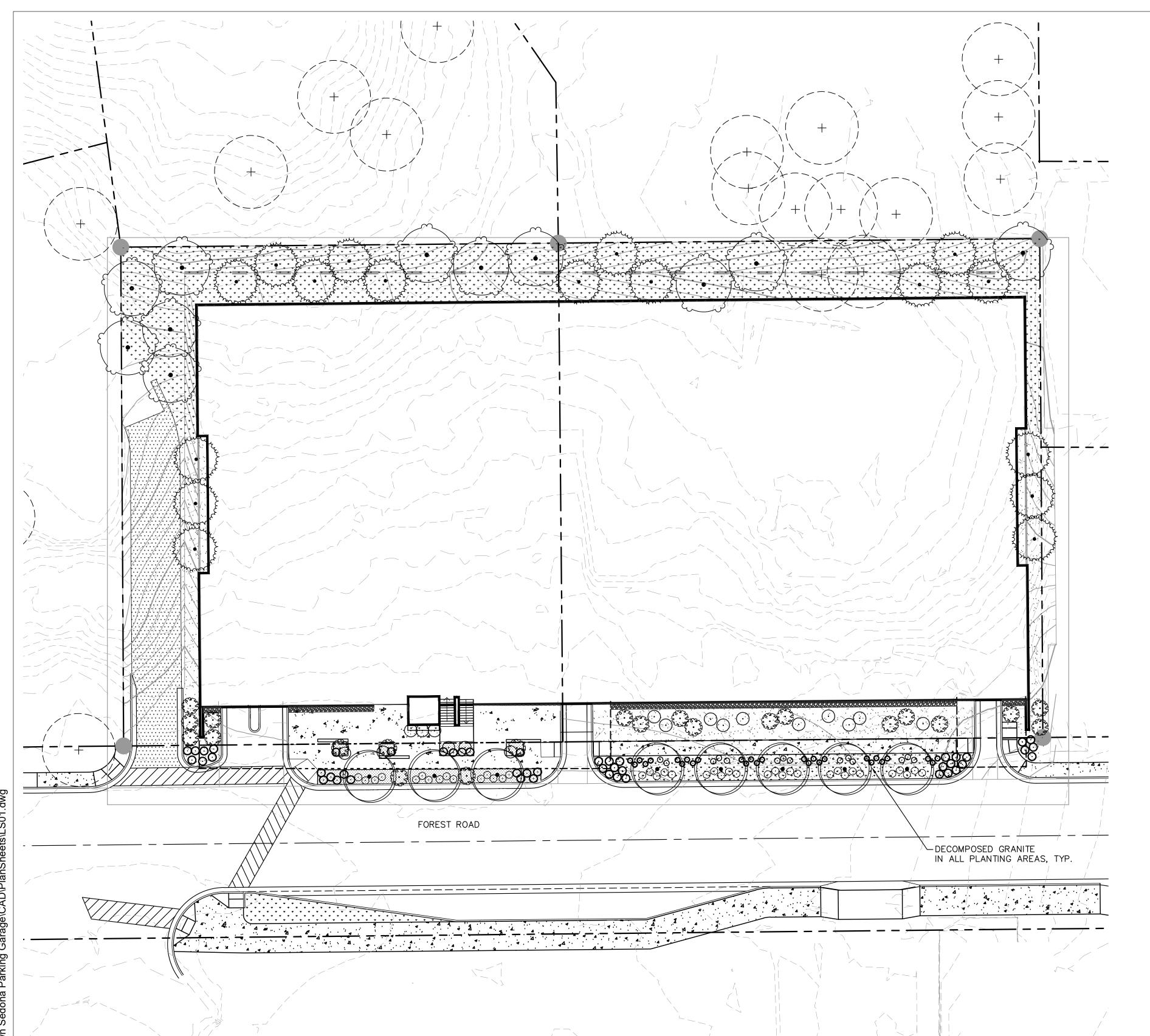


CITY OF SEDONA
PUBLIC WORKS DEPARTMENT
102 ROADRUNNER DRIVE

**SEDONA, ARIZONA 86336** 928-204-7111

**UPTOWN SEDONA** PARKING GARAGE **ROADWAY PLAN** STA 4+25 TO 7+00

SHEET ID RD02 SHEET NO. 06 OF 07



#### PLANT LEGEND

DECIDUO	US TREES		
SYMBOL	BOTANICAL NAME COMMON NAME	QTY.	SIZE
	CHILOPSIS LINEARIS DESERT WILLOW	28	15 GAI
EVERGRE	EEN TREES		
SYMBOL	Botanical Name Common Name	Qty.	Size
January Color	JUNIPERUS ARIZONICA REDBERRY JUNIPER	6	15 Gal
•	JUNIPERUS SCOPULORUM ROCKY MOUNTAIN JUNIPER	6	15 Gal
SHRUBS/	GROUNDCOVERS		
SYMBOL	Botanical Name Common Name	Qty.	Qty.
0	Asclepias tuberosa Butterfly Milkweed	89	1 Gal.
0	Bouteloua gracilis 'Blonde Ambition' Blue Grama	231	1 Gal.
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Chamaebatiaria millefolium 'Fernbush' Fernbush	89	5 Gal.
⊙	Ericamerica Iaricifolia Turpentine Bush	138	1 Gal.
•	Melampodium leucanthum Blackfoot Daisy	174	1 Gal.
$\odot$	Nolina microcarpa Bear Grass	111	5 Gal.
OJTE			

#### LEGEND

— PROPERTY LINE — PROPOSED FLOW LINE ROADWAY CENTERLINE PROPOSED MAJOR CONTOUR PROPOSED MINOR CONTOUR EXISTING MAJOR CONTOUR EXISTING MINOR CONTOUR

#### LANDSCAPE NOTES

1"-3" Rock Mulch, Color TBD

- 1. ALL LANDSCAPING WILL BE IRRIGATED.
- 2. ALL PUBLIC LANDSCAPING AND IRRIGATION WILL COMPLY WITH CITY OF SEDONA STANDARDS AND SPECIFICATIONS.
- 3. MINOR ADJUSTMENTS TO THE LANDSCAPE MATERIAL AND LOCATIONS MAY BE PROPOSED FOR CITY CONSIDERATION OF APPROVAL AT THE CONSTRUCTION DOCUMENT STAGE TO RESPOND TO MARKET AND FIELD CONDITIONS. HOWEVER, THE NUMBER AND GENERAL SIZE OF MATERIALS SHALL REMAIN

16,928sf



SYMBOL





SCHEMATIC DESIGN

$\tilde{\infty}$							
\0918	Symbol	Revisions	Date	Appr.	Designed by: DLP	Date: 07/28/2021	
аŚ						Scale:	
Roadw					ALD	HORIZ. 1"=20'	
SS F						Project Code:	
\ P					JRW	091885001	

PRELIMINARY 30% FOR REVIEW ONLY NOT FOR CONSTRUCTION **Kimley** »Horn ENGINEER <u>A. BAIRD</u> PE NO. <u>48841</u> DATE<u>07/28</u>



© 2020 KIMLEY-HORN AND ASSOCIATES, INC. 201 NORTH MONTEZUMA SUITE 206 PRESCOTT ARIZONA, 86301 PHONE: 928-458-7121 WWW.KIMLEY-HORN.COM

No service of the ser
A

CITY OF SEDONA PUBLIC WORKS DEPARTMENT 102 ROADRUNNER DRIVE

**SEDONA, ARIZONA 86336** 928-204-7111

**UPTOWN SEDONA** PARKING GARAGE

LANDSCAPE PLAN

(PER ARS 32-1129.01) THIS CONTRACT ALLOWS THE OWNER TO MAKE PAYMENT WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR PROGRESS PAYMENTS, WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVALS OF BILLINGS AND ESTIMATES FOR RELEASE OF RETENTION AND WITHIN 15 DAYS AFTER CERTIFICATION AND APPROVAL OF BILLINGS AND ESTIMATES FOR FINAL PAYMENT. SHEET ID LS01

NOTICE OF EXTENDED PAYMENT PROVISION

SHEET NO.

07 OF 07