

Structural Engineering CIVIL 310

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Executive Summary

The objective presented was to model and create a structure out of wood, steel, and concrete whether these three materials would be combined into one structure or if three different structures would be created was free of choice. This particular project uses the materials separately and so the same structure was created three times once out of wood, once out of steel and once out of concrete. The structure created was made to be a warehouse that is 100 feet wide, 180 feet long and 50 feet high, including two indoor soccer courts, a small office space and a bathroom for each gender. The structure could be made with all three materials with wood being the cheapest of the three structures made followed by concrete and then steel being the most expensive at \$589367. Each structure had dead loads and live loads applied to it with concrete undergoing the least deflection making it the most stable and wood undergoing the most deflection making it the least stable. Comparing the stability and cost effectiveness of each structure it was found that concrete would be best choice of material as it provided the most stability and a price that seems reasonable for a building such as the one chosen.

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Introduction

The purpose of this project was to design a structure in AutoCAD and then model the structure in Risa. Through this a warehouse with two indoor soccer fields, a men's and women's bathroom and a small office and front desk area was designed. The design for this project was based on an indoor soccer arena in Australia, the majority are designed like this and that is what sparked this idea. The structure was then modelled in Risa three times using different materials each time, one made from wood, one out of steel and one out of concrete. The structure was designed to be 100 feet wide by 180 feet long and 50 feet high, with a flat roof. Columns were to be placed every 20 feet to support the structure. The designed plans for the structure can be seen below and a separate plan for where the columns will be placed. Each of these structures comes with a cost analysis describing the estimated cost of how much one of these buildings would cost with their chosen materials.

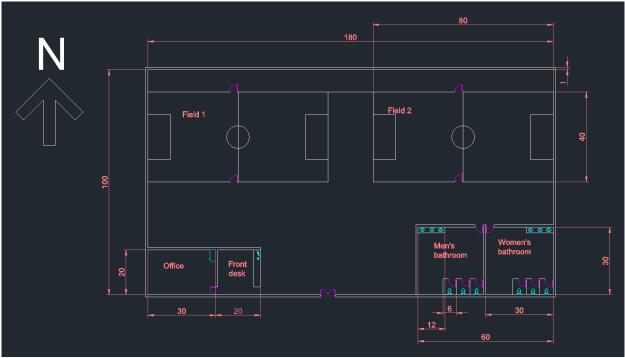


Figure 1: Structure layout

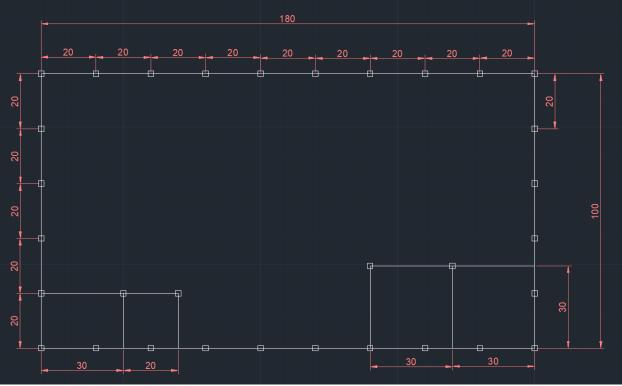


Figure 2: Placement of columns

Designed Structures

There are three structures shown and described below the first being made of wood, followed by one made of steel and then one made of concrete. Each structure was placed under a 0.1 ksf area load, a 3 k nodal load and 0.1 ksf wind load on all of the walls to test the stability of the structure and which all of these results are calculated from.

1. Wood design

The first structure that was designed was made from wood with 20 x 20 Douglas fir columns, 4 x 10 Douglas fir beams and 6 x 8 Douglas fir girders. Between the columns wood walls were placed which were also made from Douglas fir. In the figures below the model of the structure made from wood can be seen and the forces that were placed on it. There is also an analysis of each member and how the members act when they are under a force. The wood structure undergoes the most deflection which can be seen in the diagrams below for each element, this is primarily due to wood being a weaker material than steel and concrete. The entire structure would be stable if it would be created out of wood so this would be a suitable design.

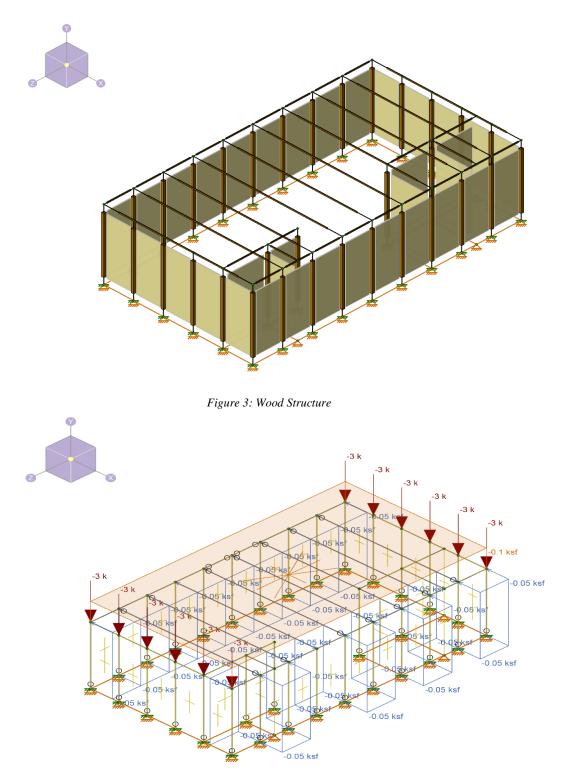


Figure 4: Loads applied to wood Structure



Figure 6: Wood column diagrams

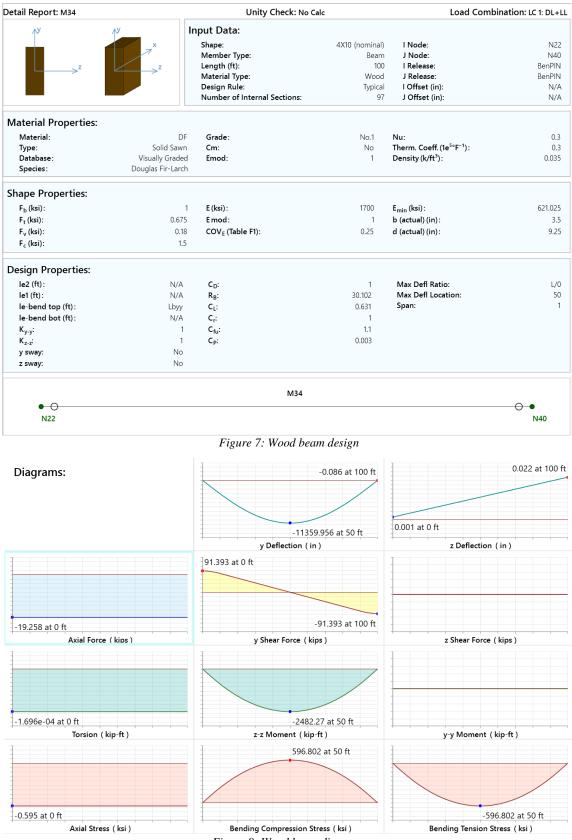
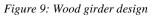


Figure 8: Wood beam diagrams

| Detail Report: M42 | | Unity Check: 13.408 (sh | ear) | Load Combi | nation: LC 1: DL+L |
|-----------------------|--------------|------------------------------|---------------|--|--------------------|
| NY NY | In | iput Data: | | | |
| | x | Shape: | 6X8 (nominal) | l Node: | N42 |
| | | Member Type: | Beam | J Node: | N40 |
| Z | . z | Length (ft): | 20 | l Release: | Fixed |
| | >⁴ | Material Type: | Wood | J Release: | Fixed |
| | | Design Rule: | Typical | l Offset (in): | N/A |
| | | Number of Internal Sections: | 97 | J Offset (in): | N/A |
| Material Properties: | | | | | |
| Material: | DF | Grade: | No.1 | Nu: | 0.3 |
| Туре: | Solid Sawn | Cm: | No | Therm. Coeff. (1e ⁵ °F ⁻¹): | 0.3 |
| Database: Visu | ally Graded | Emod: | 1 | Density (k/ft ³): | 0.035 |
| | as Fir-Larch | | | ,, | |
| Shape Properties: | | | | | |
| F _b (ksi): | 1.2 | E (ksi): | 1600 | E _{min} (ksi): | 584.494 |
| F _t (ksi): | 0.825 | Emod: | 1 | b (actual) (in): | 5.5 |
| F _v (ksi): | 0.17 | COV _F (Table F1): | 0.25 | d (actual) (in): | 7.5 |
| F _c (ksi): | 1 | | | | |
| Design Properties: | | | | | |
| le2 (ft): | N/A | C _D : | 1 | Max Defl Ratio: | L/9642 |
| le1 (ft): | N/A | R _B : | 7,714 | Max Defl Location: | 15 |
| le-bend top (ft): | Lbyy | с _L : | 0.994 | Span: | 1 |
| le-bend bot (ft): | N/A | C _r : | 1 | | |
| К _{у-у} : | 1 | C _{fu} : | 0.74 | | |
| K _{z-z} : | 1 | C _P : | 0.238 | | |
| y sway: | No | | 0.200 | | |
| z sway: | No | | | | |
| | | M42 | | | |
| • N42 | | | | | • N40 |



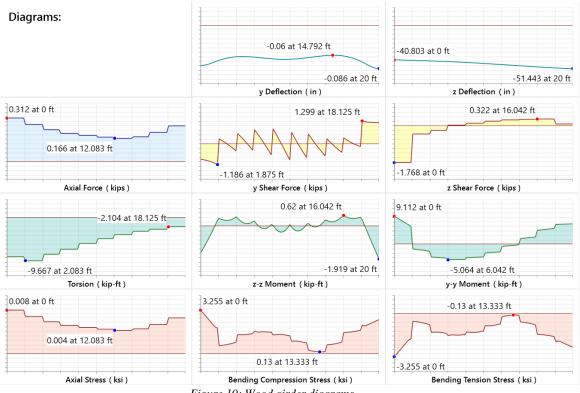


Figure 10: Wood girder diagrams

2. Steel Design

The second structure that was designed was made from Steel with W32 x 652 wide flanged hot rolled steel columns, W12 x 30 wide flanged hot rolled beams and W14 x 132 wide flanged hot rolled girders, all the hot rolled steel was made from A992 steel. Between the columns masonry walls were placed. In the figures below an analysis of each member can be seen of how the members act when a force is placed on them. The entire structure would be stable if it would be created out of steel so this would be a suitable design.

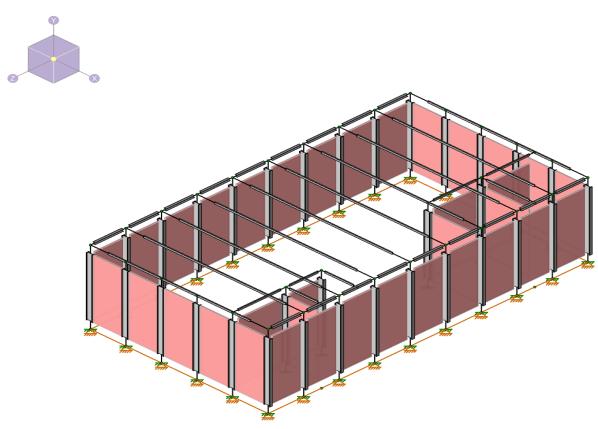


Figure 11: Steel structure

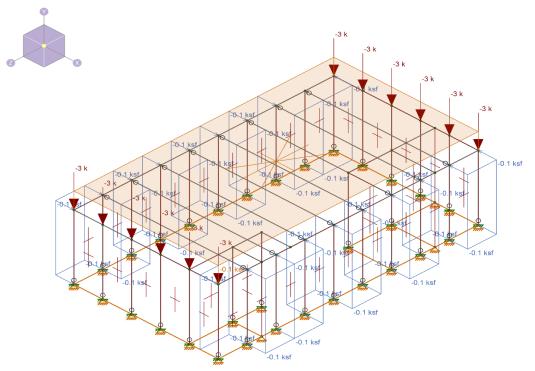


Figure 12: Loads applied to steel structure

| Detail Report: M19 | | Unity Check: 0.119 (axial/l | pending) | Load Combinat | ion: LC 1: DL+LL+W |
|-------------------------------------|------------|--|--|--|--------------------------------------|
| × × | , x , z | nput Data: Shape: Member Type: Length (ft): Material Type: Design Rule: Number of Internal Sections: | W36X652 Column 50 Hot Rolled Steel Typical 97 | l Node: J Node: I Release: J Release: I Offset (in): | N38 N37 Fixed BenPIN N/A |
| | | Number of Internal Sections: | 97 | J Offset (in): | N/A |
| Material Properties: | | | | _ | |
| Material: | A992 | Therm. Coeff. (1e ⁵ °F ⁻¹): | 0.65 | R _y : | 1.1 |
| E (ksi): | 29000 | Density (k/ft ³): | 0.49 | F _u (ksi): | 65 |
| G (ksi): | 11154 | F _y (ksi): | 50 | R _t : | 1.1 |
| Nu: | 0.3 | | | | |
| Shape Properties: | | | | | |
| d (in): | 41.1 | Area (in²): | 192 | S _w (in ⁴): | 2560 |
| b _f (in): | 17.6 | Z _{vv} (in ³): | 581 | r _T (in): | 4.684 |
| t _f (in): | 3.54 | Z_{zz} (in ³): | 2910 | J (in ⁴): | 593 |
| t _w (in): | 1.97 | C _w (in ⁶): | 1.13e+6 | k _{det} (in): | 4.812 |
| l _{yy} (in⁴): | 3230 | W _{no} (in ²): | 165 | k _{des} (in): | 4.49 |
| l _{zz} (in ⁴): | 50600 | | | | |
| Design Properties: | | | | | |
| L _{b y-y} (ft) : | N/A | К _{у-у} : | 1 | Max Defl Ratio: | L/10000 |
| L _{b z-z} (ft): | N/A | K _{z-z} : | 1 | Max Defl Location: | . 0 |
| L _{comp top} (ft): | Lbyy | y sway: | No | Span: | N/A |
| L _{comp bot} (ft): | N/A | z sway: | No | | |
| L _{torque} (ft): | N/A | Function: | Lateral | | |
| • | | Seismic DR: | None | | |
| | | M19 | | | |
| • | | | | | |
| N38 | | | | | N37 |

Figure 13: Steel column design

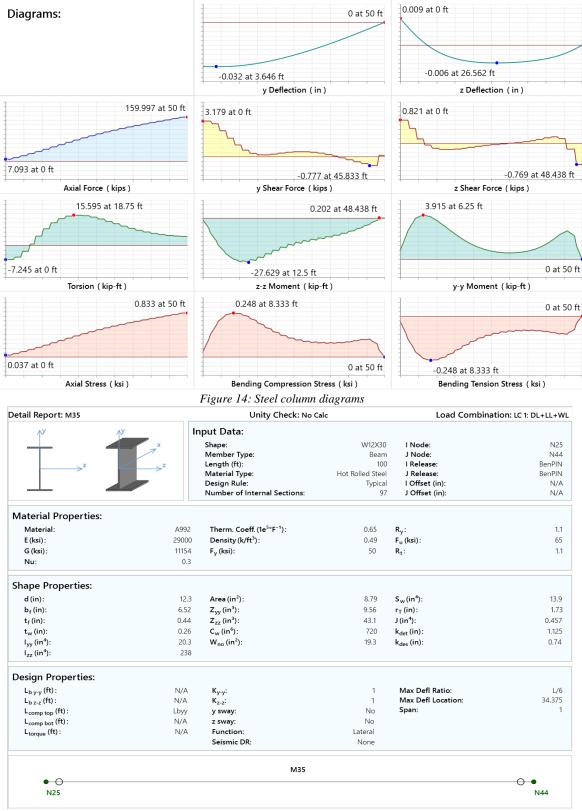


Figure 15: Steel beam design

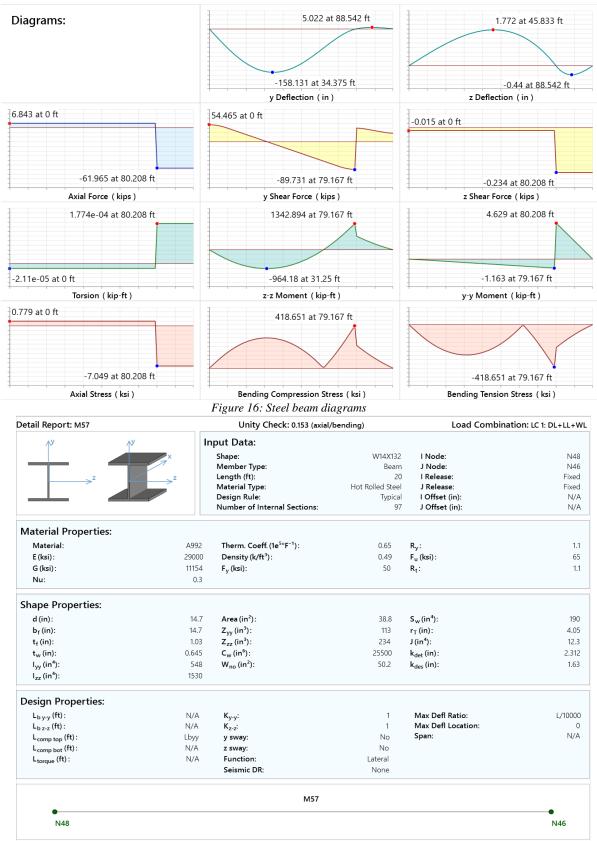


Figure 17: Steel girder design

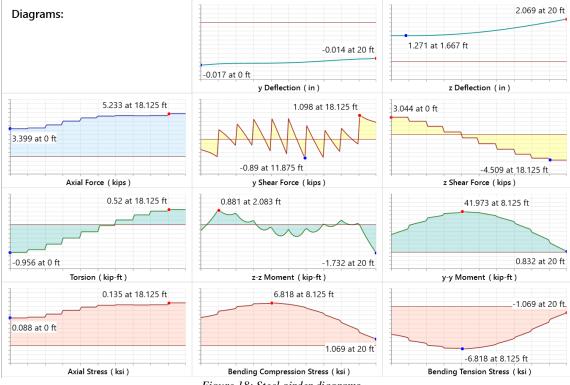


Figure 18: Steel girder diagrams

3. Concrete Design

The third structure that was designed was made from Concrete with 25in x 25in thick concrete columns, 13in x 13in thick concrete beams and 10in x 10in thick concrete girders, all the concrete was made from 3000NW concrete. Between the columns 18in thick concrete walls were placed which passed all the design tests. In the figures below an analysis of each member can be seen of how the members act when a force is placed on them, and the concrete structure looks and where the forces were placed. This structure made from concrete deflects the least out of the three as can be seen by all of the diagrams for each element. The entire structure would be stable if it would be created out of steel so this would be a suitable design.

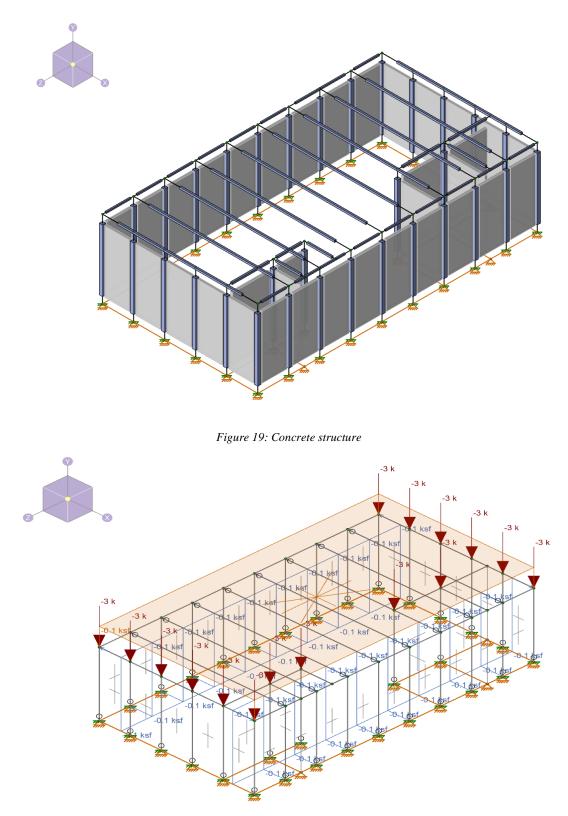


Figure 20: Loads applied to concrete structure



Figure 22: Concrete column diagrams

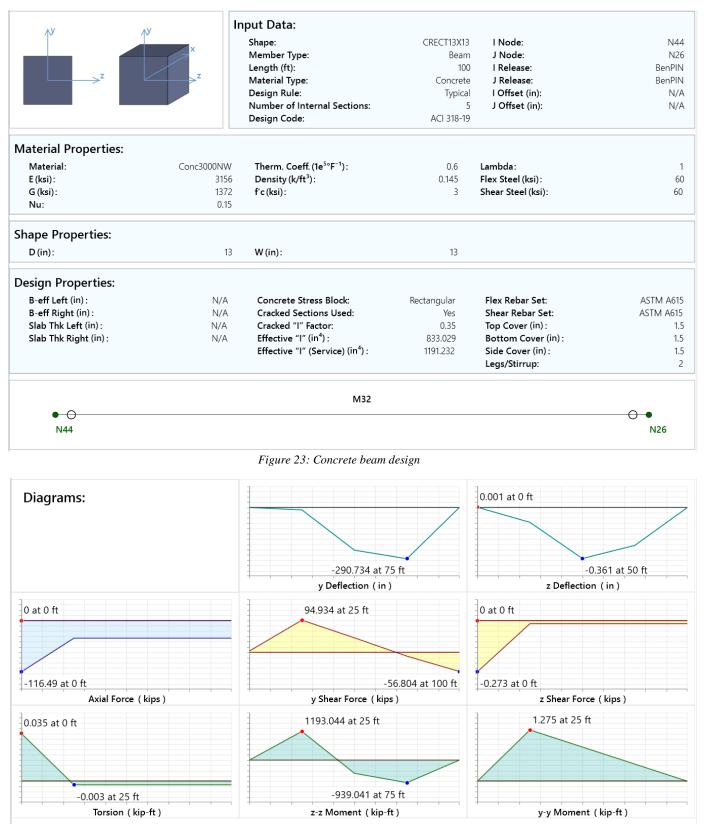


Figure 24: Concrete beam diagrams

| AM | Input Data: | | | |
|--|--|----------------|---------------------------|---|
| ^y ^y x | Shape: | CRECT10X10 | | N4 |
| | Member Type: | Beam | | N4 |
| z | Length (ft): | 20 Concrete | | Fixed |
| | Material Type: Design Rule: | Typica | | Fixed N/J |
| | Number of Internal Sections: | Typica | . , | N// |
| | Design Code: | ACI 318-19 | | |
| Material Properties: | | | | |
| Material: Conc3000NW | Therm. Coeff. (1e⁵°F⁻¹): | 0.6 | Lambda: | |
| E (ksi): 3156 | | 0.145 | Flex Steel (ksi): | 60 |
| G (ksi): 1372 | | 3 | Shear Steel (ksi): | 60 |
| Nu: 0.15 | | | | |
| Shape Properties: | | | | |
| D (in): 10 | W (in): | 10 | | |
| Design Properties: | | | | |
| B-eff Left (in): N/A | Concrete Stress Block: | Rectangular | Flex Rebar Set: | ASTM A615 |
| B-eff Right (in): N/A | Cracked Sections Used: | Yes | Shear Rebar Set: | ASTM A615 |
| Slab Thk Left (in): N/A | Cracked "I" Factor: | 0.35 | Top Cover (in) : | 1. |
| Slab Thk Right (in): N/A | Effective "I" (in ⁴) : | 291.667 | Bottom Cover (in) : | 1.5 |
| | Effective "I" (Service) (in ⁴) : | 417.083 | Side Cover (in) : | 1.5 |
| | | | Legs/Stirrup: | 2 |
| | M53 | | | |
| • N48 | | | | • N46 |
| | Figure 25: Concrete girde | er design | | |
| | | | | |
| Diagrams: | 0 at 0 ft | | | 0.427 at 20 ft |
| Diagrams: | 0 at 0 ft | | | 0.427 at 20 ft |
| Diagrams: | 0 at 0 ft | | | 0.427 at 20 ft |
| Diagrams: | 0 at 0 ft -0.013 at 0 ft | | | 0.427 at 20 ft |
| Diagrams: | | 1) | z Deflec | 0.427 at 20 ft |
| Diagrams: 5.134 at 0 ft | -0.013 at 0 ft y Deflection (in | n) at 10 ft | z Deflec 0.826 at 0 ft | |
| | -0.013 at 0 ft y Deflection (in | | | |
| | -0.013 at 0 ft y Deflection (in | | | |
| | -0.013 at 0 ft y Deflection (in | | | |
| 5.134 at 0 ft | -0.013 at 0 ft y Deflection (in 0.987 | | | tion (in) |
| | -0.013 at 0 ft y Deflection (in | | 0.826 at 0 ft | tion (in) -0.812 at 20 ft |
| 5.134 at 0 ft | -0.013 at 0 ft y Deflection (in 0.987 | at 10 ft | 0.826 at 0 ft | tion (in) -0.812 at 20 ft, proce (kips) |
| 5.134 at 0 ft -0.069 at 10 ft | -0.013 at 0 ft y Deflection (in 0.987 -0.367 at 0 ft y Shear Force (ki | at 10 ft | 0.826 at 0 ft | tion (in) -0.812 at 20 ft |
| 5.134 at 0 ft -0.069 at 10 ft Axial Force (kips) | -0.013 at 0 ft y Deflection (in 0.987 -0.367 at 0 ft y Shear Force (ki | at 10 ft | 0.826 at 0 ft | tion (in) -0.812 at 20 ft, proce (kips) |
| 5.134 at 0 ft -0.069 at 10 ft Axial Force (kips) | -0.013 at 0 ft y Deflection (in 0.987 -0.367 at 0 ft y Shear Force (ki | at 10 ft | 0.826 at 0 ft | tion (in) -0.812 at 20 ft, proce (kips) |
| 5.134 at 0 ft -0.069 at 10 ft Axial Force (kips) | -0.013 at 0 ft y Deflection (in 0.987 -0.367 at 0 ft y Shear Force (ki | at 10 ft | 0.826 at 0 ft | tion (in) -0.812 at 20 ft orce (kips) 0.789 at 10 ft |
| 5.134 at 0 ft -0.069 at 10 ft Axial Force (kips) | -0.013 at 0 ft y Deflection (in 0.987 -0.367 at 0 ft y Shear Force (ki | at 10 ft | 0.826 at 0 ft | tion (in) -0.812 at 20 ft, proce (kips) |

Figure 26: Concrete girder diagrams

4. Cost Analysis

4.1 Wood Design

The prices for the wood pieces are from Lowe's [1] where the price was multiplied by the length of the required wood pieces. The cost of 2x8 per foot was \$1.09, the cost of 20x20 was \$19.5 per foot, the cost of 2x6 was \$0.885 per foot, the cost of 4x10 was \$8.70 per foot and for 6x8 the cost was \$12.6 per foot. The total cost for the wood structure equaled to \$54367 which was by far the cheapest for any of the three structures. Since it is made completely out of wood it might also not be the strongest of structures and therefore might need to be reinforced which would increase the price of the final structure.

| wood | Size | Pieces | Length (ft) | Price |
|-------------|-------|--------|-------------|---------|
| Douglas fir | 2x8 | 140 | 5000 | \$5457 |
| Douglas fir | 20x20 | 32 | 1600 | \$31200 |
| Douglas fir | 2x6 | 70 | 2500 | \$2212 |
| Douglas fir | 4x10 | 17 | 1260 | \$10962 |
| Douglas fir | 6x8 | 18 | 360 | \$4536 |
| Total | | 304 | 17560 | \$54367 |

Table 1: Wood structure cost analysis

4.2 Steel Design

According to Midwest steel and aluminum [2] the price for one foot of W12x30 is \$58.92, the price for one foot of W14x132 is \$108.73 and the price for one foot of W36x652 is \$300. The total cost for the structure made completely out of steel the total cost would be \$589367 making it the most expensive of the three structures. The price of the structure could be reduced if a different material or size was used for the columns making the entire structure more affordable. The cost breakdown of the entire structure can be seen in table 2 below where all parts of structures cost can be seen.

Table 2: Steel structure cost analysis

| Hot rolled steel | Size | Pieces | Length (ft) | Price |
|------------------|---------|--------|-------------|----------|
| A992 | W12x30 | 17 | 1180 | \$69525 |
| A992 | W14x132 | 18 | 360 | \$39142 |
| A992 | W36x652 | 32 | 1600 | \$480700 |
| Total | | 67 | 3140 | \$589367 |

4.3 Concrete Design

According to concrete network [3] for a yard of concrete is about \$125 however there can be other fees or the region the concrete is needed can change the price. For this price the \$125 estimate has been used. Making the structure out of concrete would make the final cost \$300136 which would be the second most expensive out of the wood, steel, and concrete options. The piece of the structure that adds the most cost on is the concrete walls, since there is a large area for them to cover. If the walls could be replaced with a different material, it would make the structure more affordable.

| Concrete | Size | Pieces | Length (yd's) | Price |
|------------|-------|--------|---------------|----------|
| Conc3000NW | 10x10 | 16 | 9.3 | \$1162.5 |
| Conc3000NW | 13x13 | 17 | 51.3 | \$6412.5 |
| Conc3000NW | 25x25 | 32 | 257.2 | \$32150 |
| Conc300NW | walls | 35 | 2083.3 | \$260412 |
| Total | | 100 | 2401.1 | \$300136 |

Table 3: Concrete structure cost analysis

Summary

The project was a success as three structures were created one out of wood, one out of steel and one out of concrete and all three of them are stable and can be constructed. With steel being the most expensive of the three at \$589367 followed by the concrete structure at \$300136 and the cheapest of the three being the wood structure at \$54367. All three of the structures could become cheaper or more expensive if certain modifications are made that could greatly benefit each structure by replacing certain parts with other materials. All three structures have deflection diagrams showing how they react under the forces they were placed with wood being the weakest of the three materials and therefore deflecting the most and concrete being the strongest material and deflecting the least. In terms of the best option, it would be concrete if one looks at affordability and its ability to resist deflection.

References

- [1] Reliabilt 2-in x 8-in x 12-ft douglas fir . Lowes. (n.d.). Retrieved November 27, 2021, from <u>https://www.lowes.com/pd/Top-Choice-2-in-x-8-in-x-12-ft-Douglas-Fir-Lumber-</u> <u>Common-1-562-in-x-7-5-in-x-12-ft-Actual/1000009776</u>.
- [2] Hot Roll Steel I Beam. Midwest Steel & Aluminum. (n.d.). Retrieved November 27, 2021, from https://www.midweststeelsupply.com/store/hotrollsteelbeam.
- [3] Concrete Network. (2021, July 26). Concrete prices 2021 how much does concrete cost? The Concrete Network. Retrieved November 27, 2021, from <u>https://www.concretenetwork.com/concrete-prices.html</u>.