

GEOTECHNICAL ENGINEERING REPORT

US 491 & N30 INTERSECTION PROJECT MEXICAN SPRINGS, NEW MEXICO

PREPARED FOR:

WILSON & COMPANY, INC., ENGINEERS AND ARCHITECTS

4401 MASTHEAD STREET NE #150 ALBUQUERQUE, NM 87109

PROJECT NO.: US0033333.0562

DATE: SEPTEMBER 05, 2024

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September 05, 2024

Confidential

Wilson & Company, Inc., Engineers and Architects
4401 Masthead St. NE, Suite 150
Albuquerque, NM 87109

Attention: Mr. Thaddeus Yazzie, P.E., Civil Engineer

**Re: Geotechnical Engineering Report
US 491 & N30 Intersection
Mexican Springs, New Mexico**

Dear Mr. Yazzie,

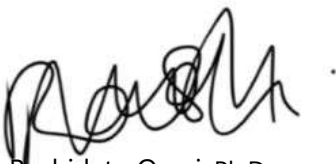
WSP USA Environment & Infrastructure Inc. (WSP) has performed a geotechnical investigation for the proposed US 491 & N30 Intersection project in Mexican Springs, New Mexico. The results of our field investigation and laboratory testing programs along with geotechnical engineering recommendations for the project are presented in this report.

Should you have any questions concerning the recommendations presented in this report, please do not hesitate in contacting us.

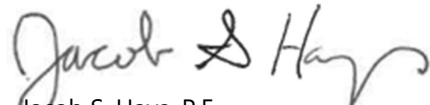
Sincerely,

WSP USA Environmental & Infrastructure Inc.

Reviewed by:



Rashidatu Ossai, Ph.D.
Staff Geotechnical Engineer



Jacob S. Hays, P.E.
Lead Consultant



Jason Alcantar, E.I.
Staff Geotechnical Engineer

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EXECUTIVE SUMMARY

This summary of the project geotechnical engineering considerations is presented for introductory purposes and should be used only in conjunction with the full text of this report.

Project Description: The project consists of pavement investigation for roadway improvements along US 491 and Indian Service Route 30 (N30) in Mexican Springs, New Mexico. The engineering properties of the subsurface soils underlying the proposed site were evaluated to provide recommendations for pavement rehabilitation and construction and to confirm that New Mexico Department of Transportation (NMDOT) standard foundation drawings for roadway lighting support structures are appropriate for the site. The approximate location of the project can be seen in the Site and Exploration Plan, Appendix A.

Exploratory Methods: We explored the subsurface conditions by drilling Four (4) hollow stem auger borings (designated B-01, B-02, B-03, and B-04). Standard Penetration Testing (SPT – ASTM D1586) was performed and split-barrel soil samples were taken at 2.5, 5, 7.5, 10 and at 5-foot intervals thereafter, advanced at the approximate locations shown on the Site and Exploration Plan.

Groundwater Conditions: At the time of exploration (May 6, 2024), groundwater was not encountered in the exploration borings.

Light Pole Foundations: WSP recommends the use of drilled shaft foundations based on the cohesive soils encountered (i.e., medium stiff lean clay) within the project site and their potential to heave. The drilled shaft lengths ranging from 6 to 8.5 feet in the NMDOT Roadway Lighting Support Structures standard drawings are acceptable to use for this site.

Pavement Design: We are providing recommendations for a flexible pavement design comprised of Hot Asphalt Mix (HMA) and Aggregate Base Course (ABC) for the roadway improvements. The in-situ site soils are clayey and have moisture contents near the tested plastic limits. Because of this, the bottom of excavation may become unstable and require stabilization to mitigate “pumping” effects. Recommendations for mechanical and chemical stabilization are provided in Section 4.2.

1 SITE AND PROJECT DESCRIPTION

This report presents a summary of the results of the geotechnical and pavement investigation for roadway improvements along US 491 and Indian Service Route 30 (N30) in Mexican Springs, New Mexico. The intersection improvements will include approximately 2,500-feet of lane expansion/roadway improvements along US 491 and 200-feet of improvements for N30. The objective of this study was to evaluate the engineering properties of the subsurface soils underlying the proposed site and provide recommendations for pavement rehabilitation and construction and also confirm if New Mexico Department of Transportation (NMDOT) standard foundation drawings for lighting structures are appropriate for the site by verifying that the conditions are within the design parameters shown in the NMDOT standard drawings (Sheets 707-07-8/10, 707-07-9/10, and 707-07-10/10).

The conclusions and recommendations contained in this report are based on our understanding of the project, written information, and verbal information supplied to us. Consequently, if any significant changes are made, we may need to modify our conclusions and recommendations contained herein to reflect those changes.

2 EXPLORATORY METHODS

The surface and subsurface conditions at the project site were explored by WSP on May 6th, 2024. Our exploration and testing program comprised the following elements:

- A review of NMDOT standard foundation drawings.
- A visual surface reconnaissance of the site.
- Four (4) hollow stem auger borings (designated B-01, B-02, B-03, and B-04) with SPT samples taken at 2.5 to 5-foot intervals. Refer to Figure 1 in Appendix A for the Site Plan and boring layout.
- Laboratory testing consisting of:
 - grain-size distribution analyses,
 - Atterberg limit tests,
 - moisture content and moisture density relationship tests,
 - Sulfate and Chlorides tests,
 - R-Value tests, and
 - pH and soil resistivity tests.

Table 1 summarizes the approximate locations, surface elevations and termination depths of the test borings performed for this investigation. Appendix A includes the boring logs and describes the field exploration procedures. Appendix B presents the laboratory testing procedures and results.

Table 1: Boring Locations, Elevations, and Depths

Borehole	Proposed Facility Location	Latitude/Longitude	Surface Elevation (feet)	Termination Depth (feet)
B-01	US 491 Southbound Shoulder (East of N30)	35° 46' 37.96"N, 108° 46' 19.11"W	6252	15
B-02	US 491 Southbound shoulder (East of N30)	35° 46'37.08"N, 108°46'18.95"W	6252	5
B-03	US 491 Southbound shoulder (West of N30)	35° 46'44.22"N, 108°46'17.97"W	6249	5
B-04	US 491 Northbound shoulder	35° 46'42.49"N, 108°46'17.63"W	6250	5

Note(s):

1. Elevation datum: Google Earth Pro, accessed 6/25/2024.

The number and locations of our test borings were developed based upon the locations of the proposed pavement improvements and lighting structures as indicated on our site and exploration plan, Appendix A. Boring locations shown in Appendix A and elevations shown in table 1 are based on the Google Earth Pro (accessed June 25, 2024) and should be considered accurate only to the degree permitted by our data sources and implied by our measuring methods.

It should be noted that the test borings performed and used for this evaluation reveal subsurface conditions only at discrete locations across the project site, and that actual conditions in other locations could vary. Furthermore, the nature and extent of these variations will not become evident until construction activities have begun. If significant variations are observed, we may need to modify the conclusions and recommendations contained in this report to reflect the actual site conditions.

3 SITE CONDITIONS

This section presents our findings and interpretations supporting our foundation recommendations.

3.1 Existing Development Conditions

The project site is located at the intersection of US 491 and N30 in Mexican Springs, New Mexico. US 491 is a four-lane highway which permitted all drilling operations and lane closures to take place in the median, the right lane, or shoulder.

3.2 Surface Conditions

The site surface grades are relatively flat, with topographic relief of one to two feet. Vegetation within the project area consists of native grasses, and shrubs.

3.3 Soil Conditions

A generalized subsurface profile was developed using the borings for this project (Table 2). It is recognized that soil properties vary within short horizontal and vertical deviations from the locations

drilled. The exploration logs included in Appendix A provide a description of the soil strata encountered in our boring locations.

Geotechnical laboratory tests revealed that the on-site soils had medium fines (silt and clay) content, ranging from 13 to 20 percent. Moisture content of these soils ranged from 4.9 to 17.3 percent.

Bulk samples, from a depth of 0 to 5 feet of borings B-01 and B-03 were tested to determine the electrical resistivity, pH, and sulfate and chloride contents of the soil in accordance with ASTM G187, ASTM D4972, AASHTO T290 and AASHTO T291, respectively.

The laboratory testing sheets presented in Appendix B show the laboratory test results, and Table 3 summarizes test results for selected samples upon which grain-size analyses were performed.

Table 2: Subsurface Geotechnical Profile

Description of Soil Unit	Layer Thickness (Feet) ⁽¹⁾	Typical Soil
Medium dense to dense clayey sand	0 – 5	A-6 (SC)
Medium stiff lean and high plasticity clay	5 – 10 ⁽²⁾	A-7 (CL)
Loose to medium dense silty clayey sand and silty sand	10 – 16.5 ⁽²⁾	A-2-4, A-2-7 (SM, SC-SM)

Note(s):

1. Borings terminated at a depth of 15 feet for B-01 and 5 feet for all other boreholes with a 1.5-foot sampler extended to 16.5 feet and 6.5 feet.

2. Based on information from Boring B-01.

Table 3: Laboratory Test Results

Borehole, Depth (feet)	Soil Type (AASHTO Soil Type)	P.I.	Moisture Content (percent)	NMDOT R-Value (60% Risk)	Grain Size Analysis		
					Gravel (percent)	Sand (percent)	Silt/Clay (percent)
B-01, 0 - 5.0	A-6	13	8.5	10	0	51	49
B-01, 5.0	A-6	20	9.2	8	0	24	76
B-01, 15.0	A-2-4	NP ⁽¹⁾	4.9	-	0	71	29
B-02, 0 - 5.0	A-6	20	15.3	8	1	50	49
B-03, 0 - 5.0	A-6	13	17.3	11	1	60	39
B-04, 0 - 5.0	A-6	15	8.8	10	6	47	47

Note(s):

1. NP = Non-plastic

The results of electrical resistivity, pH, and sulfate and chloride tests are summarized in Table 4. Soils with resistivity within the range of 0 to 1000 ohms-centimeters are considered severely corrosive and 1,000 to 2,000 ohms-centimeters are considered corrosive. Based on the laboratory results, the corrosivity of the soil in the upper 5 feet of the site varies from corrosive to severely corrosive. Laboratory test results are presented in Appendix B.

Table 4: Chemical Test Results

Borehole, Depth	pH	Minimum Resistivity (Ω -cm)	Sulfate Content (ppm)	Chloride Content (ppm)
B-01, @ 0 - 5.0 feet	7.8	804	268	353

Borehole, Depth	pH	Minimum Resistivity (Ω -cm)	Sulfate Content (ppm)	Chloride Content (ppm)
B-03, @ 0 - 5.0 feet	8.3	1340	47	107

3.4 Groundwater Conditions

At the time of exploration (May 6, 2024), groundwater was not encountered in the exploration borings. Throughout the year, groundwater levels will fluctuate in response to changing precipitation patterns, construction activities, and site use.

4 CONCLUSIONS AND RECOMMENDATIONS

This section presents our geotechnical engineering conclusions and recommendations concerning site preparation and foundations. ASTM International (ASTM) specification codes cited herein refer to the most current applicable ASTM manual. A design parameter summary of the soil is presented below:

Clayey sand (SC) layer (0 – 5 feet and 10 – 15 feet):

Soil moist unit weight, $\gamma = 110$ (pounds per cubic feet, pcf)

Soil Friction Angle, $\theta = 30^\circ$

Lean clay (CL) layer (5 – 10 feet, based on B-01):

Soil dry unit weight $\gamma_{dry} = 83$ pcf

Soil moist unit weight, $\gamma = 92$ pcf

Cohesion, $c = 880$ psf

4.1 Light Pole Foundations

WSP recommends the use of drilled shaft foundations based on the cohesive soils encountered within the project site and their potential to heave.

Analysis of the subsurface soil conditions at the site indicate that the soils at the top 5 feet and bottom 10 to 15 feet are comprised of mostly of loose to dense clayey sands with an estimated unit weight of 110 pcf and friction angle of 30° and medium stiff lean clay with a unit weight of 92 pcf and cohesion of 880 psf at depths of 5 to 10 feet. The standard penetration testing (SPT) value determined during the subsurface field exploration provides additional data for the medium stiff lean clay layer encountered. Based on the SPT values obtained within this layer we expect the unit weight for a medium stiff clay to be at or greater than the minimum requirement unit weight presented in the NMDOT Roadway Lighting Support Structures standard drawings (i.e., soil unit weight of 100 pcf). Therefore, the recommended drilled shaft lengths in the NMDOT standard drawings ranging from 6 to 8.5 feet, are feasible and light pole foundation design standards are acceptable for this project.

Should the design parameters change significantly from NMDOT Standards, we may need to modify our conclusions and recommendations contained herein to reflect those changes.

4.2 Pavement Design Recommendations

A flexible pavement design for the project area is presented in the subsequent sections. The pavement design is in accordance with the 1993 AASHTO Guide for Design of Pavement Structures and NMDOT methodology.

4.2.1 SITE CONDITIONS

Based on field investigation, the existing pavement for US 491 consists of Asphalt Cement Pavement (ACP) with a nominal thickness of four (4) to five (5) inches supported by approximately three (3) inches of base course. The pavement conditions indicated signs of distress primarily in the form of longitudinal and fatigue cracking (referred to as alligator cracking). The fatigue cracks are a series of interconnected cracks of the asphaltic concrete surface under repeated traffic loading. It is a load associated distress that is also considered to be a major structural distress. The upper five (5) of subsurface soil consisted of clayey sands as described in Section 3.3 and Appendix A.

R-value tests were performed on the clayey soils of B-02 and B-03 in accordance with AASHTO T190. Thickness of asphalt cores, estimated base course thickness, and R-value results are summarized in Table 5.

Table 5: Summary of Existing Pavement Design

Boring Designation	Location	Asphalt Thickness (inches)	Base Course Thickness ¹ (inch)	R-Value (0 to 5 feet)
B-02, 0 to 5'	US 491 Southbound shoulder (East of N30)	5	3	21
B-03, 0 to 5'	US 491 Southbound shoulder (West of N30)	4	3	45

Note(s):

1. Untreated Aggregate.

4.2.2 TRAFFICE ANALYSIS

Traffic data for US 491 and Indian Service Route 30 (N30) was provided to WSP by Wilson and Company Inc., on July 5th, 2024. The data included Average Annual Daily Traffic (AADT) for a 24-hour interval per vehicle type. The information was classified in accordance with Federal Highway Administration's (FHWA) vehicle class based on the vehicle type and number of axles. The calculation of total ESAL's for the project site using an AADT of 3,213, a 20-year pavement design life, and a 2.0% growth factor is 3,387,446 ESALs. The provided traffic information is attached in Appendix C.

Table 6 below summarizes the modified ESAL conversion factors used in WSP's pavement design analysis based on the provided traffic data.

Table 6: Vehicle Type, Percentage and ESAL Equivalency¹

FHWA Vehicle Classification	ESAL Equivalency Factor	Traffic Percentage (US 491)
Class 1,2, and 3	0.0008	79.0%
Class 4	0.33	1.1%
Class 5	0.50	5.0%
Class 6	0.85	1.8%
Class 7 and 8	1.20	4.1%
Class 9	1.55	8.0%
Class 10, 11, 12, and 13	2.24	1.0%

Note(s):

1. ESAL = Equivalent 18-kip Single Axle Load.

The calculated ESAL's are about 75% of what would be expected by WSP based on experience and review of other pavement design projects recently performed for US 491. The calculated ESAL's for US 491 were 3.4 million. Review of the ESAL's considered for recent projects along US 491, near mileposts 10 and 40, indicated ESAL's closer to 4.7 million.

It is our opinion that there could be two potential causes of the lower reported ESAL's for this project:

- 1) Vehicle counts are normally determined over a 48-hour period, and the AADT can vary significantly depending on what time of the year, month, or week the 48-hour monitoring period occurred.
- 2) Ranges of ESAL equivalency factors are provided by the FHWA and are selected based on the designer's experience.

Due to the discrepancy between the calculated ESAL's and what WSP would expect for this project, we recommend considering at least 4.7 million ESAL's for pavement design.

4.2.3 PAVEMENT DESIGN

In the section below, we have provided a flexible pavement in accordance with the 1993 AASHTO Guide for Design of Pavement Structures methodology, and NMDOT Design Manual Section 620. A design reliability of 85% was used along with an initial serviceability index (P_i) of 4.2 and a terminal serviceability index (P_t) 2.5. These are assumed values and typical for major collector and all arterial roads.

The flexible pavement design considered a 20-year ESALs value of 4,700,000 (4.7 million). Traffic analysis data presented in Section 4.2.2 were used to estimate the ESALs value and is based on data provided by the Wilson & Company Inc for the project site.

Based on the R-value data provided in Section 3.3 and 4.2.1, WSP selected a design R-value of 19 for this project. This value was determined using the lowest laboratory R-value (21) and the standard deviation (1.3) computed from the NMDOT Correlated R-Values to better represent variability within the sample population. The design R-value of 19 represents a level of confidence of 90% considering the one-tailed t-test distribution and a sample population of 5. The site soils appear to be relatively uniform, and we do not anticipate that import will be required to meet the minimum R-value requirement. If fill is necessary

for the roadway expansion, it is recommended to import materials having R-value greater than the design R-value. We recommend that the contractor remove the upper 2 feet of poor subgrade material and replace it with the import or borrow material for areas that do not meet the design R-value. The design structural number for the pavement design is based on the recommended design R-value for onsite subgrade material.

The values for various parameters used in the calculations for the flexible pavement design are provided in

Table 7 below. Additionally, the following equation from NMDOT was used to estimate the resilient modulus based on the design R-value and is presented in the table below:

$$M_R = 10^{(R_{\text{value}} + 222)/67}$$

Table 7: Flexible Pavement Design Parameters

Design Parameters	Value
Initial Serviceability	4.2
Terminal Serviceability	2.5
Reliability Level	85%
Overall Standard Deviation, S_o	0.49
Growth rate of vehicles (%)	2.0
Design R-Value	19 ¹
Resilient Modulus, M_R (psi)	3,954
Structural Number, SN	5.37

Note(s):

1. Estimated R-value based on date from B-02 and correlated NMDOT R-Value (60% Risk) data with a confidence value of 90%.

We recommend the subgrade be stabilized because the onsite soils are clayey and in wet condition. This will improve the potential unstable subgrade at the bottom of the excavation and help mitigate “pumping” effects. The following options are presented in Table 8 below based on the design parameters and expected site conditions.

Option #1: Geogrid Reinforcement

- Placement of triaxial geogrid reinforcement located at the bottom of excavation (i.e., above the untreated subgrade).
- Placement of (2) 6-inch layers of compacted aggregate base course (ABC) and (1) 5-inch layer of compacted aggregate base course (17-inches total thickness of ABC).
- Placement of two (2) layers of 3-inches of Hot Mix Asphalt (HMA) pavement (6-inches total thickness of HMA).

Option #2: Lime or Cement Chemical Stabilization

- Lime or cement treat the upper 10-inches of the clayey soils located at the bottom of excavation.
- Placement of (2) 5-inch layers of compacted aggregate base course (10-inches total thickness).
- Placement of two (2) layers of 3-inches of HMA pavement and (1) layer of 2-inches (8-inches total thickness of HMA)

We recommend aggregate base course and stabilized subgrade be compacted in maximum lift heights of 6 inches in accordance with NMDOT Section 303.

Table 8: Flexible Pavement Section

Option	Hot Mix Asphalt (HMA) Thickness (inches)	Aggregate Base Course (ABC) Thickness (inches)	Subgrade Stabilization Method	Required Structural Number, SN	Resulting SN
Option #1: Geogrid Reinforcement	6	17	Triaxial Geogrid – Tensar NX750 or Equivalent ¹	5.37	5.48
Option #2: Lime or Cement Stabilization	8	10	10" Lime or Cement Treated ²		5.42

Note(s):

1. Treated subgrade mix design required. Cement or lime mixture should achieve unconfined compressive strength of 100 psi at 28-day cure using lime or 200 psi after 7-day cure using cement (to be considered a structural layer).

2. Triaxial geogrid is recommended for mechanically stabilizing the subgrade for constructability purposes.

4.3 Pavement Construction Considerations

If the project is to be governed by NMDOT criteria, Project construction and material specifications shall conform to New Mexico Department of Transportation's 2019 Standard Specifications for Highway and Bridge Construction. At a minimum, the following sections and divisions should apply for the roadways:

- NMDOT Section 203 – Excavation, Borrow, and Embankment
- NMDOT Section 203-A – Unstable Subgrade Stabilization
- NMDOT Section 207 – Subgrade Preparation
- NMDOT Section 303 – Base Course
- NMDOT Division 400 – Surface Treatments and Pavements

It is our recommendation that SP-III and SP-IV be selected for the asphalt pavement. Binder Grade PG 70-22 should be the base binder grade per NMDOT Pavement Design Policy. Additionally, we recommend a Type I base course grading designation (NMDOT-19, Table 303.2.2:1) be specified for the aggregate base course.

It is recommended that Wilson & Company considers a quantity of unstable subgrade using pay item Unstable Subgrade Stabilization Section 203-A due to the inherent risk of variable moisture condition in the field.

The soils throughout the project area that will be encountered during earthwork operations can be excavated with normal earth moving equipment. Heavier equipment may be necessary if cobble and rock is to be excavated (neither bed rock nor cobbles were encountered during the geotechnical investigation).

4.3.1 FHWA SPECIFICATIONS

If the project is to be governed by FHWA criteria, project construction and material specifications shall conform to Standard Specification of Roads and Bridges on Federal Highway Projects FP-14. At a minimum, the following sections and divisions should apply for the roadways:

- FP-14 Section 204. - Excavation and Embankment
- FP-14 Section 301. – Untreated Aggregate Courses
- FP-14 Division 400 Asphalt Pavements and Surface Treatments

It is our recommendation that the HMA have a nominal maximum aggregate size of $\frac{3}{4}$ inches. Additionally, we recommend a "D" Grading Designation (FP-14, Section 703) be specified for the aggregate basecourse.

The soils throughout the project area that will be encountered during earthwork operations can be excavated with conventional earth moving equipment.

5 CLOSURE

The conclusions and recommendations presented in this report are based, in part, on the explorations WSP performed and used for this study; therefore, if variations in the subgrade conditions are observed, we may need to modify this report to reflect those changes. WSP is available to provide geotechnical engineering throughout the design process and to perform monitoring services throughout construction.

APPENDIX

A

FIELD EXPLORATION PROCEDURES,
SITE LOCATION AND EXPLORATION
PLAN, BORING LOGS

APPENDIX

Appendix A – Field Exploration Procedures and Logs

The following paragraphs describe the procedures used for field explorations and field tests that WSP conducted for this project. Descriptive logs of our explorations are enclosed in this appendix.

Auger boring procedures

Exploratory borings were advanced with a hollow-stem auger using a truck-mounted drill rig operated by an independent drilling firm working under subcontract to WSP. An engineer from WSP continuously observed the borings, logged the subsurface conditions, and collected representative soil samples. All samples were stored in watertight containers and later transported to the laboratory for further visual examination and testing. After each boring was completed, the borehole was backfilled with a mixture of bentonite chips and soil cuttings, and the surface was patched with asphalt or concrete (where appropriate).

Throughout the drilling operation, soil samples were obtained at 2.5- or 5-foot depth intervals by means of the Standard Penetration Test (SPT) per ASTM D-1586. This testing and sampling procedure consists of driving a standard 2-inch-diameter steel split-spoon sampler 18 inches into the soil with a 140-pound hammer free-falling 30 inches. The number of blows required to drive the sampler through each 6-inch interval was counted, and the total number of blows struck during the final 12 inches was recorded as the Standard Penetration Resistance, or "SPT blow count." If a total of 50 blows were struck within any 6-inch interval, the driving was stopped and the blow count was recorded as 50 blows for the actual penetration distance. The resulting Standard Penetration Resistance values indicate the relative density of granular soils and the relative consistency of cohesive soils.

The enclosed Boring Logs describe the vertical sequence of soils and materials encountered in each boring, based primarily on field classifications and supported by subsequent laboratory examination and testing. Where a soil contact was observed to be gradational, boring logs indicate the average contact depth. Where a soil type changed between sample intervals, we inferred the contact depth. The boring logs also graphically indicate the blow count, sample type, sample number, and approximate depth of each soil sample obtained from the borings, as well as any laboratory tests performed on these soil samples. If any groundwater was encountered in a borehole, the approximate groundwater depth is depicted on the boring log. Groundwater depth estimates are typically based on the moisture content of soil samples, the wetted height on the drilling rods, and the water level measured in the borehole after the auger has been extracted.

L:\transfer\Ossai\Rashid\U050033333\0562 Mexican Springs\CAD\Site and Exploration Plan - US0033333.0562 US 491 & N30 Intersection Improvement Project.dwg



CLIENT: Wilson & Company, Inc., Engineers and Architects	LEGEND  B-01 Boring Number and Approximate Location	DWN BY: RO	PROJECT: US 491 & N30 Road Intersection Improvements Mexican Springs, New Mexico	REV. NO.:
		CHK'D BY: JH		DATE: 06/27/2024
 Global Inc. 4221 Balloon Park Rd. N.E. Albuquerque, NM 87109		DATUM: NAD83	TITLE: Site and Exploration Plan – US 491 & N30 Road Intersection Improvements Project	PROJECT NO: US0033333.0562
		PROJECTION: N/A		CONTRACT NO: N/A
		SCALE: AS SHOWN		FIGURE NO: 1



PROJECT US 491 & N30 Intersection

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DATE 5/6/24

BORING NO. B-01

WSP PROJECT NO. US0033333.0562

LOCATION Mexican Springs

DRILLING COMPANY / DRILLER GEOMAT INC. / Jose Quintana

RIG TYPE / BORING TYPE CME-55 / H.S.A

SURFACE ELEV.

LOGGER Eric Koenig

Depth in Feet	Continuous Penetration Resistance	Graphical Log	Sample	Sample Type	Blows/ft. 140 lb. 30" free-fall drop hammer	Dry Density lbs. per cubic foot	Moisture Content Percent of Dry Weight	Unified Soil Classification	REMARKS	VISUAL CLASSIFICATION
0								SC		(SC) CLAYEY SAND fine to medium grained, medium plasticity, moist, yellowish grey brown, intermittent cobbles.
					15-28-15		9	SC	Hard	(SC) CLAYEY SAND predominately fine grained, low plasticity, dry, grey brown.
5					2-3-4		9	CL	Medium Stiff	(CL) LEAN CLAY medium plasticity, damp to moist, brown.
					5-8	78	12	CL	Medium Stiff	(CL) LEAN CLAY medium plasticity, damp to moist, brown.
10					4-7	88	7	SC-SM	Loose	(SC-SM) SILTY CLAYEY SAND predominately fine grained, low plasticity, dry, grey brown.
15					3-5-5		5	SM	Medium Dense	(SM) SILTY CLAYEY SAND predominately fine grained, low plasticity, dry, grey brown.
										Augering ended at 15.0', sampling ended at 16.5'.
20										
25										

GROUNDWATER

SAMPLE TYPE

A-Auger cuttings; NR-No Recovery
R-Ring Sample
BS-Bulk Sample.
S-2" O.D. 1.38" I.D. tube sample.
U-3" O.D. 2.42" I.D. tube sample.
T-3" O.D. thin walled Shelby tube.

WSP E&I, Inc.
4221 Balloon Park Road, NE
Albuquerque, NM 87109

GEOTECH_BH_US0033333.0562_MEXICANSPRINGS.GPJ AMEC-DATATMPL-FINAL -JR.GDT 6/27/24



DEPTH	HOUR	DATE



PROJECT US 491 & N30 Intersection

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DATE 5/6/24

BORING NO. B-02

WSP PROJECT NO. US0033333.0562

LOCATION Mexican Springs

DRILLING COMPANY / DRILLER GEOMAT INC. / Fernando Enriquez

RIG TYPE / BORING TYPE CME-55 / H.S.A

SURFACE ELEV.

LOGGER Eric Koenig

Depth in Feet	Continuous Penetration Resistance	Graphical Log	Sample	Sample Type	Blows/ft. 140 lb. 30" free-fall drop hammer	Dry Density lbs. per cubic foot	Moisture Content Percent of Dry Weight	Unified Soil Classification	REMARKS	VISUAL CLASSIFICATION
0										Pavement, 5" asphalt, 3" basecourse.
					4-4-5		15	SC	Loose	(SC) CLAYEY SAND predominately fine to medium grain sand, medium plasticity, moist, yellowish grey brown.
5					5-9-13		13	SC-SM	Medium Dense	(SC-SM) SILTY CLAYEY SAND predominately fine to medium grained sand, low plasticity, damp to moist, grey brown.
										Augering ended at 5.0', sampling ended at 6.5'.
10										
15										
20										
25										

GROUNDWATER

SAMPLE TYPE

A-Auger cuttings; NR-No Recovery
R-Ring Sample
BS-Bulk Sample.
S-2" O.D. 1.38" I.D. tube sample.
U-3" O.D. 2.42" I.D. tube sample.
T-3" O.D. thin walled Shelby tube.

WSP E&I, Inc.
4221 Balloon Park Road, NE
Albuquerque, NM 87109

GEOTECH_BH_US0033333.0562_MEXICANSPRINGS.GPJ AMEC-DATATMPL-FINAL -JR.GDT 6/27/24



DEPTH	HOUR	DATE



PROJECT US 491 & N30 Intersection

Page 1 of 1

DATE 5/6/24

BORING NO. B-03

WSP PROJECT NO. US0033333.0562

LOCATION Mexican Springs

DRILLING COMPANY / DRILLER GEOMAT INC. / Fernando Enriquez

RIG TYPE / BORING TYPE CME-55 / H.S.A

SURFACE ELEV.

LOGGER Eric Koenig

Depth in Feet	Continuous Penetration Resistance	Graphical Log	Sample	Sample Type	Blows/ft. 140 lb. 30" free-fall drop hammer	Dry Density lbs. per cubic foot	Moisture Content Percent of Dry Weight	Unified Soil Classification	REMARKS	VISUAL CLASSIFICATION
0										Pavement approximately 7" thick, approximately 4" of asphalt, approximately 3" of basecourse.
					5-5-5		17	SC-SM	Medium Dense	(SC-SM) SILTY CLAYEY SAND predominately fine to medium grained sand, low plasticity, moist, yellowish gray brown.
5					6-8-10		13	SC	Medium Dense	(SC) CLAYEY SAND fine to medium grained, medium plasticity, damp to moist, gray brown.
										Augering ended at 5.0', sampling ended at 6.5'.
10										
15										
20										
25										

GROUNDWATER

SAMPLE TYPE

A-Auger cuttings; NR-No Recovery
R-Ring Sample
BS-Bulk Sample.
S-2" O.D. 1.38" I.D. tube sample.
U-3" O.D. 2.42" I.D. tube sample.
T-3"O.D. thin walled Shelby tube.

WSP E&I, Inc.
4221 Balloon Park Road, NE
Albuquerque, NM 87109

GEOTECH_BH_US0033333.0562_MEXICANSPRINGS.GPJ AMEC-DATATMPL-FINAL -JR.GDT 6/27/24



DEPTH	HOUR	DATE



PROJECT US 491 & N30 Intersection

Page 1 of 1

DATE 5/6/24

BORING NO. B-04

WSP PROJECT NO. US0033333.0562

LOCATION Mexican Springs

DRILLING COMPANY / DRILLER GEOMAT INC. / Jose Quinatana

RIG TYPE / BORING TYPE CME-55 / H.S.A

SURFACE ELEV.

LOGGER Eric Koenig

Depth in Feet	Continuous Penetration Resistance	Graphical Log	Sample	Sample Type	Blows/ft. 140 lb. 30" free-fall drop hammer	Dry Density lbs. per cubic foot	Moisture Content Percent of Dry Weight	Unified Soil Classification	REMARKS	VISUAL CLASSIFICATION
0								SC		(SC) CLAYEY SAND fine to medium sand, no plasticity, moist, yellowish gray brown.
					9-10-8		8	SC	Medium Dense	(SC) CLAYEY SAND predominately fine to medium grained sand, medium plasticity, damp to moist, yellowish grey brown.
5					6-13-13		9	SC	Medium Dense	(SC) CLAYEY SAND predominately fine to medium grained, no plasticity, damp, gray brown.
										Augering ended at 5.0', sampling ended at 6.5'.
10										
15										
20										
25										

GROUNDWATER

SAMPLE TYPE

A-Auger cuttings; NR-No Recovery
R-Ring Sample
BS-Bulk Sample.
S-2" O.D. 1.38" I.D. tube sample.
U-3" O.D. 2.42" I.D. tube sample.
T-3"O.D. thin walled Shelby tube.

WSP E&I, Inc.
4221 Balloon Park Road, NE
Albuquerque, NM 87109

DEPTH	HOUR	DATE

GEOTECH_BH_US0033333.0562_MEXICANSPRINGS.GPJ AMEC-DATATMPL-FINAL -JR.GDT 6/27/24

APPENDIX

B

LAB PROCEDURES & RESULTS

APPENDIX

Appendix B – Laboratory Testing Procedures and Results

This appendix describes procedures associated with the laboratory tests WSP assigned for this project. Geotechnical laboratory testing was performed by a local, accredited geotechnical testing laboratory, subcontracted to WSP. Results of certain laboratory tests are enclosed in this appendix.

Visual Classification Procedures

Visual soil classifications were conducted on all samples in the field and on selected samples in the laboratory. All soils were classified in general accordance with the Unified Soil Classification System, which includes color, relative moisture content, primary soil type (based on grain size), and any accessory soil types. The resulting soil classifications are presented on the exploration logs contained in Appendix A.

Moisture Content Determination Procedures

Moisture content determinations were performed on representative samples to aid in identification and correlation of soil types. All determinations were made in general accordance with ASTM D-2216. The results of these tests are shown on the exploration logs contained in Appendix A.

Grain-size Analysis Procedures

A grain-size analysis indicates the range of soil particle diameters included in a particular sample. Grain-size analyses were performed on representative samples in general accordance with ASTM D-422. The results of these tests are presented on the enclosed grain-size distribution graphs and were used in soil classifications shown on the exploration logs contained in Appendix A.



Client: Wilson & Company
4401 Masthead St. NE, Suite 150
Albuquerque, NM 87109

Report Date: May 20, 2024

Project #: US0033333.0562

Report #: 2572

Attn: Myra Candelaria

Work Order #: 1

Project Name: Task #86 US 491 & N30 Intersection

Sampled By: Eric Koenig

Date Sampled: 5/6/2024

Mexican Springs, NM

Project Manager: Jacob Hays

SOILS / AGGREGATES

MOISTURE CONTENT OF SOIL (ASTM D2216-19) AND IN-SITU DENSITY

Lab #	Color & Type of Material	Sample Source	Test Method	Oven Temp. (C)	Mass less than Min Req.	Material Type *	Moisture (%)	Dry Density (pcf)
24-0205-02	See Boring Log	B-01 @ 2.5'	B	110	☐	☐	8.5	
24-0205-03	See Boring Log	B-01 @ 5.0'	B	110	☐	☐	9.2	
24-0205-04	See Boring Log	B-01 @ 7.5'	B	110	☐	☐	11.8	78.4
24-0205-05	See Boring Log	B-01 @ 10.0'	B	110	☐	☐	7.1	88.1
24-0205-06	See Boring Log	B-01 @ 15.0'	B	110	☐	☐	4.9	
24-0205-08	See Boring Log	B-02 @ 2.5'	B	110	☐	☐	15.3	
24-0205-09	See Boring Log	B-02 @ 5.0'	B	110	☐	☐	13.2	
24-0205-11	See Boring Log	B-03 @ 2.5'	B	110	☐	☐	17.3	
24-0205-12	See Boring Log	B-03 @ 5.0'	B	110	☐	☐	13.1	
24-0205-14	See Boring Log	B-04 @ 2.5'	B	110	☐	☐	8.2	
24-0205-15	See Boring Log	B-04 @ 5.0'	B	110	☐	☐	9.4	

*Sample contains more than one type of material.

Distribution: Client: ☐ File: ☐ Supplier: ☐ Email: ☐ Other:

WSP
4221 Balloon Park Rd NE
Albuquerque, NM 87109
Tel 5058211801

www.wsp.com



Client: Wilson & Company
4401 Masthead St. NE, Suite 150
Albuquerque, NM 87109

Report Date: May 17, 2024

Attention: Myra Candelaria
Project Name: Task #86 US 491 & N30 Intersection

Mexican Springs, NM

Project #: US0033333.0562
Work Order #: 1
Sampled By: Eric Koenig
Date Sampled: 5/6/2024

Sieve Analysis (ASTM C117-17/C136-19)
Plasticity Index (ASTM D4318-17)
Soil Classification (ASTM D2487-17)

Project Manager: Jacob Hays

SOILS / AGGREGATES

Sample Location	Soil Class.	L.L.	P.I.	Sieve Sizes												Sieve Result are as Percent Passing.							Lab Number	
				#200	#100	#50	#40	#30	#16	#10	#8	#4	1/4"	3/8"	1/2"	3/4'	1"	1 1/4"	1 1/2"	2"	2 1/2"	3"		6"
B-01 @ 0-5.0'	SC	29	14	49	73	97	99	99	99	100														24-0205-01
B-01 @ 5.0'	CL	38	20	76	91	98	99	100																24-0205-03
B-01 @ 15.0'	SM	NV	NP	29	63	97	100																	24-0205-06
B-02 @ 0-5.0'	SC	34	20	49	66	92	94	95	96	97	98	99		100										24-0205-07
B-03 @ 0-5.0'	SC	28	13	39	64	91	94	95	96	97	98	99		100										24-0205-10
B-04 @ 0-5.0'	SC	30	15	47	66	86	89	90	91	92	92	94		96	97	100								24-0205-13

Distribution: Client: ☐ File: ☐ Supplier: ☐ Email: ☐ Other:



Client: Wilson & Company
4401 Masthead St. NE, Suite 150
Albuquerque, NM 87109

Report Date: May 17, 2024

Attention: Myra Candelaria
Project Name: Task #86 US 491 & N30 Intersection

Mexican Springs, NM

Project #: US0033333.0562
Work Order #: 1
Sampled By: Eric Koenig
Date Sampled: 5/6/2024

Sieve Analysis (ASTM C117-17/C136-19)
Plasticity Index (ASTM D4318-17)
Soil Classification (ASTM D2487-17)

Project Manager: Jacob Hays

SOILS / AGGREGATES

Lab Number	Sample Location	Soil Class.	L.L.	P.I.	D10	D20	D30	D50	D60	D70	CC	CU	Cmu
24-0205-01	B-01 @ 0-5.0'	SC	29	14	0	0	0	0.077	0.103	0.138	0	0	0
24-0205-03	B-01 @ 5.0'	CL	38	20	0	0	0	0	0	0	0	0	0
24-0205-06	B-01 @ 15.0'	SM	NV	NP	0	0	0.076	0.114	0.141	0.172	0	0	0
24-0205-07	B-02 @ 0-5.0'	SC	34	20	0	0	0	0.077	0.115	0.165	0	0	0
24-0205-10	B-03 @ 0-5.0'	SC	28	13	0	0	0	0.102	0.136	0.176	0	0	0
24-0205-13	B-04 @ 0-5.0'	SC	30	15	0	0	0	0.082	0.119	0.169	0	0	0

Distribution: Client: ☐ File: ☐ Supplier: ☐ Email: ☐ Other:



Client: Wilson & Company
4401 Masthead St. NE, Suite 150
Albuquerque, NM 87109

Report Date: May 20, 2024

Attn: Myra Candelaria
Project Name: Task #86 US 491 & N30 Intersection
Mexican Springs, NM

Project #: US0033333.0562

Work Order #: 1

Lab #: 24-0205-01

Sampled By: Eric Koenig

Date Sampled: 5/6/2024

Visual Description of See Boring Log

Material:

Sample Source: B-01 @ 0-5.0'

Project Manager: Jacob Hays

SOILS / AGGREGATES

pH of Soils (ASTM D4972)

Method Used: A

pH Value Tested In Water: 7.8

Distribution: Client: ☐ File: ☐ Supplier: ☐ Email: ☐ Other:



Client: Wilson & Company
4401 Masthead St. NE, Suite 150
Albuquerque, NM 87109

Attn: Myra Candelaria

Project Name: Task #86 US 491 & N30 Intersection

Mexican Springs, NM

Project Manager: Jacob Hays

Report Date: May 20, 2024

Project #: US0033333.0562

Work Order #: 1

Lab #: 24-0205-10

Sampled By: Eric Koenig

Date Sampled: 5/6/2024

Visual Description of See Boring Log

Material:

Sample Source: B-03 @ 0-5.0'

SOILS / AGGREGATES

pH of Soils (ASTM D4972)

Method Used: A

pH Value Tested In Water: 8.3

Distribution: Client: ☐ File: ☐ Supplier: ☐ Email: ☐ Other:



Client: Wilson & Company
4401 Masthead St. NE, Suite 150
Albuquerque, NM 87109

Report Date: May 20, 2024

Attn: Myra Candelaria
Project Name: Task #86 US 491 & N30 Intersection
Mexican Springs, NM

Project #: US0033333.0562

Work Order #: 1

Lab #: 24-0205-01

Sampled By: Eric Koenig

Date Sampled: 5/6/2024

Visual Description of See Boring Log

Material:

Sample Source: B-01 @ 0-5.0'

Project Manager: Jacob Hays

SOILS / AGGREGATES

Determining Soil Resistivity Using the Two-Electrode Soil Box Method (ASTM G187-20)

Soil Resistivity: 804 Ω cm

Distribution: Client: ☐ File: ☐ Supplier: ☐ Email: ☐ Other:



Client: Wilson & Company
4401 Masthead St. NE, Suite 150
Albuquerque, NM 87109

Report Date: May 20, 2024

Attn: Myra Candelaria
Project Name: Task #86 US 491 & N30 Intersection

Mexican Springs, NM

Project #: US0033333.0562

Work Order #: 1

Lab #: 24-0205-10

Sampled By: Eric Koenig

Date Sampled: 5/6/2024

Visual Description of See Boring Log

Material:

Sample Source: B-03 @ 0-5.0'

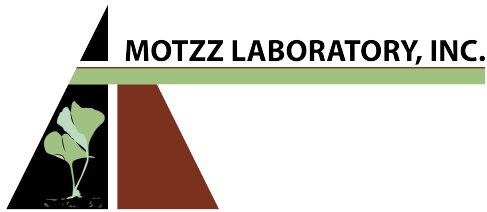
Project Manager: Jacob Hays

SOILS / AGGREGATES

Determining Soil Resistivity Using the Two-Electrode Soil Box Method (ASTM G187-20)

Soil Resistivity: 1340 Ω cm

Distribution: Client: ☐ File: ☐ Supplier: ☐ Email: ☐ Other:



Report: 950907
Reported: 5/22/2024
Received: 5/17/2024
PO: US0033333.05

Laboratory Analysis Report

WSP USA Environment & Infrastructure Inc.
Jesse Boam
8519 Jefferson NE
Albuquerque, NM 87113

Project: US0033333.0562

Lab Number	Sample ID	
950907-1	24-0205-01	B-01 (0-5.0')

Test Parameter			
<i>Test</i>	<i>Method</i>	<i>Result</i>	<i>Units</i>
Sulfate	AASHTO T290	268	ppm
Chloride	AASHTO T291	353	ppm

Lab Number	Sample ID	
950907-2	24-0205-10	B-03 (0-5.0')

Test Parameter			
<i>Test</i>	<i>Method</i>	<i>Result</i>	<i>Units</i>
Sulfate	AASHTO T290	47	ppm
Chloride	AASHTO T291	107	ppm



Client: Wilson & Company
4401 Masthead St. NE, Suite 150
Albuquerque, NM 87109

Report Date: May 28, 2024

Attn: Myra Candelaria
Project Name: Task #86 US 491 & N30 Intersection

Project #: US0033333.0562

Work Order #: 1

Lab #: 24-0205-07

Sampled By: Eric Koenig

Date Sampled: 5/6/2024

Color & Type of Material: See Boring Log

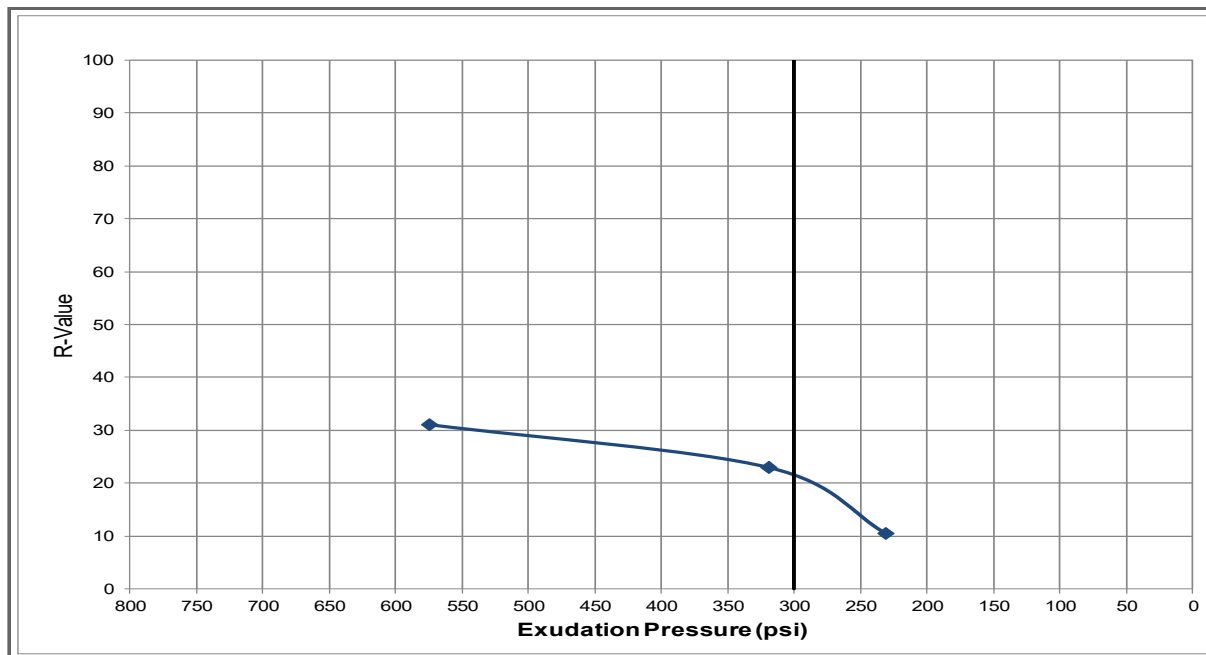
Sample Source: B-02 @ 0-5.0'

Project Manager: Jacob Hays

SOILS / AGGREGATES

RESISTANCE R-VALUE AND EXPANSION PRESSURE OF COMPACTED SOILS (ASTM D2844-18)

<u>Specimen Id.</u>	<u>A</u>	<u>B</u>	<u>C</u>
Moisture (%):	18.6%	19.9%	21.9%
Compactor Pressure (psi):	120	90	60
Specimen Height (in):	2.50	2.50	2.50
Dry Density (pcf):	108.2	107.5	106.8
Horizontal Pressure @ 1000lbs (psi):	37	44	56
Horizontal Pressure @ 2000lbs (psi):	94	108	132
Displacement:	3.90	4.05	4.52
Expansion Pressure (psi):	0.782	0.451	0.000
Exudation Pressure (psi):	574	319	231
R-Value:	31	23	11



R-Value at 300psi: 21

Distribution: Client: ☐ File: ☐ Supplier: ☐ Email: ☐ Other: ☐



Client: Wilson & Company
4401 Masthead St. NE, Suite 150
Albuquerque, NM 87109

Report Date: May 28, 2024

Attn: Myra Candelaria
Project Name: Task #86 US 491 & N30 Intersection

Project #: US0033333.0562

Work Order #: 1

Lab #: 24-0205-10

Sampled By: Eric Koenig

Date Sampled: 5/6/2024

Color & Type of Material: See Boring Log

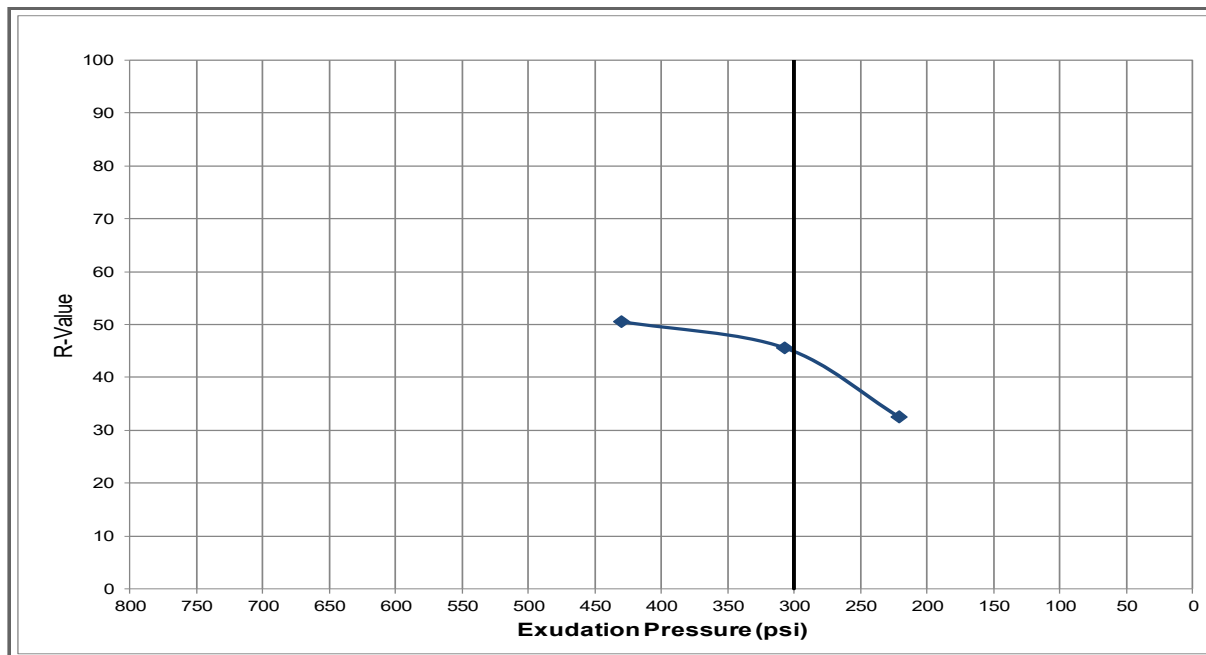
Sample Source: B-03 @ 0-5.0'

Project Manager: Jacob Hays

SOILS / AGGREGATES

RESISTANCE R-VALUE AND EXPANSION PRESSURE OF COMPACTED SOILS (ASTM D2844-18)

<u>Specimen Id.</u>	<u>A</u>	<u>B</u>	<u>C</u>
Moisture (%):	13.6%	14.8%	16.1%
Compactor Pressure (psi):	95	80	70
Specimen Height (in):	2.50	2.50	2.50
Dry Density (pcf):	113.4	112.8	112.7
Horizontal Pressure @ 1000lbs (psi):	26	28	36
Horizontal Pressure @ 2000lbs (psi):	54	60	80
Displacement:	4.80	4.97	5.20
Expansion Pressure (psi):	0.631	0.451	0.241
Exudation Pressure (psi):	430	307	221
R-Value:	51	46	32



R-Value at 300psi: 45

Distribution: Client: ☐ File: ☐ Supplier: ☐ Email: ☐ Other: ☐



Client: Wilson & Company
4401 Masthead St. NE, Suite 150
Albuquerque, NM 87109

Report Date: May 15, 2024

Attn: Myra Candelaria
Project Name: Task #86 US 491 & N30 Intersection

Project #: US0033333.0562

Work Order #: 1

Lab #: 24-0205-07

Sampled By: Eric Koenig

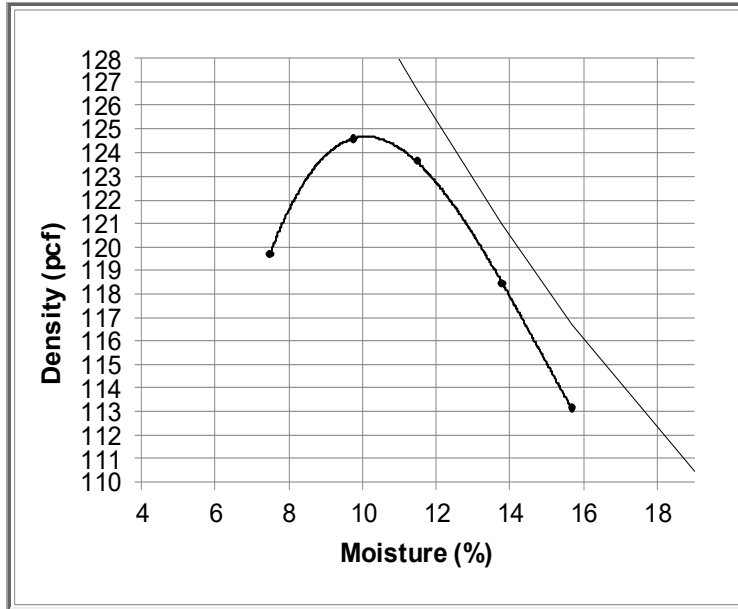
Date Sampled: 5/6/2024

Color & Type of Material: See Boring Log

Project Manager: Jacob Hays

SOILS / AGGREGATES

Sample Source: B-02 @ 0-5.0'



Moisture Density Relationship: (ASTM D1557-21) **Method:** B

Preparation Method: Dry **Rammer Type:** Mechanical

Specific Gravity: 2.651 **Assumed**

Maximum Density: 124.5

Optimum Moisture: 10.0

Distribution: Client: ☐ File: ☐ Supplier: ☐ Email: ☐ Other:

APPENDIX

C

TRAFFIC DATA



Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

Station ID:
Location 1:
Location 2:
Latitude: 35.773072
Longitude: -108.772264

File Name: Mexican Springs NB Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No

11/15/2021	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Week Average
Time	NB, B To A	NB, B To A	NB, B To A	NB, B To A	NB, B To A	NB, B To A	NB, B To A	NB, B To A
12:00 AM	*	*	34	43	*	*	*	38
1:00	*	*	26	28	*	*	*	27
2:00	*	*	16	22	*	*	*	19
3:00	*	*	23	25	*	*	*	24
4:00	*	*	38	27	*	*	*	32
5:00	*	*	43	61	*	*	*	52
6:00	*	*	90	82	*	*	*	86
7:00	*	*	230	227	*	*	*	228
8:00	*	*	133	149	*	*	*	141
9:00	*	*	142	165	*	*	*	154
10:00	*	*	138	170	*	*	*	154
11:00	*	*	176	190	*	*	*	183
12:00 PM	*	*	187	171	*	*	*	179
1:00	*	*	212	187	*	*	*	200
2:00	*	*	220	218	*	*	*	219
3:00	*	*	259	256	*	*	*	258
4:00	*	*	271	258	*	*	*	264
5:00	*	*	323	337	*	*	*	330
6:00	*	*	223	234	*	*	*	228
7:00	*	*	126	162	*	*	*	144
8:00	*	*	124	118	*	*	*	121
9:00	*	*	78	79	*	*	*	78
10:00	*	*	58	56	*	*	*	57
11:00	*	*	43	46	*	*	*	44
Total	0	0	0	0	0	0	0	0
Day	0	0	3213	3311	0	0	0	3260
AM Peak			7:00	7:00				7:00
Volume			230	227				228
PM Peak			5:00	5:00				5:00
Volume			323	337				330
Comb Total	0	0	3213	3311	0	0	0	3260
ADT	ADT: 3,271	AADT: 3,271						

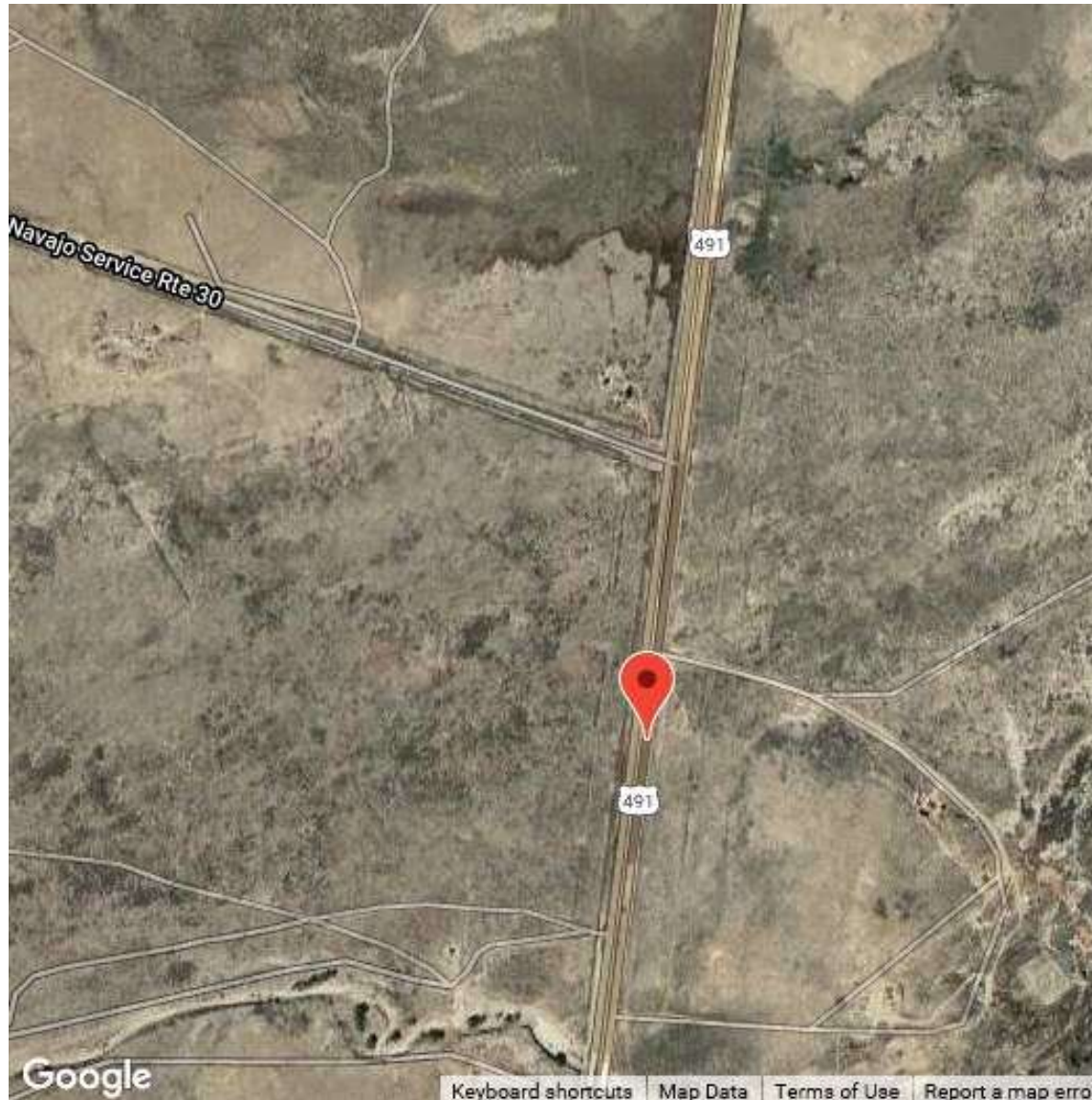


Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

Station ID:
Location 1:
Location 2:
Latitude: 35.773072
Longitude: -108.772264

File Name: Mexican Springs NB Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No





Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

File Name: Mexican Springs NB

Station ID:
Location 1:
Location 2:
Latitude: 35.773072
Longitude: -108.772264
Direction: NB, B To A

Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No

11/17/2021	0 - 35	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70 -	> 75 -	> 80 -	> 85 -	> 90	Total
Time	MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	75 MPH	80 MPH	85 MPH	90 MPH	MPH	
12:00 AM	0	0	0	1	7	7	11	6	2	0	0	0	0	34
1:00	0	0	1	0	4	5	10	4	2	0	0	0	0	26
2:00	0	0	0	0	0	1	9	2	2	1	1	0	0	16
3:00	0	0	0	0	0	2	15	5	1	0	0	0	0	23
4:00	0	0	0	0	2	11	11	9	4	1	0	0	0	38
5:00	1	0	0	0	2	5	14	6	7	4	1	1	2	43
6:00	0	0	0	0	13	16	29	19	9	1	1	2	0	90
7:00	0	0	0	0	10	29	72	51	41	18	7	2	0	230
8:00	0	0	0	1	5	17	44	37	19	6	3	0	1	133
9:00	0	0	0	2	12	32	51	38	4	3	0	0	0	142
10:00	0	0	1	0	17	31	50	27	11	0	1	0	0	138
11:00	0	0	0	3	14	34	69	40	12	4	0	0	0	176
12:00 PM	0	0	0	2	4	30	86	48	13	3	1	0	0	187
1:00	0	0	0	0	7	43	75	56	21	7	2	0	1	212
2:00	0	0	0	6	11	60	72	45	19	6	0	1	0	220
3:00	0	0	0	1	9	43	81	56	47	17	5	0	0	259
4:00	0	0	1	3	9	51	105	56	24	16	6	0	0	271
5:00	0	0	0	6	27	55	111	69	37	16	2	0	0	323
6:00	1	0	0	7	19	43	72	53	14	12	2	0	0	223
7:00	0	0	0	1	8	24	45	29	11	7	1	0	0	126
8:00	0	0	0	4	12	31	34	33	5	3	2	0	0	124
9:00	0	0	0	1	8	11	26	22	6	2	1	0	1	78
10:00	0	0	0	1	6	11	19	11	6	2	1	1	0	58
11:00	0	0	0	0	4	8	12	9	3	3	3	0	1	43
Total	2	0	3	39	210	600	1123	731	320	132	40	7	6	3213



Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

File Name: Mexican Springs NB

Station ID:
Location 1:
Location 2:
Latitude: 35.773072
Longitude: -108.772264
Direction: NB, B To A

Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No

11/18/2021	0 - 35	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70 -	> 75 -	> 80 -	> 85 -	> 90	Total
Time	MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	75 MPH	80 MPH	85 MPH	90 MPH	MPH	
12:00 AM	0	0	0	0	4	9	16	9	3	2	0	0	0	43
1:00	0	0	0	1	0	6	15	4	1	0	1	0	0	28
2:00	0	0	0	2	0	9	5	4	1	1	0	0	0	22
3:00	0	0	0	0	3	6	8	5	2	1	0	0	0	25
4:00	0	0	0	0	1	4	9	6	4	3	0	0	0	27
5:00	0	0	0	0	5	10	12	18	7	5	1	2	1	61
6:00	0	0	0	1	2	11	22	29	9	7	0	1	0	82
7:00	0	0	0	1	5	26	53	65	44	19	8	6	0	227
8:00	0	0	1	1	6	7	54	44	21	9	6	0	0	149
9:00	0	0	0	0	6	13	55	42	34	13	1	0	1	165
10:00	0	1	0	0	9	25	55	43	27	7	3	0	0	170
11:00	0	1	0	2	6	27	68	53	23	7	3	0	0	190
12:00 PM	1	0	0	1	4	10	59	46	24	19	4	0	3	171
1:00	0	1	0	1	11	19	56	53	31	11	3	0	1	187
2:00	0	0	0	0	1	21	71	70	33	17	5	0	0	218
3:00	0	0	0	2	7	22	64	66	51	24	9	6	5	256
4:00	0	0	0	0	6	21	71	81	47	21	6	1	4	258
5:00	0	0	0	1	12	48	105	97	51	14	7	1	1	337
6:00	0	0	2	2	6	39	58	59	40	15	12	1	0	234
7:00	0	0	0	2	3	25	49	48	21	12	2	0	0	162
8:00	0	0	0	0	2	15	38	36	19	5	2	1	0	118
9:00	0	1	0	0	5	11	21	20	11	8	2	0	0	79
10:00	0	0	0	0	2	6	14	12	10	5	3	3	1	56
11:00	0	0	0	0	3	8	15	10	7	0	2	1	0	46
Total	1	4	3	17	109	398	993	920	521	225	80	23	17	3311
Grand Total	3	4	6	56	319	998	2116	1651	841	357	120	30	23	6524

Stats	Percentile	15th	50th	85th	95th
Speed		58.5	64.3	71.7	77.3
Mean Speed (Average)		65.4			
10 MPH Pace Speed		60-69			
Number in Pace		3754			
Percent in Pace		57.5%			
Number > 65 MPH		3022			
Percent > 65 MPH		46.3%			



Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

Station ID:
Location 1:
Location 2:
Latitude: 35.773072
Longitude: -108.772264

File Name: Mexican Springs NB
Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No





Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

File Name: Mexican Springs NB

Station ID:
Location 1:
Location 2:
Latitude: 35.773072
Longitude: -108.772264
Direction: NB, B To A

Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No

11/17/2021 Time	Motor Cycles	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	No Class	Total
12:00 AM	0	17	7	0	0	0	0	2	6	0	0	2	0	0	34
1:00	0	11	1	0	2	1	0	4	5	0	2	0	0	0	26
2:00	0	8	2	0	0	0	0	1	4	0	0	1	0	0	16
3:00	0	7	1	0	1	2	0	3	7	0	0	1	0	1	23
4:00	0	17	3	0	1	3	0	1	10	0	1	2	0	0	38
5:00	0	22	8	0	2	1	0	4	5	0	0	1	0	0	43
6:00	1	38	21	3	8	4	0	8	6	0	0	1	0	0	90
7:00	0	136	44	5	13	2	0	12	16	0	1	1	0	0	230
8:00	0	69	33	1	7	2	0	7	12	0	1	1	0	0	133
9:00	0	59	37	4	7	7	0	7	20	0	0	0	1	0	142
10:00	0	62	36	1	8	5	0	6	20	0	0	0	0	0	138
11:00	0	94	34	4	13	8	0	9	13	0	1	0	0	0	176
12:00 PM	0	97	50	1	10	0	0	5	24	0	0	0	0	0	187
1:00	0	121	46	2	5	3	0	9	21	1	2	1	0	1	212
2:00	1	122	55	1	9	6	0	8	16	1	0	0	0	1	220
3:00	0	160	63	2	8	3	0	8	11	0	1	0	1	2	259
4:00	0	150	68	8	18	2	0	7	13	3	0	0	0	2	271
5:00	0	218	67	0	19	0	0	8	8	1	0	0	0	2	323
6:00	0	154	41	1	14	2	0	3	6	0	0	0	0	2	223
7:00	0	75	32	1	7	1	0	3	6	0	0	1	0	0	126
8:00	0	70	26	0	7	2	0	8	10	0	0	0	0	1	124
9:00	0	54	10	0	3	3	0	4	4	0	0	0	0	0	78
10:00	0	43	4	0	0	0	0	4	7	0	0	0	0	0	58
11:00	0	28	4	2	0	0	0	2	7	0	0	0	0	0	43
Total	2	1832	693	36	162	57	0	133	257	6	9	12	2	12	3213
Percent	0.1%	57.0%	21.6%	1.1%	5.0%	1.8%	0.0%	4.1%	8.0%	0.2%	0.3%	0.4%	0.1%	0.4%	
AM Peak	6:00	7:00	7:00	7:00	7:00	11:00		7:00	9:00		1:00	12:00 AM	9:00	3:00	7:00
PM Peak	1	136	44	5	13	8	*	12	20	*	2	2	1	1	230
	2:00	5:00	4:00	4:00	5:00	2:00		1:00	12:00 PM	4:00	1:00	1:00	3:00	3:00	5:00
	1	218	68	8	19	6	*	9	24	3	2	1	1	2	323



Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

File Name: Mexican Springs NB

Station ID:
Location 1:
Location 2:
Latitude: 35.773072
Longitude: -108.772264
Direction: NB, B To A

Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No

11/18/2021 Time	Motor Cycles	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	No Class	Total
12:00 AM	0	16	7	1	1	0	0	4	11	0	0	2	0	1	43
1:00	0	8	5	2	3	0	0	2	6	0	1	1	0	0	28
2:00	0	5	0	0	4	1	0	2	7	0	1	2	0	0	22
3:00	0	7	0	0	3	4	0	3	6	0	1	1	0	0	25
4:00	0	15	3	0	1	1	0	0	3	1	2	1	0	0	27
5:00	0	31	10	1	3	3	0	1	10	1	0	0	0	1	61
6:00	0	35	21	5	5	3	0	4	9	0	0	0	0	0	82
7:00	0	135	51	6	17	3	0	4	9	0	0	0	0	2	227
8:00	0	77	34	2	15	2	0	10	7	0	1	1	0	0	149
9:00	1	87	41	3	9	1	0	7	12	0	1	0	1	2	165
10:00	0	80	38	5	14	6	0	8	14	2	0	1	1	1	170
11:00	0	97	41	6	13	4	0	11	14	1	0	1	0	2	190
12:00 PM	0	94	38	1	11	4	0	9	10	1	2	0	0	1	171
1:00	0	108	45	2	14	3	0	5	9	1	0	0	0	0	187
2:00	0	122	49	4	14	2	0	14	10	1	1	1	0	0	218
3:00	0	168	49	2	16	2	0	10	6	0	1	1	0	1	256
4:00	0	140	60	9	18	4	0	15	8	0	0	0	0	4	258
5:00	0	180	95	2	37	2	0	8	8	0	0	0	0	5	337
6:00	0	142	46	3	31	1	0	4	5	0	0	0	0	2	234
7:00	0	103	25	1	14	1	0	6	11	0	0	1	0	0	162
8:00	0	60	30	3	12	3	0	3	7	0	0	0	0	0	118
9:00	0	45	17	0	9	0	0	4	4	0	0	0	0	0	79
10:00	0	25	12	3	7	3	0	3	3	0	0	0	0	0	56
11:00	0	26	9	0	3	1	0	3	3	0	0	1	0	0	46
Total	1	1806	726	61	274	54	0	140	192	8	11	14	2	22	3311
Percent	0.0%	54.5%	21.9%	1.8%	8.3%	1.6%	0.0%	4.2%	5.8%	0.2%	0.3%	0.4%	0.1%	0.7%	
AM Peak	9:00	7:00	7:00	7:00	7:00	10:00		11:00	10:00	10:00	4:00	12:00 AM	9:00	7:00	7:00
PM Peak	1	135	51	6	17	6	*	11	14	2	2	2	1	2	227
		5:00	5:00	4:00	5:00	12:00 PM		4:00	7:00	12:00 PM	12:00 PM	2:00		5:00	5:00
	*	180	95	9	37	4	*	15	11	1	2	1	*	5	337
Grand Total	3	3638	1419	97	436	111	0	273	449	14	20	26	4	34	6524
Percent	0.0%	55.8%	21.8%	1.5%	6.7%	1.7%	0.0%	4.2%	6.9%	0.2%	0.3%	0.4%	0.1%	0.5%	



Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

Station ID:
Location 1:
Location 2:
Latitude: 35.773072
Longitude: -108.772264

File Name: Mexican Springs NB
Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No





Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

File Name: Mexican Springs SB Lane

Date Printed: 11/22/2021

Start Date: 11/17/2021

End Date: 11/18/2021

GPS Accuracy: 0 ft

Location Verified: No

Station ID:
Location 1:
Location 2:
Latitude: 35.772776
Longitude: -108.772435

11/15/2021	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Week Average
Time	SB, B To A	SB, B To A	SB, B To A	SB, B To A	SB, B To A	SB, B To A	SB, B To A	SB, B To A
12:00 AM	*	*	18	17	*	*	*	18
1:00	*	*	23	20	*	*	*	22
2:00	*	*	22	18	*	*	*	20
3:00	*	*	17	21	*	*	*	19
4:00	*	*	22	28	*	*	*	25
5:00	*	*	70	73	*	*	*	72
6:00	*	*	152	135	*	*	*	144
7:00	*	*	274	251	*	*	*	262
8:00	*	*	165	170	*	*	*	168
9:00	*	*	175	186	*	*	*	180
10:00	*	*	174	198	*	*	*	186
11:00	*	*	162	196	*	*	*	179
12:00 PM	*	*	190	189	*	*	*	190
1:00	*	*	159	182	*	*	*	170
2:00	*	*	169	186	*	*	*	178
3:00	*	*	188	199	*	*	*	194
4:00	*	*	183	219	*	*	*	201
5:00	*	*	184	218	*	*	*	201
6:00	*	*	143	128	*	*	*	136
7:00	*	*	120	115	*	*	*	118
8:00	*	*	98	59	*	*	*	78
9:00	*	*	52	40	*	*	*	46
10:00	*	*	43	39	*	*	*	41
11:00	*	*	25	29	*	*	*	27
Total	0	0	0	0	0	0	0	0
Day	0	0	2828	2916	0	0	0	2875
AM Peak			7:00	7:00				7:00
Volume			274	251				262
PM Peak			12:00 PM	4:00				4:00
Volume			190	219				201
Comb Total	0	0	2828	2916	0	0	0	2875
ADT	ADT: 2,888	AADT: 2,888						

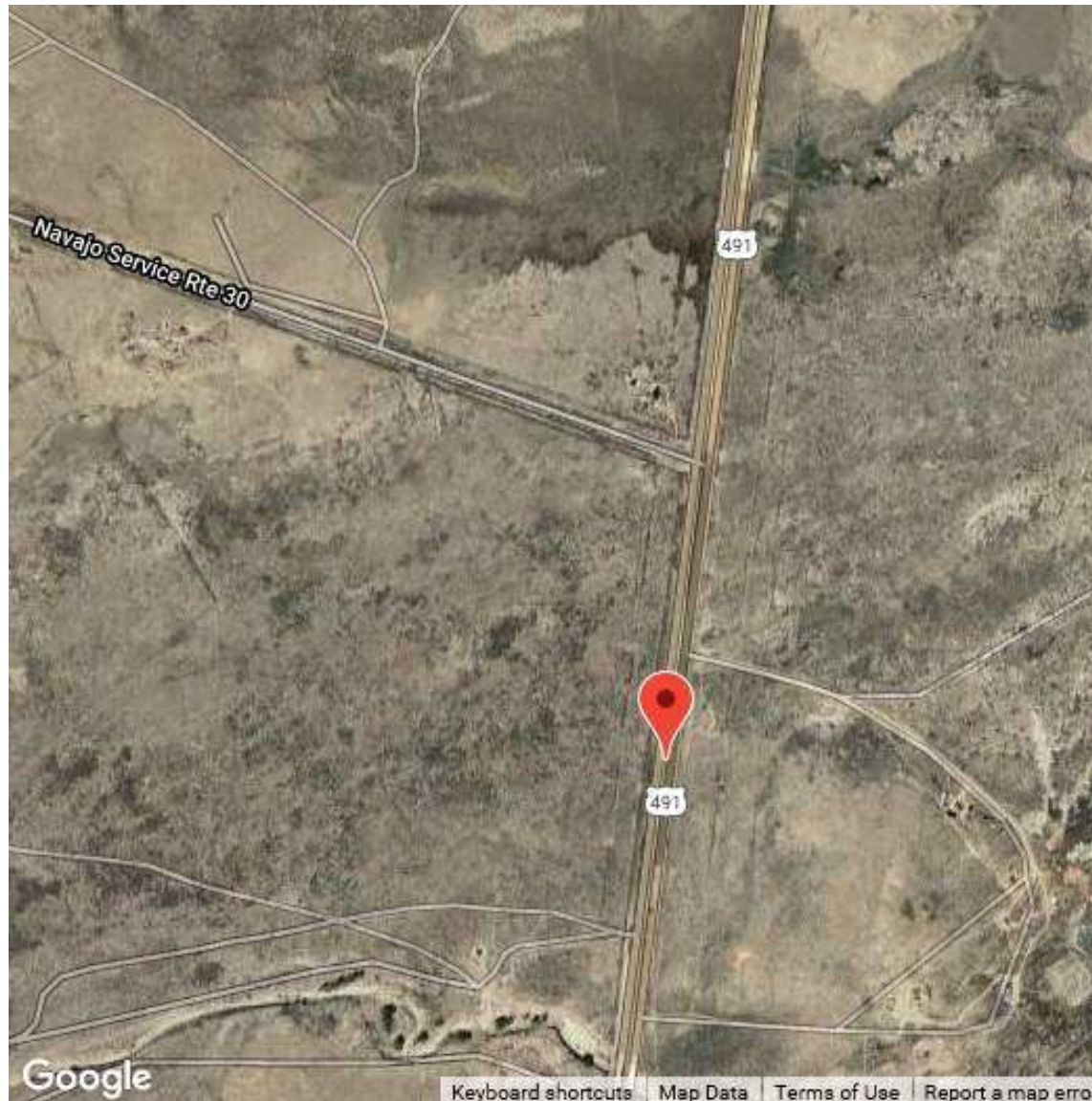


Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

Station ID:
Location 1:
Location 2:
Latitude: 35.772776
Longitude: -108.772435

File Name: Mexican Springs SB Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No





Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

File Name: Mexican Springs SB

Station ID:
Location 1:
Location 2:
Latitude: 35.772776
Longitude: -108.772435
Direction: SB, B To A

Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No

11/17/2021	0 - 35	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70 -	> 75 -	> 80 -	> 85 -	> 90	
Time	MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	75 MPH	80 MPH	85 MPH	90 MPH	MPH	Total
12:00 AM	0	0	0	0	0	0	8	10	0	0	0	0	0	18
1:00	0	0	0	0	0	1	8	11	3	0	0	0	0	23
2:00	0	0	0	0	1	4	5	11	1	0	0	0	0	22
3:00	0	0	0	0	1	4	3	6	1	2	0	0	0	17
4:00	0	0	0	1	0	3	3	11	4	0	0	0	0	22
5:00	0	0	0	0	3	17	25	16	4	2	3	0	0	70
6:00	0	0	1	1	13	19	42	45	21	8	1	0	1	152
7:00	0	0	0	1	5	28	84	86	45	15	9	1	0	274
8:00	0	0	0	0	2	10	53	54	28	15	0	1	2	165
9:00	0	0	0	0	4	17	57	57	32	4	1	2	1	175
10:00	0	0	1	4	8	18	43	62	23	7	7	1	0	174
11:00	0	0	0	0	4	15	42	59	27	10	5	0	0	162
12:00 PM	0	0	0	3	9	12	53	76	27	7	3	0	0	190
1:00	0	0	0	0	5	20	52	48	24	7	2	0	1	159
2:00	0	0	0	0	10	21	47	53	23	10	4	0	1	169
3:00	0	0	0	2	4	15	73	52	30	9	3	0	0	188
4:00	0	0	0	1	3	15	52	51	34	18	8	1	0	183
5:00	0	0	0	1	10	24	51	61	29	8	0	0	0	184
6:00	1	1	0	1	4	23	45	48	15	4	1	0	0	143
7:00	0	0	0	2	3	22	38	33	16	6	0	0	0	120
8:00	0	0	0	1	3	19	39	29	5	2	0	0	0	98
9:00	0	0	0	0	2	14	16	15	4	1	0	0	0	52
10:00	0	0	0	0	2	5	21	12	1	1	1	0	0	43
11:00	0	0	0	0	3	8	6	8	0	0	0	0	0	25
Total	1	1	2	18	99	334	866	914	397	136	48	6	6	2828



Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

File Name: Mexican Springs SB

Station ID:
Location 1:
Location 2:
Latitude: 35.772776
Longitude: -108.772435
Direction: SB, B To A

Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No

11/18/2021	0 - 35	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70 -	> 75 -	> 80 -	> 85 -	> 90	Total
Time	MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	75 MPH	80 MPH	85 MPH	90 MPH	MPH	
12:00 AM	0	0	0	0	1	1	6	2	7	0	0	0	0	17
1:00	0	0	0	0	0	4	7	6	2	1	0	0	0	20
2:00	0	0	0	0	1	1	5	8	2	1	0	0	0	18
3:00	0	0	0	0	0	0	5	12	2	2	0	0	0	21
4:00	0	0	0	1	3	1	4	12	7	0	0	0	0	28
5:00	0	0	0	1	3	13	27	18	5	2	1	2	1	73
6:00	0	0	0	1	10	27	47	35	9	5	1	0	0	135
7:00	0	0	0	0	2	16	74	93	46	14	3	3	0	251
8:00	0	0	1	1	1	19	40	52	32	14	7	3	0	170
9:00	0	0	0	1	2	11	36	68	42	20	4	2	0	186
10:00	0	0	0	1	4	12	50	74	34	11	6	4	2	198
11:00	0	0	0	2	9	17	67	58	29	10	2	1	1	196
12:00 PM	0	0	0	2	1	14	53	75	30	9	4	1	0	189
1:00	0	0	0	0	7	20	47	49	34	13	7	3	2	182
2:00	0	0	0	0	4	18	60	59	29	4	8	2	2	186
3:00	1	0	1	0	5	16	53	72	27	18	5	1	0	199
4:00	0	0	0	3	4	15	71	66	40	12	6	1	1	219
5:00	0	0	0	3	8	25	65	77	27	8	4	1	0	218
6:00	0	0	0	3	5	18	43	34	19	5	0	0	1	128
7:00	0	0	0	0	3	12	37	50	11	2	0	0	0	115
8:00	0	0	0	0	7	6	23	16	4	3	0	0	0	59
9:00	0	0	1	0	1	5	12	16	5	0	0	0	0	40
10:00	0	0	0	1	2	11	11	8	4	0	2	0	0	39
11:00	0	0	0	0	2	4	6	12	5	0	0	0	0	29
Total	1	0	3	20	85	286	849	972	452	154	60	24	10	2916
Grand Total	2	1	5	38	184	620	1715	1886	849	290	108	30	16	5744

Stats	Percentile	15th	50th	85th	95th
Speed		59.9	65.6	71.9	77.3
Mean Speed (Average)		66.2			
10 MPH Pace Speed		60-69			
Number in Pace		3576			
Percent in Pace		62.3%			
Number > 65 MPH		3179			
Percent > 65 MPH		55.3%			



Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

Station ID:
Location 1:
Location 2:
Latitude: 35.772776
Longitude: -108.772435

File Name: Mexican Springs SB
Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No





Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

File Name: Mexican Springs SB

Station ID:
Location 1:
Location 2:
Latitude: 35.772776
Longitude: -108.772435
Direction: SB, B To A

Lane
Date Printed: 11/22/2021
Start Date: 11/17/2021
End Date: 11/18/2021
GPS Accuracy: 0 ft
Location Verified: No

11/17/2021 Time	Motor Cycles	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	No Class	Total
12:00 AM	0	10	0	1	0	3	0	0	4	0	0	0	0	0	18
1:00	0	8	1	1	1	5	0	1	6	0	0	0	0	0	23
2:00	0	4	3	0	2	2	0	1	7	0	3	0	0	0	22
3:00	0	7	1	2	1	1	0	1	3	0	1	0	0	0	17
4:00	0	8	3	2	1	2	0	0	5	0	1	0	0	0	22
5:00	0	38	8	6	9	3	0	0	6	0	0	0	0	0	70
6:00	0	72	36	8	19	7	0	0	9	0	0	0	0	1	152
7:00	0	170	70	3	17	2	0	2	10	0	0	0	0	0	274
8:00	0	86	39	5	12	3	0	2	18	0	0	0	0	0	165
9:00	0	79	41	5	13	5	0	6	23	0	1	0	0	2	175
10:00	1	80	45	2	10	3	0	12	18	1	0	0	0	2	174
11:00	0	77	47	3	13	4	0	2	15	0	0	0	0	1	162
12:00 PM	0	109	31	3	13	5	0	10	17	0	0	1	0	1	190
1:00	0	86	29	5	9	4	0	10	12	0	2	1	0	1	159
2:00	0	98	38	2	11	1	0	5	14	0	0	0	0	0	169
3:00	0	107	39	6	11	5	0	2	17	0	0	0	0	1	188
4:00	0	97	55	1	12	1	0	8	9	0	0	0	0	0	183
5:00	0	98	42	1	20	3	0	7	13	0	0	0	0	0	184
6:00	0	73	32	3	13	3	0	6	12	0	0	0	0	1	143
7:00	0	72	18	3	4	1	0	5	15	0	1	0	0	1	120
8:00	0	57	14	0	5	1	0	5	15	0	1	0	0	0	98
9:00	0	29	4	0	1	0	0	5	12	0	1	0	0	0	52
10:00	0	19	9	0	0	3	0	6	4	0	0	1	0	1	43
11:00	0	15	0	1	0	2	0	2	3	0	1	1	0	0	25
Total	1	1499	605	63	197	69	0	98	267	1	12	4	0	12	2828
Percent	0.0%	53.0%	21.4%	2.2%	7.0%	2.4%	0.0%	3.5%	9.4%	0.0%	0.4%	0.1%	0.0%	0.4%	
AM Peak	10:00	7:00	7:00	6:00	6:00	6:00		10:00	9:00	10:00	2:00			9:00	7:00
	1	170	70	8	19	7	*	12	23	1	3	*	*	2	274
PM Peak		12:00	4:00	3:00	5:00	12:00		12:00	12:00		1:00	12:00		12:00	12:00
		PM				PM		PM	PM			PM		PM	PM
	*	109	55	6	20	5	*	10	17	*	2	1	*	1	190



Wilson & Company

4401 Masthead Street NE, Suite 150
Albuquerque, NM 87109

File Name: Mexican Springs SB

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12:00 AM	0	9	1	0	0	1	0	1	5	0	0	0	0	0	17
1:00	0	4	1	0	1	2	0	4	8	0	0	0	0	0	20
2:00	0	5	3	1	0	1	0	1	5	0	2	0	0	0	18
3:00	0	5	2	2	2	3	0	0	5	0	2	0	0	0	21
4:00	0	12	2	2	1	4	0	2	5	0	0	0	0	0	28
5:00	0	38	12	7	6	2	0	3	5	0	0	0	0	0	73
6:00	0	68	29	7	15	5	0	2	8	0	0	0	0	1	135
7:00	0	137	62	5	21	8	0	4	13	0	1	0	0	0	251
8:00	0	92	45	2	7	5	0	6	11	0	0	1	0	1	170
9:00	0	83	52	6	13	3	0	6	20	0	2	0	0	1	186
10:00	0	85	59	3	19	3	0	9	17	0	0	0	0	3	198
11:00	0	101	46	3	11	3	0	4	24	0	0	0	1	3	196
12:00 PM	0	95	47	3	16	1	0	6	17	0	0	1	0	3	189
1:00	0	106	49	7	4	2	0	6	8	0	0	0	0	0	182
2:00	0	104	41	8	13	1	0	6	12	0	0	0	0	1	186
3:00	0	91	57	7	19	2	0	7	15	0	0	0	0	1	199
4:00	0	116	45	5	21	5	0	11	16	0	0	0	0	0	219
5:00	0	122	49	9	19	1	0	3	15	0	0	0	0	0	218
6:00	0	73	19	4	8	3	0	7	13	1	0	0	0	0	128
7:00	0	59	17	5	7	2	0	5	15	0	1	3	0	1	115
8:00	0	26	9	2	4	2	0	7	9	0	0	0	0	0	59
9:00	0	13	10	1	3	2	0	2	9	0	0	0	0	0	40
10:00	0	21	8	1	0	1	0	3	3	0	1	1	0	0	39
11:00	0	14	4	0	1	3	0	2	3	1	1	0	0	0	29
Total	0	1479	669	90	211	65	0	107	261	2	10	6	1	15	2916
Percent	0.0%	50.7%	22.9%	3.1%	7.2%	2.2%	0.0%	3.7%	9.0%	0.1%	0.3%	0.2%	0.0%	0.5%	
AM Peak		7:00	7:00	5:00	7:00	7:00		10:00	11:00		2:00	8:00	11:00	10:00	7:00
	*	137	62	7	21	8	*	9	24	*	2	1	1	3	251
PM Peak		5:00	3:00	5:00	4:00	4:00		4:00	12:00	6:00	7:00	7:00		12:00	4:00
	*	122	57	9	21	5	*	11	17	1	1	3	*	3	219
Grand Total	1	2978	1274	153	408	134	0	205	528	3	22	10	1	27	5744
Percent	0.0%	51.8%	22.2%	2.7%	7.1%	2.3%	0.0%	3.6%	9.2%	0.1%	0.4%	0.2%	0.0%	0.5%	



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