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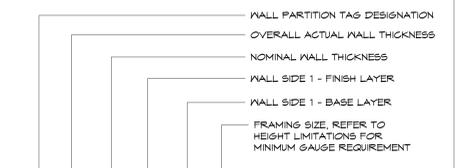
Offshore Wind Safety Center -
 Training Room
 1535 Bacharach Blvd, Atlantic
 City, NJ 08401

CONSTRUCTION DOCUMENTS
 10.27.2021

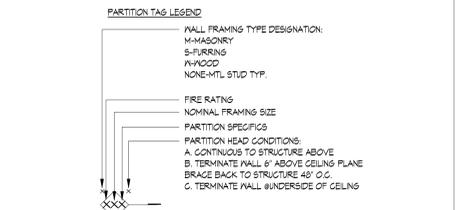
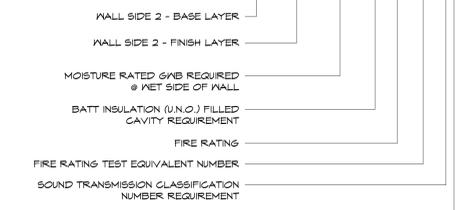
REVISIONS

DRAWING TITLE:
**PARTITION TYPES, LEGENDS, &
 NOTES**

DRAWN BY: Author
 REVIEWED BY: Checker
 PROJECT NO. **AC-010**



TYPE	OA THICK	NOM THICK	FINISH LAYER	BASE LAYER	STUD SIZE	BASE LAYER	FINISH LAYER	MOISTURE	INSUL	FIRE	TEST NO.	STC
032	0'-4 1/8"	5"	5/8" GNB	-	3 5/8"	-	5/8" GNB	-	-	-	-	-
FOOO	0'-5 5/8"	1/2"	5/8" GNB	-	-	-	-	-	-	-	-	-



PARTITION LEGEND SCALE N.T.S. **3**

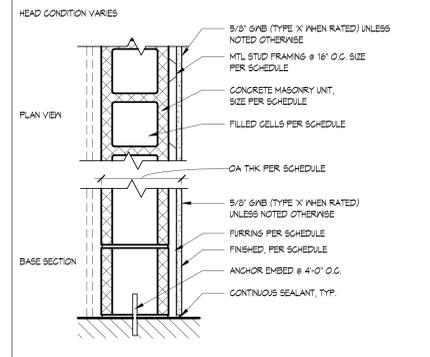
TYPICAL LIMITING HEIGHTS - INTERIOR PARTITIONS

STUD WIDTH	STUD SPACING	ALLOW DEFL.	PARTITION ONE LAYER	PARTITION TWO LAYERS	FURRING ONE LAYER
25 GAUGE STUD (0119 MIN.)					
1-5/8"	16"	L/240	9'-6"	10'-6"	8'-3"
	24"	L/240	9'-3"	8'-4"	7'-3"
2-1/2"	16"	L/240	12'-6"	13'-6"	11'-0"
	24"	L/240	10'-4"	11'-3"	9'-4"
3-5/8"	16"	L/240	16'-0"	16'-4"	14'-6"
	24"	L/240	13'-6"	13'-6"	12'-9"
6"	16"	L/240	20'-0"	20'-0"	20'-0"
	24"	L/240	15'-0"	15'-0"	15'-0"
22 GAUGE STUD (0210 MIN.)					
2-1/2"	16"	L/240	13'-0"	14'-0"	12'-0"
	24"	L/240	11'-6"	12'-3"	9'-3"
3-5/8"	16"	L/240	17'-3"	18'-0"	16'-0"
	24"	L/240	15'-0"	15'-4"	14'-0"
6"	16"	L/240	25'-3"	26'-0"	23'-4"
	24"	L/240	22'-0"	22'-4"	20'-4"
20 GAUGE STUD (0312 MIN.)					
2-1/2"	16"	L/240	13'-10"	16'-11"	13'-0"
	24"	L/240	12'-0"	13'-5"	11'-6"
3-5/8"	16"	L/240	17'-11"	20'-2"	17'-3"
	24"	L/240	15'-7"	17'-8"	15'-3"
6"	16"	L/240	26'-11"	28'-6"	25'-6"
	24"	L/240	22'-10"	24'-11"	22'-3"

PARTITION LIMITING HEIGHT CHART SCALE N.T.S. **2**

- GENERAL WALL PARTITION NOTES:
- CONTRACTOR IS RESPONSIBLE TO VERIFY WALL ASSEMBLY CONSTRUCTION WHEN PROPOSED WALL THICKNESS IS TO MATCH EXISTING. ANY DISCREPANCIES SHOULD BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO INSTALLATION.
 - REFER TO REFLECTED CEILING PLANS AND/OR FINISH SCHEDULE FOR INFORMATION ON CEILING TYPE, FINISH AND HEIGHTS. COORDINATE REQUIREMENTS FOR CEILING BLOCKING, FRAMING ETC. WITH PARTITIONS. IN NO CASE SHALL ANY FRAMING OR BLOCKING REQUIREMENTS COMPROMISE THE FIRE RATING SPECIFIED FOR ANY ASSEMBLY.
 - REFER TO INTERIOR ELEVATIONS AND/OR FINISH SCHEDULE FOR INFORMATION ON WALL FINISHES, BASE TRIM, MOLDINGS, ETC.
 - ALL PARTITIONS SHALL BE SECURELY FASTENED ABOVE AND BELOW TO THE STRUCTURAL SLAB OR STRUCTURAL FLOOR FRAMING AND STRUCTURAL DECK/CEILING / ROOF ABOVE.
 - FOR ADDITIONAL INFORMATION REGARDING EXTERIOR WALL CONSTRUCTION, REFER TO DETAIL SHEETS AND ALL SECTION.
 - AT ALL FIRE RATED ASSEMBLIES, CONTRACTOR SHALL STENCIL DURATION OF FIRE RATED ASSEMBLY ABOVE FINISHED CEILING (I.E. 1HR OR 2HR ETC).
 - PROVIDE 3-5/8" KICKERS @ 32" O.C. FOR ALL PARTITIONS TALLER THAN THEIR HEIGHT LIMITATIONS.
 - WHERE APPLICABLE REPLACE 5/8" GNB WITH 1/2" CEMENTITIOUS BOARD & TRIMSET AT TILE/STONE FINISHES.
 - 6" STUDS GREATER THAN 22'-0" TO BE STRUCTURAL STUDS.

PARTITION WALLS SCALE N.T.S. **1**

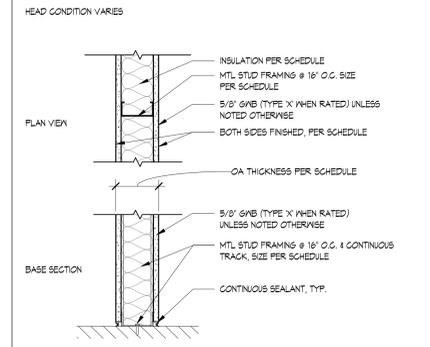


M080 PARTITION TYPES

TYPE	OA THICK	NOM THICK	FINISH LAYER	BASE LAYER	STRUCT SIZE	BASE LAYER	FINISH LAYER	MOIST RE	INSUL	FIRE	TEST NO.	STC
030	0'-4 1/8"	5"	5/8" GNB	-	3 5/8"	-	5/8" GNB	-	-	-	-	-

THICKNESS IS FOR GNB & FURRING ONLY - REFER TO PLAN FOR ADDITIONAL THICKNESS DUE TO EXTERIOR CONDITIONS

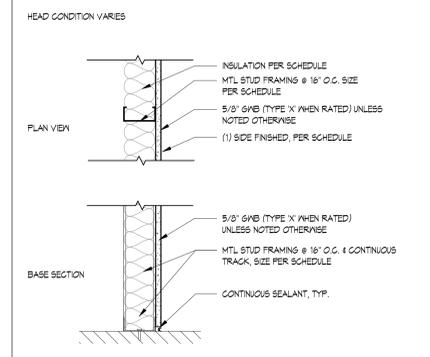
PARTITION TYPES - M080 SCALE 1 1/2" = 1'-0" **6**



030 PARTITION TYPES

TYPE	OA THICK	NOM THICK	FINISH LAYER	BASE LAYER	STRUCT SIZE	BASE LAYER	FINISH LAYER	MOIST RE	INSUL	FIRE	TEST NO.	STC
032	0'-4 1/8"	5"	5/8" GNB	-	3 5/8"	-	5/8" GNB	-	-	-	-	-

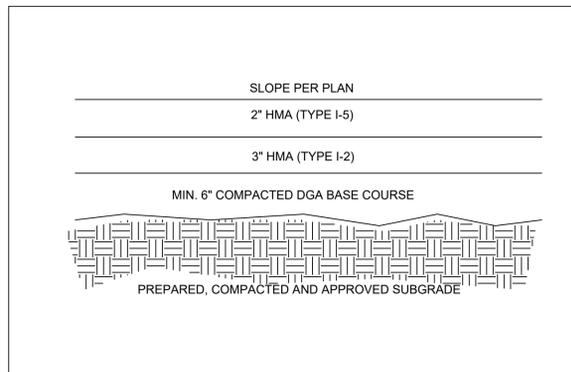
PARTITION TYPES - 030 SCALE 1 1/2" = 1'-0" **5**



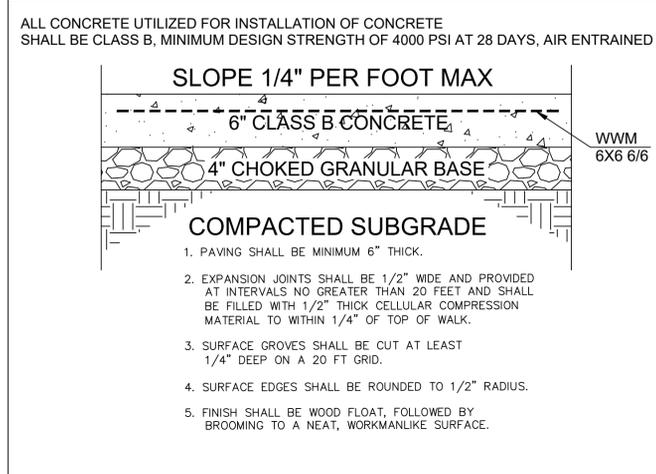
S030 PARTITION TYPES

TYPE	OA THICK	NOM THICK	FINISH LAYER	BASE LAYER	STRUCT SIZE	BASE LAYER	FINISH LAYER	MOIST RE	INSUL	FIRE	TEST NO.	STC
032	0'-4 1/4"	4"	-	-	3 5/8"	-	5/8"	-	-	-	-	-

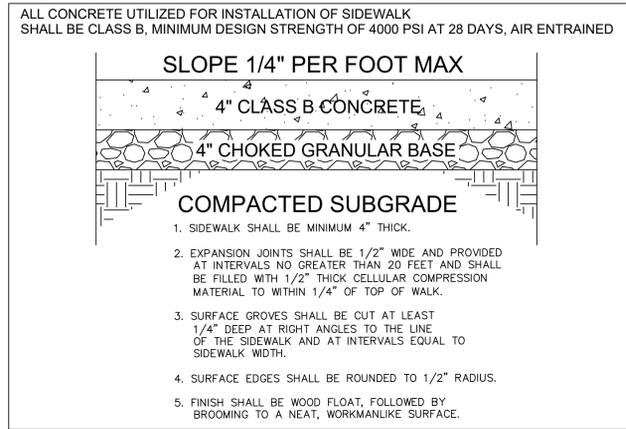
PARTITION TYPES - S030 SCALE 1 1/2" = 1'-0" **4**



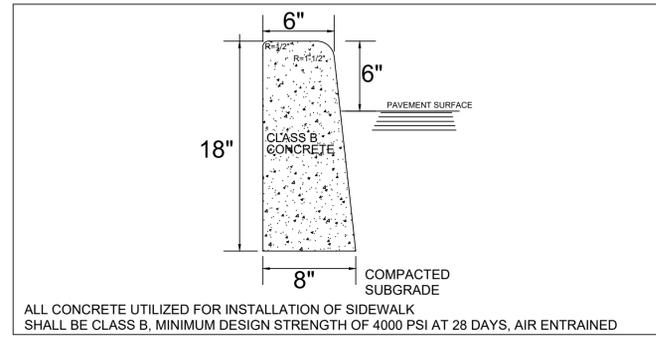
1 ON-SITE ASPHALT PAVEMENT



4 CONCRETE PAVING SECTION



2 SIDEWALK DETAIL



3 CONCRETE CURB DETAIL (ON-SITE)

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ALL WORK SHALL COMPLY WITH ALL APPLICABLE FEDERAL, STATE AND LOCAL LAWS AND SAFETY REQUIREMENTS AND SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST PROVISIONS OF THE OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA), THE HIGH VOLTAGE PROXIMITY ACT, STATE OF NEW JERSEY, ADOPTED 7/21/48 AS P.L. 1948, c.249, THE NEW JERSEY UNIFORM CONSTRUCTION CODE, ICC, ASTM SPECIFICATIONS, ALL LOCAL ORDINANCES AND PERMIT CONDITIONS.

AWP, PONZIO CO & ASSOCIATES, INC. RESPONSIBILITIES DO NOT INCLUDE ANY FIELD INSPECTION, CONSTRUCTION MANAGEMENT, CONSTRUCTION OR CONTRACTORS COMPLIANCE WITH CONSTRUCTION DOCUMENTS.

NO.	DATE	BY	DESCRIPTION	NO.	DATE	BY	DESCRIPTION
REVISIONS							

AWP

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SURVEYORS, PLANNERS, ENGINEERS
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SITE DETAILS
BLOCK 323 LOT 1
ATLANTIC CITY ATLANTIC COUNTY NEW JERSEY

SCALE: SEE PLAN
DATE: 10-28-21

BY: JJB
PROJ. NO.: 36085

SHEET NO.
C-2
SHEET 1 of 2

1. NOTES, TYPICAL DETAILS, AND SCHEDULES APPLY TO ALL STRUCTURAL WORK UNLESS NOTED OTHERWISE. TYPICAL DETAILS ARE TO BE USED FOR ALL CONDITIONS WHERE THE DETAIL IS APPLICABLE, WHETHER OR NOT NOTED ON PLAN. TYPICAL DETAILS MAY BE SLIGHTLY ALTERED IF SUCH ALTERATIONS ARE NECESSARY TO MAINTAIN THE INTENT OF THE DETAIL. ANY ALTERATIONS SHALL BE NOTED ON THE DRAWINGS AND THE ENGINEER SHALL BE NOTIFIED PRIOR TO THE START OF CONSTRUCTION.

2. ALL DIMENSIONS AND ELEVATIONS SHOWN ON STRUCTURAL DRAWINGS, WITH THE EXCEPTION OF STRUCTURAL MEMBER SIZES, ARE GENERATED BY OTHER DISCIPLINES. ANY DIMENSIONS OR ELEVATIONS OMITTED OR NOT SHOWN ON THE STRUCTURAL DRAWINGS SHOULD BE USED IN CONJUNCTION WITH, AND COORDINATED WITH, THE SPECIFICATIONS, ARCHITECTURAL DRAWINGS AND ALL OTHER DISCIPLINE'S DRAWINGS. IF THERE IS A DISCREPANCY BETWEEN DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ENGINEER AND ARCHITECT PRIOR TO PERFORMING THE WORK.

3. IF DIFFERENCES OCCUR WITHIN OR BETWEEN DRAWINGS AND SPECIFICATIONS REGARDING MATERIALS, STRENGTHS OR QUANTITIES, THE BETTER MATERIAL AND GREATER QUANTITY INDICATED, SPECIFIED OR NOTED SHOULD BE USED.

4. REPRODUCTIONS OF STRUCTURAL DRAWINGS FOR SUBMITTAL AS SHOP DRAWINGS IS PROHIBITED, UNLESS WRITTEN APPROVAL IS REQUESTED BY THE CONTRACTOR AND IT IS GRANTED BY O'DONNELL & NACCARATO, INC.

5. DO NOT SCALE DRAWINGS TO OBTAIN DIMENSIONAL INFORMATION.

6. THESE DRAWINGS DO NOT DEFINE SCOPE OF CONTRACTOR OR SUBCONTRACTOR CONTRACTS.

7. AT ALL TIMES, THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONDITIONS OF THE JOBSITE INCLUDING MEANS AND METHODS OF CONSTRUCTION AND SAFETY OF PERSONS AND PROPERTY. THE ENGINEER'S PRESENCE OR REVIEW OF WORK AT THE JOBSITE IS FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT ONLY AND NOT TO BE CONSIDERED AS A REVIEW OF MEANS AND METHODS OF CONSTRUCTION AND SAFETY METHODS.

8. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING ALLOWABLE CONSTRUCTION LOADS AND FOR PROTECTING THE COMPLETED OR INCOMPLETE STRUCTURAL FRAMING FROM DAMAGE DUE TO TEMPORARY CONSTRUCTION LOADS.

9. COSTS OF INVESTIGATION AND/OR REDSIGN DUE TO CONTRACTOR ERRORS WILL BE AT THE CONTRACTOR'S EXPENSE.

10. ANY APPROVED CONTRACTOR REQUESTED CHANGES TO THESE DRAWINGS WILL BE DONE AT NO COST TO THE OWNER. APPROVAL OF CONTRACTOR REQUESTED CHANGES IN NO WAY STATES OR IMPLIES APPROVAL OF A CHANGE IN SCOPE OR CHANGE IN CONTRACT COST.

11. UNLESS EXPPLICITLY NOTED AS "ISSUED FOR BID", THESE DRAWINGS ARE NOT SUITABLE FOR OBTAINING BIDS FROM GENERAL OR SUBCONTRACTORS. REVIEWING OR USING CONTRACTS FOR BID OR ISSUED FOR BID OR OTHER CONTRACTS SHALL BE AT THE CONTRACTORS EXPENSE. CONTRACTOR, ADDITIONS OR CORRECTIONS TO DRAWINGS THAT ARE BID PRIOR TO DESIGN COMPLETION AND "ISSUED FOR BID" WILL NOT BE CONSIDERED AS DESIGN OR DRAWING CHANGES. STRUCTURAL DESIGN, BY NATURE, CANNOT BE COMPLETE PRIOR TO COMPLETION OF ARCHITECTURAL AND MECHANICAL DRAWINGS.

12. ALL REFERENCES TO WATERDAMP-PROOFING, FIREPROOFING, AND UTILITIES ON THE STRUCTURAL DRAWINGS ARE FOR REFERENCE ONLY. SEE ARCHITECTURAL DRAWINGS, SPECIFICATIONS, AND OTHER DOCUMENTS FOR ALL WATERDAMP-PROOFING, FIREPROOFING, AND UTILITIES DETAILS AND REQUIREMENTS. COORDINATE ALL UNDERGROUND UTILITY REQUIREMENTS WITH THE CIVIL/MEP DRAWINGS. ALL UTILITIES SHALL BE ABOVE GRADE FOOTING AND SHALL NOT BE LOCATED UNDER THE FOOTINGS, UNLESS NOTED OTHERWISE.

13. IF THE EXISTING FIELD CONDITIONS DO NOT PERMIT THE INSTALLATION OF THE WORK IN ACCORDANCE WITH THE DETAILS SHOWN, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER IMMEDIATELY. THE CONTRACTOR MUST PROVIDE A SKETCH OF THE CONDITION WITH HIS PROPOSED MODIFICATION TO THE CONTRACT DOCUMENTS. THIS SKETCH MUST BE SUBMITTED TO AND APPROVAL MUST BE GRANTED BY THE ENGINEER PRIOR TO PERFORMING THE WORK.

14. SUBMIT SHOP DRAWINGS SUCH THAT BY THE TIME THEY ARE RECEIVED BY O'DONNELL & NACCARATO, INC., THERE WILL BE AT LEAST 14 DAYS BEFORE REVIEWED SUBMITTALS WILL BE NEEDED. ANY REVIEW THAT IS REQUIRED MORE EXPEDITIOUSLY WILL BE AT THE CONTRACTORS EXPENSE. SHOP DRAWINGS SHALL BE MANUFACTURED WITH THE CONTRACTORS STAMP OF APPROVAL, CERTIFYING THAT HE HAS REVIEWED ALL FIELD MEASUREMENTS, CONSTRUCTION CRITERIA, MATERIALS AND SIMILAR DATA AND HAS CHECKED EACH DRAWING FOR COMPLETENESS, COORDINATION AND COMPLIANCE WITH THE CONTRACT DOCUMENTS. IF REVIEW OF AN INCOMPLETE SHOP DRAWING IS REQUIRED, THAT SHOP DRAWING SHALL BE CLEARLY MARKED AS INCOMPLETE. THE AREA THAT NEEDS TO BE REVIEWED SHALL BE CLEARLY NOTED WITH AN EXPLANATION FOR THE REASON FOR PARTIAL APPROVAL.

15. IN NO CASE SHALL HEAVY EQUIPMENT BE PERMITTED CLOSER THAN 6'-0" FROM ANY FOUNDATION BASEMENT WALL. IF THE CONTRACTOR DEEMS IT NECESSARY TO OPERATE SUCH EQUIPMENT CLOSER THAN 6'-0", THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE AND, AT HIS OWN EXPENSE, PROVIDE ADEQUATE PROTECTION AND SUPPORTS FOR THE EXISTING CONDITIONS, WHERE NECESSARY FROM SUCH EQUIPMENT.

16. SIZE AND/OR LOCATION OF OPENINGS, SLEEVES, CONCRETE HOUSEKEEPING PADS, INSERTS, DEPRESSIONS, ETC. SHOWN ON THE STRUCTURAL DOCUMENTS ARE FOR THE CONTRACTORS CONVENIENCE ONLY. THE CONTRACTOR IS SOLELY RESPONSIBLE TO COORDINATE ALL CONTRACT DOCUMENTS TO SUBMITTER THE SIZE AND/OR LOCATION OF OPENINGS, SLEEVES, CONCRETE HOUSEKEEPING PADS, INSERTS, DEPRESSIONS, ETC.

17. SIZE AND/OR LOCATION OF EXISTING STRUCTURES AND UTILITIES SHOWN ON THE STRUCTURAL DOCUMENTS ARE FOR THE CONTRACTORS CONVENIENCE ONLY. THE CONTRACTOR IS SOLELY RESPONSIBLE TO VERIFY BY FIELD MEASUREMENTS INVESTIGATION THE SIZE AND/OR LOCATION OF ALL EXISTING UTILITIES.

18. THE CONTRACTOR SHALL SUBMIT SIGNED AND SEALED CALCULATIONS AND SHOP DRAWINGS BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED SHOWING DESIGN OF METAL STAIRS, METAL RAILINGS AND CONNECTIONS TO STRUCTURE TAKING INTO ACCOUNT ALL APPLICABLE CODES AND REGULATIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF METAL STAIRS, METAL RAILINGS AND CONNECTIONS TO STRUCTURE. MEMBERS HAVE BEEN DESIGNATED ON THE STRUCTURAL CONTRACT DOCUMENTS TO SUPPORT THE STAIRS, THE CONNECTIONS FROM THE STAIRS SHALL BE DESIGNED AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF METAL STAIRS, METAL RAILINGS AND CONNECTIONS TO STRUCTURE. CONNECTIONS ARE USED, CONTRACTOR SHALL PROVIDE BRACING ELEMENTS FOR ALL SUPPORTING STEEL TO ELIMINATE THE TORSIONAL EFFECTS OF THE STAIRS. CONTRACTOR SHALL BE RESPONSIBLE FOR FURNISHING AND INSTALLING ALL EMBEDDED ITEMS AND HARDWARE AS REQUIRED PER THE STAIR DESIGN.

19. STRUCTURAL COMPONENTS ARE NOT DESIGNED FOR VIBRATING EQUIPMENT. MOUNT VIBRATING EQUIPMENT ON VIBRATION ISOLATORS, INERTIA PADS, ETC.

20. EXACT LOCATIONS OF ROOF PENETRATIONS TO BE COORDINATED BY THE GENERAL CONTRACTOR BETWEEN STEEL JOIST/DECK/HVAC SUBCONTRACTORS. SEE DETAIL FOR ROOF FRAME REQUIREMENTS.

EXISTING CONDITIONS/DEMOLITION

1. SHORING, BRACING, PROTECTION, AND UNDERPINNING OF EXISTING AND ADJACENT STRUCTURES DURING CONSTRUCTION, INCLUDING ALL DESIGN RESPONSIBILITIES, IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. PROVIDE SIGNED AND SEALED CALCULATIONS AND DRAWINGS TO OWNER, ARCHITECT AND MAINTAIN THE INTEGRITY OF EXISTING AND ADJACENT STRUCTURES, BUILDINGS AND STREETS.

2. ALL EXISTING DIMENSIONS, ELEVATIONS, AND LOCATIONS OF EXISTING STRUCTURES, OR RELATIVE TO EXISTING STRUCTURES, THAT ARE SHOWN ON THE STRUCTURAL DOCUMENTS WILL BE VERIFIED BY FIELD MEASUREMENTS PERFORMED BY THE CONTRACTOR. ANY DISCREPANCIES SHALL BE REPORTED TO THE ARCHITECT AND ENGINEER IMMEDIATELY.

3. THE STRUCTURAL DOCUMENTS HAVE BEEN PREPARED BASED ON AVAILABLE KNOWLEDGE OF EXISTING CONDITIONS. IF, DURING DEMOLITION, EXCAVATION OR CONSTRUCTION, ACTUAL CONDITIONS ARE DISCOVERED TO DIFFER FROM THOSE INDICATED ON THE DOCUMENTS, THE ARCHITECT AND ENGINEER SHALL BE NOTIFIED.

4. ALL STRUCTURAL DEMOLITION MUST BE COORDINATED WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.

5. SELECTIVELY DEMOLISH STRUCTURAL COMPONENTS AS REQUIRED TO CONSTRUCT NEW WORK. PRIOR TO ANY DEMOLITION WORK, AN ENGINEERING SURVEY REPORT OF THE STRUCTURE SHALL BE PREPARED BY THE CONTRACTOR TO DOCUMENT THE CONDITION OF THE FRAMING, FLOORS, ROOFS, AND UTILITIES. ANY ADJACENT STRUCTURES OR NEARBY OCCUPANTS MAY BE SIMILARLY REVIEWED.

6. WHERE NEW FRAMING IS TO BE CONNECTED TO AN EXISTING STRUCTURE WITH BRICK OR CMU VENEER, THE VENEER SHALL BE REMOVED SUFFICIENTLY TO PERMIT CONNECTION OF THE NEW FRAMING DIRECTLY TO THE BUILDING SUPERSTRUCTURE. NEW BRICK OR CMU SHALL BE INSTALLED TO MATCH THE EXISTING ADJACENT VENEER. MAINTAIN A 1/2" SEPARATION BETWEEN THE BRICK OR CMU AND THE NEW FRAMING, UNLESS NOTED OTHERWISE ON DRAWINGS. FILL GAPS WITH BACKER RODS AND SEALANTS.

7. CONTRACTOR TO FIELD VERIFY ALL EXISTING FINISHED FLOOR ELEVATIONS PRIOR TO FABRICATION OF STEEL BEAMS, PROVIDE ALLOWANCE FOR ADDITIONAL LEVELING MATERIAL IN AREAS OF BREAK THROUGH TO THE EXISTING STRUCTURE TO ENSURE FINISHED FLOOR ELEVATIONS OF NEW MATCHES EXISTING.

8. CONTRACTOR SHALL RETAIN INDIVIDUAL TO PERFORM SITE SAFETY DEMOLITION PLAN, ENGINEERING STUDY AND ALL OTHER SERVICES RELATED TO DEMOLITION IN ACCORDANCE WITH LOCAL JURISDICTION REQUIREMENTS.

STRUCTURAL SPECIAL INSPECTIONS

1. THE QUALIFIED AGENCY RETAINED BY THE OWNER FOR THESE SPECIAL INSPECTION SERVICES SHALL BE APPROVED BY THE OWNER, THE ARCHITECT, AND THE ENGINEER OF RECORD PRIOR TO START OF CONSTRUCTION. AN OUTLINE OF THE SCOPE OF SERVICES TO BE PERFORMED BY THE INSPECTING AGENCY IS TO BE SUBMITTED PRIOR TO THE START OF CONSTRUCTION.

2. IN ACCORDANCE WITH SECTION 1704 OF THE INTERNATIONAL BUILDING CODE, AND ALL APPLICABLE STATE AND LOCAL REQUIREMENTS, AN INDEPENDENT APPROVED AGENCY SHALL MAKE PERIODIC/ANNUAL CONTINUOUS INSPECTIONS OF THE CONSTRUCTION PROGRESS IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS:

STEEL CONSTRUCTION	SECTION 1704.3, TABLE 1704.3	SECTION 1705.2, TABLE 1705.2.3, ASCE 380
CONCRETE CONSTRUCTION	SECTION 1704.4, TABLE 1704.4	SECTION 1705.3, TABLE 1705.3
SOILS	SECTION 1704.7, TABLE 1704.7	SECTION 1705.6, TABLE 1705.6

FOUNDATIONS

1. PERFORM ALL SITE PREPARATION AND EXCAVATION WORK IN STRICT ACCORDANCE WITH THE REPORT ON GEOTECHNICAL INVESTIGATION PREPARED BY GEOTECH, INC. (N.J. DATED 02/10/2021).

2. EXCAVATE THE BUILDING SITE TO THE DEPTH AND EXTENT INDICATED IN THE SOILS REPORT. ALL SUBGRADES SHALL BE APPROVED IN WRITING BY THE SOILS ENGINEER PRIOR TO BACKFILLING.

3. BOTTOM OF FOOTINGS SHALL BEAR ON SUBGRADE PER THE GEOTECHNICAL REPORT.

4. SUBGRADE OF ALL FOOTINGS MUST BE INSPECTED UNDER THE SUPERVISION OF AND APPROVED BY A REGISTERED SOILS ENGINEER BEFORE PLACING ANY CONCRETE. APPROVAL IN WRITING MUST INDICATE THE SOIL IS ADEQUATELY COMPACTED TO SATISFY THE SPECIFIED SOIL BEARING PRESSURE.

5. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 14 FEET BELOW EXTERIOR FINISH GRADE. ALL FOOTING ELEVATIONS SHOWN ON PLAN ARE THE BEST APPROXIMATIONS BASED ON AVAILABLE DATA. GENERAL CONTRACTOR MAY ALTER FOOTING ELEVATIONS FOR REASONS INCLUDING, BUT NOT LIMITED TO, REVISIONS, GEOTECHNICAL OR CIVIL INFORMATION, UNDESIRABLE CONDITIONS, ACTUAL INHERENT ELEVATIONS, CONTRACTIBILITY, ETC. CONTRACTOR SHALL NOTIFY ARCHITECT AND OBTAIN WRITTEN APPROVAL PRIOR TO ANY MODIFICATIONS.

6. DO NOT BACKFILL ANY BASEMENT WALLS WITH AN UNBALANCED HEIGHT OF SOIL GREATER THAN THREE FEET UNTIL ELEVATED FLOOR IS IN PLACE AND THE WALL HAS REACHED ITS DESIGN STRENGTH OR THE WALLS ARE ADEQUATELY BRACED.

7. CONCRETE/CMU WALLS EXPOSED TO WIND (INTERIOR OR EXTERIOR) SHALL HAVE CONTROL JOINTS AT 30 FEET MAXIMUM ON CENTER UNLESS NOTED OTHERWISE. WALLS WITH METAL COLUMN PIERS OR PLASTERS SHALL BE POURING MONOLITHICALLY AND SHALL HAVE A FORKED CONTROL JOINT ON ONE SIDE OF EACH PIER ON THE EXPOSED FACE OF THE WALL. JOINTS SHALL BE FILLED WITH AN APPROVED SEALANT.

CONCRETE

1. REINFORCING STEEL SHALL BE WITHIN TOLERANCES SET FORTH IN ACI 117, AND HAVE THE SPECIFIED CLEAR COVER, UNLESS NOTED OTHERWISE ON DRAWINGS.

2. CONCRETE POURED AGAINST EARTH:

- #3 OR SMALLER 1 1/2"
- #4 OR LARGER 2"

CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:

- COLUMNS (TIES AND MAIN REINFORCING) 1 1/2"
- SLAB WALLS, JOISTS, #1 OR #2 BARS 1 1/2"
- BEAMS (STIRRUPS AND MAIN REINFORCING) 1 1/2"

3. CLEAR COVER SHALL BE CLEARLY SHOWN ON ALL REINFORCING BAR DETAIL DRAWINGS.

4. ALL CONCRETE SHALL BE READY-MIX AND HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF:

- A. SPREAD FOOTINGS/WALL FOOTINGS/FOUNDATION WALL 3,000 PSI
- B. BASEMENT WALLS/RETAINING WALLS (MINIMUM OF 3,000 PSI)
- C. PERIMETER WALL STRENGTH 4,000 PSI
- D. SLAB-ON-GRADE 4,000 PSI

OR AS SHOWN ON DRAWINGS.

5. HAVE A MINIMUM OF 500 LBS. OF CEMENT PER CUBIC YARD.

6. SLOPE AT POINT OF CONCRETE PLACEMENT SHALL BE 3 INCH MINIMUM AND 6 INCH MAXIMUM.

7. CONCRETE EXPOSED TO WEATHER SHALL HAVE A PERCENT AIR ENTRAINMENT. CONCRETE NOT EXPOSED TO WEATHER SHALL NOT CONTAIN AIR-ENTRAINING AGENT.

8. SUBMIT MIX DESIGNS FOR REVIEW.

9. NORMAL-WEIGHT CONCRETE TO BE GIVEN A HARD-TROWELED FINISH SHALL NOT CONTAIN AN AIR-ENTRAINING AGENT. TOTAL AIR CONTENT FOR THIS CONCRETE SHOULD NOT EXCEED 3 PERCENT AT POINT OF CONCRETE PLACEMENT. ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE LATEST ACI BUILDING CODE (ACI 318), THE ACI DETAILING MANUAL (ACI 315), AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301).

10. ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL, CONFORMING TO THE ASTM DESIGNATION A615 GRADE 60.

11. LAP ALL REINFORCING BARS #2 DIAMETERS. LAP ALL WVF A MINIMUM OF SIX INCHES.

12. ALL INSERTS AND SLEEVES SHALL BE CAST-IN-PLACE. THE CONTRACTOR SHALL VERIFY THE DIMENSIONS AND LOCATIONS OF ALL OPENINGS, PIPE SLEEVES, ETC. AS REQUIRED BY ALL TRADES BEFORE THE CONCRETE IS POURED. THE CONTRACTOR SHALL CONSULT THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS, AS WELL AS THE STRUCTURAL DRAWINGS FOR THE LOCATION, NUMBER, AND SIZE OF ALL OPENINGS, SLEEVES, ETC. HOWEVER, OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE INSTALLED ONLY AFTER APPROVAL BY THE STRUCTURAL ENGINEER IS OBTAINED. DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL OF THE LOCATION AND DIMENSIONS OF ALL OPENINGS, SLEEVES, ETC. IN CAST-IN-PLACE CONCRETE SLABS, BEAMS, WALLS, COLUMNS, AND FOUNDATIONS. THESE DRAWINGS SHALL BE COORDINATED BY THE CONTRACTOR. OPENINGS AND SLEEVES THROUGH CAST-IN-PLACE CONCRETE SHALL BE INSTALLED WHERE SHOWN IN THE DRAWINGS AND OPENINGS ARE SHOWN ON THE STRUCTURAL DRAWINGS OR WHERE THEY ARE SHOWN ON THE APPROVED SLEEVE AND OPENING DRAWINGS THAT HAVE BEEN SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. SPLICING CORERS OR DRILLING OF SLEEVES OR OPENING THROUGH PREVIOUSLY CAST CONCRETE IS NOT PERMITTED EXCEPT WHERE SPECIFICALLY REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER.

13. LOCATION OF CONSTRUCTION JOINTS IN THE SLAB ON GRADE SHALL BE SUBMITTED FOR APPROVAL BY THE STRUCTURAL ENGINEER & ARCHITECT.

14. SUBMIT ALL REINFORCING SHOP DRAWINGS FOR REVIEW PRIOR TO ANY FABRICATION.

15. THE CONTRACTOR SHALL INSTALL FLOOR LEVELING MATERIAL AND PERFORM OTHER CORRECTIVE MEASURES IN ALL AREAS, INCLUDING BUT NOT LIMITED TO, AREAS WHERE FLOOR FINISH PROVISIONS DO NOT COMPLY WITH THE FLATNESS AND LEVELNESS REQUIREMENTS.

16. THE CONTRACTOR SHALL DELIVER TO THE ENGINEER, AT THE END OF THE JOB, ONE (1) ELECTRONIC VERSION OF THE FINAL FIELD COPIES OF ALL STEEL REINFORCING SHOP DRAWINGS.

17. RIGID INSULATION USED AS FLOOR FILL SHALL BE STYROFOAM HIGH-LOAD 40 EXTRUDED POLYSTYRENE INSULATION (40 PSI COMPRESSIVE STRENGTH) ASTM C278, TYPE V, MANUFACTURED BY DOW CHEMICAL COMPANY, OR APPROVED EQUAL.

DOCUMENTS

1. ALL STRUCTURAL STEEL SHALL BE FABRICATED AND ERRECTED IN ACCORDANCE WITH THE LATEST AISI CODE. ALL CONNECTIONS, INCLUDING AT HPS SECTIONS, SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH THE LATEST AISI CODE. UNLESS INDICATED OTHERWISE ON CONTRACT DOCUMENTS, IN ADDITION TO THE SHEAR CONNECTION, INCLUDE AS A MINIMUM, 45X30X8 ANGLES TOP AND BOTTOM OR ENPLATE AT ALL HPS BEAMS/JOISTS TO COLUMN CONNECTIONS. ALL WIDE FLANGE SHAPES SHALL BE ASTM A992. ALL OTHER STRUCTURAL STEEL SHALL BE ASTM A572 UNLESS NOTED OTHERWISE.

2. ALL STEEL RECTANGULAR/ROUNDED HOLLOW STRUCTURAL SECTIONS SHALL BE ASTM A500 GRADE C, FY = 50 KSI.

3. ALL STEEL ROUND HOLLOW STRUCTURAL SECTIONS SHALL BE ASTM A500 GRADE C, FY = 48 KSI.

4. ALL STEEL SHALL HAVE A SHOP COAT OF RUST INHIBITING PAINT.

5. ALL STEEL PAINT ON ALL STEEL TO RECEIVE SPRAYED-ON FIREPROOFING OR CONCRETE ENCASEMENT, AS NOTED ON ARCHITECTURAL ORIENT ALL MILL CAMBER UPWARD DURING FABRICATION AND ERECTION.

6. ALL STEEL SHALL BE THOROUGHLY CLEANED IN ACCORDANCE WITH SSPC-SP9 PRIOR TO PAINTING.

7. SHOP AND FIELD WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS, AS DESCRIBED IN "AMERICAN WELDING SOCIETY'S STANDARD QUALIFICATION PROCEDURE"; AWS D1.1 LATEST EDITION, TO PERFORM THE TYPE OF WORK REQUIRED.

8. UNLESS OTHERWISE NOTED, ALL CONNECTIONS SHALL BE BOLTED WITH MINIMUM 3/4-INCH DIAMETER A325 OR A490 HIGH STRENGTH BEARING TYPE BOLTS OR WELDED. THE FABRICATOR IS RESPONSIBLE FOR THE SELECTION, DESIGN, AND DETAILING OF ALL CONNECTIONS, INCLUDING BUT NOT LIMITED TO MOMENT CONNECTIONS, BRACED FRAME CONNECTIONS, AND TRUSS CONNECTIONS, NOT FULLY DETAILED ON THE CONTRACT DRAWINGS. THIS INCLUDES TO DESIGN, DETAIL, FURNISH, AND INSTALL STIFFENERS, CONTINUITY PLATES, DOUBLER PLATES, OR OTHER NECESSARY ADDITIONAL LOCAL STRENGTHENING MEASURES AS REQUIRED. MEMBER SIZES INDICATED ON THE DRAWINGS ARE BASED ON MEMBER BEARING AWAY FROM CONNECTIONS.

9. UNLESS OTHERWISE NOTED, DETAILS INDICATED ON DRAWINGS INDICATE GENERAL CRITERIA FOR DESIGN AND DETAILING OF CONNECTIONS. DETAILS INDICATED ON DRAWINGS ARE NOT INTENDED TO CONVEY COMPLETE CONNECTION SIZES, PLATE SIZES, WELD SIZES, NUMBER OF BOLTS, OR ANY OTHER SPECIFIC INFORMATION THAT IS OBTAINED THROUGH DESIGNING OF AN INDIVIDUAL CONNECTION FOR A GIVEN SET OF LOADS. THESE DETAILS DO NOT SHOW ERECTION AIDS. PROVIDE ERECTION AIDS AS REQUIRED AND REMOVE THEM AFTER WORK IS COMPLETE.

10. ALL ANCHOR RODS TO BE ASTM F1554 GRADE 36, UNLESS NOTED OTHERWISE.

11. ALL ALUMINUM AND STEEL MEMBERS SHALL BE TREATED OR PROPERLY SEPARATED TO PREVENT GALVANIC AND CORROSION EFFECTS.

12. ALL STEEL WELDING RODS SHALL BE E70XX.

13. SUBMIT ALL STEEL SHOP DRAWINGS FOR REVIEW PRIOR TO ANY FABRICATION. SHOP DRAWINGS SHALL SHOW COMPLETE BOLTING AND WELDING INFORMATION, BOTH SHOP AND FIELD. ALL WELDING INFORMATION SHALL USE AMERICAN WELDING SOCIETY SYMBOLS. SHOP OR FIELD DETAILS DO NOT SHOW ERECTION AIDS. PROVIDE ERECTION AIDS AS REQUIRED AND REMOVE THEM AFTER WORK IS COMPLETE.

14. UNLESS OTHERWISE NOTED, DETAILS INDICATED ON DRAWINGS INDICATE GENERAL CRITERIA FOR DESIGN AND DETAILING OF CONNECTIONS. DETAILS INDICATED ON DRAWINGS ARE NOT INTENDED TO CONVEY COMPLETE CONNECTION SIZES, PLATE SIZES, WELD SIZES, NUMBER OF BOLTS, OR ANY OTHER SPECIFIC INFORMATION THAT IS OBTAINED THROUGH DESIGNING OF AN INDIVIDUAL CONNECTION FOR A GIVEN SET OF LOADS. THESE DETAILS DO NOT SHOW ERECTION AIDS. PROVIDE ERECTION AIDS AS REQUIRED AND REMOVE THEM AFTER WORK IS COMPLETE.

15. ALL EXPOSED STEEL, INCLUDING BUT NOT LIMITED TO DRAINAGE FRAMING, SCREEN WALL FRAMING, CANOPY FRAMING, LINTELS/SHELF ANGLES EXTERIOR WALLS, ETC.) SHALL BE HOT-DIP GALVANIZED. ANY POINTS OF WELDING SHALL BE TOUCHED UP IN THE FIELD WITH A ZINC-RICH PAINT BY THE STEEL ERECTOR.

16. SPANDREL ANGLE AT PERIMETER EDGE OF FLOOR SLAB/ROOF SHALL BE ADJUSTABLE. SHIP ANGLE LOOSE AND SET WITH STRIKE LINE IN FIELD FOR VERTICAL AND HORIZONTAL ALIGNMENT AFTER STEEL IS FULLY ERECTED TO A MAXIMUM TOLERANCE OF 1/8 INCH HORIZONTAL PER BAY/FLOOR AND MUST BE SET PLUMB BY STEEL ERECTOR PRIOR TO STUD ERECTION. ANGLE MUST BE INSTALLED IN ONE LENGTH PER BAY. SEE TYPICAL SPANDREL ANGLE DETAIL.

17. PROVIDE WELDED STIFFENER PLATES ON BOTH SIDES OF THE WEB OF BEAMS AT POINTS OF CONCENTRATED LOADS INCLUDING BEAMS SUPPORTING COLUMNS OR RUNNING OVER THE TOPS OF COLUMNS, OR OTHER BEAMS. MINIMUM STIFFENER PLATE THICKNESS SHALL BE 3/8 INCH FLOOR THICKNESS OF COLUMN ABOVE OR BELOW OR BEAM WEB THICKNESS ABOVE OR BELOW, WHICHEVER IS GREATER.

18. ALL POST-INSTALLED EXPANSION ANCHORS FASTENED INTO CONCRETE SHALL BE HELIX-TORQ BOLT-TZ WITH MATERIAL TYPE, DIAMETER, AND EMBEDMENT PER DOCUMENTS, UNLESS NOTED OTHERWISE. ALL POST-INSTALLED ADHESIVE ANCHORS FASTENED INTO CONCRETE AND REINFORCING BAR DOWELING INTO CONCRETE SHALL USE HELIX-TIF-RE 500A EPOXY ADHESIVE ANCHORING SYSTEM IN HAMMER-DRILLED HOLES WITH NO. 2 TYPE, DIAMETER, EMBEDMENT, AND SPACING/DISTANCE PER DOCUMENTS, UNLESS NOTED OTHERWISE.

19. ALL PIPING RUNS LARGER THAN 4" DIAMETER SHALL BE HUNG DIRECTLY FROM STEEL BEAMS AND NOT THE CONCRETE SLAB ON METAL DECK SYSTEM (NO UTILITIES SHALL BE HUNG FROM METAL ROOF DECK). ANY SUPPLEMENTAL STEEL REQUIRED FOR BUILDING SYSTEMS (MECHANICAL, ELECTRICAL, PLUMBING, ETC.) IS NOT BY O'DONNELL & NACCARATO.

20. THE CONTRACTOR SHALL DELIVER TO THE ENGINEER, AT THE END OF THE JOB, ONE (1) ELECTRONIC VERSION OF THE FINAL FIELD COPIES OF ALL STEEL ERECTION DRAWINGS SHOP DRAWINGS.

DECK

1. STEEL ROOF DECK SHALL BE GALVANIZED 117" 23 GAUGE TYPE B METAL DECK GRADE 33 (MINIMUM FY = 33 KSI) AS MANUFACTURED BY CANAM OR APPROVED EQUAL. MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE. ROOF DECK FABRICATION AND INSTALLATION MUST COMPLY WITH STEEL DECK INSTITUTE STANDARDS. ALL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS. SUPERSIDED CEILING, LIGHT FIXTURES, DUCTS, PIPES (INCLUDING FIRE PROTECTION), OR OTHER UTILITIES SHALL NOT BE SUPPORTED BY THE STEEL DECK.

2. ATTACH TYPE B METAL ROOF DECK TO STRUCTURAL STEEL SUPPORTS WITH 5/8" DIAMETER PLUDDLE WELDS, OR EQUIVALENT MECHANICAL FASTENER (4 CONNECTIONS (MIN. UNO) PER 36" WIDE SHEET PER SUPPORT) FASTEN SIDE JOINTS TOGETHER WITH #10 SELF-DRILLING SCREWS, OR WELD AT MID-SPAN BETWEEN SUPPORTS.

3. USE WELDING WASHERS ON ALL CONNECTIONS OF STEEL DECK WITH METAL THICKNESS LESS THAN 22 GAUGE TO STRUCTURAL STEEL SUPPORTS.

4. IN AREAS OF WARPED ROOF DECK USE SELF-DRILLING SCREWS FOR CONNECTIONS OF STEEL ROOF DECK TO STRUCTURAL STEEL SUPPORTS. SCREWS SHALL COMPLY WITH MANUFACTURER'S REQUIREMENTS. ATTACH DECK TO ALL SUPPORTING MEMBERS.

5. THE CONTRACTOR SHALL DELIVER TO THE ENGINEER, AT THE END OF THE JOB, ONE (1) ELECTRONIC VERSION OF THE FINAL FIELD COPIES OF ALL DECK LAYOUT SHOP DRAWINGS.

MASONRY

1. MASONRY UNITS SHALL BE TYPE 1-N MEDIUM WEIGHT ASTM C90 HOLLOW, WITH MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI. ALL CMU SHALL BE Laid IN A FULL BED OF MORTAR. CONSTRUCT COLUMN PIERS INTEGRALLY WITH FOUNDATION WALLS AND CONTIGUOUS WALL REINFORCEMENT THROUGH THE PIER. GROUT COLUMN PIERS AND WALLS MONOLITHICALLY.

2. FOLLOWING ARE THE BLOCK STRENGTHS REQUIRED: ASTM C90 HOLLOW 2000 PSI ON NET AREA OF INDIVIDUAL UNITS.

3. ALL MORTAR SHALL BE ASTM C270 TYPE S WITH A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS.

4. GROUT SHALL BE A HIGH SLUMP MIX IN ACCORDANCE WITH ASTM SPECIFICATION C476, HAVING A MINIMUM COMPRESSIVE STRENGTH OF 1000 PSI BUT NOT LESS THAN 2000 PSI.

5. Laid UP MASONRY DESIGN (M) IS 2000 PSI FOR STANDARD CONCRETE MASONRY.

6. VERTICAL REINFORCING SHALL BE ASTM A615, GRADE 60 DEFORMED BARS. MINIMUM DEVELOPMENT AND LAP SPICE LENGTHS TO BE PER "TENSION DEVELOPMENT AND LAP SPICE LENGTH FOR MASONRY" TYPICAL DETAIL TABLE (U.N.O. ON PLAN). MECHANICAL SPLICING DEVICES WHICH ARE RATED TO DEVELOP 75 PERCENT OF FY OF THE BAR MAY BE SUBSTITUTED. SUBMIT PRODUCT DATA FOR ENGINEER'S APPROVAL.

7. ALL CONCRETE MASONRY SHALL BE CONSTRUCTED AND ERRECTED IN ACCORDANCE WITH THE GOVERNING MASONRY CODE (TMS 402) AND SPECIFICATIONS (TMS 602).

8. PROVIDE HOT-DIPPEL GALVANIZED TRUSS TYPE OR LADDER TYPE HORIZONTAL JOINT REINFORCEMENT, MINIMUM 9 GA. AT 18 INCHES ON CENTER BETWEEN ALL MASONRY WALLS. BRACE HORIZONTAL JOINT REINFORCEMENT AT 8 INCHES ON CENTER IN ALL PARAPETS. USE SHOP FABRICATED SPECIAL PIERES AT ALL CORNERS AND TEES.

9. AS A MINIMUM, ALL CORES CONTAINING VERTICAL REINFORCING ARE TO BE GROUTED SOLID.

NOTES:

1) ALL LENGTHS ARE IN INCHES.

2) FOR BAR PLACEMENT, EDGE DISTANCE, "CENTER" INDICATES CENTER OF BAR ALONGS WITH CENTER OF BLOCK THICKNESS. "EDGE" INDICATES CENTER OF BAR TO OUTSIDE FACE OF BLOCK. IS AS FOLLOWS:

- #3-#6 = 2.375"
- #7 = 2.625"
- #8 = 3.0"
- #9 = 3.375"

3) WHERE (2) BARS PER CELL OCCUR THEY SHALL BE PLACED PER EDGE CONDITION, SEE NOTE #2.

4) #1 CMU (2) BARS PER CELL IS NOT PERMITTED.

5) CMU (2) BARS PER CELL UP TO #5 ARE PERMITTED. 10" CMU (2) BARS PER CELL UP TO #7 ARE PERMITTED.

6) "NP" INDICATES NOT PERMITTED.

7) #10 AND #11 BARS WHERE SHOWN ON PLAN OR DETAILS REQUIRE A MECHANICAL SPLICE.

8) FOR EPOXY-COATED BARS INCREASE LENGTHS SHOWN BY 50%.

MASONRY DESIGN STRENGTH	f'm =												
	1500 psi	f'm = 2000 psi	f'm = 2500 psi	f'm = 3000 psi	f'm = 3500 psi	f'm = 4000 psi							
CMU THICKNESS	3"	BAR PLACEMENT	CENTER	EDGE	CENTER	EDGE	CENTER	EDGE	CENTER	EDGE	CENTER	EDGE	
		#3	12	13	12	12	12	12	12	12	12	12	
	#4	14	24	12	21	12	18	12	17	12	16	12	
	#5	22	38	19	33	17	30	16	27	15	25	14	
	8"	#6	43	74	37	64	33	57	30	52	28	48	26
		#7	59	92	51	79	46	71	42	65	34	60	36
#8	81	121	79	105	71	94	64	85	60	79	56	74	
12"	#9	BAR PLACEMENT	CENTER	EDGE	CENTER	EDGE	CENTER	EDGE	CENTER	EDGE	CENTER	EDGE	
		#9	116	NP	102	NP	91	NP	83	NP	77	NP	72
	#10	12	13	12	12	12	12	12	12	12	12	12	
	#11	12	24	12	21	12	18	12	17	12	16	12	
	#12	14	38	12	33	12	30	12	27	12	25	12	
	#13	27	74	23	64	21	57	19	52	18	48	17	
#14	37	92	32	79	29	71	26	65	24	60	23		
#15	57	121	49	105	44	94	40	85	37	79	35		
#16	73	136	63	118	56	105	51	96	48	89	45		

A.B.	ANCHOR BOLT	L.P.	LOW POINT
A.F.F.	ABOVE FINISH FLOOR	L.W.	LIGHT WEIGHT
ADDL.	ADDITIONAL	LLH	LONG LEG HORIZONTAL
ALT.	ALTERNATE	LLV	LONG LEG VERTICAL
ARCH.	ARCHITECT	LWB	LONG WAY BOTTOM
B.C.E.	BOTTOM CHORD EXTENSION	M.E.P.	MECHANICAL ELECTRICAL PLUMBING
B.O.	BOTTOM OF	M.S.T.	METAL STUD TRUSS
BLDG.	BUILDING	MAX.	MAXIMUM
BM.	BEAM	MECH.	MECHANICAL
BOTT.	BOTTOM	MEZZ.	MEZZANINE
BRG.	BEARING	MFR.	MANUFACTURER
BSMT.	BASEMENT	MIN.	MINIMUM
BP.	BEARING PLATE	MISC.	MISCELLANEOUS
BTHN.	BETWEEN	MP.	MASONRY PIER
C.	CENTERLINE	NBL	NON BEARING LINTEL
CANT.	CANTILEVER	N.T.S.	NOT TO SCALE
CMU	CONCRETE MASONRY UNIT	N.W.	NORMAL WEIGHT
COL.	COLUMN	o/c	ON CENTER
CONC.	CONCRETE	P.A.F.	POWDER ACTUATED FASTENER
CONN.	CONNECTION	P.	PLATE
CONT.	CONTINUOUS	PC	PILE CAP
CTRD.	CENTERED	P/C	PRECAST
ø	DIAMETER	PSF	POUNDS PER SQUARE FOOT
DWG.	DRAWING	PSI	POUNDS PER SQUARE INCH
E.F.	EACH FACE	PTN.	PARTITION
E.O.D.	EDGE OF DECK	REINF.	REINFORCEMENT
E.O.S.	EDGE OF SLAB	REQ'D.	REQUIRED
E.W.	EACH WAY	RET'G.	RETAINING
EA.	EACH	S.F.	STEP FOOTING
ELEV.	ELEVATION	S.O.G.	SLAB ON GRADE
ELV.	ELEVATOR	SCHED.	SCHEDULE
EMBED.	EMBEDMENT	SECT.	SECTION
EQ.	EQUAL	SIM.	SIMILAR
EQUIP.	EQUIPMENT	SPECS.	SPECIFICATIONS
EWB	EACH WAY BOTTOM	STIFF.	STIFFENER
EWY	EACH WAY TOP	STRUCT.	STRUCTURAL
EX.	EXISTING	SWB	SHORT WAY BOTTOM
EXIST.	EXISTING	T/B	TOP AND BOTTOM
EXP.	EXPANSION	T.	TOP
EXT.	EXTERIOR	T.O.	TOP OF
FDN.	FOUNDATION	T.O.C.	TOP OF CONCRETE
FIN.	FINISH	T.O.S.	TOP OF STEEL
FLR.	FLOOR	T.S.	THICKENED SLAB
FT.	FEET	TCELE	TOP CHORD EXTENSION LEFT END
FTG.	FOOTING	TCERE	TOP CHORD EXTENSION RIGHT END
GA.	GAGE	TDS	TURN DOWN SLAB
GALV.	GALVANIZED	THK.	THICK OR THICKENED
GB.	GRADE BEAM	TYP.	TYPICAL
H.P.	HIGH POINT	U.N.O.	UNLESS NOTED OTHERWISE
HORIZ.	HORIZONTAL	V.I.F.	VERIFY IN FIELD
I.F.	INSIDE FACE	VERT.	VERTICAL
IN.	INCHES	W.R.T.	WOOD ROOF TRUSS
INFO.	INFORMATION	w/	WITH
INT.	INTERIOR	WC	WET COLUMN
JT.	JOINT	WP	WALL PLATE
k	KIP	WWF	WELDED WIRE FABRIC
k-ft	KIP-FEET		

